

**APPENDIX 1: W OG WRD GEOTECHNICAL INVESTIGATION REPORT**

**Reference No: 2172/g**

**REPORT ON A GEOTECHNICAL INVESTIGATION  
FOR THE  
PROPOSED FAR WEST WASTE ROCK DUMP 2  
FOR THE THARISA MINE**



**SEPTEMBER 2021**

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**REPORT ON A GEOTECHNICAL INVESTIGATION FOR  
THE PROPOSED FAR WEST WASTE ROCK DUMP 2 FOR THE  
THARISA MINE  
NORTH WEST PROVINCE**

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# **REPORT ON A GEOTECHNICAL INVESTIGATION FOR THE PROPOSED FAR WEST WASTE ROCK DUMP 2 FOR THE THARISA MINE NORTH WEST PROVINCE**

## **1. INTRODUCTION**

Tharisa Mine is an open-cast chrome mine situated adjacent to the N4 national route, some 25km east of Rustenburg in North West Province, and is owned by the South Africa-based company Tharisa Minerals.

Tharisa Minerals intends to build the Tailings Storage Facility (TSF3) and associated infrastructure, including a Return Water Dam (RWD) and two Waste Rock Dumps. The latter are referred to as West Waste Rock Dump (W WRD2) and Far West Waste Rock Dump (FW WRD2).

This report presents the results of a geotechnical investigation carried out within the FW WRD2 site.

## **2. TERMS OF REFERENCE**

Inroads Consulting was requested by Epoch Resources (Pty) Ltd on behalf of Tharisa Minerals to offer a proposal for undertaking a geotechnical investigation at the site of the FW WRD2.

The investigation was to be undertaken with the following primary objectives:

- to establish the stratigraphy and engineering characteristics of the subsoils underlying the waste rock dump and;
- to determine the shear strength and permeability properties of the underlying soil or rock

The proposal was subsequently prepared by Inroads Consulting cc and submitted to Epoch on 6 July 2021 and was accepted by them.

## **3. SCOPE OF ACTIVITIES**

### **3.1 Literature Review**

Before undertaking the fieldwork, discussed below, a literature survey was carried out in which all information pertaining to the engineering geological and geotechnical conditions was obtained and reviewed. This included the 1:250 000 scale Geological Map<sup>(1)</sup> and Volume 1 of the series Engineering Geology of Southern Africa<sup>(2)</sup>.

Details of these publications are presented in the References attached to this Report as Appendix A.

### **3.2 Fieldwork**

The fieldwork was undertaken from 16<sup>th</sup> to 17<sup>th</sup> July 2021 and entailed setting out and excavating 19 test pits to depths of between 1,2 to 5,3 m averaging 3,0 m. The pits are located along the northern and western boundary of the existing and presently mined pit. Most of the southern area of the FW WRD is occupied by dump rock and no pits were excavated in this area. Limited profiling of an about 5 to 7 m deep face of an excavation of the mine pit, located in the vicinity of TP18 and TP19, was also undertaken at points referenced PT1 to PT3.

The test pits, of which the positions were determined and set out by Epoch, were put down employing a Komatsu PC300 excavator and their final positions were coordinated using a hand-held Garmin GPS. The sidewall of the exposed soil was also photographed.

The pits and the excavation face were profiled following standard methods and procedures prescribed in the document *Guidelines for Soil and Rock Logging in South Africa (2002)*<sup>(3)</sup> and their positions are presented in the site plan attached as Appendix B.

Samples were recovered from certain of the soil horizons and sent to Specialised Testing Laboratory and SGS Matrolab, both ISO accredited civil engineering testing laboratories in Pretoria, South Africa.

#### 4. SITE DESCRIPTION

The area of the FW WRD is largely occupied by the presently mined open pit and dump rock located along its southern boundary. Due to this, the test pits were only located along the northern and western boundary of the pit, which is accessible by the mine road running alongside it. The overburden or topsoil is stockpiled along the boundary.

Photographs of the site are presented in Appendix C of this report.

#### 5. GEOLOGY

According to the 1:250 000 geological series map referenced 2526 Rustenburg, the area is largely underlain by leuconorite, anorthosite, pyroxenite and chromitite of the Matlagame Norite Anorthosite of the Schilpadnest Sub-suite, Rustenburg Layered Suite, Bushveld Complex. Some parts of the site to the north are shown to be underlain by anorthosite.

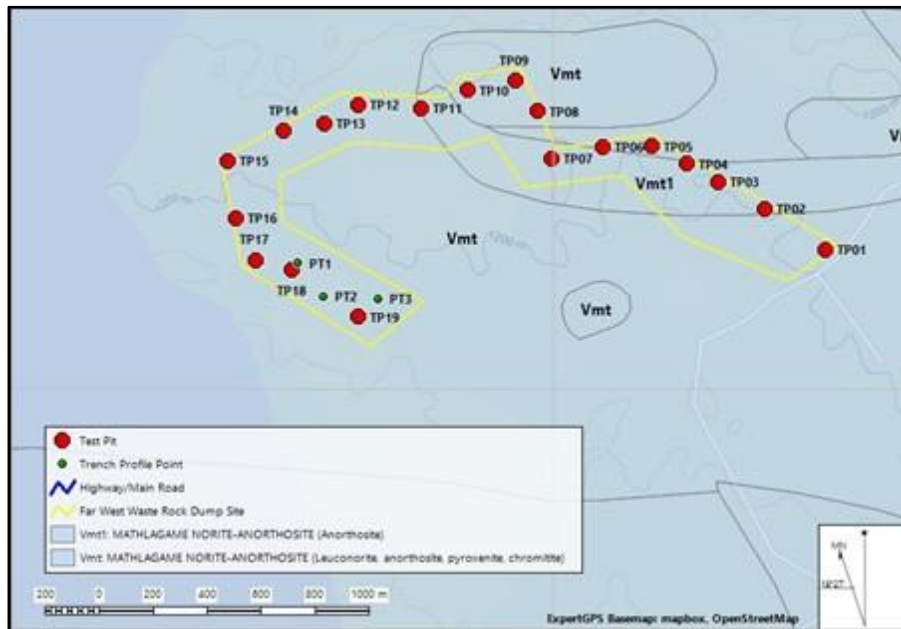


Figure 1: Surface geology in the region (from 1:250 000 Geological series).

## **6. GROUND CONDITIONS**

### **6.1 Subsoils**

The subsoil conditions within the site are described below and are summarised in Table 1 overleaf and in Appendix D attached to this report. The test pits with similar soil profiles are grouped into Zones A and B.

#### Zone A

The subsoil conditions in Zone A, as represented by test pits TP01 to TP12, are characterised by a black, soft to firm, sandy clay, or black turf. This overlies medium dense to very dense silty sand of residual norite origin at depths averaging 1,4 m. The residual sand often grades to very soft rock within which the excavator refused at depths ranging from 1,6 to 3,7 m. In test pit TP12, however, the sand extends to a depth of 5,3 m with no refusal occurring. In test pit TP09, fill, comprising loose, silty sand with gravel and cobbles was encountered to a depth of 5,0 m. This is probably an excavation that has been backfilled.

#### Zone B

The subsoil conditions in Zone B are represented by test pits TP13 to TP19 and the three points, PT1 to PT3, profiled on the excavation face of the mine pit and in the vicinity of TP18 and TP19. The subsoils are characterised by a very stiff, or very dense, clayey sand horizon of alluvial origin. It becomes more clayey with depth and in test pits TP15 and T16 is underlain by a residual clayey sandy silt horizon of unknown origin at depths of between 2,5 to 3,0 m.

On the excavation face, represented by points PT1 and PT2, the alluvium extends to depths of between 4,5 to 5,5 m and is separated from the underlying sandy residual norite, or very soft rock, by a thin pebble marker horizon.

In test pit TP14 the alluvium extends to the bottom of the hole at 4,5 m, however, in test pits TP13, TP17, TP18 and TP19, it is very stiff and the excavator refused on it or on the pebble marker and nodular ferricrete located beneath it at depths of between 1,2 to 2,1 m, averaging 1,5 m below the present ground surface.

To the south of the site, the alluvium is cemented with nodular and hardpan ferricrete occurring at about 1,7 m. The excavator refused on the ferricrete horizon in test pit TP19 at about 2,1 m. On the excavation face at point PT3, the nodular ferricrete extends to the bottom of the excavation at 5,0 m.

The detailed soil profiles of the test pits and excavation face are attached to this report as Appendix D, and a summary of the test pit profiles are also attached with these. Photographs of the pits are presented in Appendix E.

### **6.2 Groundwater**

No groundwater was encountered in any of the pits.

Table 6.1: Summary of profiles

Zone	Test Pit no.	Soil Description									Refusal
		Fill - Silty sand/ Clay sand + gravels & cobble	Hillw ash Silty sand	Black Turf Clay	Alluvium Clayey/ silty sand	Pebble marker - Gravel	Nodular ferricrete - Gravel	Residual - Clayey sandy silt/Sandy silty clay	Residual - Silty sand	Norite/ Chromitite - Very soft rock	
A		<i>SM-SC</i>		<i>MH/CH</i>					<i>WS-SC, SM</i>		
	TP01			0.0-0.8					0.8-3.7		Refusal on very soft rock norite
	TP02			0.0-1.4						1.4-1.6	Refusal on very soft rock norite
	TP03			0.0-1.3					1.3-3.3		Refusal on very soft rock norite
	TP04			0.0-1.4					1.4-2.6		Refusal on very soft rock norite
	TP05	0.0-0.4		0.4-1.9					1.9-3.4		Refusal on very soft rock norite
	TP06			0.0-0.8					0.8-1.6	1.6-1.8	Refusal on very soft rock norite
	TP07	0.0-0.1		0.1-1.5					1.5-1.8	1.8-1.9	Refusal on very soft rock norite
	TP08	0.0-0.2		0.2-1.4					1.4-1.6	1.6-1.7	Refusal on very soft rock norite
	TP09	0.0-5.0									
	TP10			0.0-0.7						0.7-2.8	Refusal on very soft rock chromitite
	TP11			0.0-0.7					0.7-2.5		Refusal on very soft rock norite
TP12			0.0-2.7					2.7-5.3			
B		<i>SM-SC</i>	<i>SP-SM</i>		<i>SC</i>	<i>GC</i>	<i>GC</i>	<i>CL, MH</i>	<i>SW-SC, SM</i>		
	TP13				0.0-1.3						Refusal on very stiff/very dense clayey sand.
	TP14				0.0-4.5						
	TP15				0.0-2.4	2.4-2.5		2.5-4.8			
	TP16	0.0-0.2			0.2-3.0			3.0-5.3			
	TP17				0.0-1.2	1.2-1.5					Refusal on very dense cobbles and gravels.
	TP18				0.0-1.2						Refusal on very stiff/very dense clayey sand.
	TP19	0.0-0.9			0.9-1.7		1.7-2.1				Refusal on very stiff/very dense clayey sand and nodular ferricrete.
	PT1				0.0-4.5			4.5-6.0			
PT2	0.0-1.0			1.0-5.5				5.5-7.0		Gravels and highly weathered rock encountered at the bottom of the excavation	
PT3		0.0-0.9		0.9-1.7		1.7-5.0					

## 7. LABORATORY TESTING

Disturbed and undisturbed samples were recovered from the underlying soil horizons and a range of tests were carried out on them to assess their engineering characteristics. The tests were undertaken to TMH, SANS and ASTM specifications and the results are presented in Appendix F and discussed in more detail below.

### 7.1 Indicator Tests

To more accurately identify and classify the soil horizons encountered, particle size distribution analysis and Atterberg limit determinations were carried out on the samples of black turf, alluvium and the residual horizons which cover the site.

With the exception of the sandy residual norite, it is evident from the test results that the black turf, alluvium and residual norite silt, encountered in test pit TP15, are of medium to high expansive potential, according to the method of Van der Merwe<sup>(4)</sup>, having weighted plasticity index of between 13 and 25. Their grading moduli are all less than 1,0 which is indicative of their fine-grained texture.

The sandy residual norite has a weighted plasticity index of 8 and is also denoted as “NP”, which suggests that it is of low expansive potential and non-plastic, respectively. Its grading modulus of between 1,44 to 2,09 indicates that it is medium to coarse grained. It also classifies as “SW-SC” and “SM” soil types according to the Unified Soil Classification System (USCS)<sup>(5)</sup>, which is well-graded sand to clayey sand and silty sand respectively. It also classifies as an A-2-4 to A-1-b soil type according to the AASHTO classification system<sup>(6)</sup>, these being silty sands with gravel and have a “good” rating for use as subgrade.

The AASHTO classification system groups the black turf, alluvium and the silty residual encountered in test pit TP15 as A-7-5 to A-7-6 soil types, which are generally clayey soil having a “poor” rating for use as subgrade. Except for the alluvium, these soils classify as MH and occasionally a CL soil type according to the USCS, which is silt of high plasticity and clay of low plasticity respectively. The alluvium classifies as SC and SM soil types which are clayey sand and silty sand respectively.

Table 7.1: Summary of indicator tests

Test pit	Depth (m)	Soil Description	LL	PI	PI <sub>ws</sub>	LS	GM	MIT Size Fraction - %				Classification	
								Gravel	Sand	Silt	Clay	AASHTO	USCS
TP01	2.1 – 2.4	Residual norite - Slightly silty sand	-	NP	-	0	1.44	11	82	7	1	A-2-4	SW-SC
TP05	1.0 – 1.3	Black turf – Sandy clay	69	28	25	13	0.38	4	26	28	42	A-7-5	MH
TP05	1.9 – 3.4	Residual norite - Silty sand & gravel	29	8	2	4.5	2.09	47	45	6	2	A-2-4	SW-SC
TP12	2.7 – 5.0	Residual norite - Silty sand	-	NP	-	0	1.52	16	73	10	1	A-1-b	SM
TP13	0.4 – 1.3	Alluvium - Clayey sand	34	18	13	9.5	0.97	0	73	9	18	A-2-6	SC
TP14	3.1 – 3.4	Alluvium - Clayey sand	44	14	10	7	0.82	2	58	12	27	A-7-5	SM
TP15	2.5 – 3.0	Residual - sandy clay	49	23	19	11.5	0.56	4	35	17	44	A-7-6	CL
TP15	3.2 – 3.5	Residual - clayey sandy silt	56	20	18	9	0.33	1	28	32	39	A-7-5	MH

LL = liquid limit; PI = plasticity index; LS = linear shrinkage; GM = grading modulus, USCS = Unified Soil Classification System, AASHTO = American Association of State Highway and Transportation Officials

Table 7.2: Summary of consolidated undrained triaxial tests

Sample Type	Test pit no.	Depth (m)	Soil Description	Dry density (kg/m <sup>3</sup> )	Moisture Content (%)	Cohesion c' (kPa)	Angle of friction φ' (degrees)
Remoulded	TP01	2.1 – 2.4	Residual norite - Slightly silty sand	1801	8	5	31
Undisturbed	TP05	1.0 – 1.3	Black turf – sandy clay	1280	34	7	16
	TP14	3.1 – 3.4	Alluvium - Clayey sand	1857	13.6	10	20
	TP15	3.2 – 3.5	Residual - clayey sandy silt	1446	26	1	28

Table 7.3: Coefficient of permeability

Sample Type	Test pit no.	Depth (m)	Soil Type	Moisture Content (%)	Dry Density $\rho_d$ (kg/m <sup>3</sup> )	Coefficient of Permeability k (m/sec)
Remoulded	TP01	2.1 – 2.4	Residual norite - Slightly silty sand	6.8	1773	$6.92 \times 10^{-7}$
Undisturbed	TP05	1.0 – 1.3	Black turf – Sandy clay	34.5	1225	$3.09 \times 10^{-10}$
	TP14	3.1 – 3.4	Alluvium - Clayey sand	14.8	1841	$8.71 \times 10^{-10}$
	TP15	3.2 – 3.5	Residual - clayey sandy silt	31.1	1231	$2.25 \times 10^{-8}$

## 7.2 Shear Strength Tests

Consolidated undrained triaxial tests were carried out on undisturbed samples of the black turf, alluvium and the clayey sandy silt residual horizon encountered in test pit TP15. Due to the friable nature of the sandy residual norite sample taken from test pit TP01, triaxial specimens could not be cut and as a result, the specimens were prepared by remoulding the soil to a dry density and moisture content determined using the lump density method.

The results of the remoulded and undisturbed samples are summarised in Table 7.2 above.

The black turf and alluvium have an effective friction angle of 16 and 20 degrees and effective cohesion of 7 kPa and 10 kPa, respectively, and this agrees with the A-7-5 and MH to SC material they have been classified as belonging to.

The residual soils have an effective friction angle of 28 to 31 degrees and effective cohesion of 1 to 7 kPa. This also agrees with the typical shear strength of the material tested, which are the MH to SW-SC soil types. The cohesion of 1 kPa for the clayey sandy silt sample taken from TP15, however, is surprisingly low for the MH soil type tested.

## 7.3 Permeability Tests

The permeability of the remoulded and undisturbed samples of the selected soil horizons was determined in the flexible wall triaxial cell and their results are summarised in Table 7.3 above.

The permeability coefficients of the sandy residual norite, silty residual norite and black turf at  $10^{-7}$  m/sec,  $10^{-8}$  m/sec and  $10^{-10}$  m/sec is typical for the SW-SC and MH soil types respectively.

The alluvium has a permeability coefficient of  $10^{-10}$  m/sec and seems to be low for the material classified as an SM soil type.

# 8. SUMMARY AND RECOMMENDATIONS

## 8.1 Soil Profile

The subsoil conditions within the site as described in detail in section 6.1 are divided into two zones, namely Zone A and B.

Zone A comprises a 1,4 m thick black turf layer as soft to firm, sandy clay which overlies medium dense to very dense silty sand of residual norite origin. The latter often grades to very soft rock within which the excavator refused at depths ranging from 1,6 to 3,7 m. No refusal occurred in test pit TP12 at a depth of 5,3 m.

Zone B is characterised by a very stiff, or very dense, clayey sand horizon of alluvial origin underlain by a residual clayey sandy silt horizon of unknown origin. On the excavation face, the alluvium extends to depths of between 4,5 to 5,5 and to the south of the site is cemented with nodular and hardpan ferricrete occurring below about 1,7 m. The excavator refused on the ferricrete horizon in test pit TP19 at about 2,1 m, however, on the excavation face at point PT3, the nodular ferricrete extends to the bottom of the excavation at 5,0 m.



In places, the black turf and alluvium are overlain by fill or topsoil and/or hillwash horizons comprising mainly silty sand with abundant roots and gravels. These horizons are of variable thickness ranging from 0,1 to 1,0 m, and it is suggested that they be incorporated into those of the black turf or alluvium underlying them when carrying out a stability analysis. The pebble marker encountered beneath the alluvium is also very thin and irregular and can be ignored when in any stability analysis.

## 8.2 Design Parameters

Given the above, for design purposes, the following shear strength parameters and coefficients of permeability are considered appropriate for the in-situ soil types encountered within the site.

The nodular ferricrete occurring sporadically to the south of the site is granular and no samples could be taken to determine its shear strength parameters and permeability, however, typical values have been estimated and they are presented in Table 8.1 below.

Table 8.1: Soil engineering parameters for design

Soil Horizon	Soil description	USCS	$\phi'$ (degrees)	$c'$ (kN/m <sup>2</sup> )	$\rho_d$ (kg/m <sup>3</sup> )	k (m/sec)
Black turf	Sandy clay	MH	16	7	1300	10 <sup>-10</sup>
Alluvium	Clayey sand	SC	20	10	1800	10 <sup>-10</sup>
Nodular ferricrete	Silty/clayey gravel	GC*	35*	0*	1900*	10 <sup>-8</sup> *
Residual norite	Clayey sandy silt	MH	28	1	1400	10 <sup>-8</sup>
	Silty sand	SW-SC & SM	31	5	1800	10 <sup>-7</sup>

USCS = Unified Soil Classification System;  $\phi'$  = effective friction angle;  $c'$  = effective cohesion;  $\rho_d$  = dry density; \* = estimated; k = coefficient of permeability.

## 8.3 Stability of Waste Rock Dump

For design purposes, Table 8.1 above presents an estimate of the engineering parameters of the soils discussed in this report and the attached summary of the profiles.



Moruti Shuping B.Sc. Honours – Geotech  
for Inroads Consulting cc



Brian Harrison Pr Eng  
for Inroads Consulting cc



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## **APPENDIX A**

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### *References*

<b>Ref</b>	<b>Title</b>	<b>Author</b>	<b>Publisher</b>	<b>Year</b>
1	1:250 000 Geological Series, 2526 Rustenburg	The Chief Director Geological Survey	The Government Printer, Pretoria, South Africa	
2	Engineering Geology of Southern Africa. Volume 1.	Brink A. B. A.	Building Publications, South Africa.	1979
3	Guidelines for Soil and Rock Logging in South Africa.	Brink A. B. A. Bruin R. M. H.	Association of Engineering Geologists South Africa Section. South African Institution of Civil Engineers. South African Institute of Engineering Geologists.	2002
4	The Prediction of Heave from the Plasticity Index and Percentage Clay Fraction.	Van der Merwe D.H.	Trans. S.A. Ins. Civ. Eng. No. 6,	1964
5	Unified Soil Classification System for Engineering Purposes.	ASTM	ASTM Designation D-2487	1967
6	AASHTO – Classification of Soils and Soil-Aggregate Mixtures	AASHTO	AASHTO Designation M-145	1970



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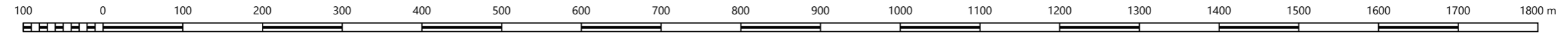
## **APPENDIX B**

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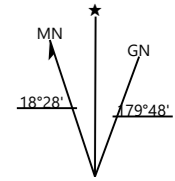
### *Site Plan*



### SITE PLAN SHOWING POSITIONS OF TEST PITS



Scale: 1 : 6000. Hartebeesthoek94 Lo27 (meters) coordinate grid at 1 km spacing.







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## APPENDIX C

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### *Photographic Record*



<b>PHOTOGRAPHIC RECORD</b>	
<b>SITE NAME: Tharisa FW WRD2</b>	
PHOTOGRAPH  <b>1</b>	
<b>Comments: View of overburden/topsoil stockpile from near TP01 looking west</b>	
PHOTOGRAPH  <b>2</b>	
<b>Comments: View of overburden/topsoil stockpile in the vicinity of TP09 looking east</b>	

**PHOTOGRAPHIC RECORD**

**SITE NAME: Tharisa FW WRD2**

PHOTOGRAPH

3



**Comments: View of mine pit from near PT2 looking north-west**

PHOTOGRAPH

4



**Comments: View of excavation face near PT3 with waste rock dump in the background looking south-east**



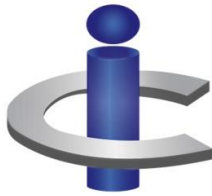


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## APPENDIX D

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### *Soil Profiles*



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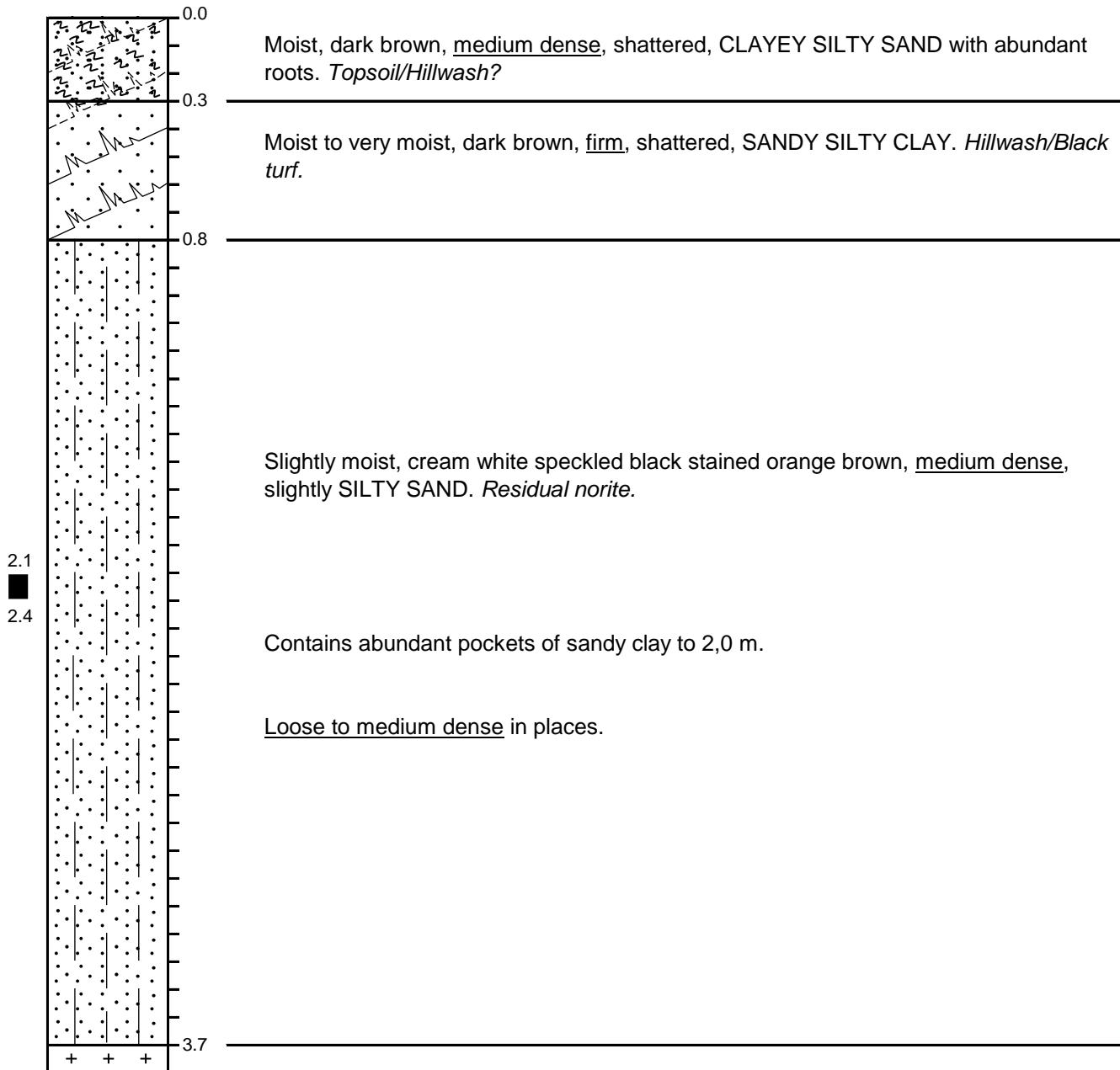
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Houghton        Fax: (011) 443 2951  
2041              e-mail: admin@inroads-sa.co.za

**PROFILE SHEET**

**TP01**

**Epoch Resources (Pty) Ltd**  
**Tharisa Far West Waste Rock Dump**

**X 2846866**  
**Y -047837**



**NOTES:**

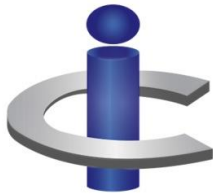
1. Bottom of hole at 3,7 m. Refusal on very soft rock norite
2. No groundwater seepage encountered.
3. Undisturbed sample taken from 2,1 to 2,4 m.
4. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: Andru Mining  
Machine: Komatsu PC300

Profiled by: MC Shuping  
Date profiled: 16-Jul-21

▽ Water seepage	■ Undisturbed sample	I Bulk sample
▼ Standing water	● Disturbed sample	┌─┐ In-situ test

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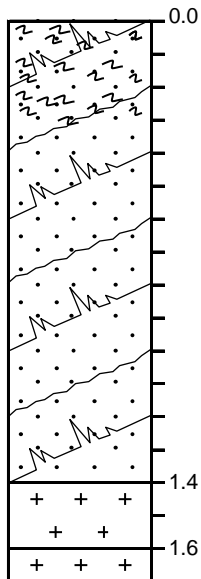
## PROFILE SHEET

TP02

Epoch Resources (Pty) Ltd  
Tharisa Far West Waste Rock Dump

X 2846715

Y -047612



Moist to very moist, dark brown grey, firm, shattered, slickensided, SANDY CLAY. *Black turf*.

Contains abundant sand and roots to 0,3 m.

Highly weathered, cream white speckled black stained light brown, coarse grained, highly to moderately fractured, very soft rock. NORITE.

### NOTES:

1. Bottom of hole at 1,6 m. Refusal on very soft rock norite
2. No groundwater seepage encountered.
3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

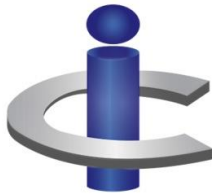
Contractor: Andru Mining  
Machine: Komatsu PC300

Profiled by: MC Shuping  
Date profiled: 16-Jul-21

Water seepage	Undisturbed sample	Bulk sample
Standing water	Disturbed sample	In-situ test

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Sheet 1 of 1



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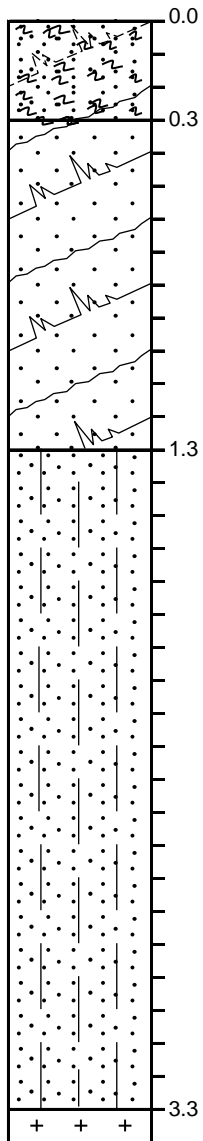
PO Box 87318    Tel: (011) 443 7811  
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**PROFILE SHEET**

**TP03**

**Epoch Resources (Pty) Ltd**  
**Tharisa Far West Waste Rock Dump**

**X 2846617**  
**Y -047439**



Moist, dark brown, medium dense to dense, shattered, CLAYEY SILTY SAND with abundant roots. *Topsoil/Hillwash?*

Moist to very moist, dark brown grey, firm, shattered, slickensided, SANDY CLAY. *Black turf.*

Contains abundant sand at base.

Slightly moist, cream white speckled black stained light brown, dense to very dense becoming very dense to very soft rock, slightly SILTY coarse SAND. *Completely weathered norite.*

Contains abundant pockets of weathered gravels and very soft rock.

Below 1,8 m appears as very dense to very soft rock.

**NOTES:**

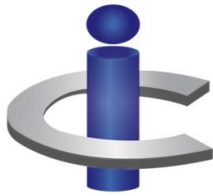
1. Bottom of hole at 3,3 m. Refusal on very soft rock norite
2. No groundwater seepage encountered.
3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: Andru Mining  
Machine: Komatsu PC300

Profiled by: MC Shuping  
Date profiled: 16-Jul-21

Water seepage	Undisturbed sample	Bulk sample
Standing water	Disturbed sample	In-situ test

**Ref: 2172/g**  
**Sheet 1 of 1**



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## PROFILE SHEET

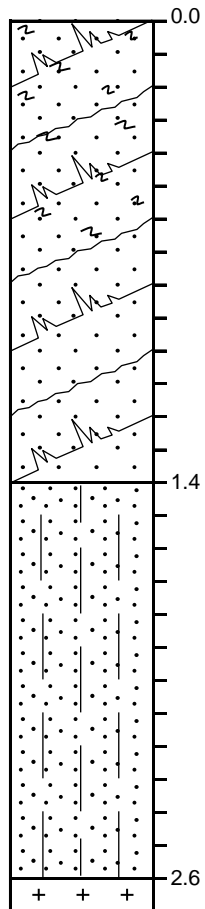
TP04

Epoch Resources (Pty) Ltd

Tharisa Far West Waste Rock Dump

X 2846549

Y -047323



Very moist, dark grey/black, soft, shattered, slickensided, SANDY CLAY with abundant roots to 0,7 m. *Black turf/Residual*.

Slightly moist, cream white speckled grey, loose to medium dense becoming dense below 2,4 m, slightly SILTY coarse SAND. *Residual norite*.

Contains abundant pockets of clay to 2,0 m.

### NOTES:

1. Bottom of hole at 2,6 m. Refusal on very soft rock norite
2. No groundwater seepage encountered.
3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: Andru Mining  
Machine: Komatsu PC300

Profiled by: MC Shuping  
Date profiled: 16-Jul-21

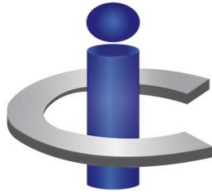
▽ Water seepage  
▼ Standing water

■ Undisturbed sample  
● Disturbed sample

I Bulk sample  
| In-situ test

Ref: 2172/g

Sheet 1 of 1



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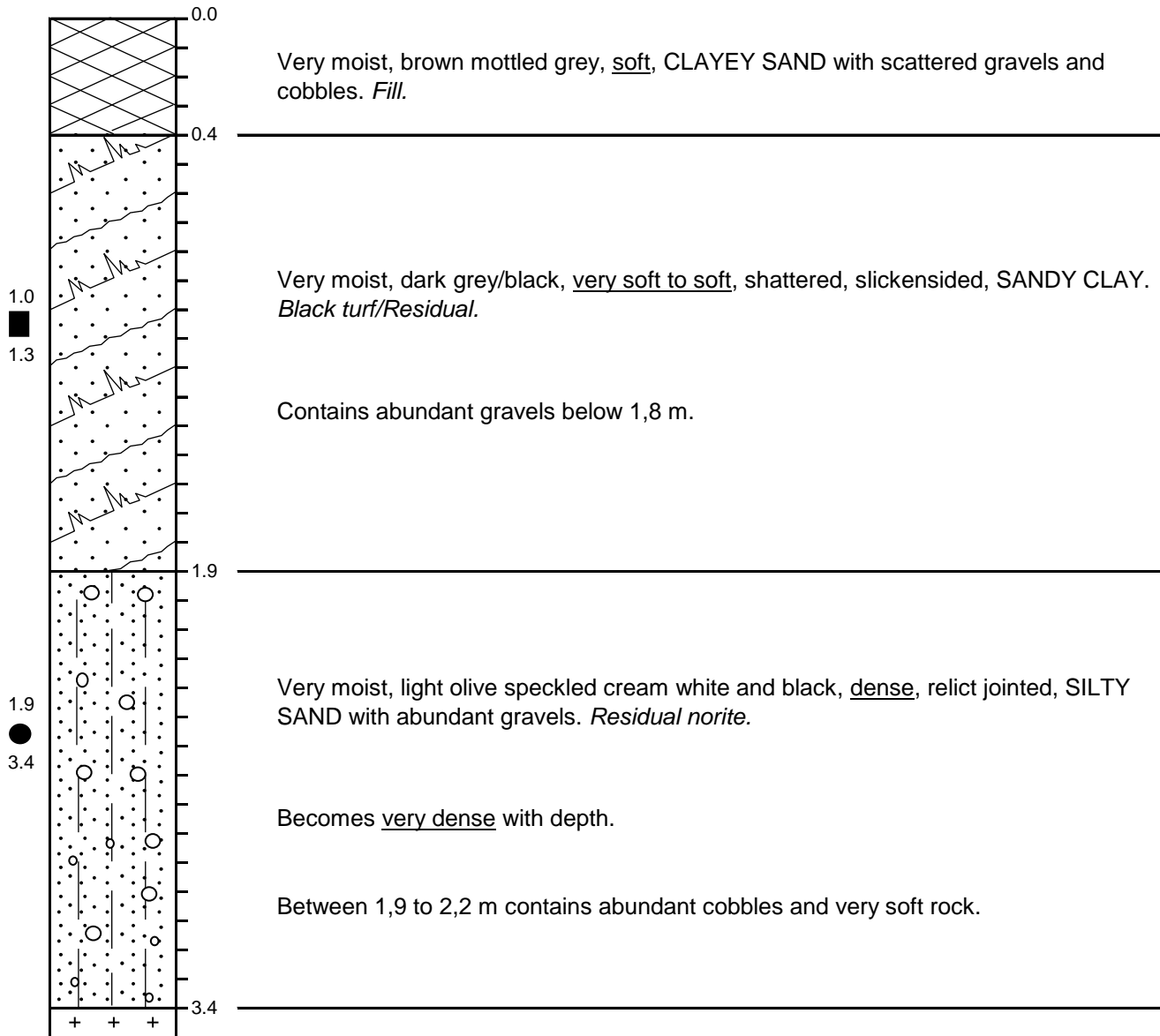
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**PROFILE SHEET**

**TP05**

**Epoch Resources (Pty) Ltd**  
**Tharisa Far West Waste Rock Dump**

**X 2846483**  
**Y -047197**



**NOTES:**

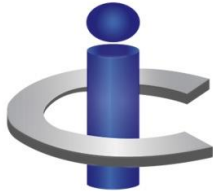
1. Bottom of hole at 3,4 m. Refusal on very soft rock norite
2. No groundwater seepage encountered.
3. Undisturbed sample taken from 1,0 to 1,3 m.
4. Disturbed sample taken from 1,9 to 3,4 m.
5. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: Andru Mining  
Machine: Komatsu PC300

Profiled by: MC Shuping  
Date profiled: 16-Jul-21

Water seepage	Undisturbed sample	Bulk sample
Standing water	Disturbed sample	In-situ test

**Ref: 2172/g**  
**Sheet 1 of 1**



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**PROFILE SHEET**

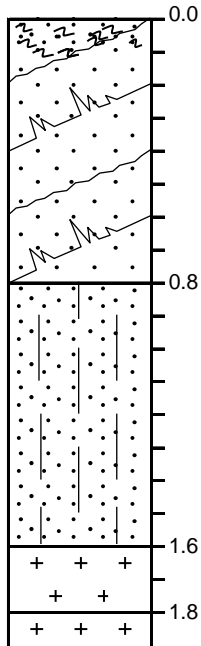
**TP06**

**Epoch Resources (Pty) Ltd**

**X 2846484**

**Tharisa Far West Waste Rock Dump**

**Y -047014**



Very moist, dark grey/black, soft to firm, shattered, slickensided, SANDY CLAY. *Black turf/Residual.*

Contains abundant sand and roots to 0,1 m.

Very moist, cream white speckled black and stained brown, medium dense, SILTY SAND with abundant very dense and friable pockets, slightly SILTY SAND. *Residual norite.*

Contains abundant clay pockets to 1,1 m.

Highly weathered, cream white speckled black and stained brown, coarse grained, highly to moderately fractured, very soft rock. NORITE.

**NOTES:**

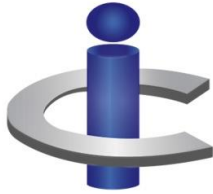
1. Bottom of hole at 1,8 m. Refusal on very soft rock norite
2. No groundwater seepage encountered.
3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: Andru Mining  
Machine: Komatsu PC300

Profiled by: MC Shuping  
Date profiled: 16-Jul-21

▽ Water seepage	■ Undisturbed sample	I Bulk sample
▼ Standing water	● Disturbed sample	┌─┐ In-situ test

**Ref: 2172/g**  
**Sheet 1 of 1**



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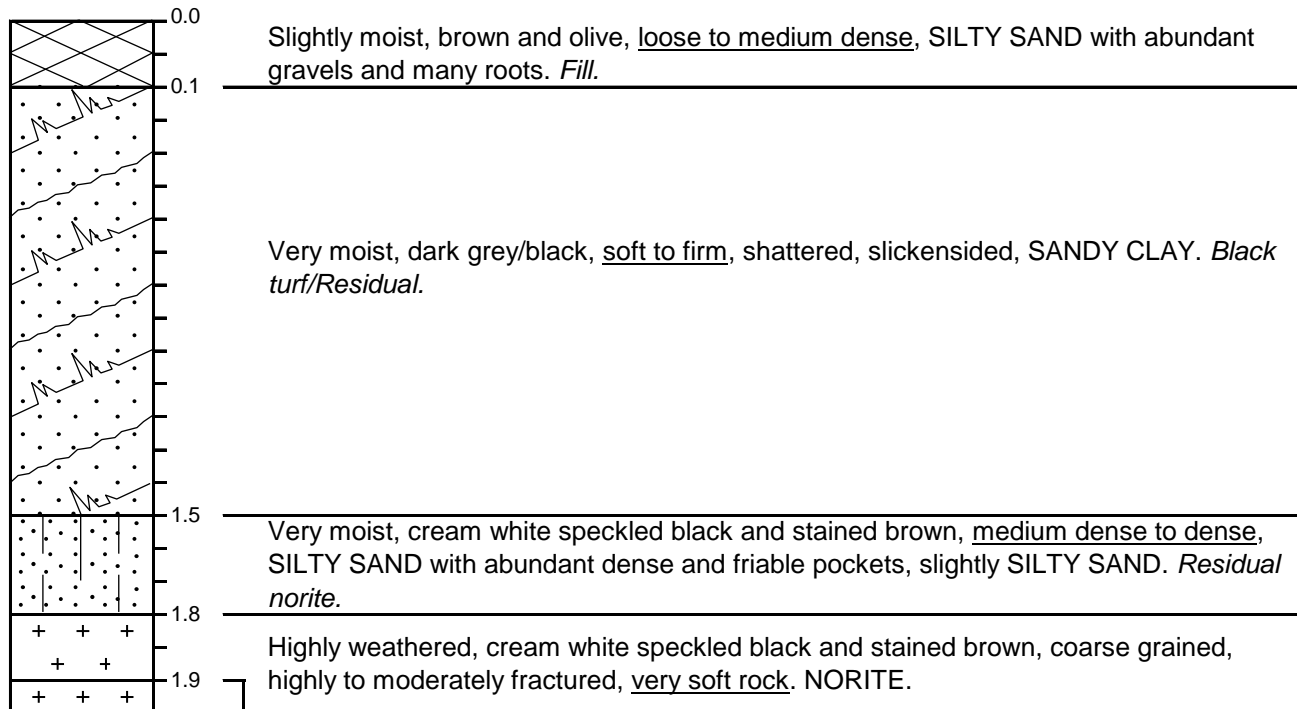
## PROFILE SHEET

TP07

Epoch Resources (Pty) Ltd  
Tharisa Far West Waste Rock Dump

X 2846526

Y -046826



### NOTES:

1. Bottom of hole at 1,9 m. Refusal on very soft rock norite
2. No groundwater seepage encountered.
3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: Andru Mining  
Machine: Komatsu PC300

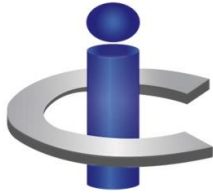
Profiled by: MC Shuping  
Date profiled: 16-Jul-21

Water seepage	Undisturbed sample	Bulk sample
Standing water	Disturbed sample	In-situ test

Ref: 2172/g

Sheet 1 of 1





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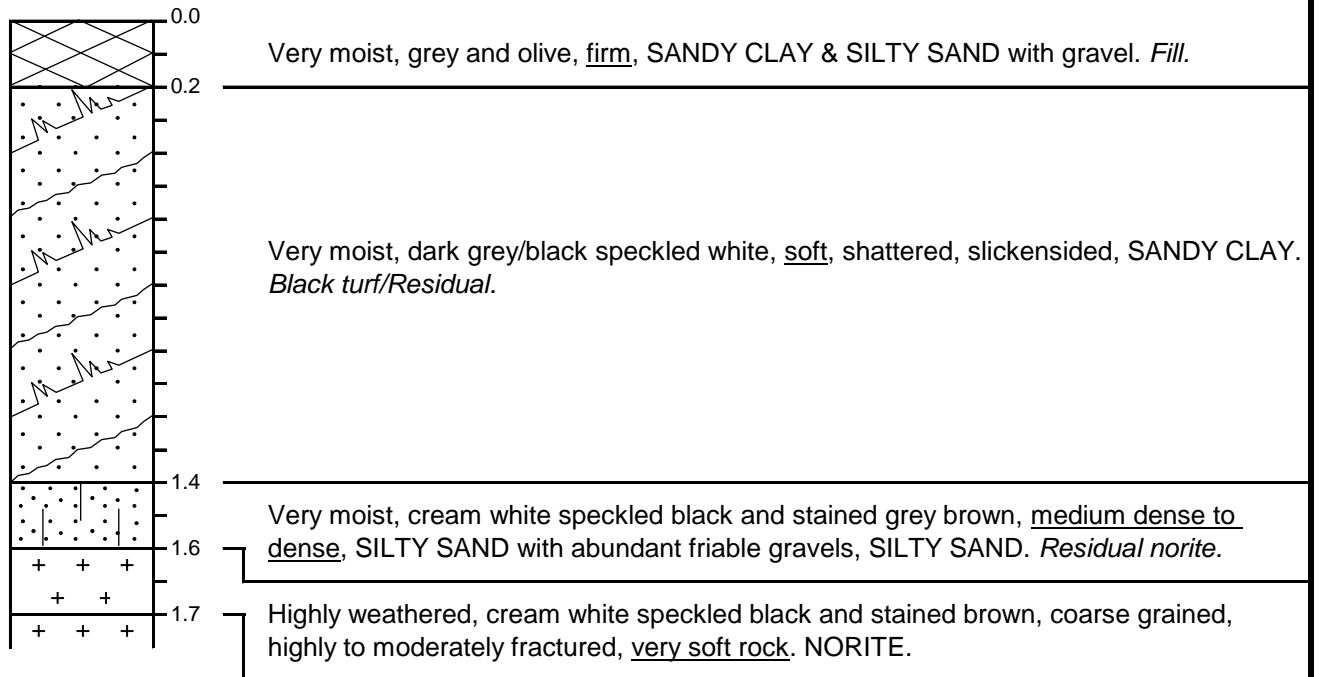
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## PROFILE SHEET

TP08

Epoch Resources (Pty) Ltd  
Tharisa Far West Waste Rock Dump

X 2846352  
Y -046775



### NOTES:

1. Bottom of hole at 1,7 m. Refusal on very soft rock norite
2. No groundwater seepage encountered.
3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

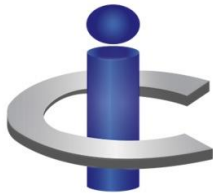
Contractor: Andru Mining  
Machine: Komatsu PC300

Profiled by: MC Shuping  
Date profiled: 16-Jul-21

Water seepage	Undisturbed sample	Bulk sample
Standing water	Disturbed sample	In-situ test

Ref: 2172/g

Sheet 1 of 1



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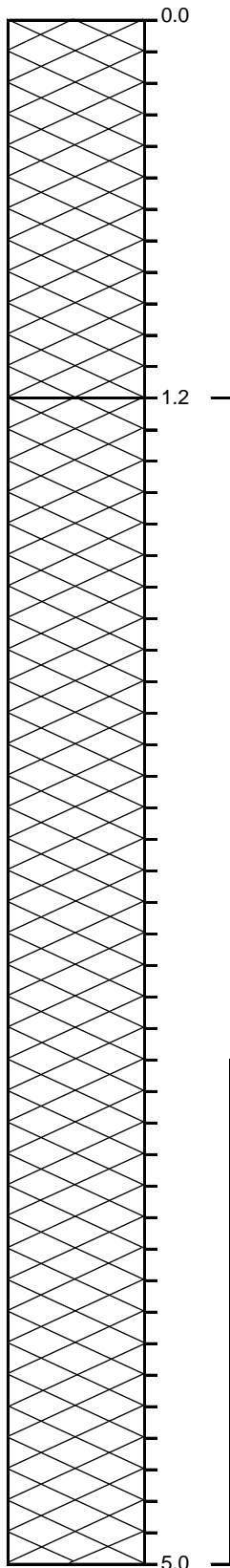
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**PROFILE SHEET**

**TP09**

**Epoch Resources (Pty) Ltd**  
**Tharisa Far West Waste Rock Dump**

**X 2846239**  
**Y -046692**



Very moist, dark grey and dark brown speckled white and mottled reddish brown, firm, SANDY CLAY with abundant sand pockets and scattered cobbles. *Fill.*

Slightly moist, light yellow olive and banded grey, loose, SILTY coarse SAND with scattered fine to coarse gravels and cobbles. *Fill.*

Contains occasional ash bands in places.

**NOTES:**

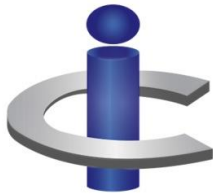
1. Bottom of hole at 5,0 m. Not to refusal.
2. No groundwater seepage encountered.
3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: Andru Mining  
Machine: Komatsu PC300

Profiled by: MC Shuping  
Date profiled: 16-Jul-21

- |                  |                      |                  |
|------------------|----------------------|------------------|
| ▽ Water seepage  | ■ Undisturbed sample | I Bulk sample    |
| ▼ Standing water | ● Disturbed sample   | ┌─┐ In-situ test |

**Ref: 2172/g**  
**Sheet 1 of 1**



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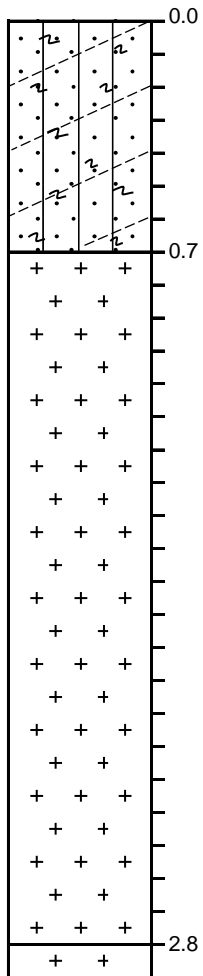
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**PROFILE SHEET**

**TP10**

**Epoch Resources (Pty) Ltd**  
**Tharisa Far West Waste Rock Dump**

**X 2846274**  
**Y -046517**



Very moist, dark brown, firm to stiff, intact, SANDY CLAYEY SILT with scattered roots. Hillwash.

Highly weathered, dark grey stained orange and light yellow, coarse grained, highly fractured, very soft to medium hard rock. Chromitite?

Contains abundant pockets of dense sand to 1,1 m.

**NOTES:**

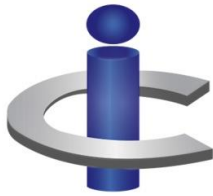
1. Bottom of hole at 2,8 m. Refusal on very soft rock chromitite
2. No groundwater seepage encountered.
3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: Andru Mining  
Machine: Komatsu PC300

Profiled by: MC Shuping  
Date profiled: 16-Jul-21

▽ Water seepage	■ Undisturbed sample	I Bulk sample
▼ Standing water	● Disturbed sample	┌─┐ In-situ test

**Ref: 2172/g**  
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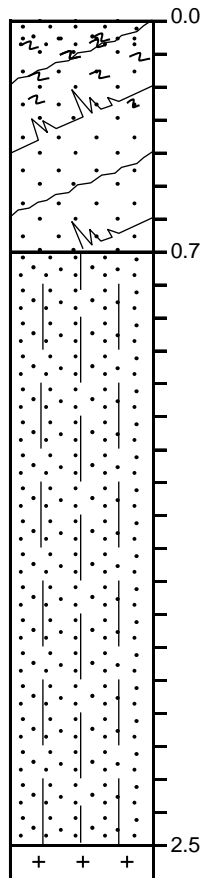
## PROFILE SHEET

TP11

Epoch Resources (Pty) Ltd  
Tharisa Far West Waste Rock Dump

X 2846342

Y -046345



Very moist, dark grey/black speckled white, soft, shattered, slickensided, SANDY CLAY. *Black turf/Residual.*

Contains abundant sand and roots to 0,3 m.

Slightly moist, cream white speckled black and stained brown, dense becoming very dense with depth, relict jointed, SILTY SAND with abundant very dense and friable pockets, slightly SILTY SAND. *Residual norite.*

### NOTES:

1. Bottom of hole at 2,5 m. Refusal on very soft rock norite
2. No groundwater seepage encountered.
3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: Andru Mining  
Machine: Komatsu PC300

Profiled by: MC Shuping  
Date profiled: 16-Jul-21

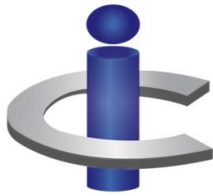
▽ Water seepage  
▼ Standing water

■ Undisturbed sample  
● Disturbed sample

I Bulk sample  
| In-situ test

Ref: 2172/g

Sheet 1 of 1



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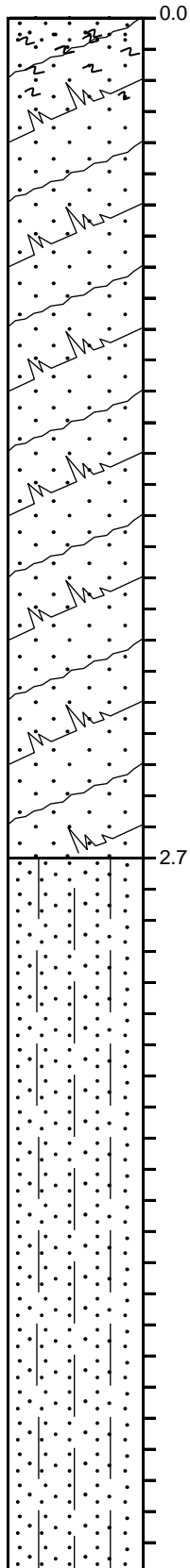
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**PROFILE SHEET**

**TP12**

**Epoch Resources (Pty) Ltd**  
**Tharisa Far West Waste Rock Dump**

**X 2846328**  
**Y -046112**



Very moist, dark grey/black, firm to stiff, shattered, slickensided, SANDY CLAY. *Black turf/Residual.*

Contains abundant sand and roots to 0,3 m.

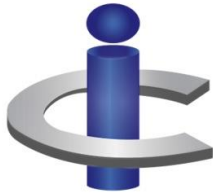
Slightly moist, cream white speckled black and stained brown, loose to medium dense becoming medium dense below 4,3 m, relict jointed, SILTY SAND . *Residual norite.*

Contractor: Andru Mining  
Machine: Komatsu PC300

Profiled by: MC Shuping  
Date profiled: 17-Jul-21

- ▽ Water seepage      ■ Undisturbed sample      I Bulk sample
- ▼ Standing water    ● Disturbed sample        | In-situ test

**Ref: 2172/g**  
**Sheet 1 of 2**



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## PROFILE SHEET

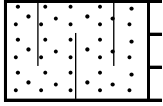
TP12 cont

Epoch Resources (Pty) Ltd

X 2846328

Tharisa Far West Waste Rock Dump

Y -046112









Slightly moist, cream white speckled black and stained brown, medium dense, relict jointed, SILTY SAND . *Residual norite*.

### NOTES:

1. Bottom of hole at 5,3 m. Not to refusal.
2. No groundwater seepage encountered.
3. Disturbed sample taken from 2,7 to 5,0 m.
4. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: Andru Mining  
Machine: Komatsu PC300

Profiled by: MC Shuping  
Date profiled: 17-Jul-21

 Water seepage    
  Undisturbed sample    
  Bulk sample  
 Standing water    
  Disturbed sample    
  *In-situ* test

Ref: 2172/g

Sheet 2 of 2



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## PROFILE SHEET

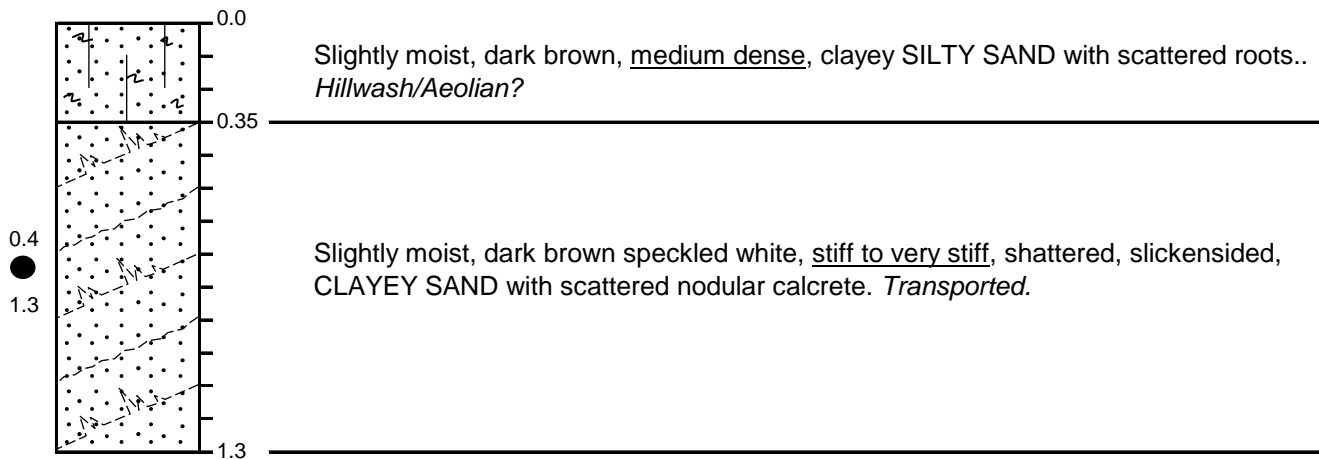
TP13

Epoch Resources (Pty) Ltd

Tharisa Far West Waste Rock Dump

X 2846395

Y -045987



### NOTES:

1. Bottom of hole at 1,3 m. Refusal on very stiff/very dense clayey sand.
2. No groundwater seepage encountered.
3. Disturbed sample taken from 0,4 to 1,3 m.
4. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: Andru Mining  
Machine: Komatsu PC300

Profiled by: MC Shuping  
Date profiled: 17-Jul-21

▽ Water seepage

■ Undisturbed sample

I Bulk sample

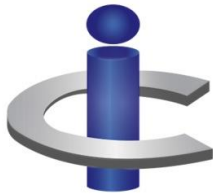
Ref: 2172/g

▼ Standing water

● Disturbed sample

┌─┐ In-situ test

Sheet 1 of 1



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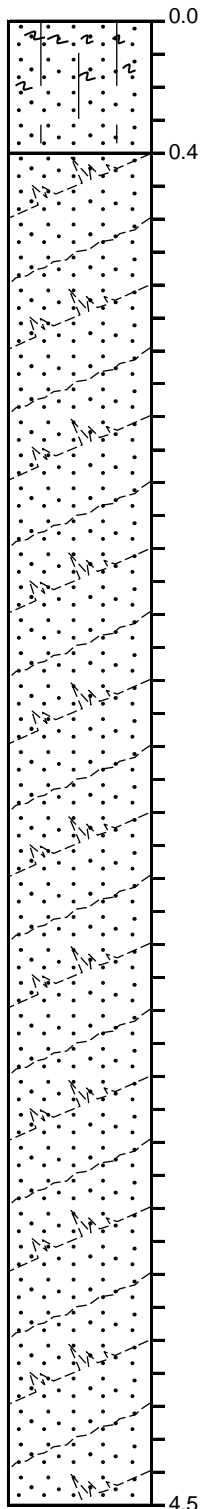
**PROFILE SHEET**

**TP14**

**Epoch Resources (Pty) Ltd**  
**Tharisa Far West Waste Rock Dump**

**X 2846421**

**Y -045837**



Slightly moist, light brown, loose, SILTY SAND with many roots. *Hillwash/Aeolian?*

Contains abundant roots to 0,1 m.

Slightly moist becoming moist with depth, brown becoming mottled orange brown and light grey below 0,6 m, stiff to very stiff, shattered, slickensided, CLAYEY SAND with scattered ferruginous nodules. *Alluvium?*

Becomes firm to stiff below 2,0 m.

**NOTES:**

1. Bottom of hole at 4,5 m. Not to refusal.
2. No groundwater seepage encountered.
3. Undisturbed sample taken from 3,1 to 3,4 m.
4. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

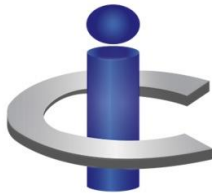
Contractor: Andru Mining  
Machine: Komatsu PC300

Profiled by: MC Shuping  
Date profiled: 17-Jul-21

▽ Water seepage	■ Undisturbed sample	┄ Bulk sample
▼ Standing water	● Disturbed sample	┄┄ In-situ test

**Ref: 2172/g**  
**Sheet 1 of 1**





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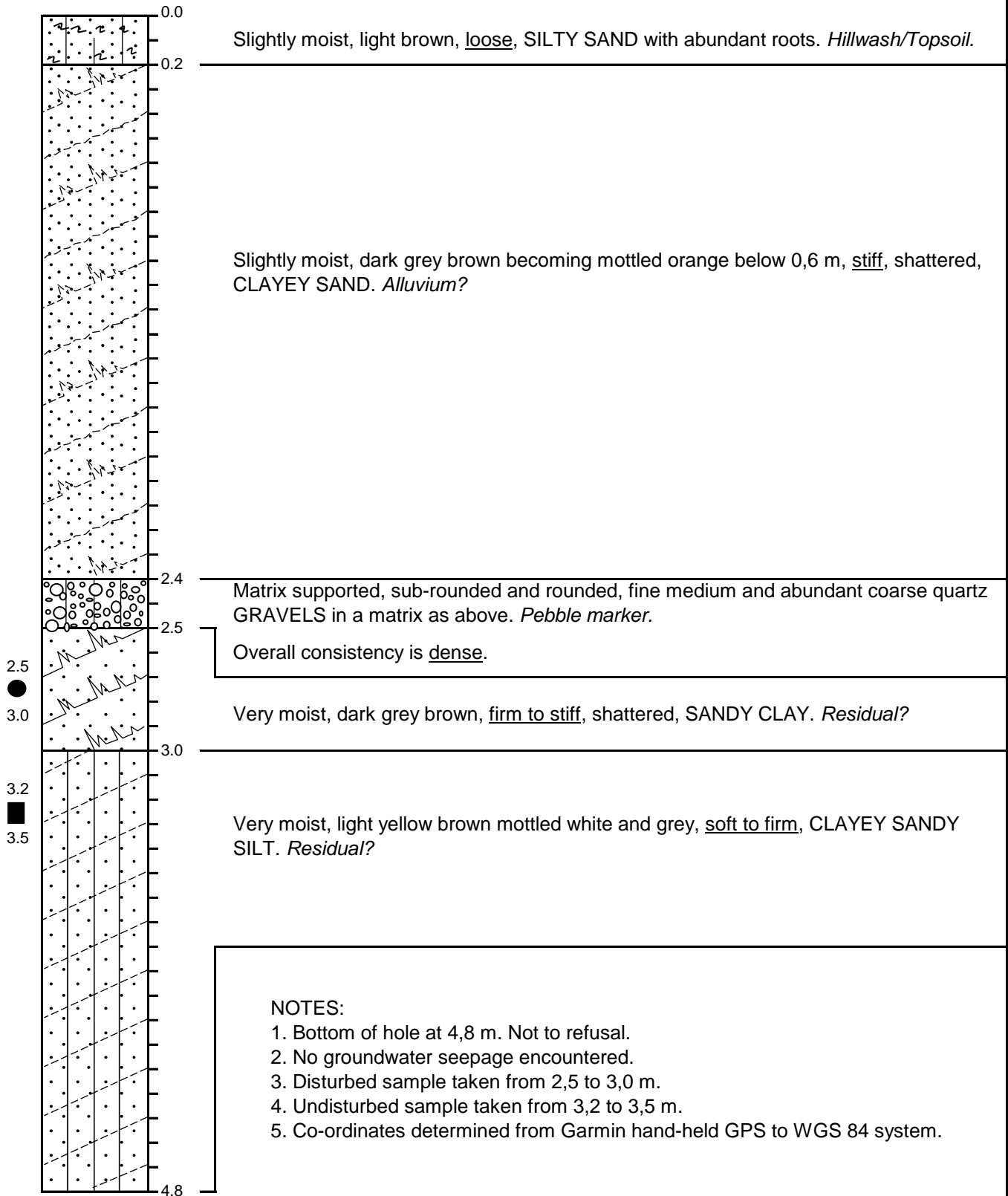
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**PROFILE SHEET**

**TP15**

**Epoch Resources (Pty) Ltd**  
**Tharisa Far West Waste Rock Dump**

**X 2846532**  
**Y -045629**

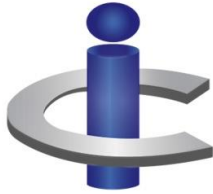


Contractor: Andru Mining  
Machine: Komatsu PC300

Profiled by: MC Shuping  
Date profiled: 17-Jul-21

- ▽ Water seepage
- ▼ Standing water
- Undisturbed sample
- Disturbed sample
- ┌ Bulk sample
- └ In-situ test

**Ref: 2172/g**  
**Sheet 1 of 1**



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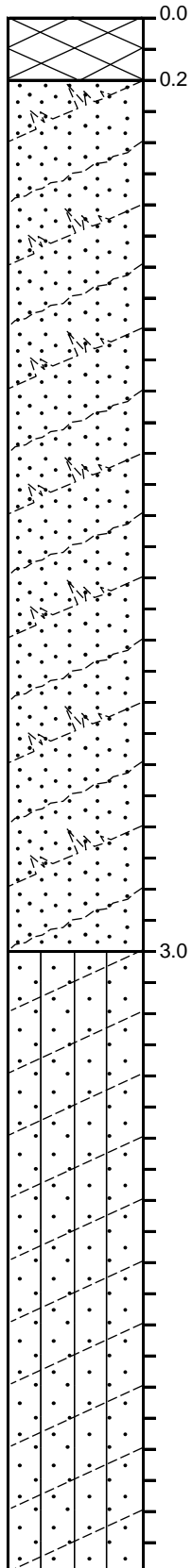
**PROFILE SHEET**

**TP16**

**Epoch Resources (Pty) Ltd**  
**Tharisa Far West Waste Rock Dump**

**X 2846744**

**Y -045656**



Clast supported, sub-angular and angular, fine medium and abundantly coarse quartz GRAVELS in a matrix of dry, brown silty sand. *Fill.*

Overall consistency is medium dense.

Moist becoming very moist with depth, dark brown mottled dark grey, stiff to very stiff, shattered, slickensided, CLAYEY SAND. *Alluvium?*

Contains scattered rounded cobbles at about 1,0 m.

Becomes firm to stiff below 2,0 m.

Very moist, light orange brown mottled cream white and brown, firm, pinholed, CLAYEY SANDY SILT. *Residual?*

Becomes light yellow brown with depth.

Becomes more sandy with depth.

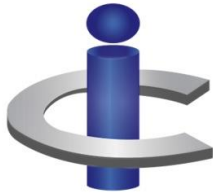
Contractor: Andru Mining  
Machine: Komatsu PC300

Profiled by: MC Shuping  
Date profiled: 17-Jul-21

- ▽ Water seepage      ■ Undisturbed sample      I Bulk sample
- ▼ Standing water    ● Disturbed sample        | In-situ test

**Ref: 2172/g**

**Sheet 1 of 2**



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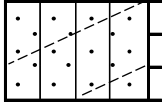
## PROFILE SHEET

TP16 cont

Epoch Resources (Pty) Ltd  
Tharisa Far West Waste Rock Dump

X 2846744

Y -045656









Very moist, light orange brown mottled cream white and brown, firm, pinholed, CLAYEY SANDY SILT. *Residual?*

### NOTES:

1. Bottom of hole at 5,3 m. Not to refusal.
2. No groundwater seepage encountered.
3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

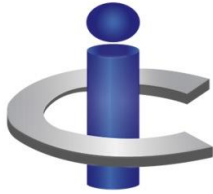
Contractor: Andru Mining  
Machine: Komatsu PC300

Profiled by: MC Shuping  
Date profiled: 17-Jul-21

 Water seepage    
  Undisturbed sample    
  Bulk sample  
 Standing water    
  Disturbed sample    
  In-situ test

Ref: 2172/g

Sheet 2 of 2



INROADS CONSULTING  
CIVIL ENGINEERS

PO Box 87318  
Houghton  
2041

Tel: (011) 443 7811  
Fax: (011) 443 2951  
e-mail: admin@inroads-sa.co.za

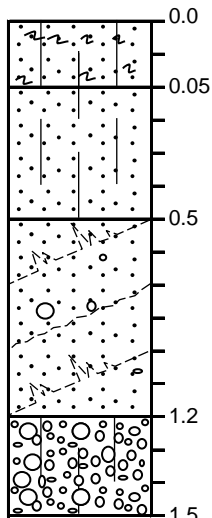
## PROFILE SHEET

TP17

Epoch Resources (Pty) Ltd  
Tharisa Far West Waste Rock Dump

X 2846897

Y -045730



Dry, dark grey brown, loose to medium dense, SILTY SAND with many roots. *Topsoil.*

Slightly moist, dark brown, dense, pinholed and shattered, slightly clayey SILTY SAND. *Hillwash/Aeolian?*

Slightly moist, dark brown mottled dark orange, stiff to very stiff, CLAYEY SAND with abundant gravels. *Alluvium?*

Clast and matrix supported, rounded, fine medium and coarse GRAVELS in a matrix as above. *Pebble marker.*







Overall consistency is very dense.

### NOTES:

1. Bottom of hole at 1,5 m. Refusal on very dense tightly packed cobbles and gravels.
2. No groundwater seepage encountered.
3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

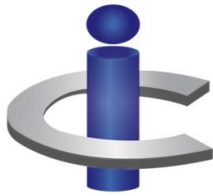
Contractor: Andru Mining  
Machine: Komatsu PC300

Profiled by: MC Shuping  
Date profiled: 17-Jul-21

 Water seepage    
  Undisturbed sample    
  Bulk sample  
 Standing water    
  Disturbed sample    
  In-situ test

Ref: 2172/g

Sheet 1 of 1



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## PROFILE SHEET

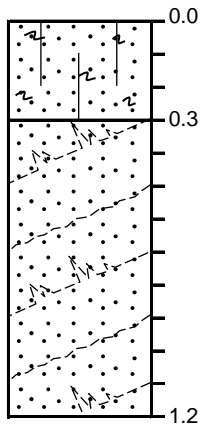
TP18

Epoch Resources (Pty) Ltd

Tharisa Far West Waste Rock Dump

X 2846932

Y -045865



Dry, dark brown, medium dense to dense, pinholed, slightly clayey SILTY SAND with abundant coarse roots. *Hillwash/Aeolian?*

Slightly moist, dark brown mottled dark orange, stiff to very stiff, CLAYEY SAND.  
*Alluvium?*

### NOTES:

1. Bottom of hole at 1,2 m. Refusal on very stiff/very dense clayey sand.
2. No groundwater seepage encountered.
3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

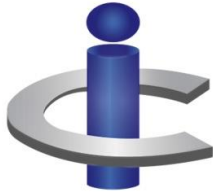
Contractor: Andru Mining  
Machine: Komatsu PC300

Profiled by: MC Shuping  
Date profiled: 17-Jul-21

Water seepage    
 Undisturbed sample    
 Bulk sample  
 Standing water    
 Disturbed sample    
 In-situ test

Ref: 2172/g

Sheet 1 of 1



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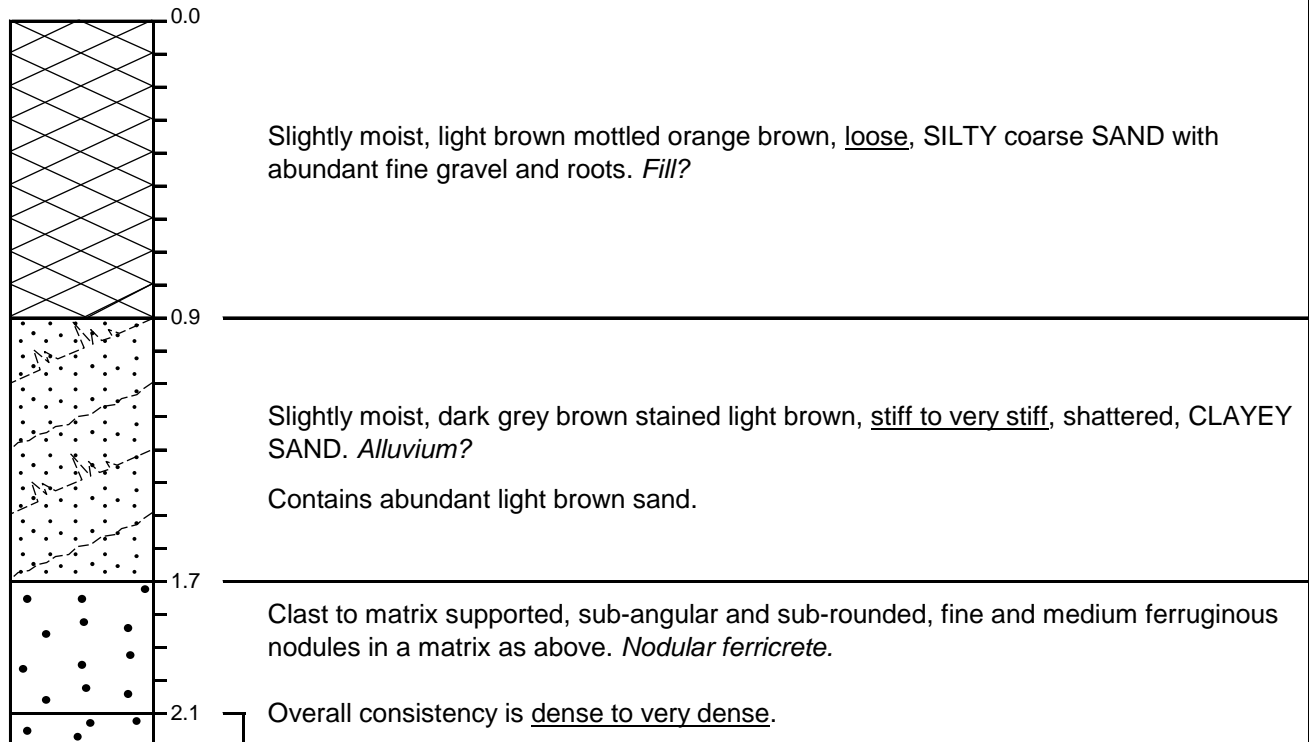
## PROFILE SHEET

TP19

Epoch Resources (Pty) Ltd  
Tharisa Far West Waste Rock Dump

X 2847108

Y -046109









### NOTES:

1. Bottom of hole at 2,1 m. Refusal on very stiff/very dense clayey sand and abundant ferruginous nodules.
2. No groundwater seepage encountered.
3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: Andru Mining  
Machine: Komatsu PC300

Profiled by: MC Shuping  
Date profiled: 17-Jul-21

 Water seepage    
  Undisturbed sample    
  Bulk sample  
 Standing water    
  Disturbed sample    
  *In-situ* test

Ref: 2172/g

Sheet 1 of 1



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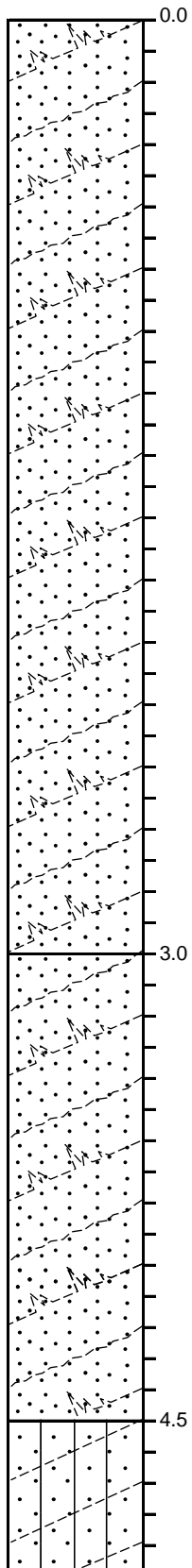
**PROFILE SHEET**

**PT1**

**Epoch Resources (Pty) Ltd**  
**Tharisa Far West Waste Rock Dump**

**X 2846909**

**Y -045885**



Dry, brown, shattered, CLAYEY SAND. *Alluvium.*

Dry, brown, shattered, CLAYEY SAND/SANDY CLAY. *Alluvium.*

Dry, brown and orange brown, CLAYEY SANDY SILT. *Residual.*

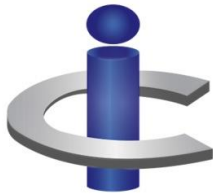
Contractor: -  
Machine: -

Profiled by: MC Shuping  
Date profiled: 17-Jul-21

- ▽ Water seepage      ■ Undisturbed sample      I Bulk sample
- ▼ Standing water    ● Disturbed sample        | In-situ test

**Ref: 2172/g**

**Sheet 1 of 2**



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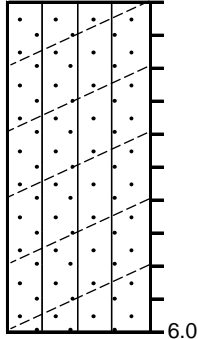
## PROFILE SHEET

PT1 cont

Epoch Resources (Pty) Ltd  
Tharisa Far West Waste Rock Dump

X 2846909

Y -045885



Dry, brown and orange brown, CLAYEY SANDY SILT. *Residual.*

### NOTES:

1. Bottom of hole at 6,0 m. Gravels and highly weathered rock encountered at the bottom of the excavation.
2. Co-ordinates determined from Garmin hand-held GPS to (WGS 84) system.

Contractor: -  
Machine: -

Profiled by: MC Shuping  
Date profiled: 17-Jul-21

▽ Water seepage  
▼ Standing water

■ Undisturbed sample  
● Disturbed sample

┆ Bulk sample  
┆ In-situ test

Ref: 2172/g

Sheet 2 of 2





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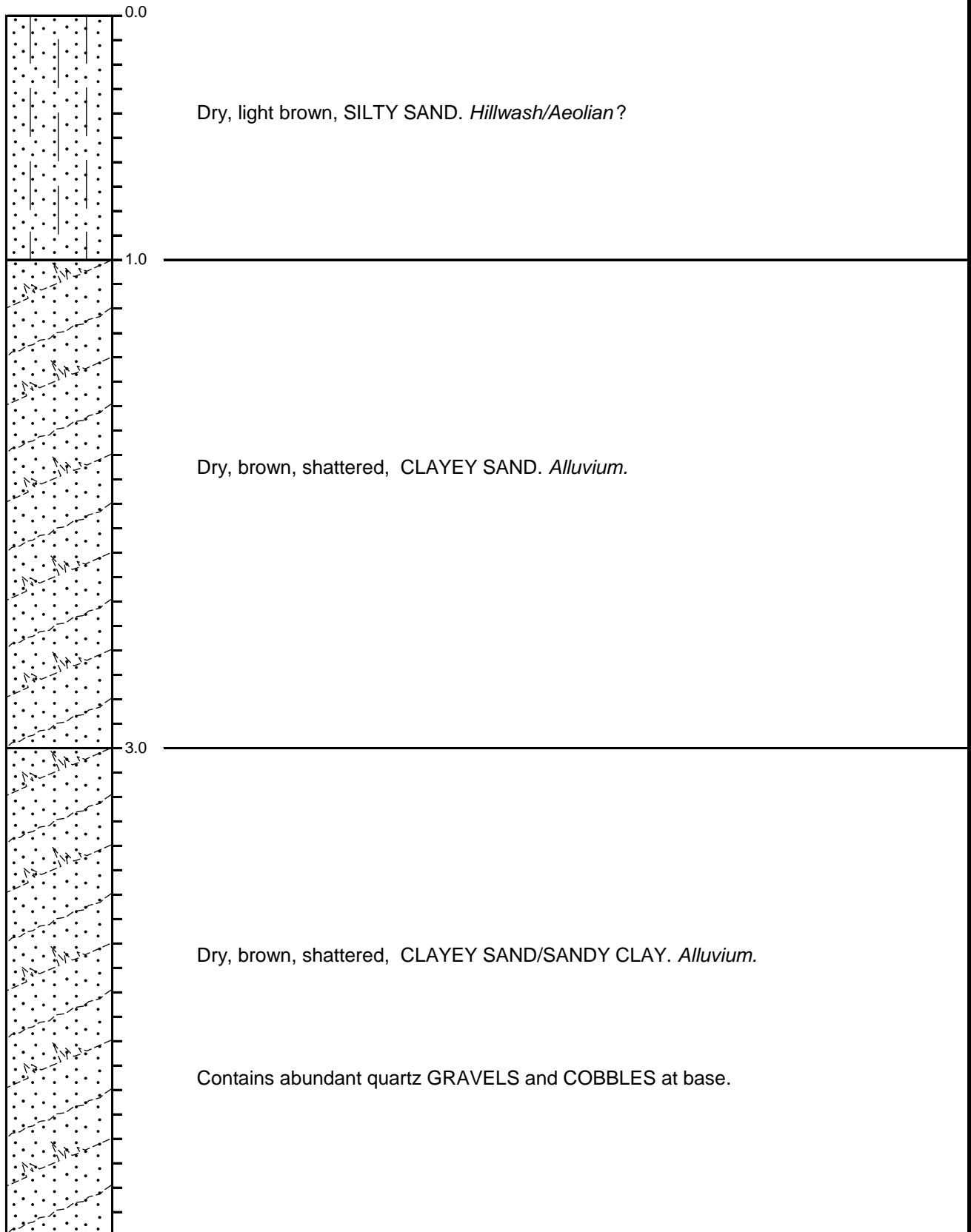
**PROFILE SHEET**

**PT2**

**Epoch Resources (Pty) Ltd**  
**Tharisa Far West Waste Rock Dump**

**X 2847032**

**Y -045978**



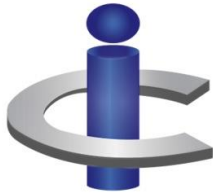
Contractor: -  
Machine: -

Profiled by: MC Shuping  
Date profiled: 17-Jul-21

- ▽ Water seepage      ■ Undisturbed sample      I Bulk sample
- ▼ Standing water    ● Disturbed sample        H In-situ test

**Ref: 2172/g**

**Sheet 1 of 2**



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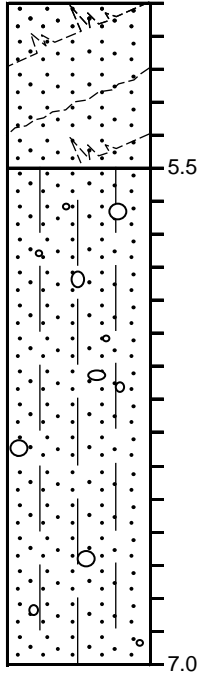
PO Box 87318      Tel: (011) 443 7811  
Houghton          Fax: (011) 443 2951  
2041                e-mail: admin@inroads-sa.co.za

**PROFILE SHEET**

**PT2 cont**

**Epoch Resources (Pty) Ltd**  
**Tharisa Far West Waste Rock Dump**

**X 2847032**  
**Y -045978**



Dry, orange brown, SILTY SAND with scattered gravels. *Residual?*

**NOTES:**

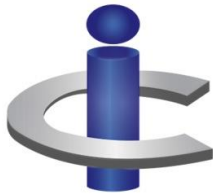
1. Bottom of hole at 7,0 m. Gravels and highly weathered rock encountered at the bottom of the excavation.
2. Co-ordinates determined from Garmin hand-held GPS to (WGS 84) system.

Contractor: -  
Machine: -

Profiled by: MC Shuping  
Date profiled: 17-Jul-21

Water seepage	Undisturbed sample	Bulk sample
Standing water	Disturbed sample	In-situ test

**Ref: 2172/g**  
**Sheet 2 of 2**



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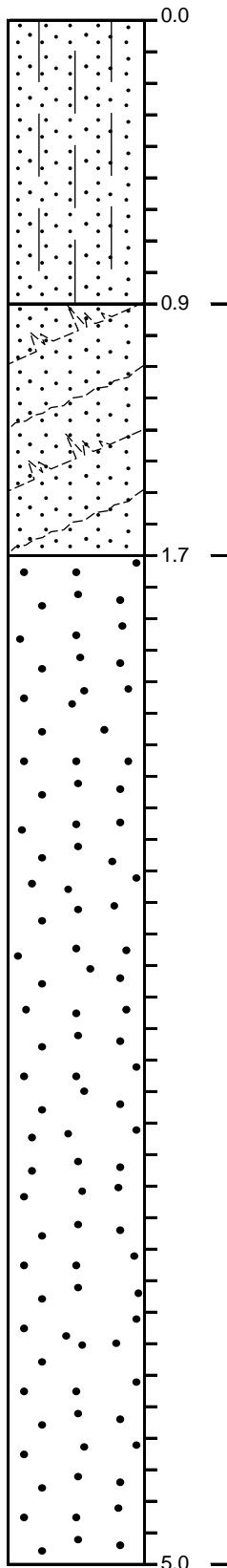
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Houghton        Fax: (011) 443 2951  
2041              e-mail: admin@inroads-sa.co.za

**PROFILE SHEET**

**PT3**

**Epoch Resources (Pty) Ltd**  
**Tharisa Far West Waste Rock Dump**

**X 2847040**  
**Y -046180**



Dry, light brown, SILTY SAND. *Hillwash.*

Dry, brown, shattered, CLAYEY SAND with abundant ferruginous nodules. *Alluvium.*

Clast supported, sub-rounded and rounded, ferruginous NODULES in a matrix of clayey sand. *Nodular ferricrete.*

Grades to weak hardpan ferricrete.

**NOTES:**

1. Bottom of hole at 5,0 m. Weakly cemented ferricrete encountered at the bottom of the excavation.
2. Co-ordinates determined from Garmin hand-held GPS to (WGS 84) system.

Contractor: -  
Machine: -

Profiled by: MC Shuping  
Date profiled: 17-Jul-21

- |                |                    |              |
|----------------|--------------------|--------------|
| Water seepage  | Undisturbed sample | Bulk sample  |
| Standing water | Disturbed sample   | In-situ test |

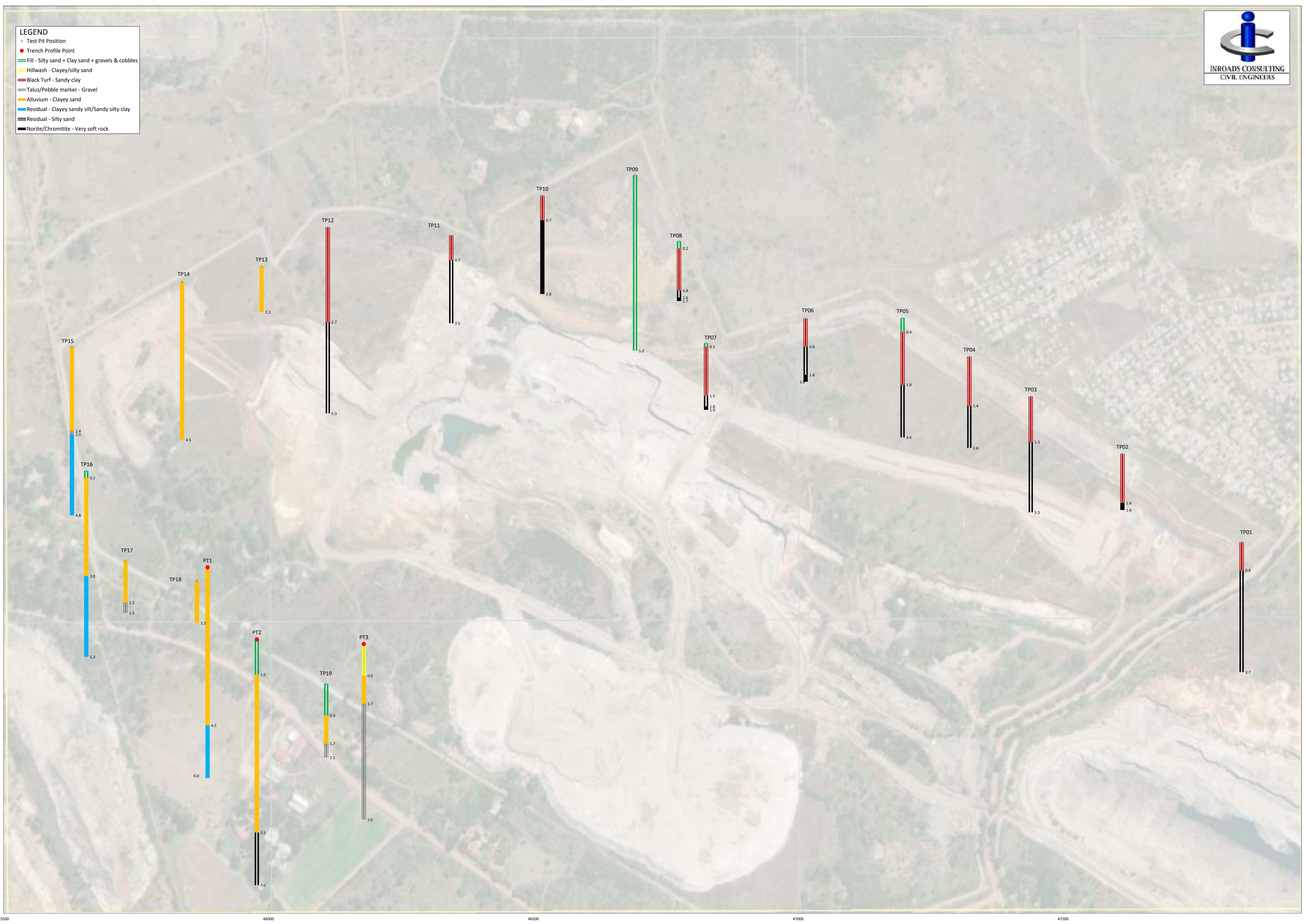
**Ref: 2172/g**  
**Sheet 1 of 1**



# Summary of Profiles - Tharisa FW WRD2



- LEGEND**
- + Test Pit Position
  - Trench Profile Point
  - █ Fill - Silty sand + Clay sand + gravels & cobbles
  - █ Hillwash - Clayey/silty sand
  - █ Black Turf - Sandy clay
  - █ Talus/Pebble marker - Gravel
  - █ Alluvium - Clayey sand
  - █ Residual - Clayey sandy silt/Sandy silty clay
  - █ Residual - Silty sand
  - █ Norite/Chromitite - Very soft rock











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## **APPENDIX E**



---

### *Photographs of Test Pits*



<b>TEST PIT PHOTOGRAPHS</b>	
<b>SITE NAME:</b> Tharisa FW WRD	
<b>PHOTOGRAPH</b>  <b>1</b>	
<b>Comments: TP01</b>	
<b>PHOTOGRAPH</b>  <b>2</b>	
<b>Comments: TP02</b>	


<b>TEST PIT PHOTOGRAPHS</b>	
<b>SITE NAME:</b> Tharisa FW WRD	
PHOTOGRAPH  <b>3</b>	
<b>Comments:</b> TP03	
PHOTOGRAPH  <b>4</b>	
<b>Comments:</b> TP04	




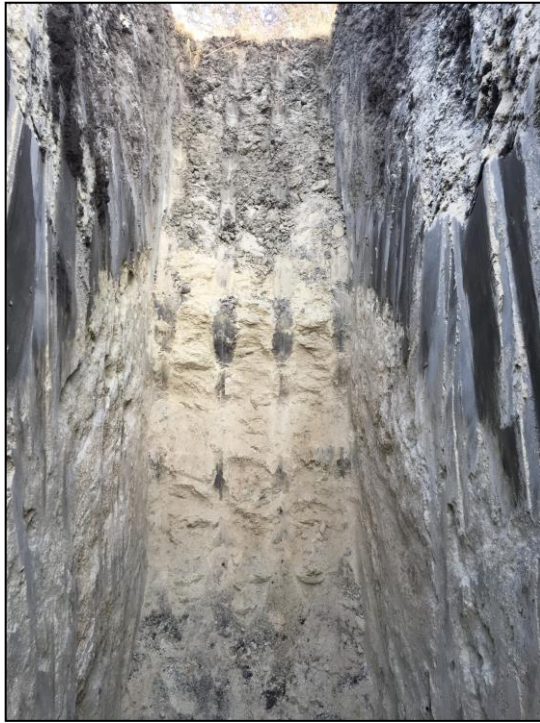
<b>TEST PIT PHOTOGRAPHS</b>	
<b>SITE NAME: Tharisa FW WRD</b>	
<p>PHOTOGRAPH</p> <p style="font-size: 1.2em;"><b>5</b></p>	
<p><b>Comments: TP05</b></p>	
<p>PHOTOGRAPH</p> <p style="font-size: 1.2em;"><b>6</b></p>	
<p><b>Comments: TP06</b></p>	





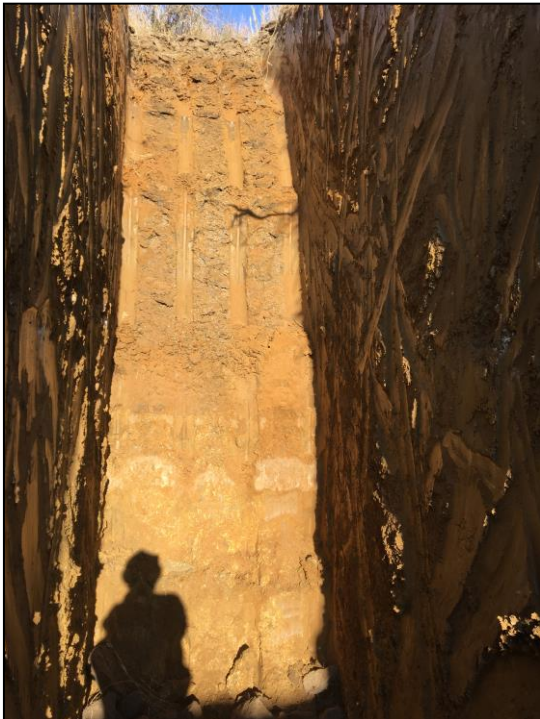

<b>TEST PIT PHOTOGRAPHS</b>	
<b>SITE NAME: Tharisa FW WRD</b>	
<p>PHOTOGRAPH</p> <p style="font-size: 24px; font-weight: bold;">7</p>	
<p><b>Comments: TP07</b></p>	
<p>PHOTOGRAPH</p> <p style="font-size: 24px; font-weight: bold;">8</p>	
<p><b>Comments: TP08</b></p>	

<b>TEST PIT PHOTOGRAPHS</b>	
<b>SITE NAME: Tharisa FW WRD</b>	
<p>PHOTOGRAPH</p> <p><b>9</b></p>	
<p><b>Comments: TP09</b></p>	
<p>PHOTOGRAPH</p> <p><b>10</b></p>	
<p><b>Comments: TP10</b></p>	





<b>TEST PIT PHOTOGRAPHS</b>	
<b>SITE NAME: Tharisa FW WRD</b>	
<p>PHOTOGRAPH</p> <p><b>11</b></p>	
<p><b>Comments: TP11</b></p>	
<p>PHOTOGRAPH</p> <p><b>12</b></p>	
<p><b>Comments: TP12</b></p>	

<b>TEST PIT PHOTOGRAPHS</b>	
<b>SITE NAME: Tharisa FW WRD</b>	
<b>PHOTOGRAPH</b>  <b>13</b>	
<b>Comments: TP13</b>	
<b>PHOTOGRAPH</b>  <b>14</b>	
<b>Comments: TP14</b>	

<b>TEST PIT PHOTOGRAPHS</b>	
<b>SITE NAME: Tharisa FW WRD</b>	
<b>PHOTOGRAPH</b>  <b>15</b>	
<b>Comments: TP15</b>	
<b>PHOTOGRAPH</b>  <b>16</b>	
<b>Comments: TP16</b>	



<b>TEST PIT PHOTOGRAPHS</b>	
<b>SITE NAME: Tharisa FW WRD</b>	
<b>PHOTOGRAPH</b>  <b>17</b>	
<b>Comments: TP17</b>	
<b>PHOTOGRAPH</b>  <b>18</b>	
<b>Comments: TP18</b>	


**TEST PIT PHOTOGRAPHS**

**SITE NAME: Tharisa FW WRD**

<p>PHOTOGRAPH</p> <p><b>19</b></p>	 A vertical photograph of a test pit. The top edge shows some dry grass and soil. Below that is a layer of light-colored, silty soil. Further down, there are several distinct, darker, and more textured layers, possibly representing different soil types or geological strata. The pit is narrow and appears to be hand-dug.
------------------------------------	--



**Comments: TP19**



<p>PHOTOGRAPH</p> <p><b>20</b></p>	 A photograph of a soil bank or embankment. The soil is a reddish-brown color and shows vertical erosion marks. A yellow measuring tape is stretched vertically against the face of the bank to indicate its height. The top of the bank is uneven, and some trees are visible in the background under a clear blue sky.
------------------------------------	--

**Comments: PT1**



<b>TEST PIT PHOTOGRAPHS</b>	
<b>SITE NAME: Tharisa FW WRD</b>	
<p>PHOTOGRAPH</p> <p><b>21</b></p>	
<p><b>Comments: PT2</b></p>	
<p>PHOTOGRAPH</p> <p><b>22</b></p>	
<p><b>Comments: PT3</b></p>	





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## APPENDIX F

---

### *Laboratory Test Results*



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 P.O Box 912387, Silverton, 0127  
 Tel. : (012) 800 1299  
 Fax :  
 Email : martinus.schwartz@sgs.com

**TEST RESULTS**

EPOCH RESOURCES  
 Attention: Mr Stephan Brakhuizen

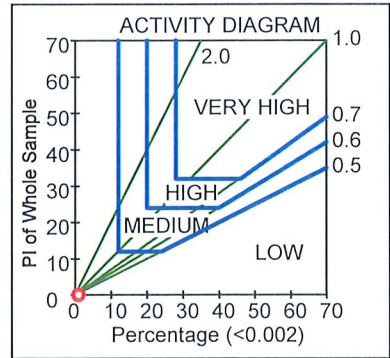
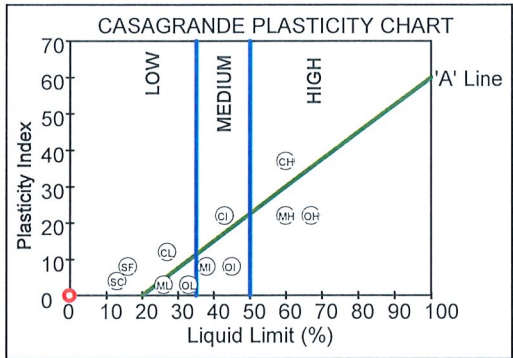
Project : Tharisa FW WRD2  
 Your Ref :  
 Our Ref : PL/45639  
 Date Reported : 21.09.2021

**FOUNDATION INDICATOR (ASTM: D422)**

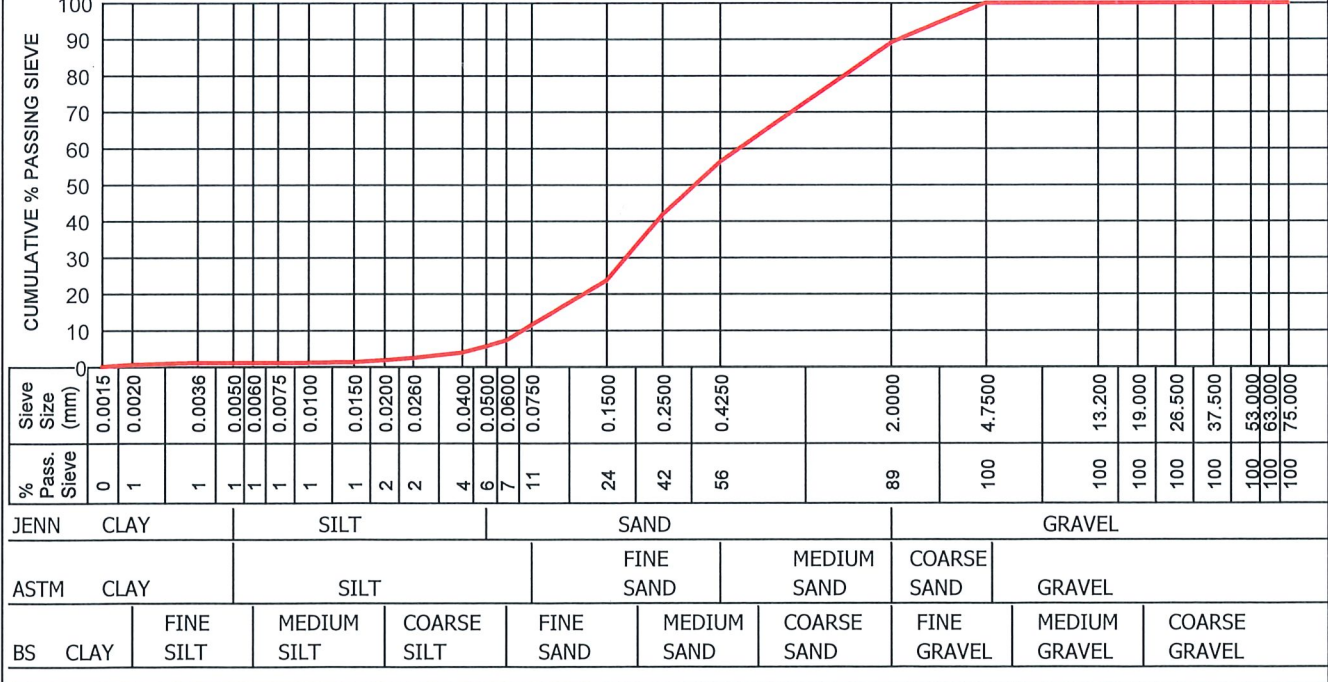
Sample No. : A21/2950(G21-056)  
 Hole No. : TP01  
 Depth : 2100-2400  
 Liquid Limit (%) : -  
 Plasticity Index : NP  
 Linear Shrinkage (%) : 0.0  
 PI of Whole Sample : 0  
 P.R.A. Classification : A-2-4(0)  
 Unified Soil Classificati: SW-SC  
 Activity : 0.00  
 Heave Classification : LOW  
 Grading Modulus : 1.44  
 Percentage (<0.002) : 1.0  
 Moisture Content (%) : 15.8

Material Description : SAND

	Clay (%)	Silt (%)	Sand (%)	Gravel (%)	Classification
Jennings	1.1	4.4	83.6	10.9	SAND
Astm	1.1	10.3	88.6	0.0	SAND
British Standard	0.6	6.7	81.9	10.9	SAND



**PARTICLE SIZE DISTRIBUTION**



Remarks : Sampled by client.  
 FORM: A6  
 4.4.1(SGS)(2019.12.04) Technical Signatory : Martinus Schwartz/Lizette Breiting

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 Fax :  
 Email : martinus.schwartz@sgs.com

**TEST RESULTS**

EPOCH RESOURCES  
 Attention: Mr Stephan Brakhuizen

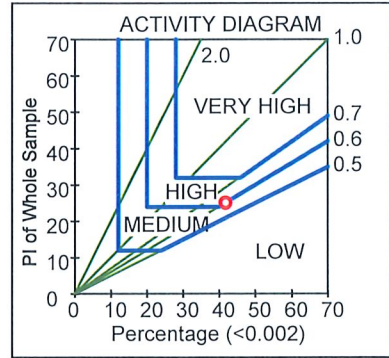
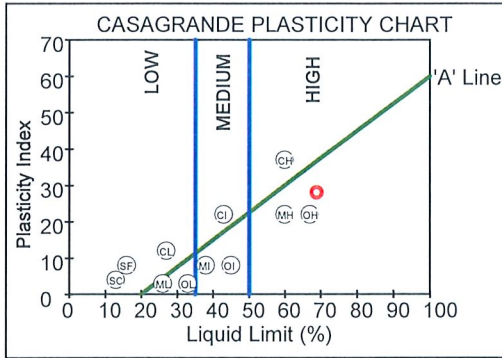
Project : Tharisa FW WRD2  
 Your Ref :  
 Our Ref : PL/45639  
 Date Reported : 21.09.2021

**FOUNDATION INDICATOR (ASTM: D422)**

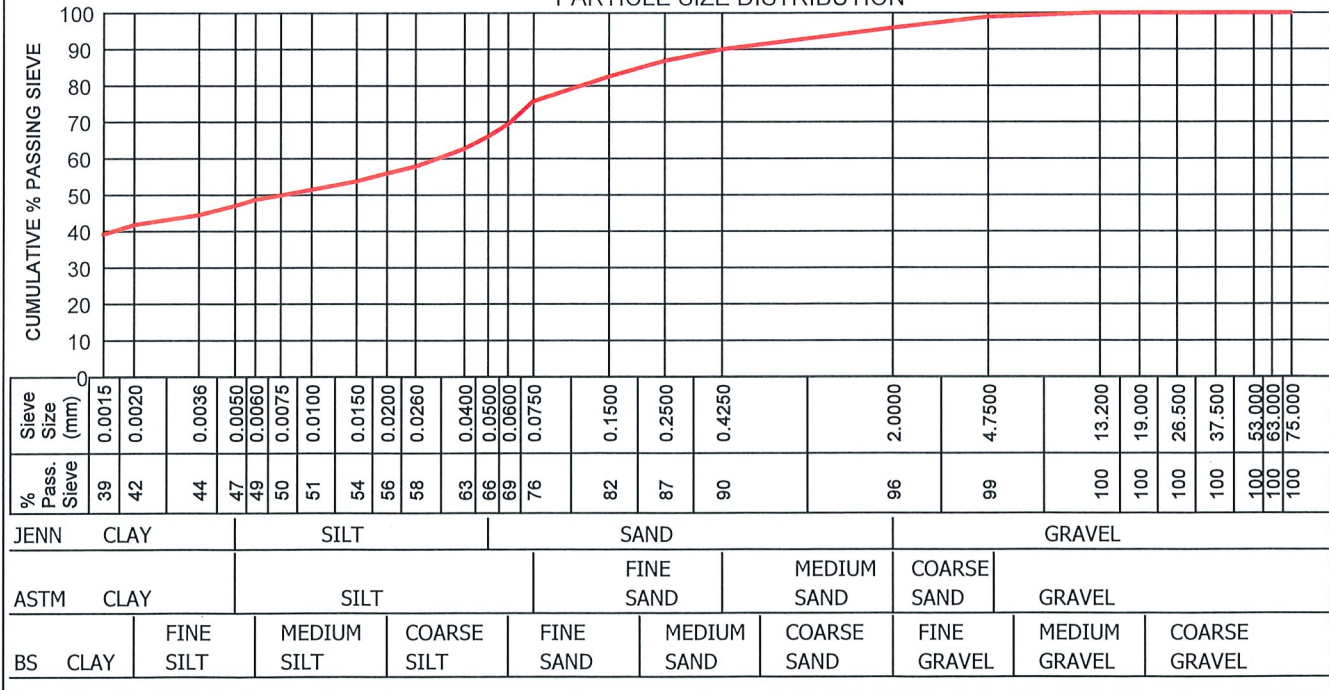
Sample No. : A21/2951(G21-0564)  
 Hole No. : TP05  
 Depth : 1000-1300  
 Liquid Limit (%) : 69  
 Plasticity Index : 28  
 Linear Shrinkage (%) : 13.0  
 PI of Whole Sample : 25  
 P.R.A. Classification : A-7-5(19)  
 Unified Soil Classification: MH  
 Activity : 0.60  
 Heave Classification : HIGH  
 Grading Modulus : 0.38  
 Percentage (<0.002) : 42.0  
 Moisture Content (%) : 29.5

Material Description : SILTY CLAY

	Clay (%)	Silt (%)	Sand (%)	Gravel (%)	Classification
Jennings	47.0	19.0	29.8	4.2	SANDY CLAY
Astm	47.0	28.6	23.2	1.2	SILTY CLAY
British Standard	41.7	27.7	26.4	4.2	SILTY CLAY



**PARTICLE SIZE DISTRIBUTION**



Remarks : Sampled by client.

FORM: A6

4.4.1(SGS)(2019.12.04)

*M Schwartz*  
 Technical Signatory : Martinus Schwartz/Lizette Breiting

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 Email : martinus.schwartz@sgs.com

a SANAS Accredited Testing Laboratory, No. T0025

**TEST RESULTS**

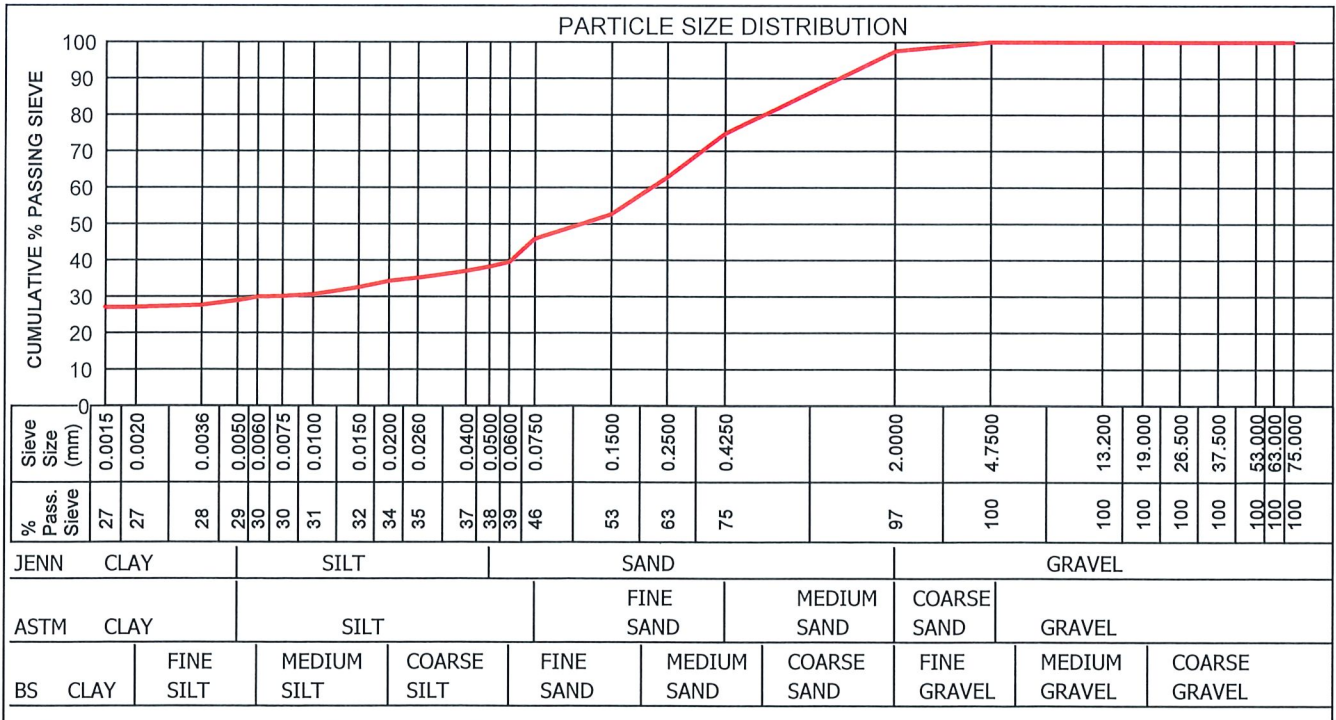
EPOCH RESOURCES  
 Attention: Mr Stephan Brakhuizen

Project : Tharisa FW WRD2  
 Your Ref :  
 Our Ref : PL/45639  
 Date Reported : 21.09.2021

**FOUNDATION INDICATOR (ASTM: D422)**

Sample No. : A21/2952(G21-056)	Material Description : CLAYEY SAND
Hole No. : TP14	
Depth : 3100-3400	
Liquid Limit (%) : 44	
Plasticity Index : 14	
Linear Shrinkage (%) : 7.0	
PI of Whole Sample : 10	
P.R.A. Classification : A-7-5(4)	
Unified Soil Classificati: SM	
Activity : 0.37	
Heave Classification : LOW	
Grading Modulus : 0.82	
Percentage (<0.002) : 27.0	
Moisture Content (%) : 13.0	

	Clay (%)	Silt (%)	Sand (%)	Gravel (%)	Classification
Jennings	28.9	9.3	59.3	2.5	CLAYEY SAND
Astm	28.9	16.9	54.3	0.0	CLAYEY SAND
British Standard	27.0	12.5	58.0	2.5	CLAYEY SAND



Remarks : Sampled by client.

FORM: A6

4.4.1(SGS)(2019.12.04)

Technical Signatory : Martinus Schwartz/Lizette Breiting

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256 Brander Street, Jan Niemand Park, Pretoria.  
 P.O Box 912387, Silverton, 0127  
 Tel. : (012) 800 1299  
 Fax :  
 Email : martinus.schwartz@sgs.com

a SANAS Accredited Testing Laboratory, No. T0025

**TEST RESULTS**

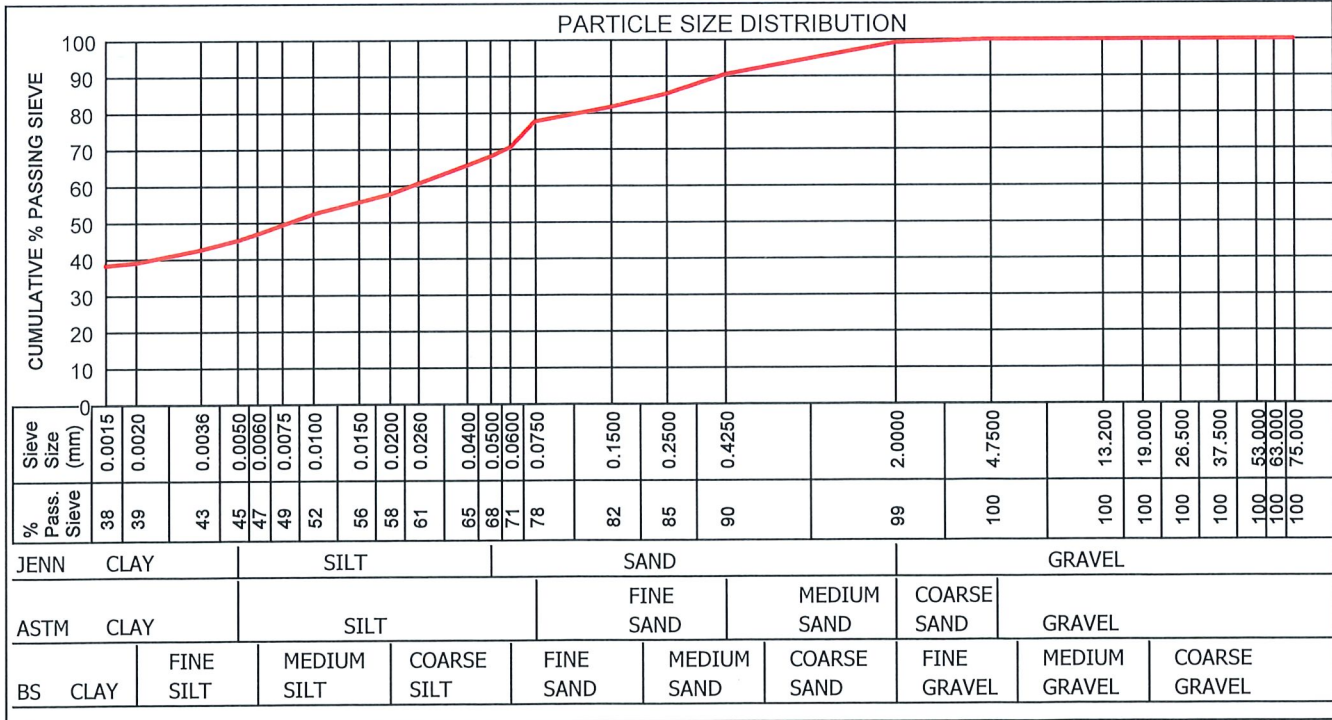
EPOCH RESOURCES  
 Attention: Mr Stephan Brakhuizen

Project : Tharisa FW WRD2  
 Your Ref :  
 Our Ref : PL/45639  
 Date Reported : 21.09.2021

**FOUNDATION INDICATOR (ASTM: D422)**

Sample No. : A21/2953(G21-056)	Material Description : SILTY CLAY
Hole No. : TP15	
Depth : 3200-3500	
Liquid Limit (%) : 56	
Plasticity Index : 20	
Linear Shrinkage (%) : 9.0	
PI of Whole Sample : 18	
P.R.A. Classification : A-7-5(15)	
Unified Soil Classificati: MH	
Activity : 0.46	
Heave Classification : LOW	
Grading Modulus : 0.33	
Percentage (<0.002) : 39.0	
Moisture Content (%) : 22.9	

	Clay (%)	Silt (%)	Sand (%)	Gravel (%)	Classification
Jennings	45.1	22.9	31.1	0.9	SANDY CLAY
Astm	45.1	32.5	22.4	0.0	SILTY CLAY
British Standard	39.1	31.5	28.5	0.9	SILTY CLAY



Remarks : Sampled by client.  
 FORM: A6  
 4.4.1(SGS)(2019.12.04)  
 Technical Signatory : Martinus Schwartz/Lizette Breiting





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**Client Name:** Inroads Consulting  
**Project Name:** Tharisa TSF - FW WRD2  
**Job Number:** IRC-18  
**Date:** 2021-09-15  
**Method:** SANS 3001 GR1, GR3, GR10 GR12 & BS 1377 (where applicable)

**FOUNDATION INDICATOR****Sheet Reference:**  
R-STL-011 Rev02**Grading & Hydrometer Analysis**  
(Particle Size (mm) & % Passing)**Atterberg Limits & Classification**

Sample	TP05		TP12	Sample	TP05		TP12
Depth (m)	1.9 - 3.4		2.7 - 5.0	Depth (m)	1.9 - 3.4		2.7 - 5.0
Lab No	IRC-18-158		IRC-18-160	Lab No	IRC-18-158		IRC-18-160
53.0	<b>100</b>		<b>100</b>	Liquid Limit (%)	<b>29</b>		-
37.5	<b>100</b>		<b>100</b>	Plastic Limit (%)	<b>21</b>		-
26.5	<b>100</b>		<b>100</b>	Plasticity Index (%)	<b>8</b>		<b>NP</b>
19.0	<b>100</b>		<b>100</b>	Linear Shrinkage (%)	<b>4.5</b>		<b>0.0</b>
13.2	<b>100</b>		<b>100</b>	PI of whole sample	<b>2</b>		-
9.5	<b>100</b>		<b>100</b>				
6.7	<b>96</b>		<b>100</b>	% Gravel	<b>47</b>		<b>16</b>
4.75	<b>88</b>		<b>100</b>	% Sand	<b>45</b>		<b>73</b>
2.00	<b>53</b>		<b>84</b>	% Silt	<b>6</b>		<b>10</b>
1.00	<b>37</b>		<b>60</b>	% Clay	<b>2</b>		<b>1</b>
0.425	<b>28</b>		<b>50</b>	Activity	<b>4.0</b>		<b>0.0</b>
0.250	<b>22</b>		<b>35</b>				
0.150	<b>16</b>		<b>24</b>	% Soil Mortar	<b>53</b>		<b>84</b>
0.075	<b>10</b>		<b>14</b>				
0.060	<b>8</b>		<b>11</b>	Grading Modulus	<b>2.09</b>		<b>1.52</b>
0.050	<b>7</b>		<b>10</b>	Moisture Content (%)	<b>N / T</b>		<b>N / T</b>
0.035	<b>5</b>		<b>7</b>	Relative Density (SG)*	<b>2.65</b>		<b>2.65</b>
0.020	<b>4</b>		<b>4</b>				
0.006	<b>2</b>		<b>2</b>	Unified (ASTM D2487)	<b>SW-SC</b>		<b>SM</b>
0.002	<b>2</b>		<b>1</b>	AASHTO (M145-91)	<b>A - 2 - 4</b>		<b>A - 1 - b</b>

Remarks: \*: Assumed

N / T: Not Tested



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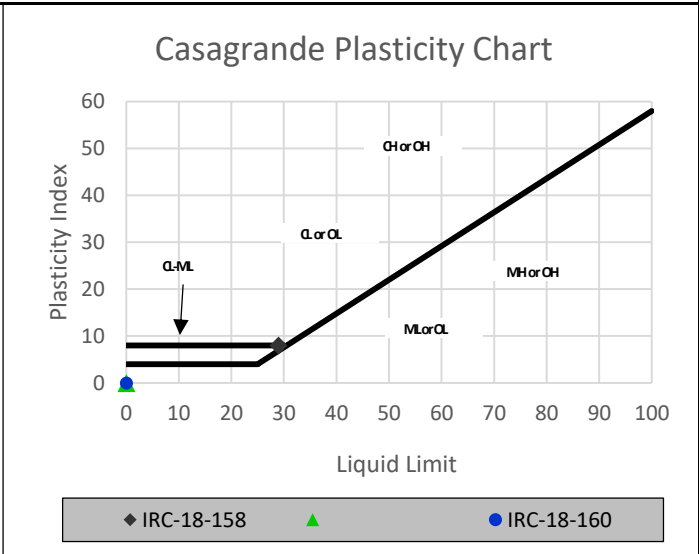
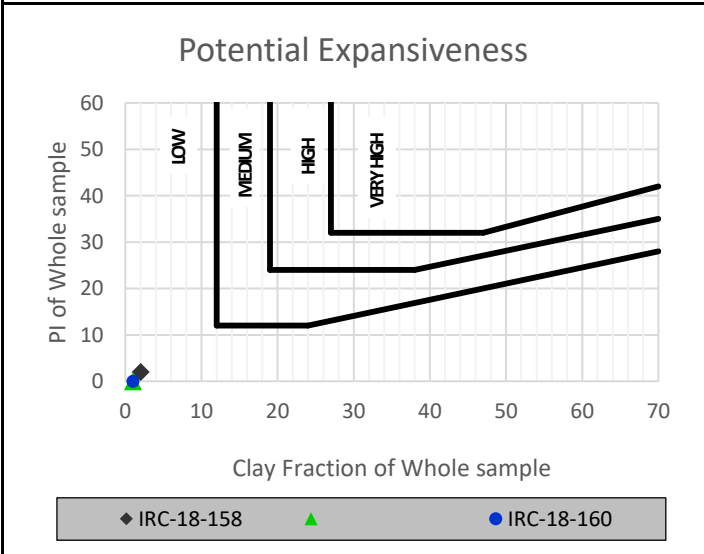
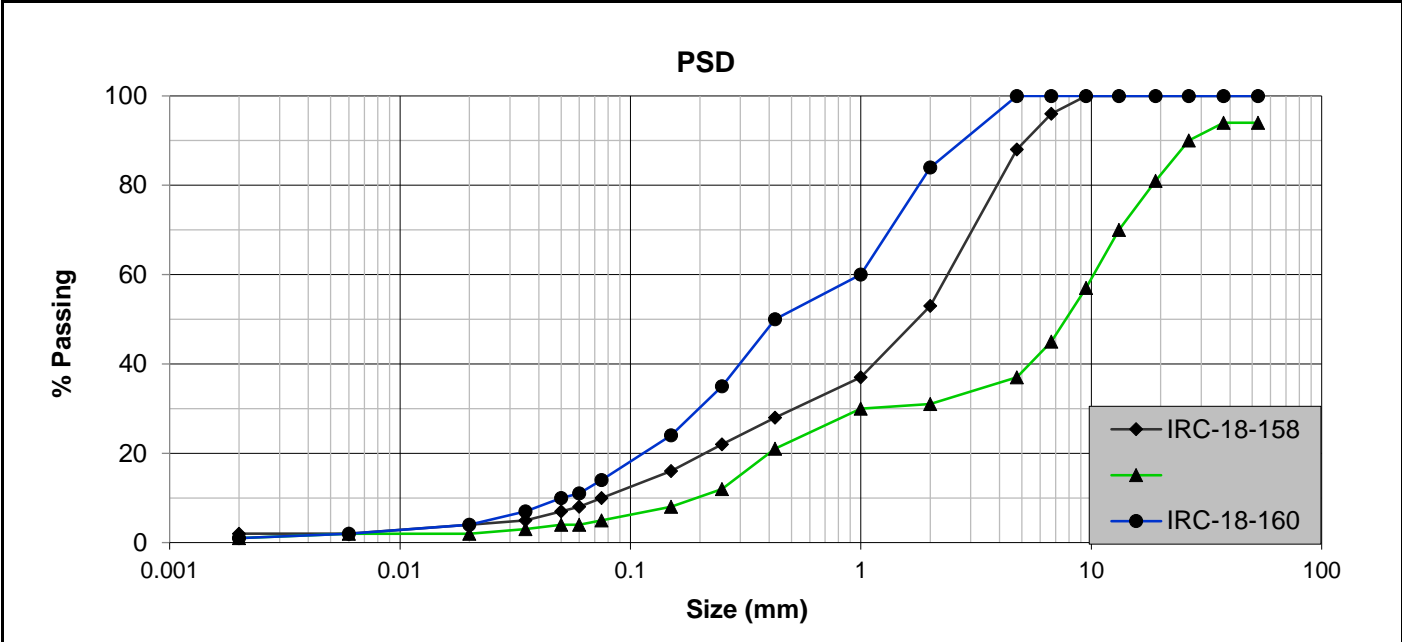
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**Client Name:** Inroads Consulting  
**Project Name:** Tharisa TSF - FW WRD2  
**Job Number:** IRC-18  
**Date:** 2021-09-15  
**Method:** SANS 3001 GR1, GR3, GR10 GR12 & BS 1377 (where applicable)

**FOUNDATION INDICATOR** **Sheet Reference:**  
R-STL-011 Rev02



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**Date:** 2021-09-15  
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FOUNDATION INDICATOR						Sheet Reference: R-STL-011 Rev02	
Grading & Hydrometer Analysis (Particle Size (mm) & % Passing)				Atterberg Limits & Classification			
Sample	TP13	TP15		Sample	TP13	TP15	
Depth (m)	0.4 - 1.3	2.5 - 3.0		Depth (m)	0.4 - 1.3	2.5 - 3.0	
Lab No	IRC-18-161	IRC-18-162		Lab No	IRC-18-161	IRC-18-162	
53.0	100	100		Liquid Limit (%)	34	49	
37.5	100	100		Plastic Limit (%)	16	26	
26.5	100	100		Plasticity Index (%)	18	23	
19.0	100	100		Linear Shrinkage (%)	9.5	11.5	
13.2	100	100		PI of whole sample	13	19	
9.5	100	100					
6.7	100	100		% Gravel	0	4	
4.75	100	99		% Sand	73	35	
2.00	100	96		% Silt	9	17	
1.00	97	92		% Clay	18	44	
0.425	72	83		Activity	1.0	0.5	
0.250	52	75					
0.150	41	70		% Soil Mortar	100	96	
0.075	31	65					
0.060	27	61		Grading Modulus	0.97	0.56	
0.050	25	59		Moisture Content (%)	N / T	N / T	
0.035	22	54		Relative Density (SG)*	2.65	2.65	
0.020	20	51					
0.006	19	47		Unified (ASTM D2487)	SC	CL	
0.002	18	44		AASHTO (M145-91)	A - 2 - 6	A - 7 - 6	
Remarks: *: Assumed							
N / T: Not Tested							
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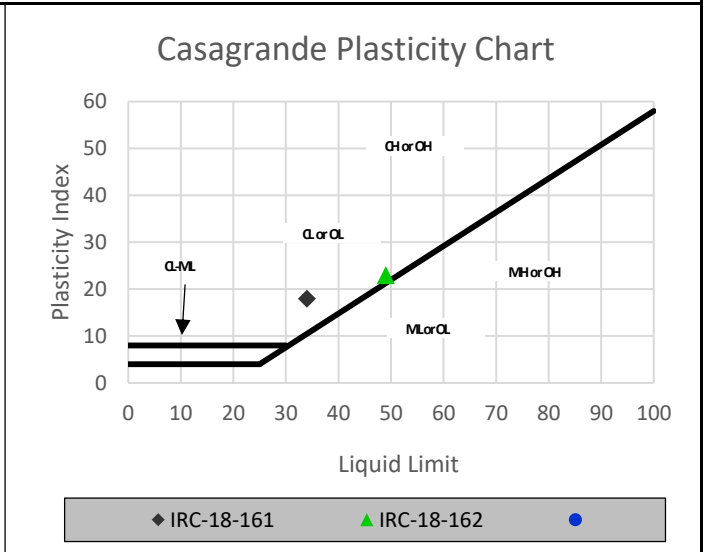
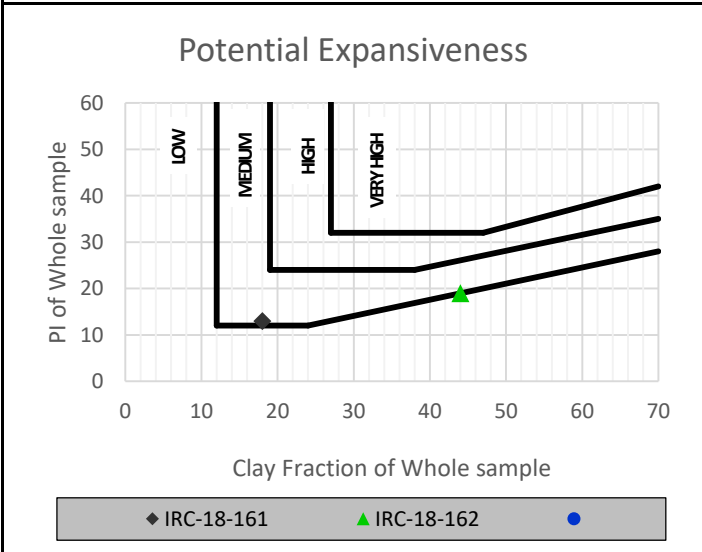
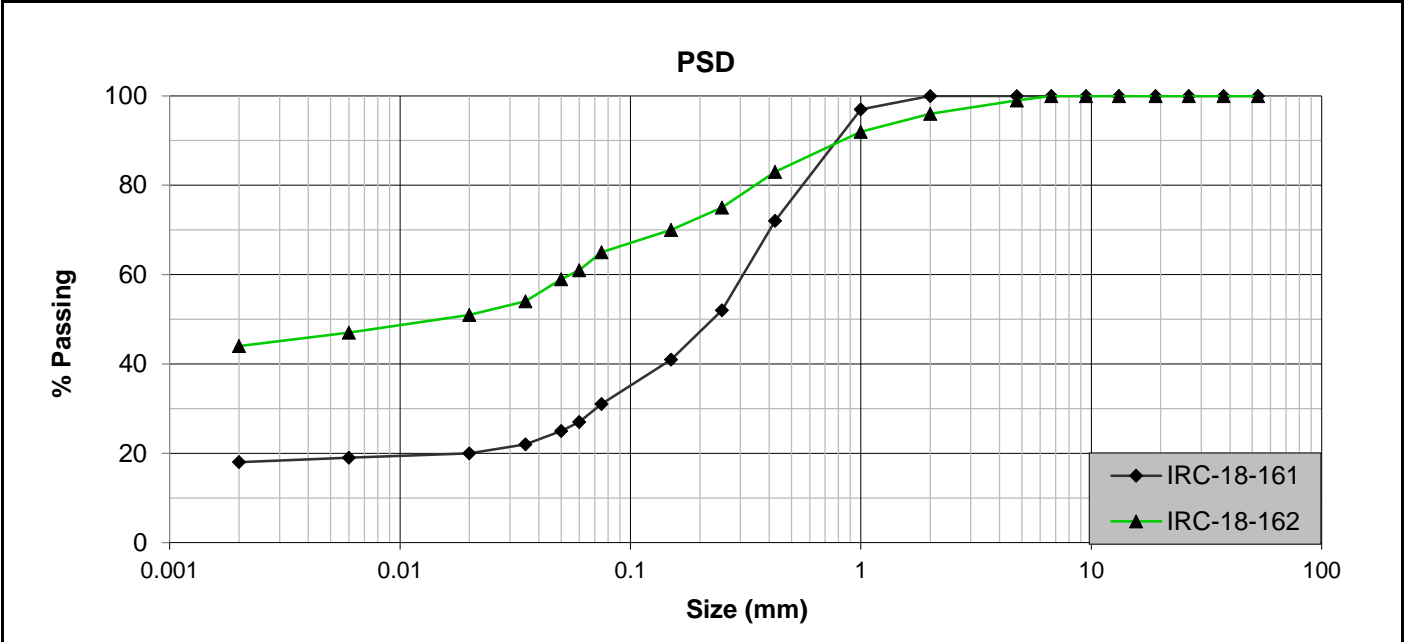
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**Job Number:** IRC-18  
**Date:** 2021-09-15  
**Method:** SANS 3001 GR1, GR3, GR10 GR12 & BS 1377 (where applicable)

**FOUNDATION INDICATOR** **Sheet Reference:**  
R-STL-011 Rev02



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## CONSOLIDATED UNDRAINED TRIAXIAL TEST

**BS 1377  
Part 8**

Client: EPOCH RESOURCES

Project: THARISA FW WRD2

Job no: 39458

Sample no: TP 01

Date: 15/09/2021

Lab no: G21-0564

Depth (m): 2.1-2.4

Page 1 of 5
-------------


Test Information		
Test Type	-	Consolidated Undrained with PWP measurements, saturated
Sample Condition	-	Remoulded
Saturation Method		Increments of Cell- and Backpressure
Consolidation Pressures	kPa	50, 100, 200
Rate of Strain	%/min	0.0104
Failure Criterion	-	Maximum Deviator Stress
Side Drains Used	-	No
Drainage Conditions	-	To One End
Comments	-	Test 1, Test 2 & Test 3 - Failure Criterion taken at Maximum Change in Pore Pressure
		-

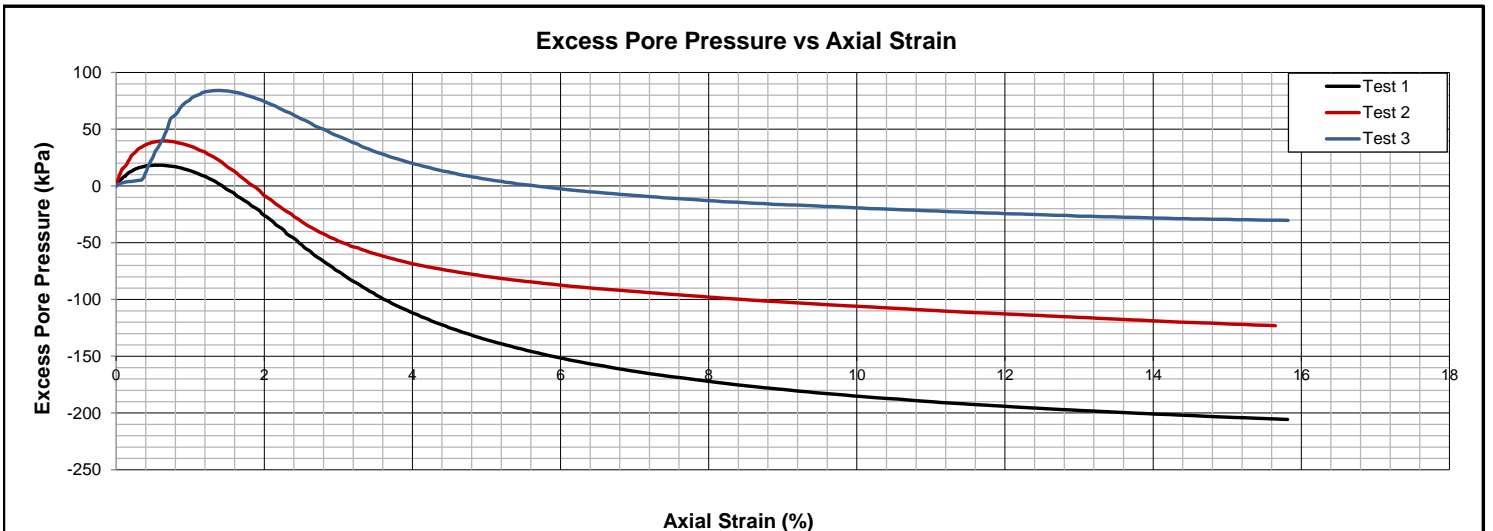
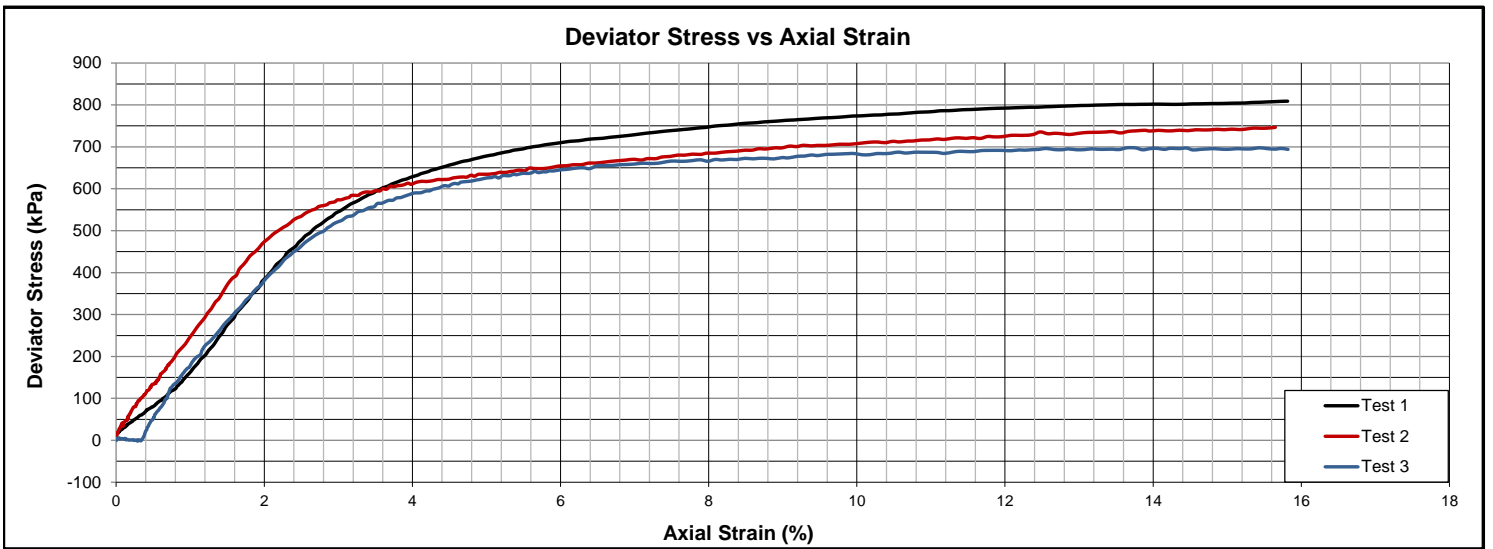
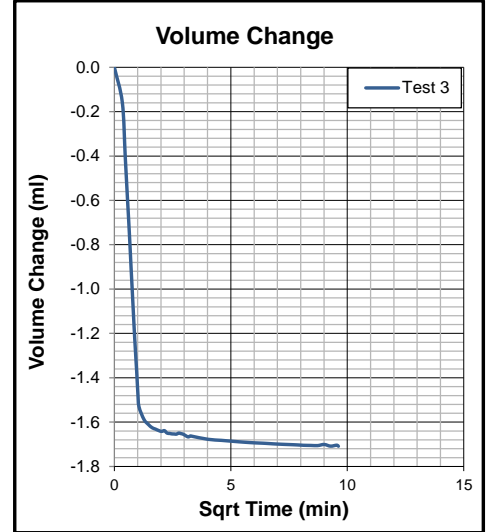
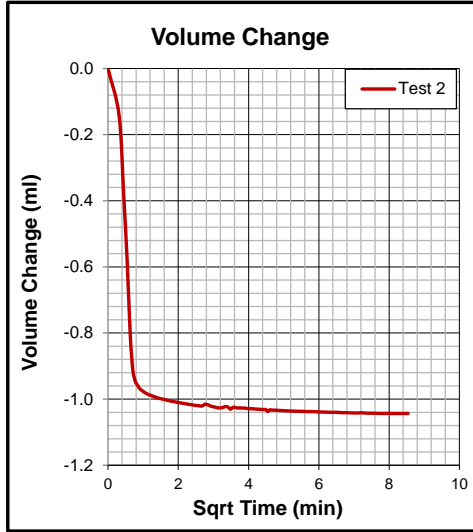
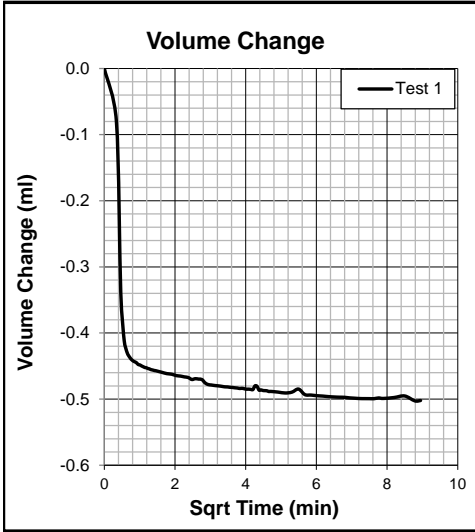
Initial Sample Parameters	Unit	Test 1	Test 2	Test 3	Remarks
Moisture Content	%	6.8	6.9	9.5	Complete test specimen
Dry Density	Kg/m <sup>3</sup>	1816	1811	1781	
Void Ratio	-	0.531	0.535	0.560	
Degree of Saturation	%	35.8	35.7	47.4	
Initial Height	cm	7.7	7.7	7.7	
Initial Diameter	cm	3.8	3.8	3.8	
Area (After Consolidation)	cm <sup>2</sup>	11.413	11.374	11.258	Calculated
Relative Density (SG)	-		2.779		Determined

Final Sample Parameters	Unit	Test 1	Test 2	Test 3	Remarks	
Moisture Content	%	19.8	19.8	20.8	Complete test specimen	
Dry Density	Kg/m <sup>3</sup>	1826	1832	1817		
Void Ratio	-	0.522	0.517	0.530		
Area	cm <sup>2</sup>	13.557	13.484	13.374	Calculated	
Eff. Consolidation Pressures	kPa	54	104	200		
Total Backpressure used	kPa	300	300	300	Saturation	
Final B Parameter	-	0.98	1.00	0.98		
Cell Pressure	kPa	350	400	500	Consolidation & Shear	
Axial Strain at Max. Deviator Stress	%	0.52	0.67	1.37		
Volume Change	ml	0.5	1.0	1.7	During Consolidation	
Principal Stresses at Max. Deviator Stress	$\sigma_1$	kPa	138	275	457	Corrected
	$\sigma_3$	kPa	54	104	200	Corrected
	$\sigma_1'$	kPa	119	235	373	Corrected
	$\sigma_3'$	kPa	36	64	116	Corrected

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	<h2 style="margin: 0;">CONSOLIDATED UNDRAINED TRIAXIAL TEST</h2>	<b>BS 1377</b> <b>Part 8</b>
Client: EPOCH RESOURCES    Project: THARISA FW WRD2	Job no: 39458	
Sample no: TP 01	Date: 15/09/2021	
Lab no: G21-0564    Depth (m): 2.1-2.4    Sample Condition: Remoulded		Page 2 of 5



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## CONSOLIDATED UNDRAINED TRIAXIAL TEST

**BS 1377**  
**Part 8**

Client: EPOCH RESOURCES

Project: THARISA FW WRD2

Job no: 39458

Sample no: TP 01

Date: 15/09/2021

Lab no: G21-0564

Depth (m): 2.1-2.4

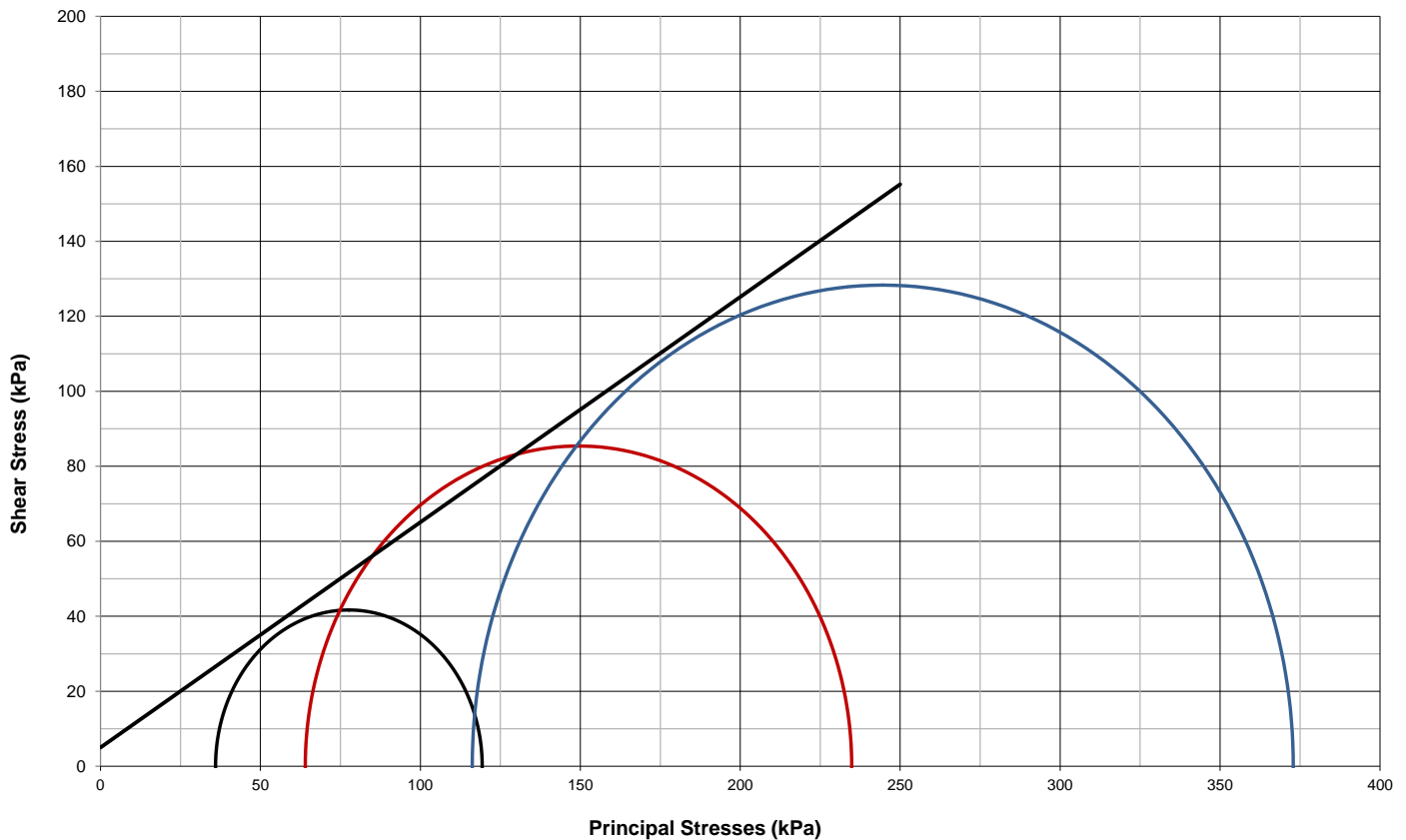
Sample Condition: Remoulded

Page 3 of 5
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### Shear Parameters of Effective Stresses


Angle of Internal Friction	Deg.	31
Cohesion	kPa	5

### Effective Shear Strength

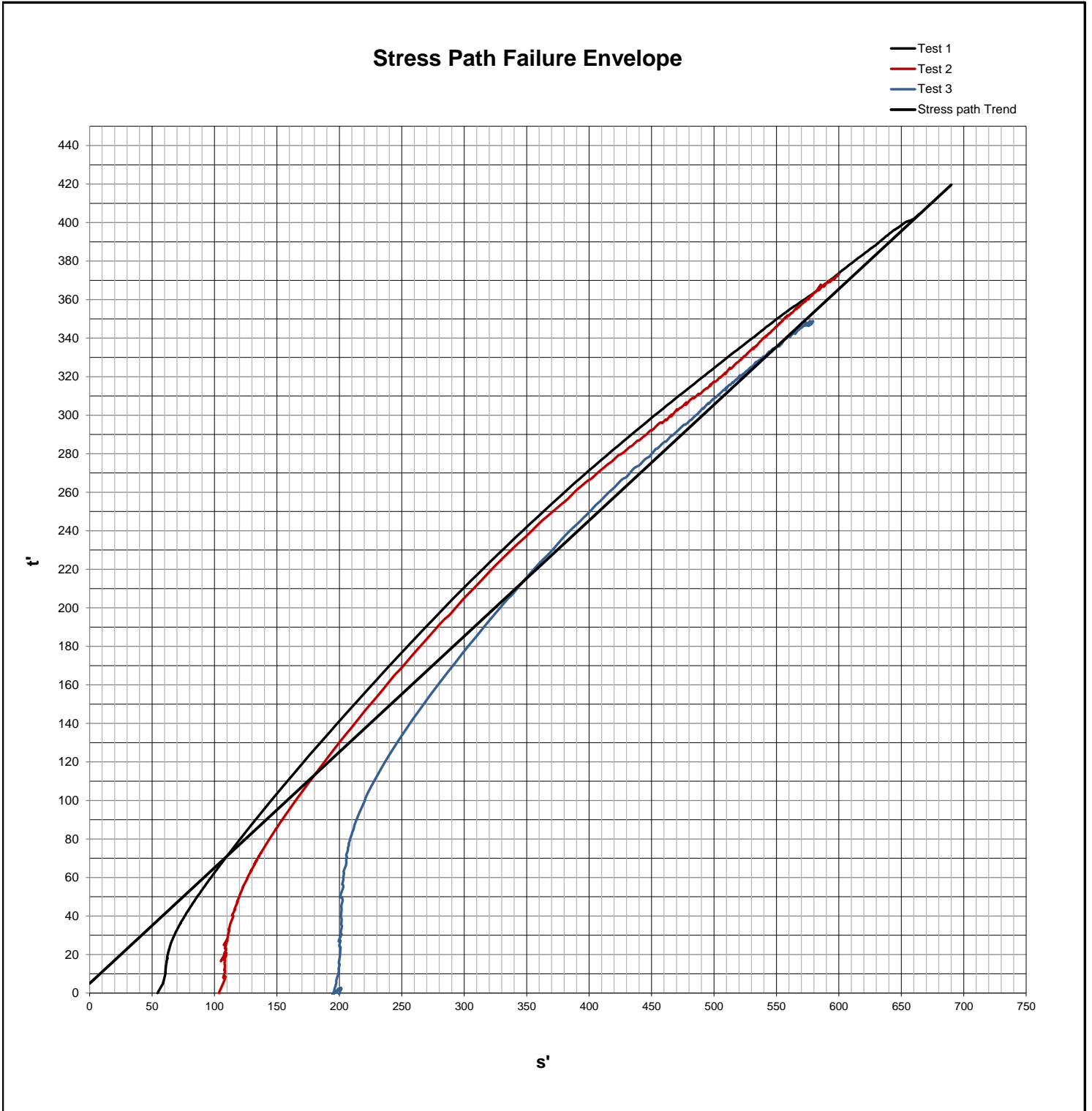


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	<h2 style="margin: 0;">CONSOLIDATED UNDRAINED TRIAXIAL TEST</h2>	<b>BS 1377</b> <b>Part 8</b>
<b>Client:</b> EPOCH RESOURCES <b>Project:</b> THARISA FW WRD2	<b>Job no:</b> 39458	<div style="border: 1px solid black; padding: 2px; display: inline-block;">Page 4 of 5</div>
<b>Sample no:</b> TP 01	<b>Date:</b> 15/09/2021	
<b>Lab no:</b> G21-0564 <b>Depth (m):</b> 2.1-2.4 <b>Sample Condition:</b> Remoulded		

Shear Parameters at Failure		
Angle of Internal Friction	Deg.	31
Cohesion	kPa	5



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**Client:** EPOCH RESOURCES    **Project:** THARISA FW WRD2  
**Sample no:** TP 01  
**Lab no:** G21-0564    **Depth (m):** 2.1-2.4

**Job no:** 39458  
**Date:** 15/09/2021

**Test 1**



**BEFOR TEST**



**AFTER TEST**

**Test 2**



**BEFOR TEST**

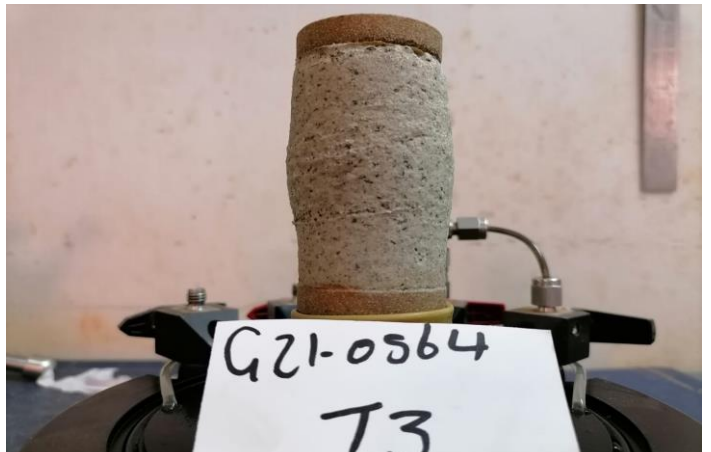


**AFTER TEST**

**Test 3**



**BEFOR TEST**



**AFTER TEST**





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## CONSOLIDATED UNDRAINED TRIAXIAL TEST

**BS 1377  
Part 8**

**Client:** EPOCH RESOURCES  
**Sample no:** TP 05  
**Lab no:** G21-0565

**Project:** THARISA FW WRD 2  
**Depth (m):** 1.0-1.3

**Job no:** 39458  
**Date:** 15/09/2021

Page 1 of 5
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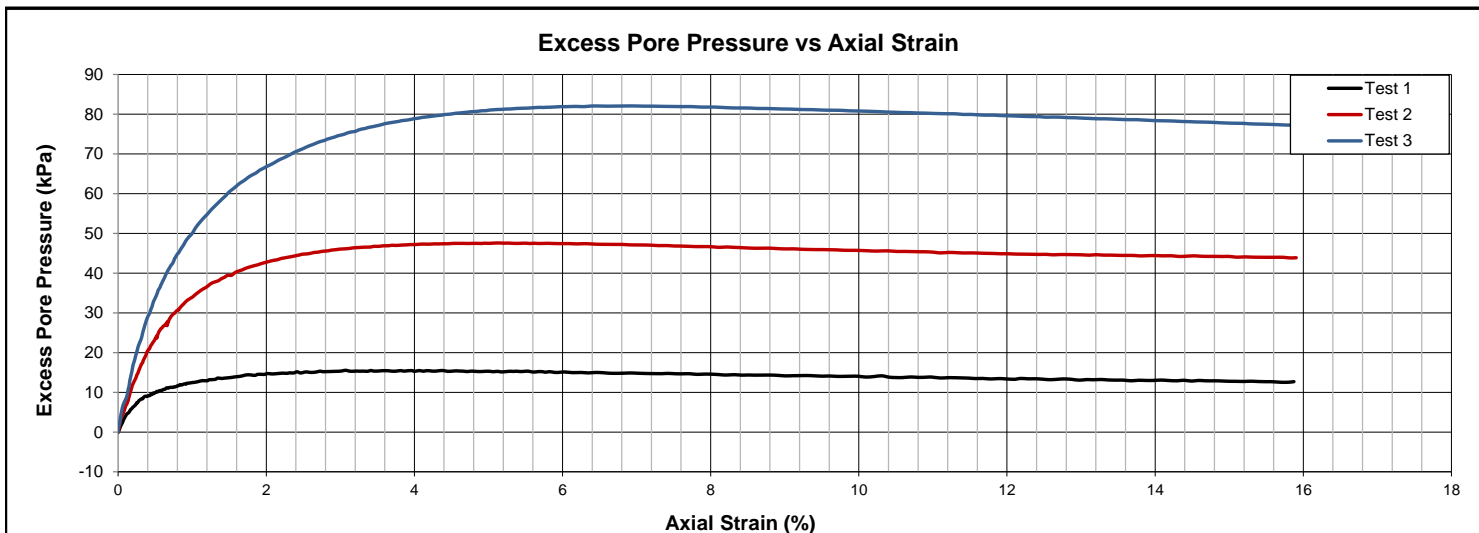
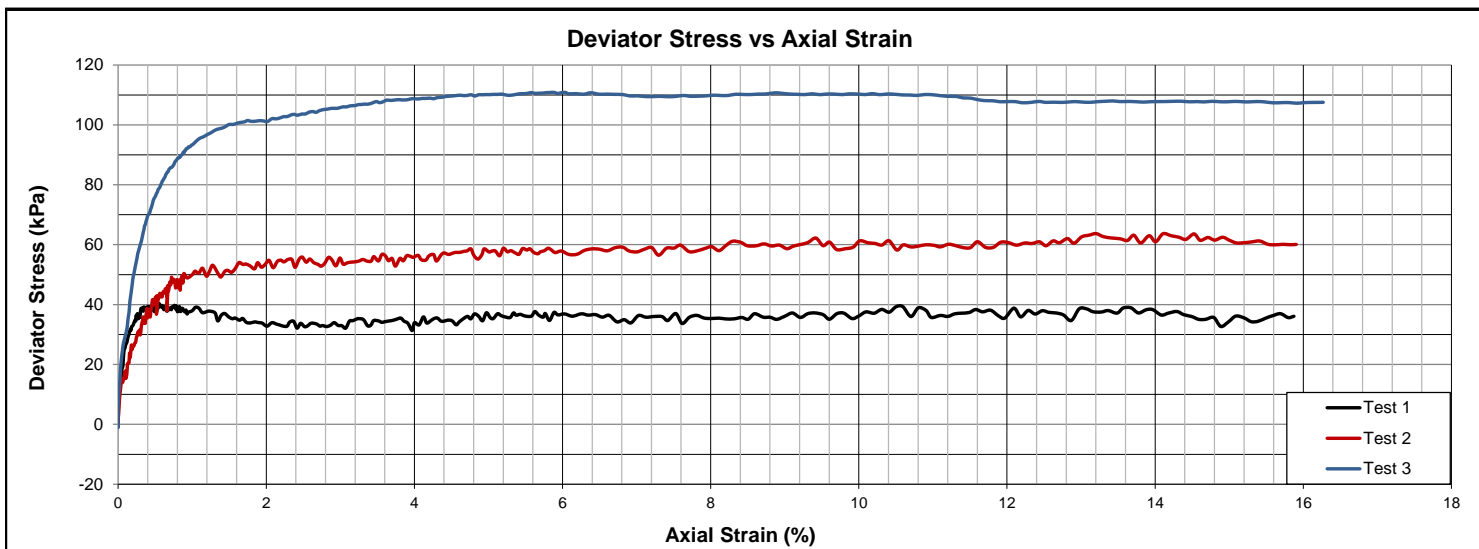
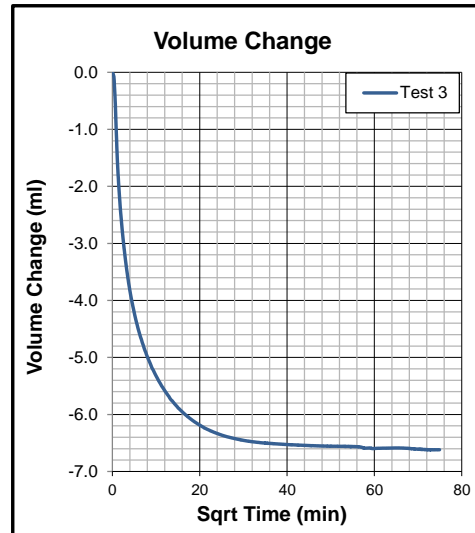
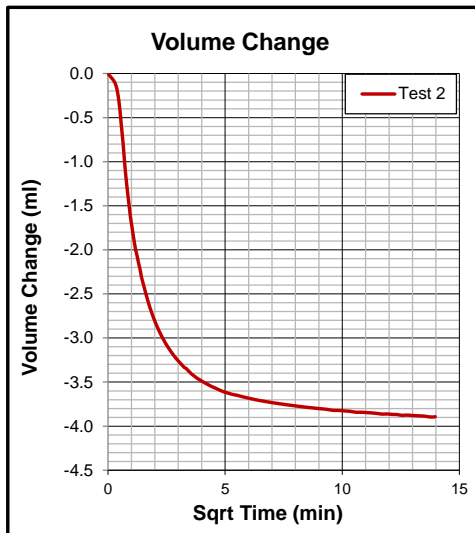
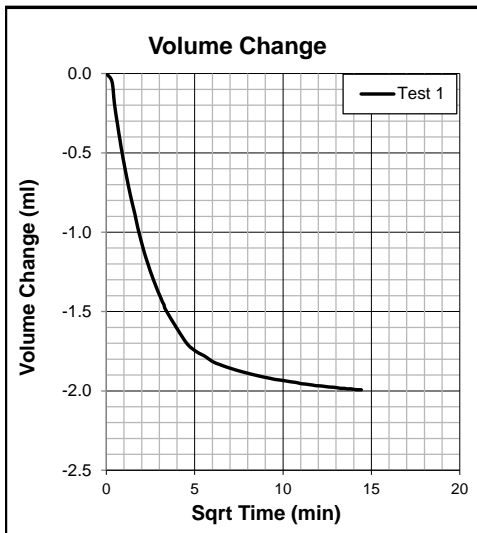
Test Information		
Test Type	-	Consolidated Undrained with PWP measurements, saturated
Sample Condition	-	Undisturbed
Saturation Method		Increments of Cell- and Backpressure
Consolidation Pressures	kPa	50, 100, 200
Rate of Strain	%/min	0.0104
Failure Criterion	-	Maximum Deviator Stress
Side Drains Used	-	Yes
Drainage Conditions	-	To One End
Comments	-	-

Initial Sample Parameters	Unit	Test 1	Test 2	Test 3	Remarks
Moisture Content	%	34.5	32.9	35.4	Complete test specimen
Dry Density	Kg/m <sup>3</sup>	1274	1291	1276	
Void Ratio	-	1.002	0.975	0.998	
Degree of Saturation	%	87.7	86.1	90.5	
Initial Height	cm	7.7	7.7	7.7	
Initial Diameter	cm	3.8	3.8	3.8	
Area (After Consolidation)	cm <sup>2</sup>	10.987	10.858	10.825	Calculated
Relative Density (SG)	-		2.550		Determined


Final Sample Parameters	Unit	Test 1	Test 2	Test 3	Remarks	
Moisture Content	%	38.4	35.2	36.0	Complete test specimen	
Dry Density	Kg/m <sup>3</sup>	1304	1352	1381		
Void Ratio	-	0.956	0.886	0.847		
Area	cm <sup>2</sup>	13.060	12.912	12.928	Calculated	
Eff. Consolidation Pressures	kPa	38	99	200		
Total Backpressure used	kPa	300	300	300	Saturation	
Final B Parameter	-	0.98	0.96	0.96		
Cell Pressure	kPa	350	400	500	Consolidation & Shear	
Axial Strain at Max. Deviator Stress	%	0.54	13.20	5.88		
Volume Change	ml	2.0	3.9	6.6	During Consolidation	
Principal Stresses at Max. Deviator Stress	$\sigma_1$	kPa	79	163	311	Corrected
	$\sigma_3$	kPa	38	99	200	Corrected
	$\sigma_1'$	kPa	69	118	229	Corrected
	$\sigma_3'$	kPa	28	54	119	Corrected

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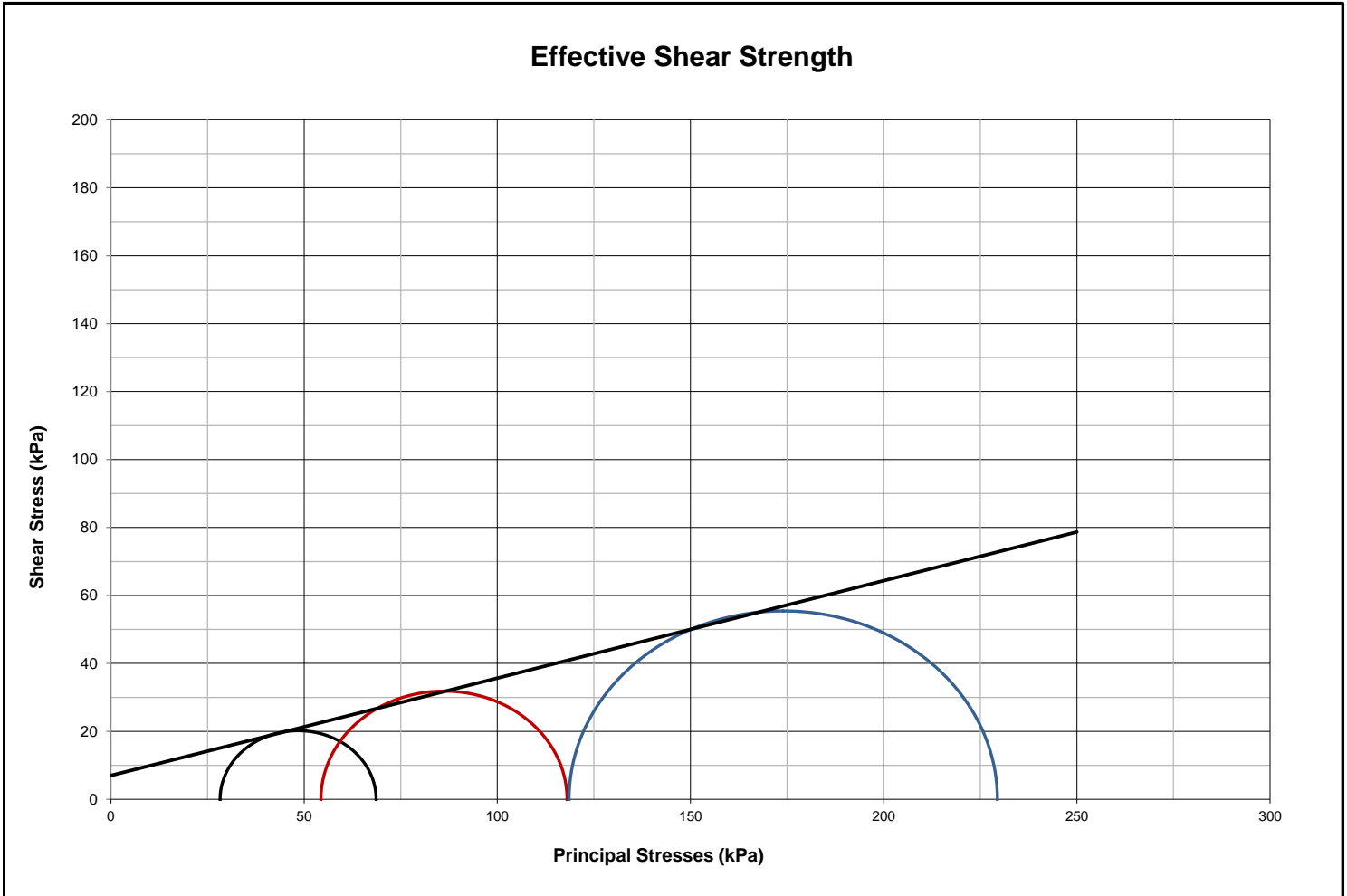
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		<b>CONSOLIDATED UNDRAINED TRIAXIAL TEST</b>		<b>BS 1377 Part 8</b>
<b>Client:</b> EPOCH RESOURCES <b>Sample no:</b> TP 05 <b>Lab no:</b> G21-0565	<b>Project:</b> THARISA FW WRD 2 <b>Depth (m):</b> 1.0-1.3 <b>Sample Condition:</b> Undisturbed	<b>Job no:</b> 39458 <b>Date:</b> 15/09/2021	<div style="border: 1px solid black; padding: 2px;">Page 3 of 5</div>	

Shear Parameters of Effective Stresses		
Angle of Internal Friction	Deg.	16
Cohesion	kPa	7

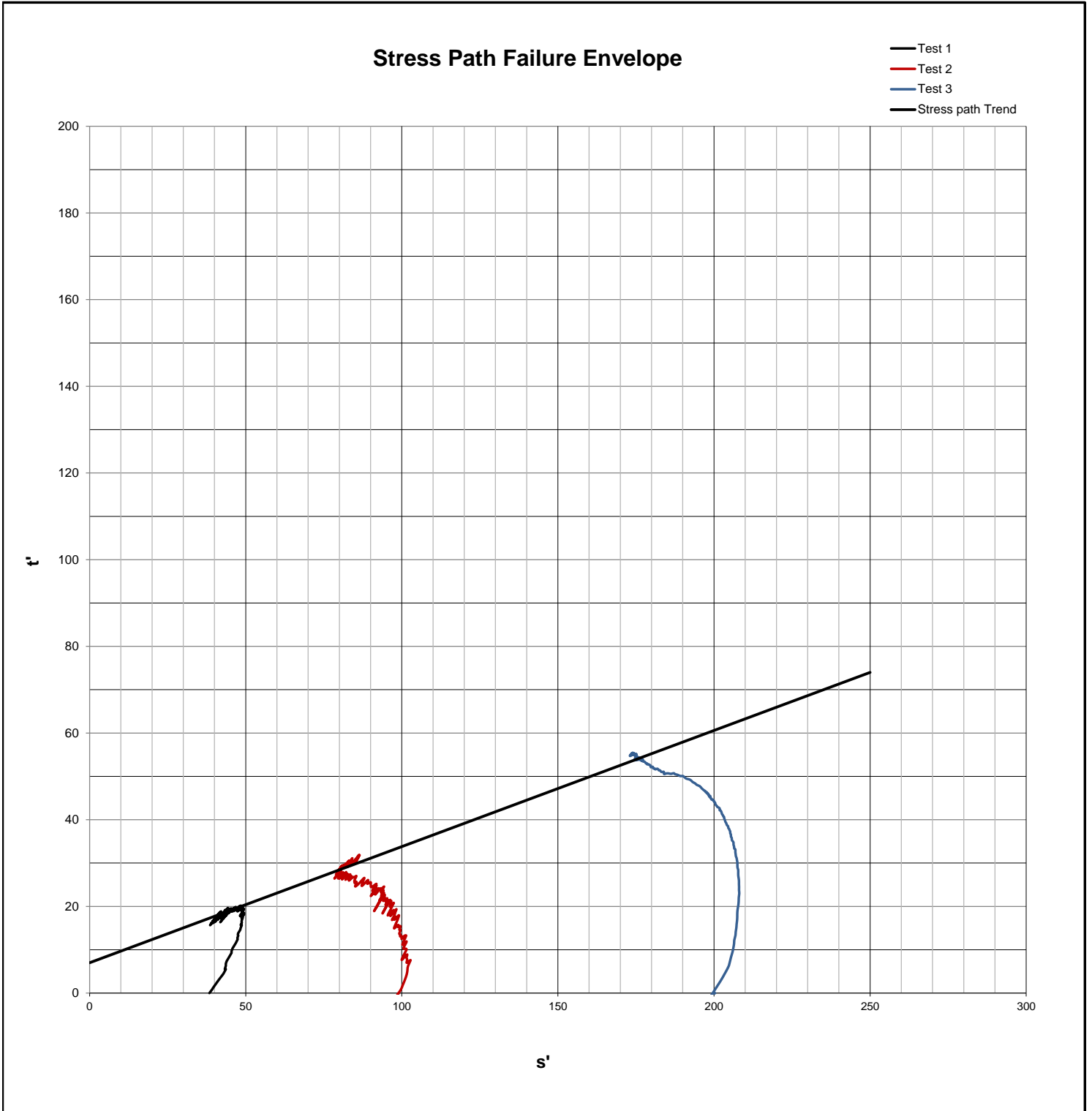


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	<h2 style="margin: 0;">CONSOLIDATED UNDRAINED TRIAXIAL TEST</h2>	<b>BS 1377</b> <b>Part 8</b>
<b>Client:</b> EPOCH RESOURCES <b>Sample no:</b> TP 05 <b>Lab no:</b> G21-0565	<b>Project:</b> THARISA FW WRD 2 <b>Depth (m):</b> 1.0-1.3 <b>Sample Condition:</b> Undisturbed	<b>Job no:</b> 39458 <b>Date:</b> 15/09/2021
		Page 4 of 5

Shear Parameters at Failure		
Angle of Internal Friction	Deg.	15
Cohesion	kPa	7



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Client: EPOCH RESOURCES  
Sample no: TP 05  
Lab no: G21-0565

Project: THARISA FW WRD 2  
Depth (m): 1.0-1.3

Job no: 39458  
Date: 15/09/2021

**Test 1**



BEFOR TEST



AFTER TEST

**Test 2**



BEFOR TEST



AFTER TEST

**Test 3**



BEFOR TEST



AFTER TEST



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## CONSOLIDATED UNDRAINED TRIAXIAL TEST

**BS 1377  
Part 8**

Client: EPOCH RESOURCES

Project: THARISA FW WRD 2

Job no: 39458

Sample no: TP 14

Depth (m): 3.1-3.4

Date: 15/09/2021

Lab no: G21-0566

Page 1 of 5
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Test Information		
Test Type	-	Consolidated Undrained with PWP measurements, saturated
Sample Condition	-	Undisturbed
Saturation Method		Increments of Cell- and Backpressure
Consolidation Pressures	kPa	50, 100, 200
Rate of Strain	%/min	0.0104
Failure Criterion	-	Maximum Deviator Stress
Side Drains Used	-	Yes
Drainage Conditions	-	To One End
Comments	-	Test 1, Test 2 & Test 3 - Failure Criterion taken at Maximum Change in Pore Pressure
		-

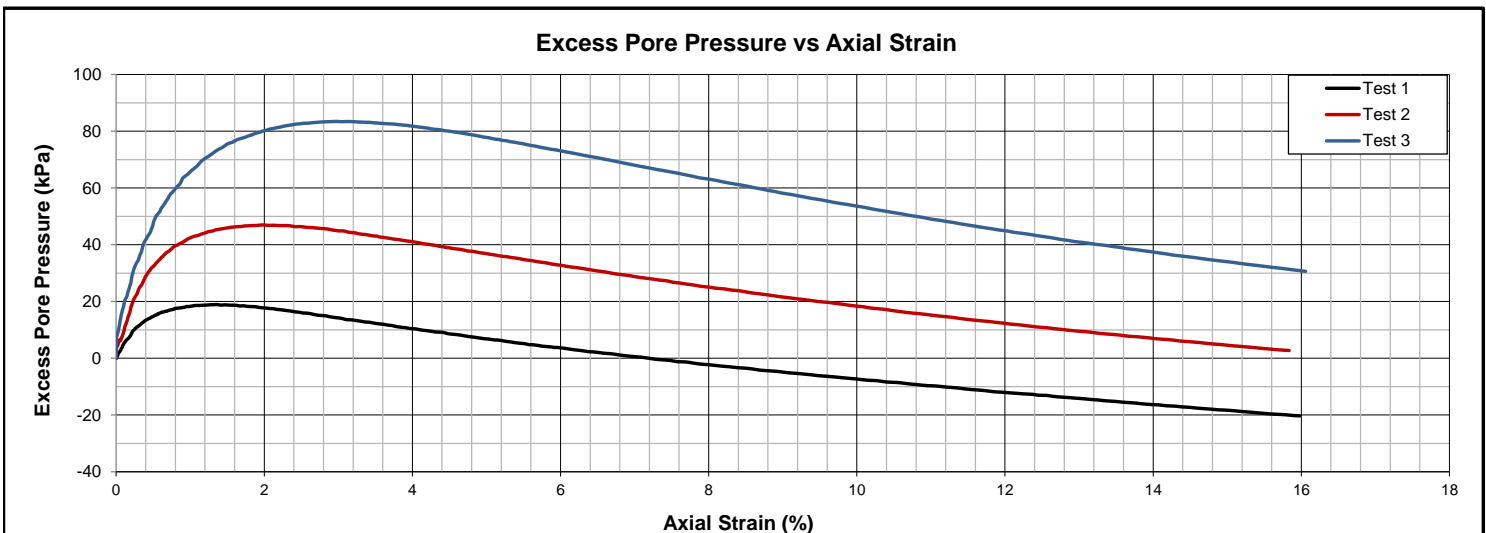
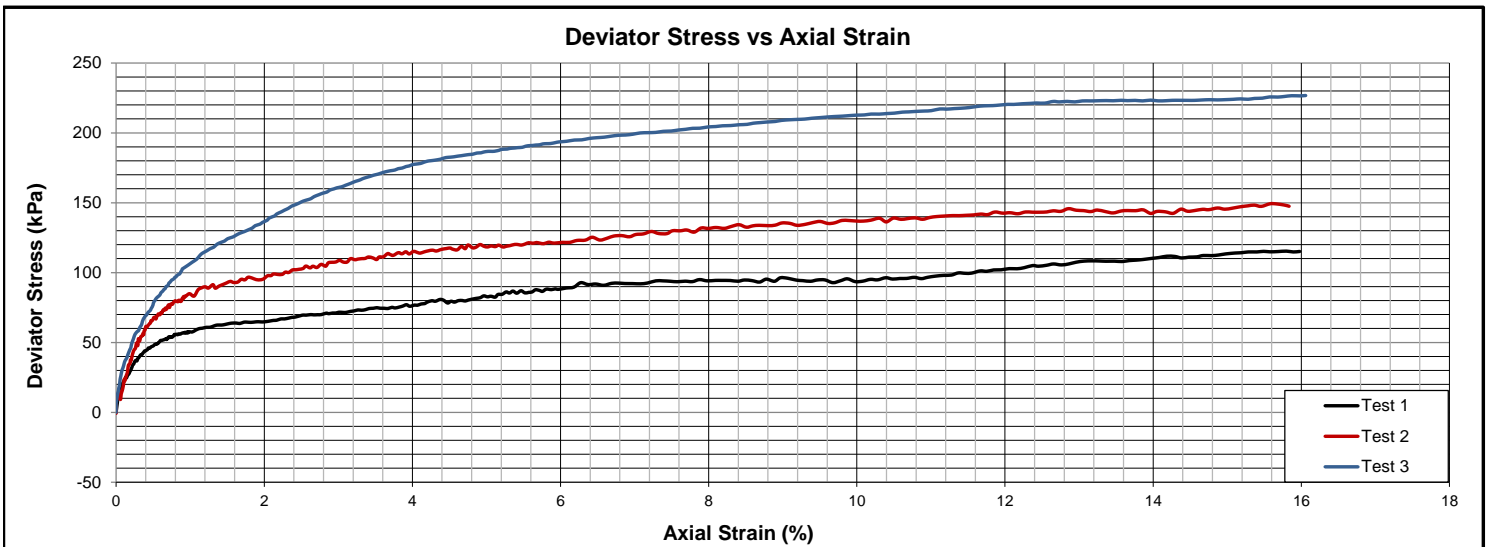
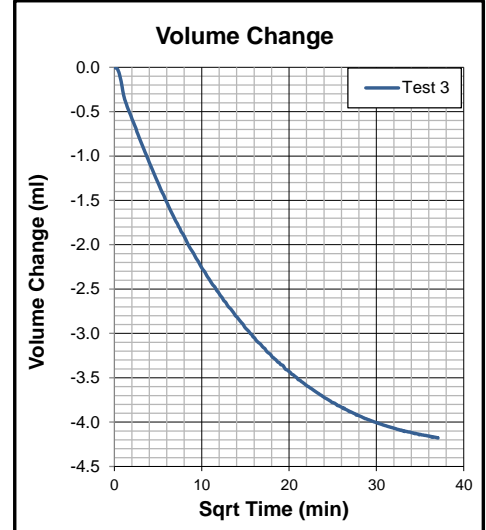
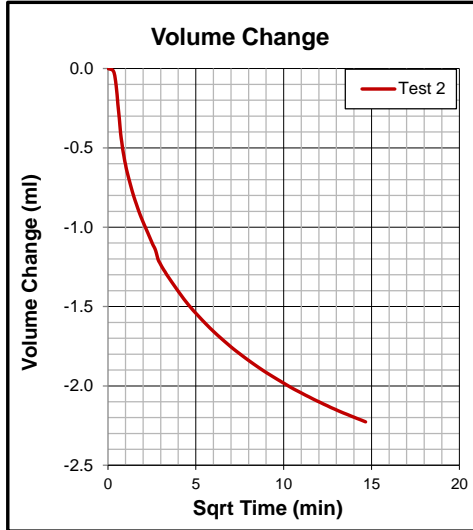
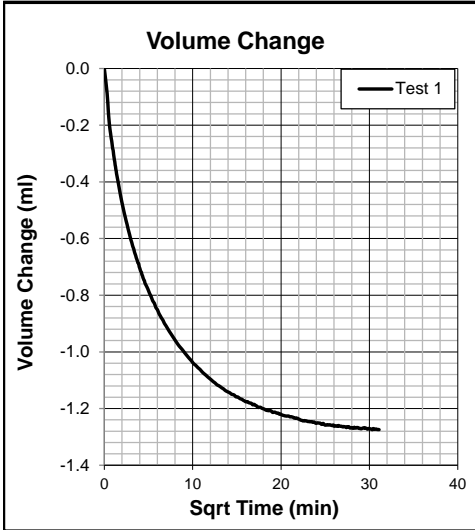
Initial Sample Parameters	Unit	Test 1	Test 2	Test 3	Remarks
Moisture Content	%	14.2	13.6	13.0	Complete test specimen
Dry Density	Kg/m <sup>3</sup>	1838	1865	1868	
Void Ratio	-	0.442	0.421	0.419	
Degree of Saturation	%	85.1	85.2	82.4	
Initial Height	cm	7.6	7.7	7.7	
Initial Diameter	cm	3.8	3.8	3.8	
Area (After Consolidation)	cm <sup>2</sup>	11.206	11.077	11.005	Calculated
Relative Density (SG)	-		2.651		Determined

Final Sample Parameters	Unit	Test 1	Test 2	Test 3	Remarks	
Moisture Content	%	18.2	17.5	16.7	Complete test specimen	
Dry Density	Kg/m <sup>3</sup>	1866	1914	1962		
Void Ratio	-	0.421	0.385	0.351		
Area	cm <sup>2</sup>	11.361	11.303	11.343	Calculated	
Eff. Consolidation Pressures	kPa	51	100	205		
Total Backpressure used	kPa	300	300	300	Saturation	
Final B Parameter	-	0.98	0.96	0.96		
Cell Pressure	kPa	350	400	500	Consolidation & Shear	
Axial Strain at Max. Deviator Stress	%	1.36	1.78	2.98		
Volume Change	ml	1.3	2.2	4.2	During Consolidation	
Principal Stresses at Max. Deviator Stress	$\sigma_1$	kPa	113	196	366	Corrected
	$\sigma_3$	kPa	51	100	205	Corrected
	$\sigma_1'$	kPa	94	150	282	Corrected
	$\sigma_3'$	kPa	32	53	122	Corrected

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	<h2 style="margin: 0;">CONSOLIDATED UNDRAINED TRIAXIAL TEST</h2>	<b>BS 1377</b> <b>Part 8</b>
<b>Client:</b> EPOCH RESOURCES <b>Sample no:</b> TP 14 <b>Lab no:</b> G21-0566	<b>Project:</b> THARISA FW WRD 2 <b>Depth (m):</b> 3.1-3.4 <b>Sample Condition:</b> Undisturbed	<b>Job no:</b> 39458 <b>Date:</b> 15/09/2021
		Page 2 of 5



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## CONSOLIDATED UNDRAINED TRIAXIAL TEST

**BS 1377  
Part 8**

Client: EPOCH RESOURCES

Project: THARISA FW WRD 2

Job no: 39458

Sample no: TP 14

Depth (m): 3.1-3.4

Date: 15/09/2021

Lab no: G21-0566

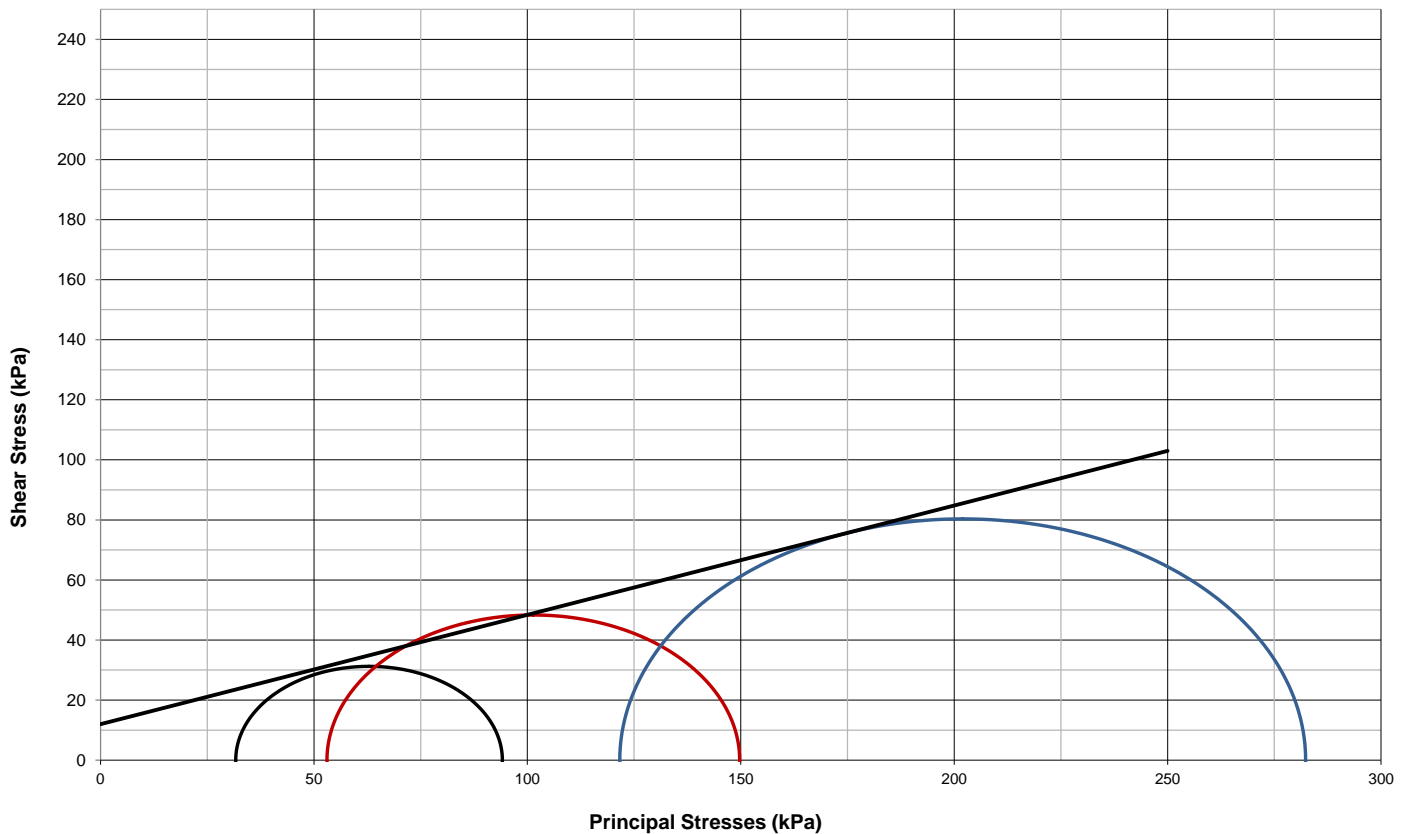
Sample Condition: Undisturbed

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### Shear Parameters of Effective Stresses

Angle of Internal Friction	Deg.	20
Cohesion	kPa	12

### Effective Shear Strength



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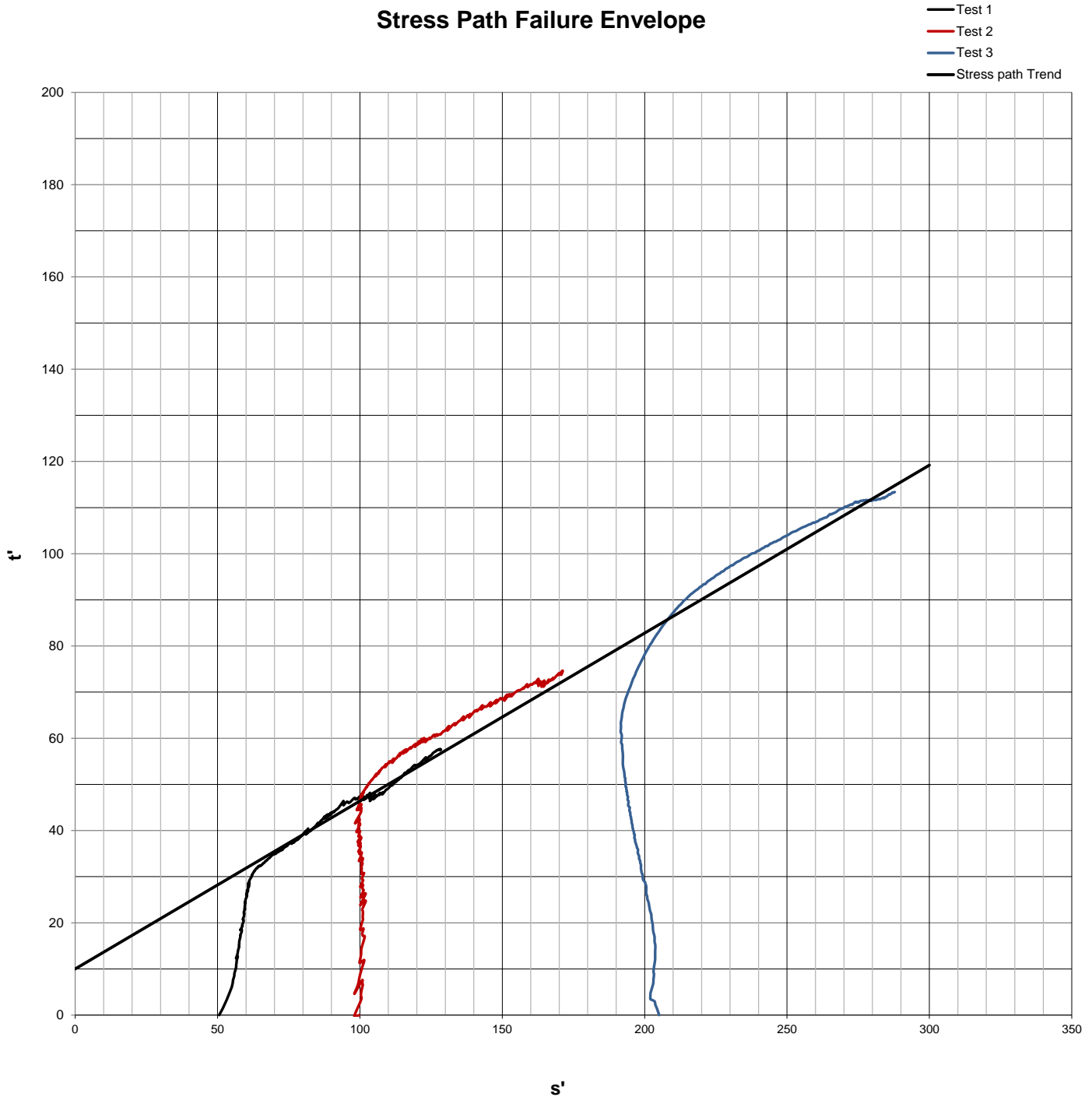
**Client:** EPOCH RESOURCES  
**Sample no:** TP 14  
**Lab no:** G21-0566

**Project:** THARISA FW WRD 2  
**Depth (m):** 3.1-3.4  
**Sample Condition:** Undisturbed

**Job no:** 39458  
**Date:** 15/09/2021

Shear Parameters at Failure		
Angle of Internal Friction	Deg.	20
Cohesion	kPa	10

**Stress Path Failure Envelope**



Client: EPOCH RESOURCES  
Sample no: TP 14  
Lab no: G21-0566

Project: THARISA FW WRD 2  
Depth (m): 3.1-3.4

Job no: 39458  
Date: 15/09/2021

**Test 1**



BEFOR TEST



AFTER TEST

**Test 2**



BEFOR TEST



AFTER TEST

**Test 3**



BEFOR TEST



AFTER TEST



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## CONSOLIDATED UNDRAINED TRIAXIAL TEST

**BS 1377  
Part 8**

**Client:** EPOCH RESOURCES  
**Sample no:** TP 15  
**Lab no:** G21-0567

**Project:** THARISA FW WRD 2  
**Depth (m):** 3.2-3.5

**Job no:** 39458  
**Date:** 26/08/2021

Page 1 of 5


Test Information		
Test Type	-	Consolidated Undrained with PWP measurements, saturated
Sample Condition	-	Undisturbed
Saturation Method		Increments of Cell- and Backpressure
Consolidation Pressures	kPa	50, 100, 200
Rate of Strain	%/min	0.0104
Failure Criterion	-	Maximum Deviator Stress
Side Drains Used	-	Yes
Drainage Conditions	-	To One End
Comments	-	Test 1, Test 2 & Test 3 - Failure Criterion Taken at Maximum Change in Pore Pressure
		-

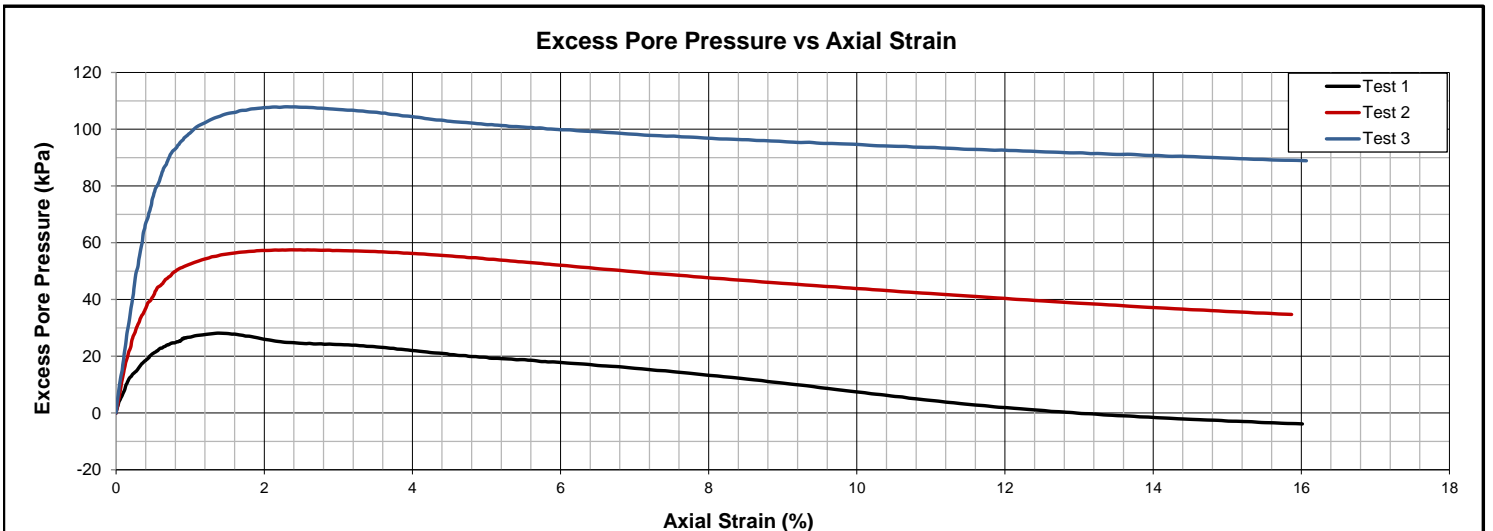
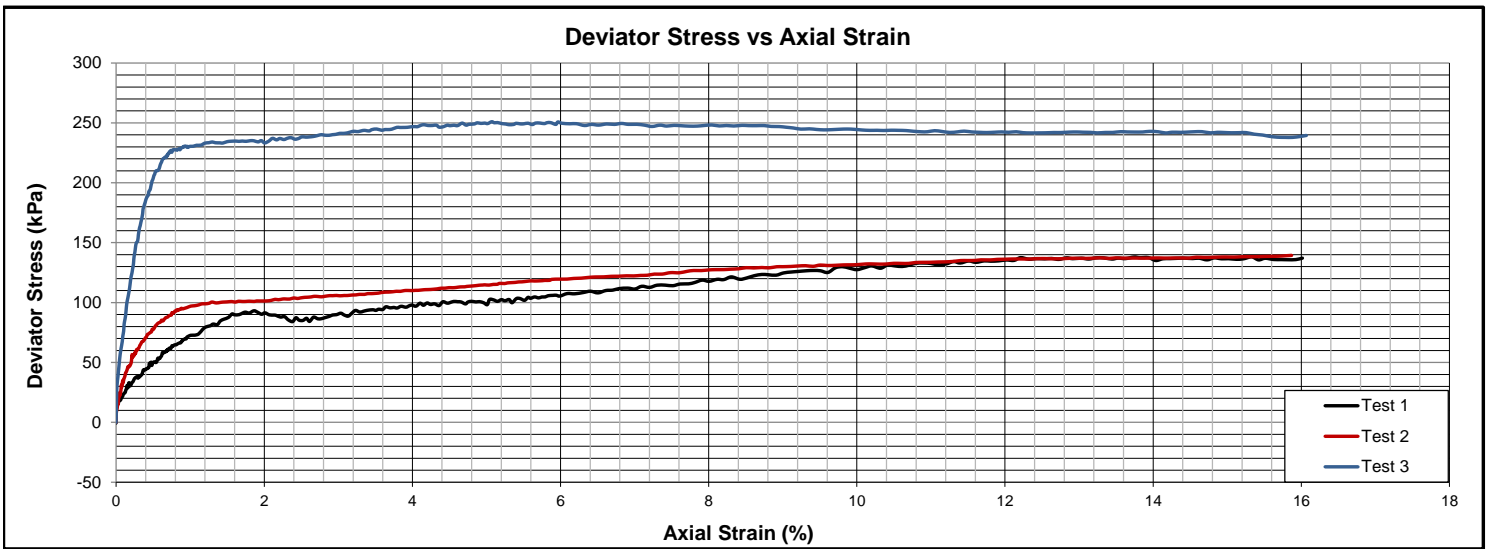
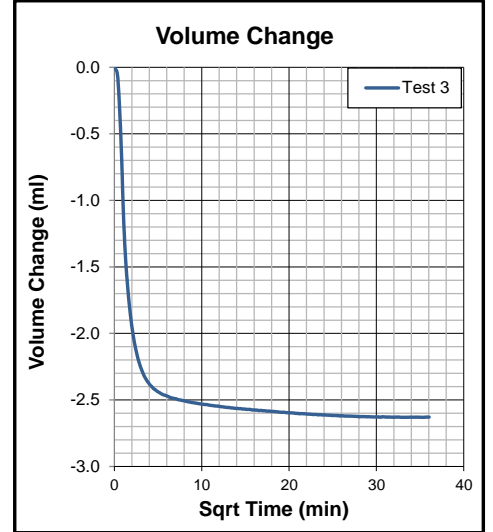
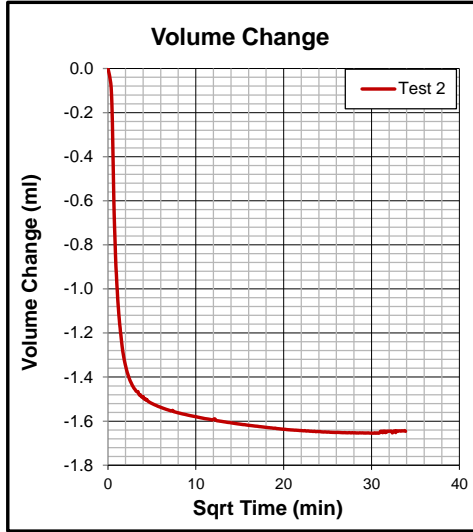
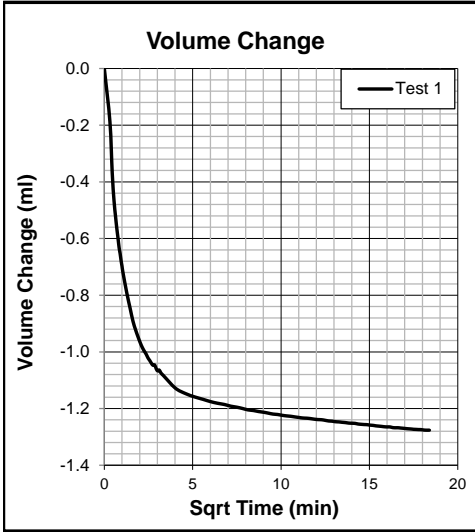
Initial Sample Parameters	Unit	Test 1	Test 2	Test 3	Remarks
Moisture Content	%	26.6	24.6	25.4	Complete test specimen
Dry Density	Kg/m <sup>3</sup>	1414	1477	1448	
Void Ratio	-	0.882	0.801	0.838	
Degree of Saturation	%	80.3	81.6	80.7	
Initial Height	cm	7.6	7.7	7.6	
Initial Diameter	cm	3.8	3.8	3.8	
Area (After Consolidation)	cm <sup>2</sup>	11.226	11.245	11.104	Calculated
Relative Density (SG)	-		2.661		Determined

Final Sample Parameters	Unit	Test 1	Test 2	Test 3	Remarks	
Moisture Content	%	31.9	29.0	29.5	Complete test specimen	
Dry Density	Kg/m <sup>3</sup>	1435	1506	1493		
Void Ratio	-	0.854	0.767	0.782		
Area	cm <sup>2</sup>	13.366	13.366	13.230	Calculated	
Eff. Consolidation Pressures	kPa	49	103	200		
Total Backpressure used	kPa	300	300	300	Saturation	
Final B Parameter	-	0.98	0.98	0.98		
Cell Pressure	kPa	350	400	500	Consolidation & Shear	
Axial Strain at Max. Deviator Stress	%	1.31	2.28	2.32		
Volume Change	ml	1.3	1.7	2.6	During Consolidation	
Principal Stresses at Max. Deviator Stress	$\sigma_1$	kPa	131	206	437	Corrected
	$\sigma_3$	kPa	49	103	200	Corrected
	$\sigma_1'$	kPa	103	148	329	Corrected
	$\sigma_3'$	kPa	21	45	92	Corrected

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	<h2 style="margin: 0;">CONSOLIDATED UNDRAINED TRIAXIAL TEST</h2>	<p><b>BS 1377</b> <b>Part 8</b></p>
<p><b>Client:</b> EPOCH RESOURCES <b>Sample no:</b> TP 15 <b>Lab no:</b> G21-0567</p>	<p><b>Project:</b> THARISA FW WRD 2 <b>Depth (m):</b> 3.2-3.5 <b>Sample Condition:</b> Undisturbed</p>	<p><b>Job no:</b> 39458 <b>Date:</b> 26/08/2021</p>
		<p>Page 2 of 5</p>



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## CONSOLIDATED UNDRAINED TRIAXIAL TEST

**BS 1377**  
**Part 8**

Client: EPOCH RESOURCES

Project: THARISA FW WRD 2

Job no: 39458

Sample no: TP 15

Depth (m): 3.2-3.5

Date: 26/08/2021

Lab no: G21-0567

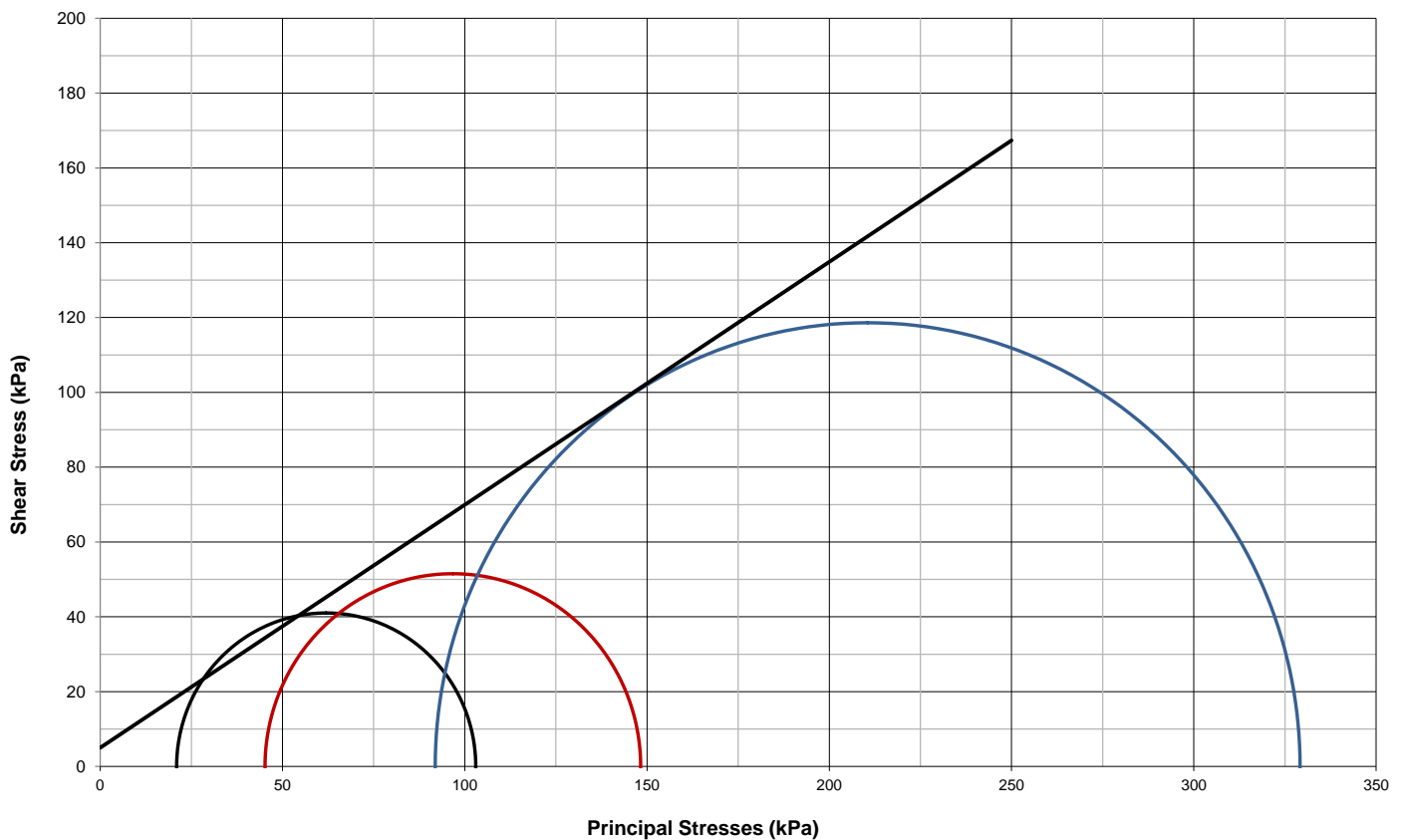
Sample Condition: Undisturbed

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### Shear Parameters of Effective Stresses


Angle of Internal Friction	Deg.	33
Cohesion	kPa	5

### Effective Shear Strength

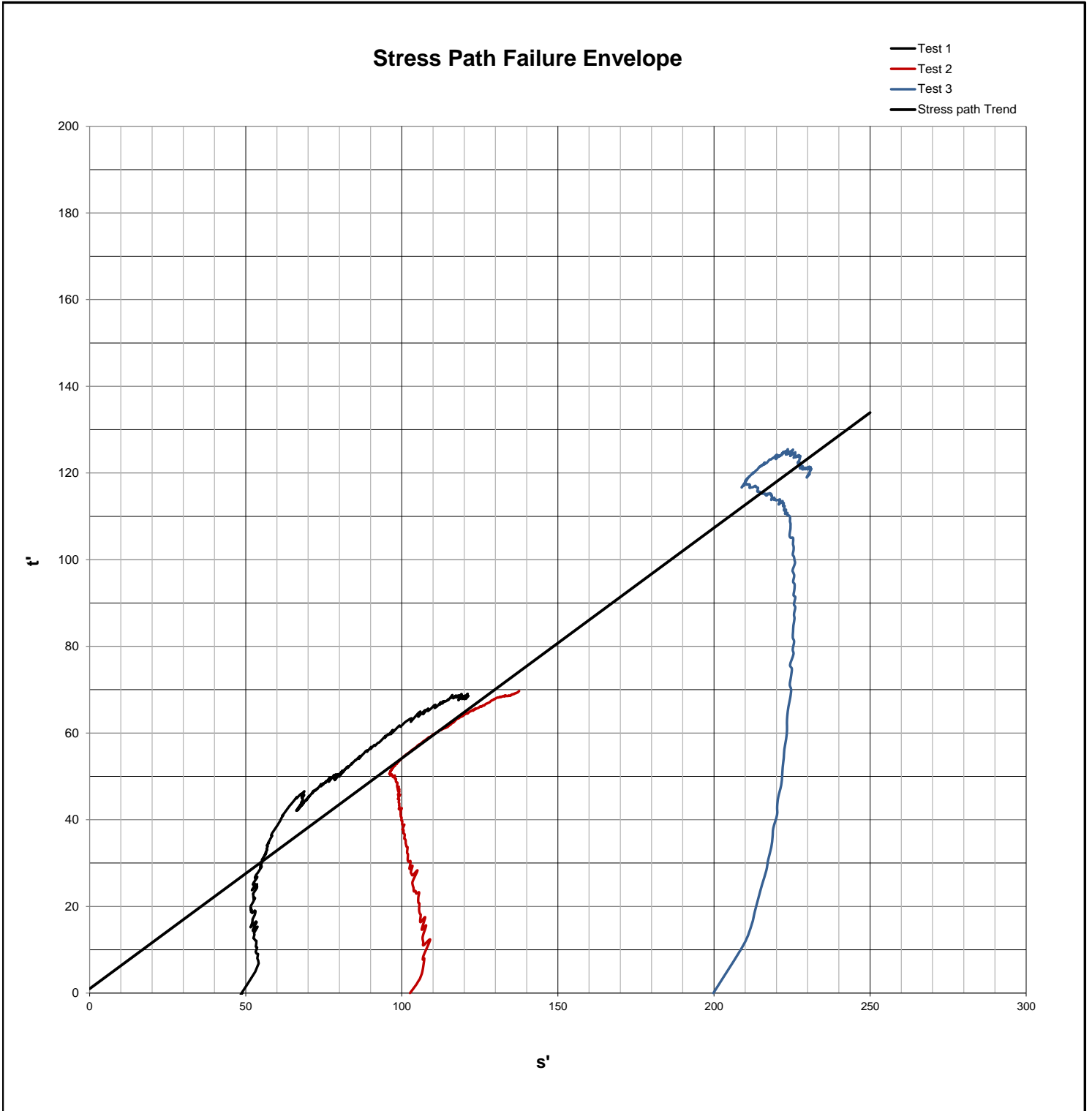


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	<h2 style="margin: 0;">CONSOLIDATED UNDRAINED TRIAXIAL TEST</h2>	<b>BS 1377</b> <b>Part 8</b>
<b>Client:</b> EPOCH RESOURCES <b>Sample no:</b> TP 15 <b>Lab no:</b> G21-0567	<b>Project:</b> THARISA FW WRD 2 <b>Depth (m):</b> 3.2-3.5 <b>Sample Condition:</b> Undisturbed	<b>Job no:</b> 39458 <b>Date:</b> 26/08/2021
		Page 4 of 5

Shear Parameters at Failure		
Angle of Internal Friction	Deg.	28
Cohesion	kPa	1



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Client: EPOCH RESOURCES  
Sample no: TP 15  
Lab no: G21-0567

Project: THARISA FW WRD 2  
Depth (m): 3.2-3.5

Job no: 39458  
Date: 26/08/2021

**Test 1**



BEFOR TEST



AFTER TEST

**Test 2**



BEFOR TEST



AFTER TEST


**Test 3**



BEFOR TEST



AFTER TEST


		<b>Flexible Wall Permeability Test</b>		<b>BS 1377 Part 6</b>
<b>Client</b> EPOCH RESOURCES <b>Sample no</b> TP 01 <b>Lab no</b> G21-0564	<b>Project</b> THARISA FW WRD 2 <b>Depth (m)</b> 2.1-2.4	<b>Job no</b> 39458 <b>Date</b> 29/09/2021		

Initial Sample Parameters		
Sample Condition	-	Remoulded by hand
Proctor	kg / m <sup>3</sup>	1845
OMC	%	6.7
Consolidation Pressure	kPa	100
Pressure Difference	kPa	10

Test Information			
Moisture Content	Before	%	6.8
	After	%	20.6
Dry Density		Kg/m <sup>3</sup>	1773
Initial Void Ratio		-	0.567
Relative Density (SG)		-	2.779 - Determined
Initial Degree of Saturation		%	33.1
Final B Parameter		-	0.98
Co-efficient of Permeability	Min.	m/s	2.40E-07
	Max.	m/s	1.45E-06
	Ave.	m/s	<b>6.92E-07</b>

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		<b>Flexible Wall Permeability Test</b>		<b>BS 1377 Part 6</b>
<b>Client</b> EPOCH RESOURCES <b>Sample no</b> TP 05 <b>Lab no</b> G21-0565	<b>Project</b> THARISA FW WRD2 <b>Depth (m)</b> 1.0-1.3	<b>Job no</b> 39458 <b>Date</b> 15/09/2021		

Initial Sample Parameters		
Sample Condition	-	UNDISTURBED
Proctor	kg / m <sup>3</sup>	N/A
OMC	%	N/A
Consolidation Pressure	kPa	100
Pressure Difference	kPa	10

Test Information			
Moisture Content	Before	%	34.5
	After	%	37.2
Dry Density		Kg/m <sup>3</sup>	1225
Initial Void Ratio		-	1.081
Relative Density (SG)		-	2.55 - Determined
Initial Degree of Saturation		%	81.3
Final B Parameter		-	0.96
Co-efficient of Permeability	Min.	m/s	1.39E-11
	Max.	m/s	7.19E-10
	Ave.	m/s	<b>3.09E-10</b>


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		<b>Flexible Wall Permeability Test</b>		<b>BS 1377 Part 6</b>
<b>Client</b> EPOCH RESOURCES <b>Sample no</b> TP 14 <b>Lab no</b> G21-0566	<b>Project</b> THARISA FW WRD 2 <b>Depth (m)</b> 3.1-3.4	<b>Job no</b> 39458 <b>Date</b> 15/10/2021		

Initial Sample Parameters		
Sample Condition	-	UNDISTURBED
Proctor	kg / m <sup>3</sup>	N/A
OMC	%	N/A
Consolidation Pressure	kPa	100
Pressure Difference	kPa	10

Test Information			
Moisture Content	Before	%	14.8
	After	%	16.9
Dry Density		Kg/m <sup>3</sup>	1841
Initial Void Ratio		-	0.440
Relative Density (SG)		-	2.651 - Determined
Initial Degree of Saturation		%	89.3
Final B Parameter		-	0.98
Co-efficient of Permeability	Min.	m/s	2.57E-10
	Max.	m/s	5.54E-10
	Ave.	m/s	<b>4.06E-10</b>


	<b>Flexible Wall Permeability Test</b>		<b>BS 1377 Part 6</b>
	<b>Client</b> EPOCH RESOURCES <b>Sample no</b> TP 14 <b>Lab no</b> G21-0566	<b>Project</b> THARISA FW WRD 2 <b>Depth (m)</b> 3.1-3.4	<b>Job no</b> 39458 <b>Date</b> 15/10/2021

Initial Sample Parameters		
Sample Condition	-	UNDISTURBED
Proctor	kg / m <sup>3</sup>	N/A
OMC	%	N/A
Consolidation Pressure	kPa	100
Pressure Difference	kPa	20

Test Information			
Moisture Content	Before	%	14.8
	After	%	16.9
Dry Density		Kg/m <sup>3</sup>	1841
Initial Void Ratio		-	0.440
Relative Density (SG)		-	2.651 - Determined
Initial Degree of Saturation		%	89.3
Final B Parameter		-	0.98
Co-efficient of Permeability	Min.	m/s	7.06E-11
	Max.	m/s	6.97E-09
	Ave.	m/s	<b>1.60E-09</b>

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	<b>Flexible Wall Permeability Test</b>		<b>BS 1377 Part 6</b>
	<b>Client</b> EPOCH RESOURCES <b>Sample no</b> TP 14 <b>Lab no</b> G21-0566	<b>Project</b> THARISA FW WRD 2 <b>Depth (m)</b> 3.1-3.4	<b>Job no</b> 39458 <b>Date</b> 15/10/2021


Initial Sample Parameters		
Sample Condition	-	UNDISTURBED
Proctor	kg / m <sup>3</sup>	N/A
OMC	%	N/A
Consolidation Pressure	kPa	100
Pressure Difference	kPa	50

Test Information			
Moisture Content	Before	%	14.8
	After	%	16.9
Dry Density		Kg/m <sup>3</sup>	1841
Initial Void Ratio		-	0.440
Relative Density (SG)		-	2.651 - Determined
Initial Degree of Saturation		%	89.3
Final B Parameter		-	0.98
Co-efficient of Permeability	Min.	m/s	2.86E-11
	Max.	m/s	2.24E-09
	Ave.	m/s	<b>6.07E-10</b>

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	<b>Flexible Wall Permeability Test</b>		<b>BS 1377 Part 6</b>
	<b>Client</b> EPOCH RESOURCES <b>Sample no</b> TP 15 <b>Lab no</b> G21-0567	<b>Project</b> THARISA FW WRD 2 <b>Depth (m)</b> 3.2-3.5	<b>Job no</b> 39458 <b>Date</b> 21/09/2021

Initial Sample Parameters		
Sample Condition	-	UNDISTURBED
Proctor	kg / m <sup>3</sup>	N/A
OMC	%	N/A
Consolidation Pressure	kPa	100
Pressure Difference	kPa	10

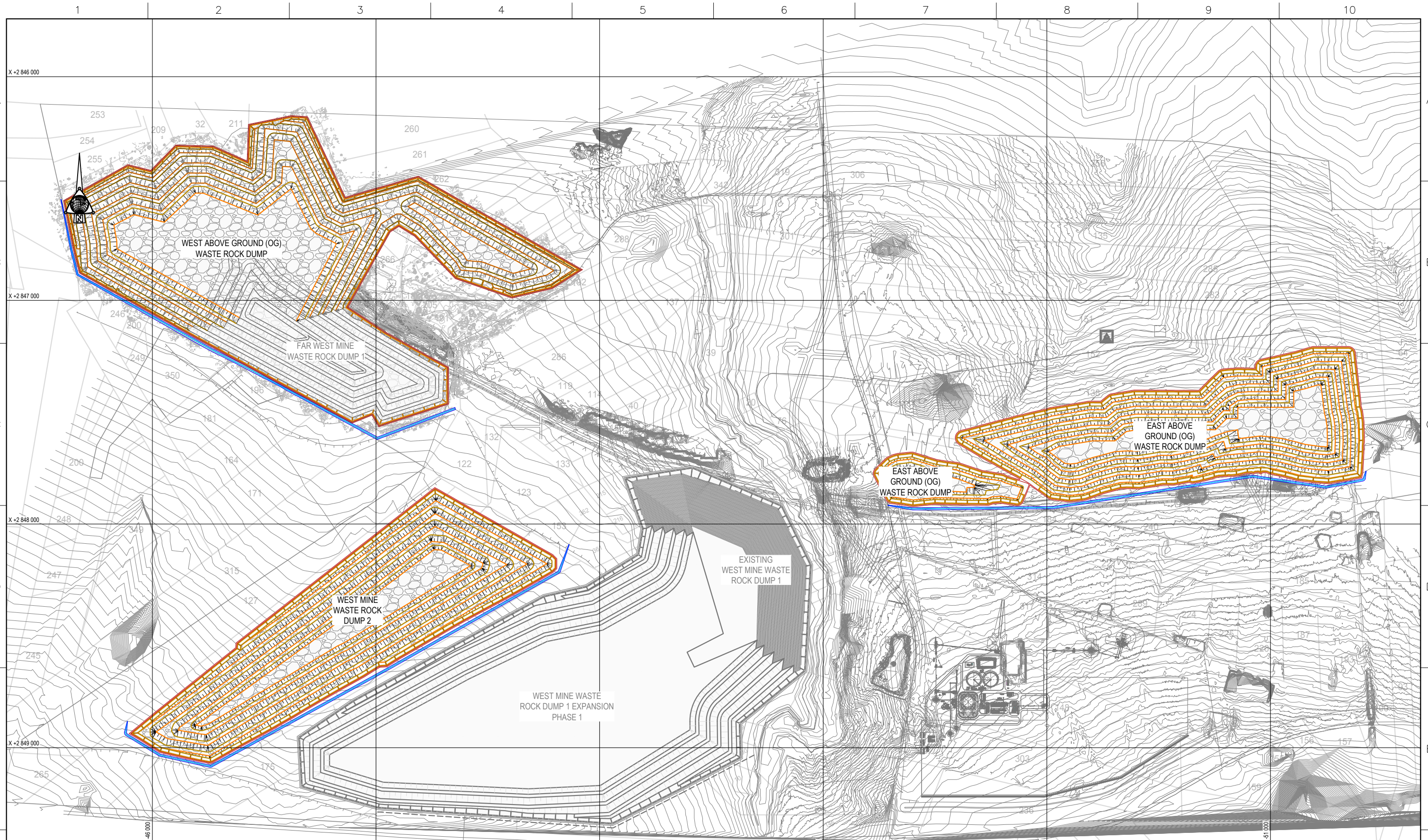
Test Information			
Moisture Content	Before	%	31.1
	After	%	39.1
Dry Density		Kg/m <sup>3</sup>	1231
Initial Void Ratio		-	1.162
Relative Density (SG)		-	2.661 - Determined
Initial Degree of Saturation		%	71.3
Final B Parameter		-	0.96
Co-efficient of Permeability	Min.	m/s	1.46E-08
	Max.	m/s	3.80E-08
	Ave.	m/s	<b>2.25E-08</b>

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**APPENDIX 2: DRAWINGS**







**DO NOT SCALE**

**FOR INFORMATION**

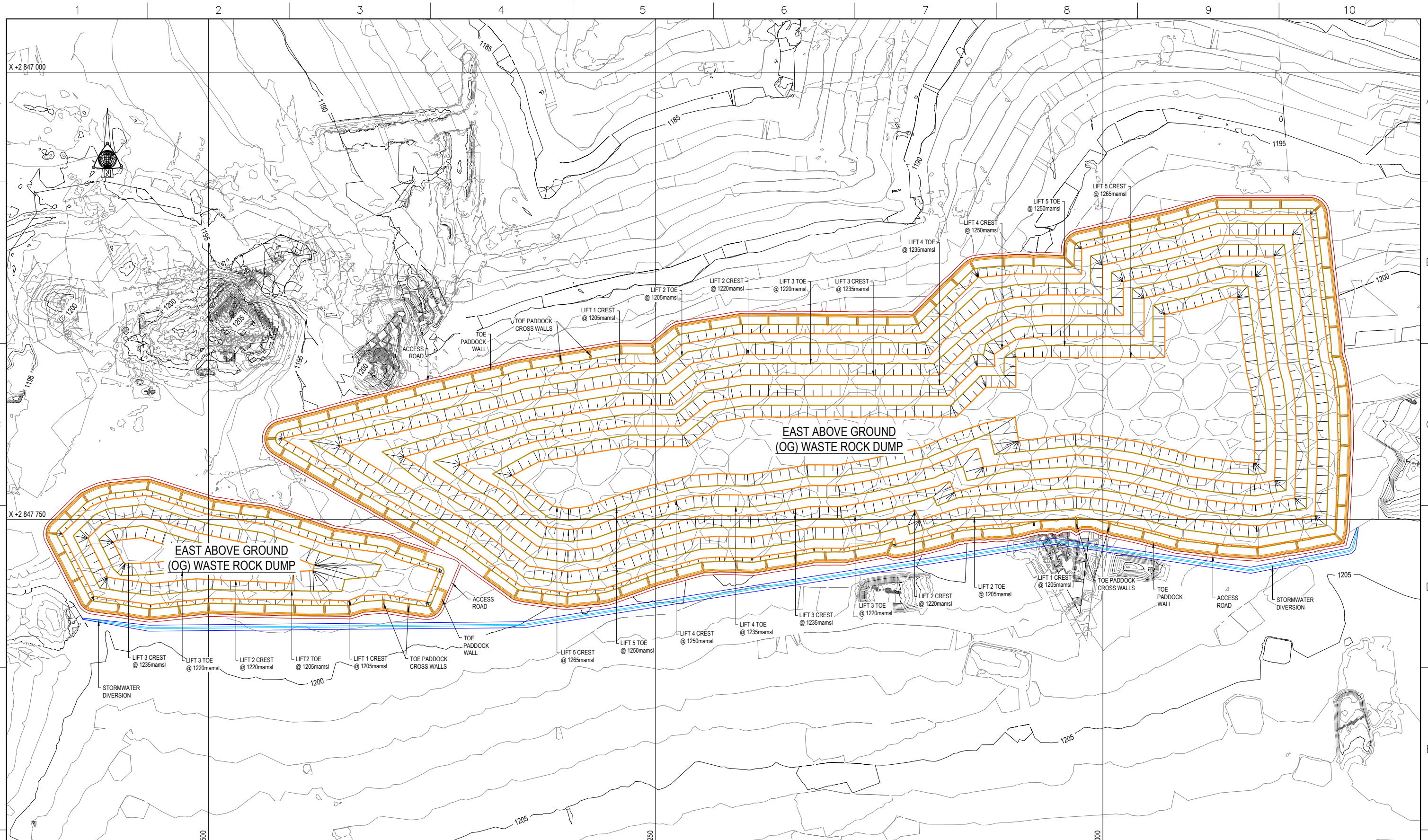
144-016-951	FAR WEST WASTE ROCK DUMP 2 - GENERAL ARRANGEMENT
144-016-954	EAST PIT WASTE ROCK DUMP - GENERAL ARRANGEMENT
144-016-957	WEST MINE WASTE ROCK DUMP 2 - GENERAL ARRANGEMENT
DRAWING No.	DRAWING TITLE
	REFERENCE DRAWINGS


3	MADE	DATE	2	MADE	DATE	1	MADE	DATE	0	MADE	DATE	C	MADE	DATE	B	MADE	DATE	A	MADE	DATE
	APPD			APPD			APPD			APPD			APPD			APPD			APPD	

This drawing is confidential and the property of Epoch Resources (PTY)Limited and is not to be communicated to any third party nor must it be used except in relation to the contract in connection with which it has been supplied.		DRAWN MUHAMMED DATE MARCH, 2022
PROFESSIONAL REGISTRATION NAME: G.J. WIID PR.ENG.No. 940269 DATE: 29/03/2022	ISSUED FOR: MANUFACTURE <input type="checkbox"/> CONSTRUCTION <input type="checkbox"/> APPROVAL <input type="checkbox"/> COMMENT <input type="checkbox"/> INFORMATION <input checked="" type="checkbox"/>	CHECKED BY A. STOFFBERG DATE MARCH, 2022
SIGNED:  SIGNED:	APPROVED (ENG) DATE APPROVED (CLIENT) DATE	CO-ORD SYSTEM UTM WGS 84 L27
		SCALE A1 1:8000 A3 1:16000

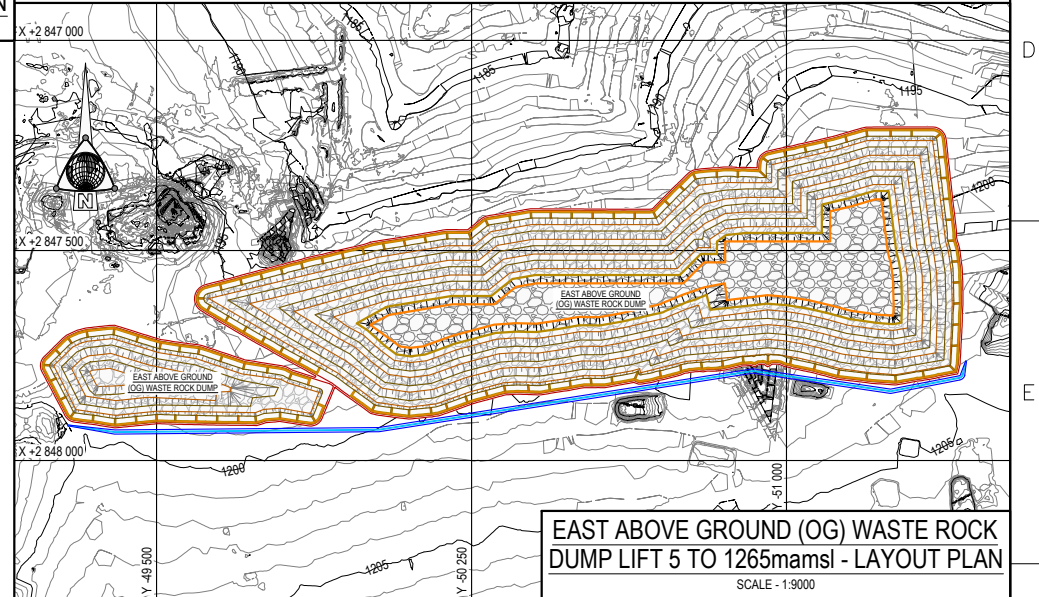
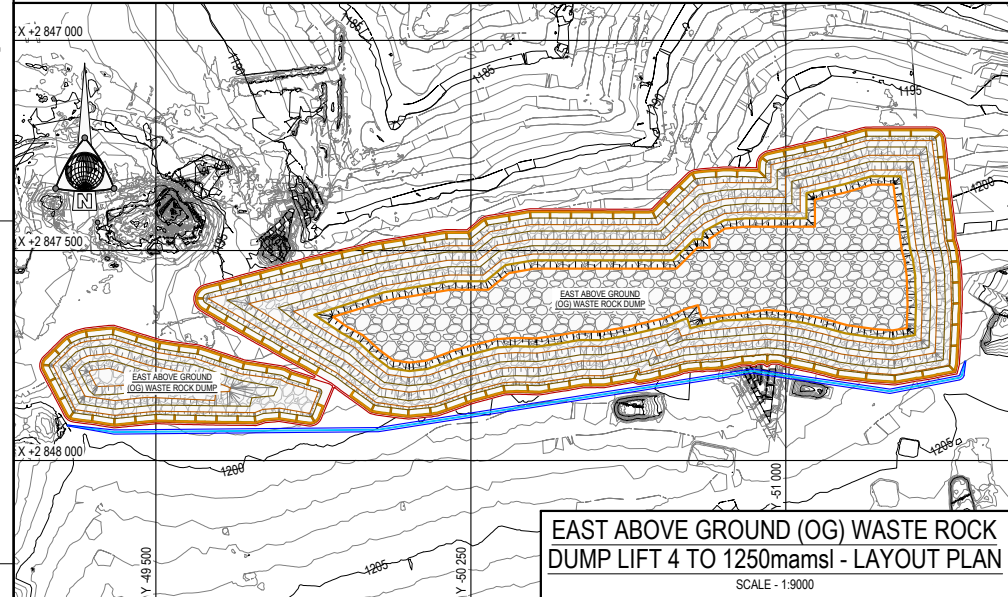
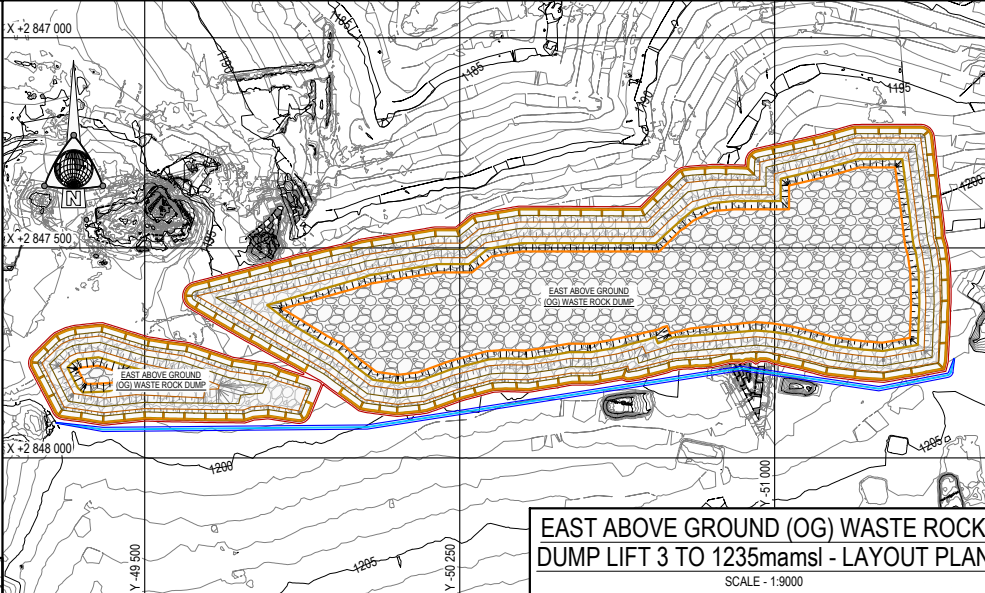
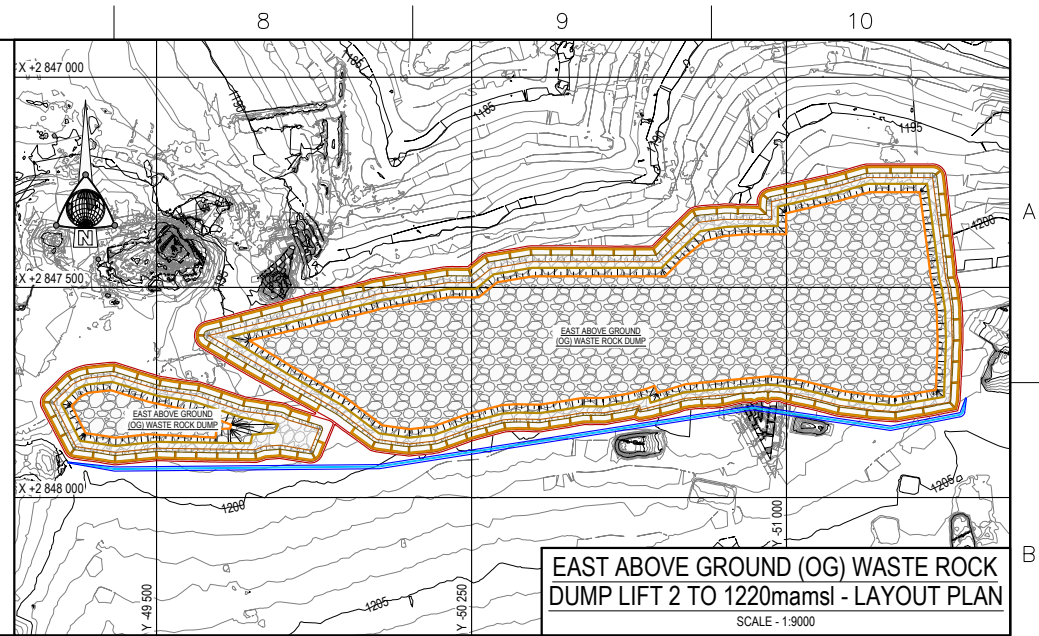
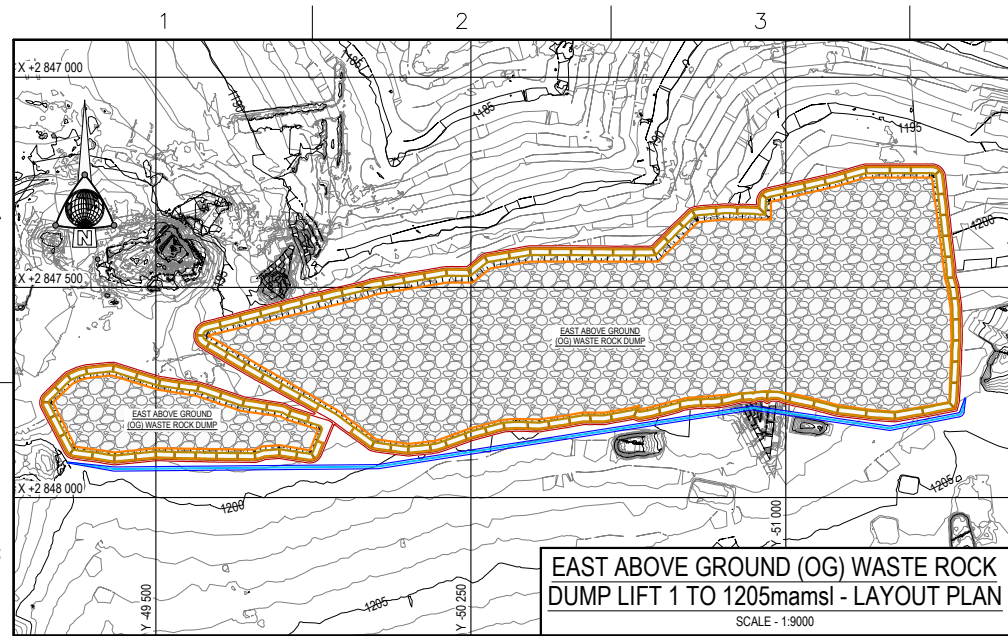
 mine residue and environmental engineering consultants	
TITLE THARISA MINERALS WASTE ROCK DUMPS SITE OVERVIEW GENERAL ARRANGEMENT	
DRG No. 144-016-950	REV. A





<b>DO NOT SCALE</b>		<b>FOR INFORMATION</b>																										
144-016-950	WASTE ROCK DUMPS - SITE OVERVIEW - GENERAL ARRANGEMENT	3	MADE	DATE	2	MADE	DATE	1	MADE	DATE	0	MADE	DATE	C	MADE	DATE	B	MADE	DATE	A	MADE	DATE	23.09.21	This drawing is confidential and the property of Epoch Resources (PTY)Limited and is not to be communicated to any third party nor must it be used except in relation to the contract in connection with which it has been supplied.	DRAWN	L. STEINHOBEL	 mine residue and environmental engineering consultants	
144-016-955	EAST PIT WASTE ROCK DUMP - PHASE DEVELOPMENT - GENERAL ARRANGEMENT		APPD	DATE		APPD	DATE		APPD	DATE		APPD	DATE		APPD	DATE		APPD	DATE		APPD	DATE			DATE	MARCH, 2022		TITLE
144-016-956	EAST PIT WASTE ROCK DUMP - TYPICAL SECTIONS AND DETAILS																							APPROVED (ENG)				
																								APPROVED (CLIENT)				
																								DATE				
																								CO-ORD SYSTEM	SCALE			
																								UTM	A1	1:3000	DRG No.	
																								WGS 84	A3	1:6000	144-016-954	REV.
																											A	





**DO NOT SCALE**

144-016-950	WASTE ROCK DUMPS - SITE OVERVIEW - GENERAL ARRANGEMENT	3	MADE	DATE	2	MADE	DATE	1	MADE	DATE	0	MADE	DATE	C	MADE	DATE	B	MADE	DATE	A	MADE	DATE	23.09.21
144-016-954	EAST PIT WASTE ROCK DUMP - GENERAL ARRANGEMENT		APPD	DATE		APPD	DATE		APPD	DATE		APPD	DATE		APPD	DATE		APPD	DATE		APPD	DATE	
144-016-956	EAST PIT WASTE ROCK DUMP - TYPICAL SECTIONS AND DETAILS																						
DRAWING No.	DRAWING TITLE																						
	REFERENCE DRAWINGS																						

This drawing is confidential and the property of Epoch Resources (PTY)Limited and is not to be communicated to any third party nor must it be used except in relation to the contract in connection with which it has been supplied.		DRAWN	L. STEINHOBEL
PROFESSIONAL REGISTRATION	ISSUED FOR: MANUFACTURE <input type="checkbox"/> CONSTRUCTION <input type="checkbox"/> APPROVAL <input type="checkbox"/> COMMENT <input type="checkbox"/> INFORMATION <input checked="" type="checkbox"/>	CHECKED BY	MARCH, 2022
NAME: G.J. WIID	DATE: 29/03/2022	DATE	
PR.ENG.No. 940269	SIGNED:	APPROVED (ENG)	
DATE: 29/03/2022		DATE	
SIGNED:		APPROVED (CLIENT)	
		DATE	
		CO-ORD SYSTEM	SCALE
		UTM	A1 1:9000
		WGS 84	A3 1:18000
		L27	

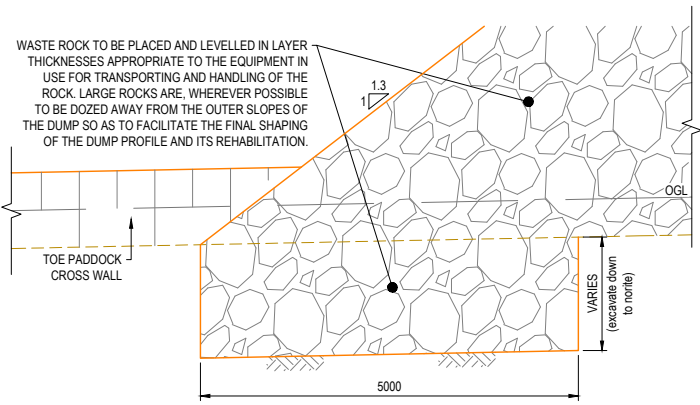
**FOR INFORMATION**

 mineral residue and environmental engineering consultants	
TITLE <b>THARISA MINERALS EAST ABOVE GROUND (OG) WASTE ROCK DUMP PHASE DEVELOPMENT GENERAL ARRANGEMENT</b>	
DRG No.	144-016-955
REV.	A

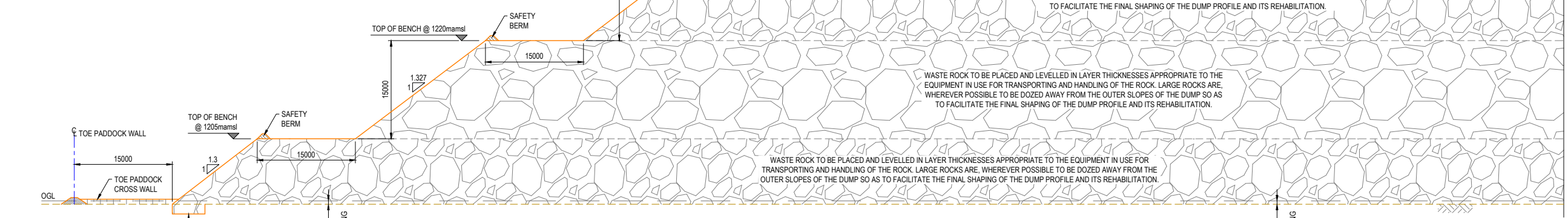


**LEGEND:**  
 RIP AND RE-COMPACT IN-SITU MATERIAL 300mm DEEP TO AT LEAST 98% OF STANDARD PROCTOR DENSITY, AND WITHIN -X1 AND +X2% OF O.M.C.

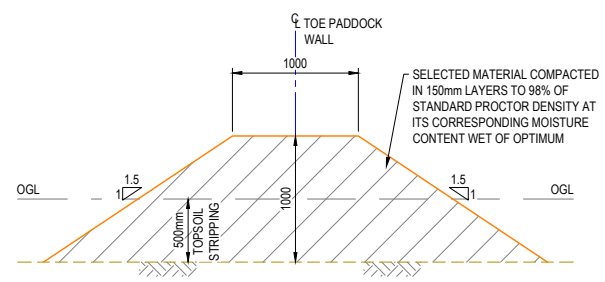
A  
B  
C  
D  
E  
F  
A1



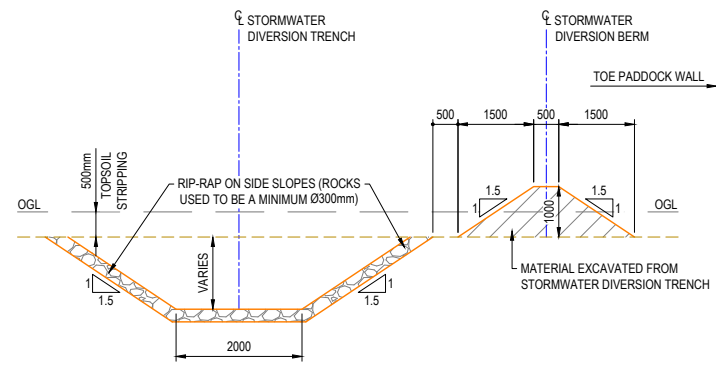
**TYPICAL CLAY KEY EXCAVATION DETAIL**  
SCALE - 1:50



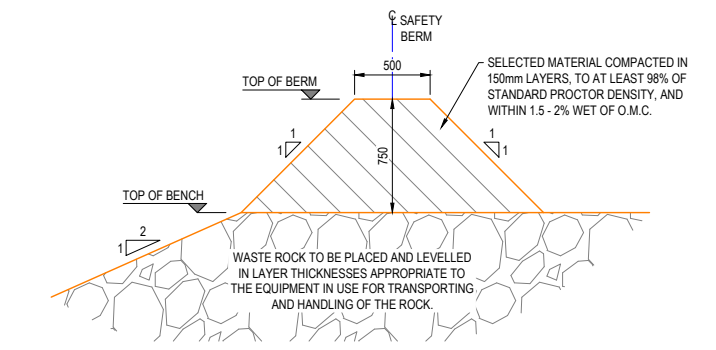
**TYPICAL SECTION THROUGH PHASED DEVELOPMENT OF EAST ABOVE GROUND (OG) WASTE ROCK DUMP**  
SCALE - 1:350



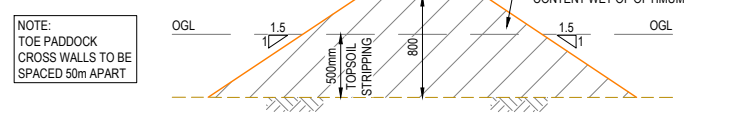
**TYPICAL TOE PADDOCK WALL DETAIL**  
SCALE 1:30



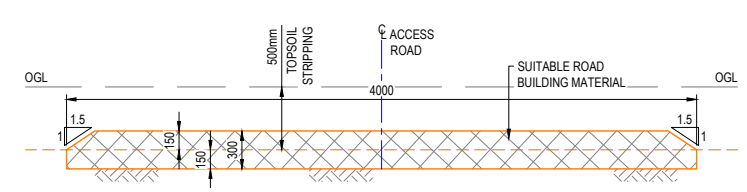
**TYPICAL SECTION THROUGH STORMWATER DIVERSION**  
SCALE - 1:75



**TYPICAL SECTION THROUGH WRD SAFETY BERM**  
SCALE - 1:25



**TYPICAL TOE PADDOCK CROSS-WALL DETAIL**  
SCALE 1:30



**TYPICAL ACCESS ROAD DETAIL**  
SCALE - 1:30

NOTE:  
TOE PADDOCK  
CROSS WALLS TO BE  
SPACED 50m APART

**DO NOT SCALE**

**FOR INFORMATION**

144-016-954	EAST PIT WASTE ROCK DUMP - GENERAL ARRANGEMENT	3	MADE APPD	DATE	2	MADE APPD	DATE	1	MADE APPD	DATE	0	MADE APPD	DATE	C	MADE APPD	DATE	B	MADE APPD	DATE	A	MADE APPD	DATE	23.09.21	This drawing is confidential and the property of Epoch Resources (PTY)Limited and is not to be communicated to any third party nor must it be used except in relation to the contract in connection with which it has been supplied.	DRAWN MUHAMMED DATE MARCH, 2022 CHECKED BY S. BARKHUIZEN DATE MARCH, 2022 APPROVED (ENG) DATE APPROVED (CLIENT) DATE CO-ORD SYSTEM UTM WGS 84 L27 SCALE A1 AS SHOWN A3 AS SHOWN x 2	TITLE THARISA MINERALS EAST ABOVE GROUND (OG) WASTE ROCK DUMP TYPICAL SECTIONS AND DETAILS DRG No. 144-016-956 REV. A
DRAWING No.	DRAWING TITLE																									
	REFERENCE DRAWINGS																									



THARISA MINERALS  
EAST ABOVE GROUND (OG) WASTE ROCK DUMP  
TYPICAL SECTIONS AND DETAILS

144-016-956

REV. A



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