



HILLARDIA PV (PTY) LTD

HILLARDIA PV

Stormwater Management Plan

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| Author: | Merchandt Le Maitre (Pr. Tech Eng.) | |
| Signature: |  Pr. N°: 2018300094 | Date: 18 th July 2022 |
| Reviewed: | Richard Hirst (Pr Tech Eng.) | |
| Signature: |  Pr. N°: 2018300110 | Date: 18 th July 2022 |
| For: | HILLARDIA PV (PTY) LTD | |
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EXECUTIVE SUMMARY

Objective

The Applicant, Hillardia PV (Pty) Ltd, proposes the construction of a photovoltaic (PV) solar energy facility, known as the Hillardia PV facility located on a site ±10 km northwest of the town of Lichtenburg in the North West Province. The development area is situated within the Ditsobotla Local Municipality within the Ngaka Modiri Molema District Municipality and is accessible via the R505, located ±2.3 km east of the development area. The solar PV facility will comprise several arrays of PV panels and associated infrastructure and will have a contracted capacity of up to 120 MW.

The main objective of the 'Stormwater Management Plan' is to determine the impact/s of the proposed development on the immediate and greater area concerning stormwater and to include these findings in the Environmental Impact Assessment (EIA) submission. The assessment will comprise a desktop assessment and include preliminary stormwater-related matters arising during the construction phase, through the Operation & Maintenance Phase, up to and including the decommissioning phase of the development.

The proposed Hillardia PV Facility forms part of cluster development with two additional developments adjacent to this facility as separate EIA applications: - Verbena PV and Euphorbia PV. Although this report only focuses on the Hillardia PV Facility, all three developments are considered for this study as they share common boundaries adjacent to each other.

Key Findings

We don't foresee any significant risks concerning the proposed development and therefore include our recommendations below to take note of before and during the detailed design and construction stages. However, it should be noted that several recommendations were highlighted and therefore noted as important.

The proposed development / infrastructure will have a minimal impact on the stormwater quality and quantities post-development (operational phase). This development's construction phase typically generates the highest surface run-off during the wet season. However, it will be temporary, and impacts can be mitigated and considered nominal. The post-development stormwater flow from the operation phase will have a minimal impact on the immediate environment if adequate stormwater designs are implemented to maintain existing drainage patterns and flows in the catchment.

Many mitigation measures are proposed to accommodate the development and reduce the impact on the surrounding area.

Recommendation

Concerning this report, associated assessment and the findings made within, it is SiVEST's opinion that the Hillardia PV and associated grid infrastructure will have a nominal impact on the existing stormwater catchment. The project is therefore deemed acceptable from a stormwater perspective, provided the recommendations and mitigation measures in this report are implemented. Hence, Environmental Authorisations (EAs) should be granted for the EIA applications.

This document should also be read in conjunction with the EMP. The developer, owner, and professional team shall ensure that the requirements and conditions set out in the EMP are adhered to.

DECLARATION BY SPECIALIST

I, MERCHANDT LE MAITRE, declare that –

- I act as the independent specialist in this application;
- I will perform the work relating to the application in an objective manner, even if this results in views and findings that are not favourable to the applicant;
- I declare that there are no circumstances that may compromise my objectivity in performing such work;
- I have expertise in conducting the specialist report relevant to this application, including knowledge of the Act, Regulations and any guidelines that have relevance to the proposed activity;
- I will comply with the Act, Regulations and all other applicable legislation;
- I have no, and will not engage in, conflicting interests in the undertaking of the activity;
- I undertake to disclose to the applicant and the competent authority all material information in my possession that reasonably has or may have the potential of influencing - any decision to be taken with respect to the application by the competent authority; and - the objectivity of any report, plan or document to be prepared by myself for submission to the competent authority;
- all the particulars furnished by me in this form are true and correct; and
- I realise that a false declaration is an offence in terms of regulation 48 and is punishable in terms of section 24F of the Act.

Signature of Specialist:



Name of Company: SiVEST SA (PTY) Ltd

Date: 18th July 2022

HILLARDIA PV (PTY) LTD

HILLARDIA PV FACILITY

STORMWATER MANAGEMENT PLAN

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1 INTRODUCTION

SiVEST Civil Engineering Division was appointed by Hillardia PV (Pty) Ltd (hereafter referred to as "Hillardia PV") to complete a Stormwater Management Plan (SWMP) for the proposed 120MW Hillardia PV Facility and associated grid infrastructure (hereafter the "proposed facility / facilities") situated ± 10 km north-west of the town Lichtenburg and is within the Ditsobotla Local Municipality, the greater Ngaka Modiri Molema District Municipality of the North West Province.

The proposed facility and associated grid infrastructure situated north of Vryburg & Klerksdorp will not be located within Renewable Energy Development Zones (REDZ). The development is however located between the 'Existing Vryburg REDZ' and 'Existing Klerksdorp REDZ'.

The proposed Hillardia PV Facility forms part of cluster development with two additional developments adjacent to this facility as separate EIA applications: - Verbena PV and Euphorbia PV. Although this report only focuses on the Hillardia PV Facility, all three developments are considered for this study as they share common boundaries adjacent to each other.

2 PV FACILITY COMPONENTS

The PV facility will consist of the following:

2.1 Solar Farm Components

The proposed Hillardia PV will comprise photovoltaic (PV) panels with a maximum total energy generation capacity of up to 120MW. The electricity generated by the proposed PV development will be fed into the national grid via a 132kV overhead power line. In summary, the proposed Hillardia PV will include the following components:

- PV panels (number of will be determined in the design phase), connected in series to form a 'string' of panels. Several strings are connected in parallel to form an 'array of modules / panels', each type between 4MW and 7MW, with a maximum export capacity of 120MW. However, the final number of panels and layout of the PV will depend on the outcome of the Specialist Studies conducted during the EIA process.
- Mounting structures that are either fixed, north-facing at a defined angle or single-axis tracking modules rotating in an east-west direction will be considered. (Will be determined at the design stage)
- Electrical transformers (690/11V to 33kV) adjacent to each inverter station (typical footprint of up to approximately 3m x 2.5m) step up the voltage to between 11kV and 33kV.
- One (1) new 11-33kV / 132kV on-site substation including associated equipment and infrastructure.
- A single inverter station is connected to several 'solar arrays' placed within the internal 'Right of Way' servitude.
- The inverter station will be connected to the proposed substation via medium voltage (33kV) cables. Cables will be buried underground along access roads wherever technically feasible.
- An overhead line servitude of 22m wide for the 33kV line and 31m wide for the 132kV line.
- Internal roads up to approximately 5m wide will provide access to each PV panel and inverter station. Existing site roads will be used wherever possible, although new site roads will be constructed where necessary.
- One (1) construction laydown area of up to approximately ± 3.0 ha. It should be noted that no construction camps will be required to house workers overnight as all workers will be accommodated in the nearby town.

- Operation and Maintenance (O&M) buildings, including offices, a guard house, operational control centre, O&M area / warehouse / workshop, canteen, visitor centre, and ablution facilities to be located on the site identified as Auxiliary Buildings. This site area is approximately ±1.0 ha.
- A new 2.0m high electric fence around the site perimeter will be erected.
- Water will either be sourced from existing boreholes within the application site or trucked in, should the boreholes within the application site be limited.

3 OBJECTIVE & SCOPE OF WORK

The main objective of the study is to develop a conceptual stormwater management plan for the proposed development during the operation & maintenance phase. To achieve this objective, the following will be assessed and discussed under their relevant headings in this report: -

- Climate
- Surface Hydrology
- Development Stormwater Management
- Development run-off Calculations
- Conclusions & Recommendations

The scope of work consists of the following:

- a) A site investigation was completed on March 30 2022.
- b) Consultations with the relevant authorities and / or stakeholders.
- c) Extract the climate of the area from sources commonly available
- d) Desktop analysis of the existing surface hydrology
- e) Evaluate the impact of the proposed development on the existing catchment and propose a suitable SWMP.
- f) Conclude & propose possible mitigation measures.
- g) Seasonal impacts affect this assessment.

3.1 Legal Requirement & Guidelines

Key legal requirements and guidelines for the proposed facilities are as follows:

- Government Notice 509 (GN509) as published in Government Gazette 40229 of 2016 and refers to the National Water Act, 1998 (Act No. 36 of 1998)
- National Environmental Management Act, 1998 (Act No 107 of 1998) (NEMA)
- National Water Act, 1998 (Act No 36 of 1998) (NWA)

4 SPECIALIST CREDENTIALS

Merchandt Le Maitre from SiVEST Consulting Engineers compiled this Stormwater Management Plan. He has a B Tech (Baccalaureus Technologiae) in Civil Engineering with over 17 years of experience, with 12 years in renewable energy. His extensive experience in the different facets of Civil Engineering means he can advise clients in the renewable energy sector in; geotechnical engineering, topographical

studies, stormwater management, water demand, transportation studies, access / layout designs and glint & glare assessments. A full Curriculum Vitae is included in 'Appendix A.

Table 4:1 Specialist Credentials & Experience

| | |
|--|--|
| Company | SiVEST (Pty) Ltd |
| Contact Details | merchandtm@sivest.co.za |
| Qualifications | B Tech (Baccalaureus Technologiae) in Civil Engineering |
| Professional Registrations & Memberships | <ul style="list-style-type: none"> • Pr. Tech Eng – Engineering Council of South Africa • MSAICE – Member of South African Institute of Civil Engineers • SAWEA – South African Wind Energy Association |
| Expertise to carry out the Stormwater Management Plan | <ul style="list-style-type: none"> • Dyansons Klip 5 • De Aar Solar • Droogfontein Solar • Mierdam Solar • Prieska PV • Hoekplaas PV • Noupoot WEF • Copperton PV • Klipgats PV |

5 ASSUMPTIONS AND LIMITATIONS

The following assumptions and limitations are to be noted:

- The analysis is based on the information provided at the time by Hillardia PV and its representatives.
- Digital Terrain Model: 25m DEM from NGI (2014) & 2m DEM from GeoSmart (2016:3222DA)
- Technical Specifications for the Facility:

Table 5:1 Technical Specification for Hillardia PV Facility

| Technical Component | Dimensions |
|---|---|
| PV Tracking System | Fixed axis tracking has been used for these simulations as it has the most significant impact on the surrounding road network |
| Height of PV Panels | ≤ 5.5 m |
| Area of PV Array | ±183 ha |
| Number of Panels and Inverters | To be determined at the detailed design phase |
| Area of Inverter / Transformer stations / substations / | The inverter / transformer stations will be located within the area of the PV array, while the main HV transformers will be located within the substation complex |
| Voltage of Substation Complex | 11kV/132kV - 33kV/132kV |
| Area of Substation Complex | ≤ 3 ha |
| Height of Substation Complex | ≤ 25m |

| Technical Component | Dimensions |
|---|--|
| The area occupied by laydown areas (Permanent and Construction) | Temporary Laydown Area: ± 5 ha (per facility) Permanent Laydown Area: Less than ± 1 ha will remain in place for operations (per facility) |
| The area occupied by Buildings | ≤ 1 ha for a site office and O&M buildings |
| Length of Access Road | ≤ 7 km |
| Width of Access Road | Up to 8m |
| Length of Internal Roads | ≤ 20km |
| Width of Internal Roads | Up to 8m |
| Construction Period | ±12 months |

6 PROJECT DESCRIPTION

6.1 Locality

Hillardia PV facility and associated infrastructure are located ±10km northwest of Lichtenburg in the North West Province. The facility is near the R505 regional road (R50506) between Lichtenburg and Bakerville (Refer to **Figure 6:2**) in the Ditsobotla Local Municipality and greater Ngaka Modiri Molema District Municipality, as indicated in **Figure 6:1**.

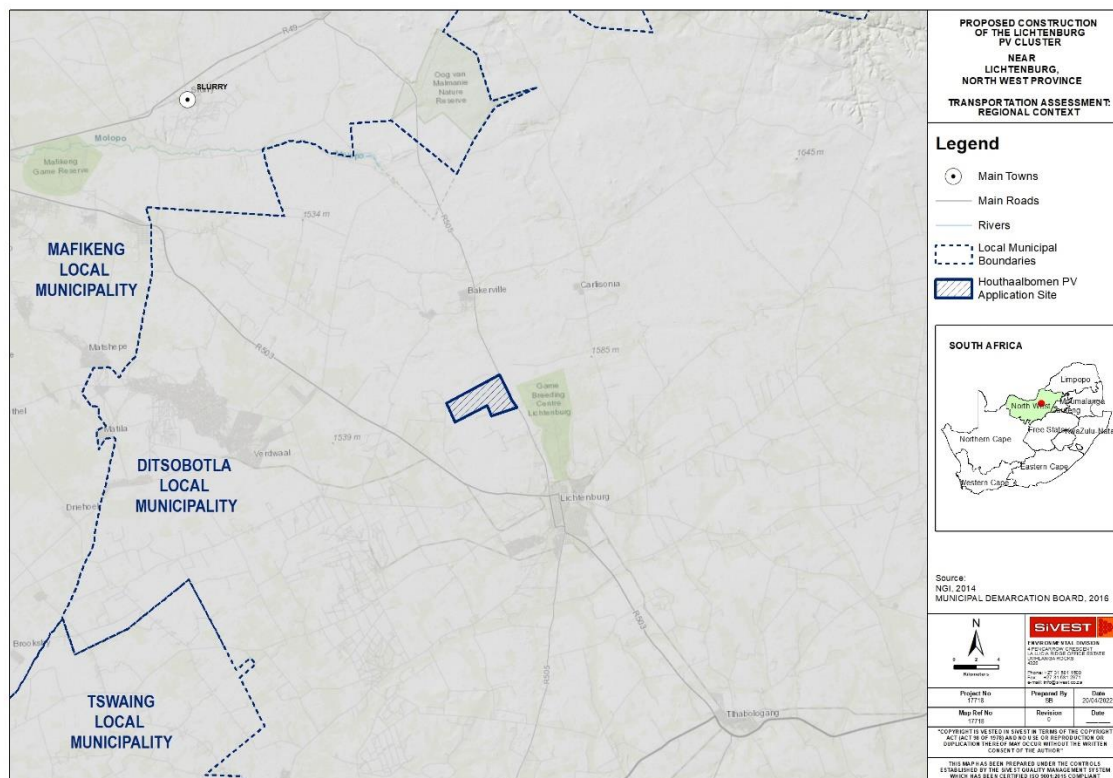


Figure 6:1 Hillardia PV - Regional Context

The development area for the PV facility and associated infrastructure will be located on the following properties (Refer to **Figure 6:2**):

- Portion 2 of the Farm Houthaalboomen 31
- Portion 3 of the Farm Houthaalboomen 31, and
- Portion 4 of the Farm Houthaalboomen 31

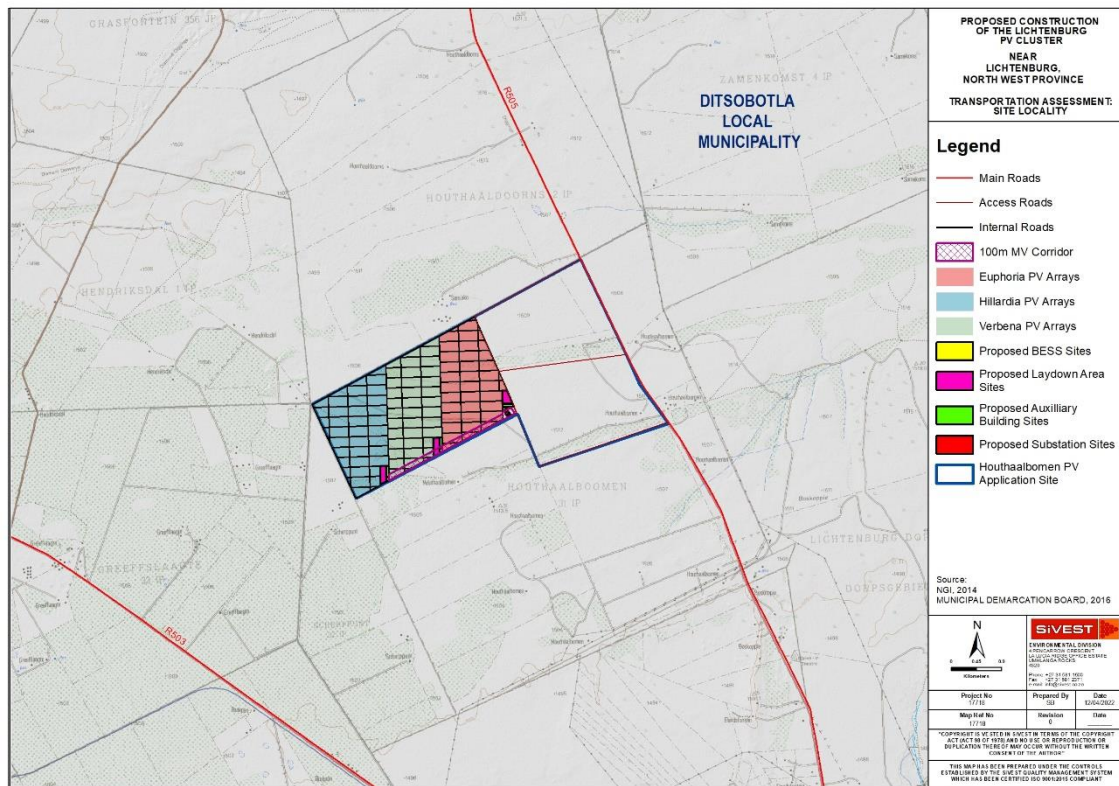


Figure 6:2 Hillardia PV - Site Locality (Hillardia PV in blue)

7 GEOTECHNICAL STUDY

A comprehensive Palaeontological Impact Assessment¹ for the proposed development was completed in February 2022 by Prof Marion Bamford for Messrs' Beyond Heritage (Pty) Ltd on the proposed sites indicated in **Section 6**.

A summary extract from the Palaeontological Impact Assessment confirms the site comprises the following geological context. Refer to **Figure 7:1** and **Table 7:1**:

¹ Van Der Walt, J (2022). Heritage Baseline Report: For the Houthaalboomen North PV Cluster (Euphorbia PV, Hillardia PV AND Verbena PV), Lichtenburg, North-West Province. Beyond Heritage.



Figure 7:1 Geological Map of Proposed Development Area

Abbreviations of the rock types are explained in **Table 7:1** below:

Table 7:1 Explanation of Figure 7:1 and Approximate Ages

| Symbol | Group / Formation | Lithology | Approximate Age |
|-------------|--|---|---|
| Qs | Quaternary | Alluvium, Sand, and Calcrete | Neogene, ca 2.5 Million years (Ma) to present |
| Qc | Quaternary Calcrete | Calcrete and Sand | Neogene, ca 2.5 Ma to present |
| C-Pd | Dwyka Group | Diamictites, Tillites, Mudstone, and Shales | Early Permian, Middle Ecca. Ca 280-270 Ma |
| Vml | Transvaal Super Group, Chuniespoort Group, Malmani Subgroup, and Littleton Formation | Dark Chert (Poor Dolomite) | Ca 2585 – 2480 Ma |
| Vmm | Transvaal Super Group, Chuniespoort Group, Malmani Subgroup, and Monte Christo Formation | Dark Chert (Poor Dolomite) | Ca 2585 – 2480 Ma |
| Vmo | Transvaal Super Group, Chuniespoort Group, Malmani Subgroup, and | Dark Chert (Free Dolomite) | Ca 2585 – 2480 Ma |

| Symbol | Group / Formation | Lithology | Approximate Age |
|------------|---|------------------------------------|-----------------|
| | Oaktree Formation | | |
| Bbr | Transvaal Super Group, and Black Reef Formation | Quartzite, Conglomerate, and Shale | < 2618 Ma |

In summary, the facility will have the following typical soil profile: -

- *Alluvium, Sand, and Calcrete (red soil) covering the site*
- *Chert-rich & poor dolomite underlying the red alluvium sand with exposed dolomite in patches*

Material excavation (soils and sand) is expected to be soft in the upper layers of alluvium / sand with *intermediate to hard* excavation techniques below in the dolomites.

We recommend that a comprehensive Geotechnical Report be carried out to form part of the detailed design stage and refinement of the SWMP.

8 CLIMATE

8.1 Climate Classification²

Hillardia PV is located ±10km northwest of Lichtenburg in the North West Province. Referring to the Klöppen-Geiger climate classification system, the North West Province has a variety of climates and is predominantly dominated by hot semi-arid climates (type 'BSh'). However, the Lichtenburg area is classified as a cold semi-arid climate (type 'BSk').

8.2 Average Temperature³

The Average Maximum temperatures range between 17.9° and 28.5°C. December is the warmest month of the year, with an average high temperature of 28.5°C. The coldest month of the year with an average low temperature of 5.4°C is the month of July. Refer to **Figure 8:1** below.

² en-climate-data

³ Weather Atlas

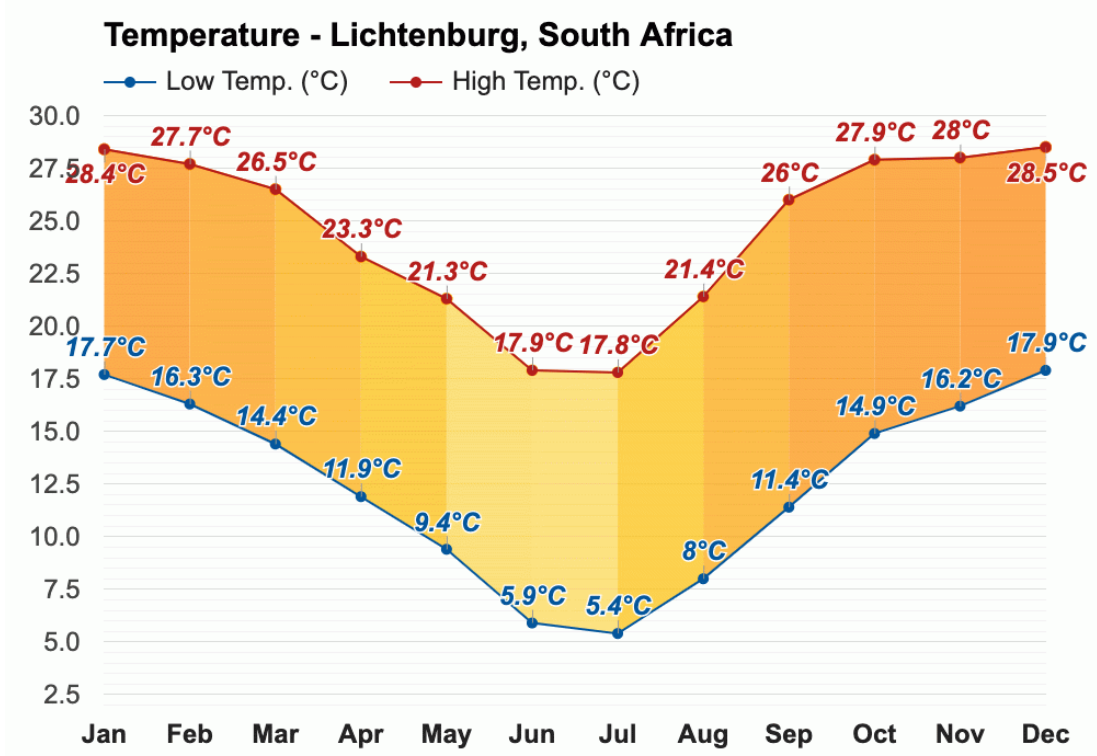


Figure 8:1 Average Temperature – Lichtenburg, South Africa

8.3 Mean Annual Precipitation (MAP)⁴

As mentioned in **Section 8.1** above, the Lichtenburg region is a cold semi-arid climate with an annual average rainfall of $\pm 351\text{mm}$, mainly between October and April. The month of December is on average, the wettest month of the year, with $\pm 72\text{mm}$ accumulated for the month. The driest months with the least amount of rainfall of $\pm 1\text{mm}$ accumulated for the month is the month of July and August, respectively. Refer to **Figure 8:2** below.

⁴ Weather Atlas

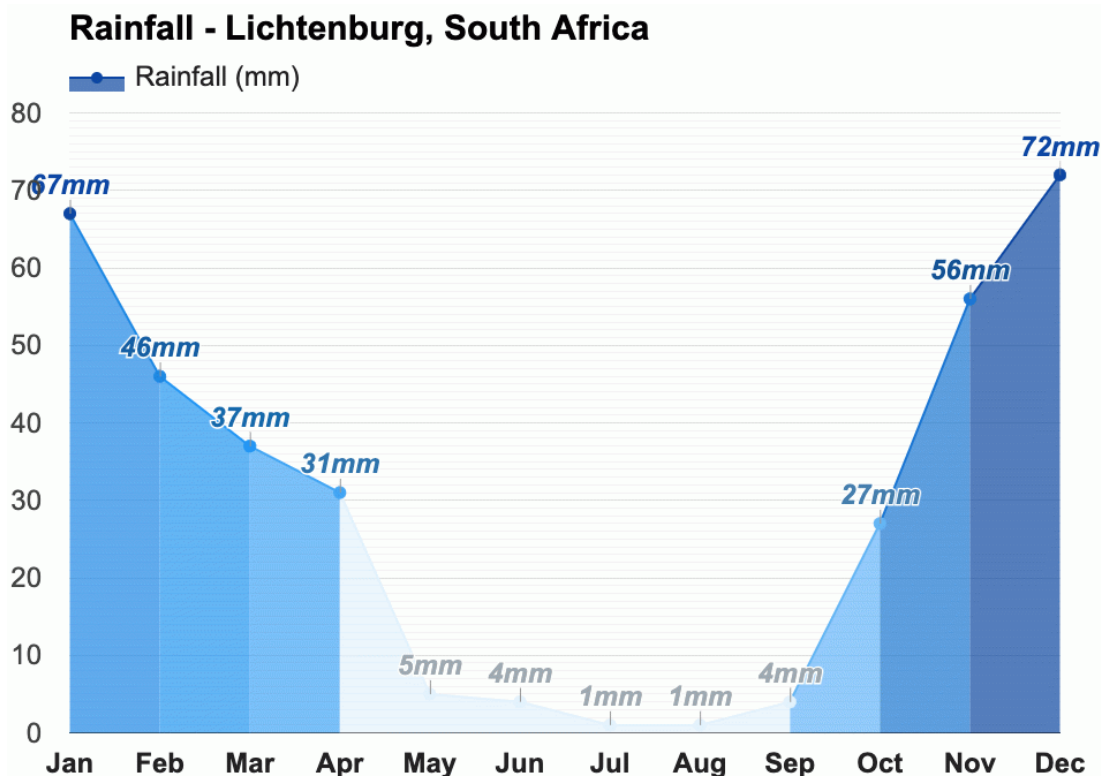


Figure 8:2 Average Rainfall – Lichtenburg, South Africa

The average rainfall days per annum is ±101 days with January having the highest number of rainfall days (17.8 days). The month with the least number of rainfall days is August (0.8 days). Refer to **Figure 8:3** below.

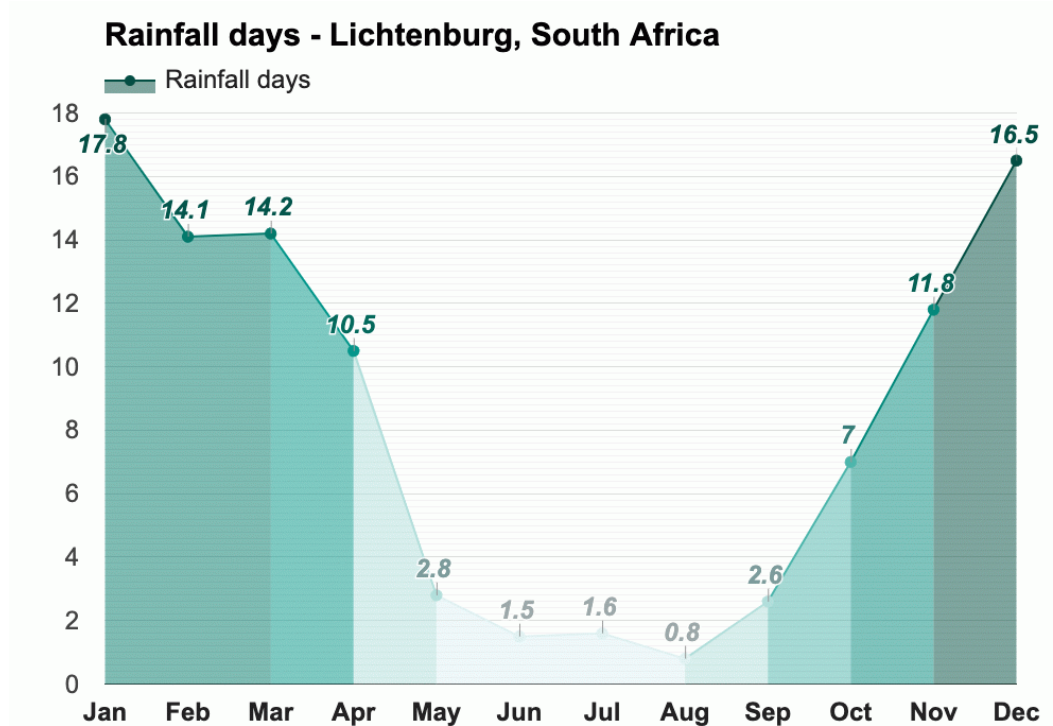


Figure 8:3 Average Rainfall Days – Lichtenburg, South Africa

8.4 Humidity⁵

The region's relative humidity ranges from a maximum of 61% in February to a minimum of 31% in September.

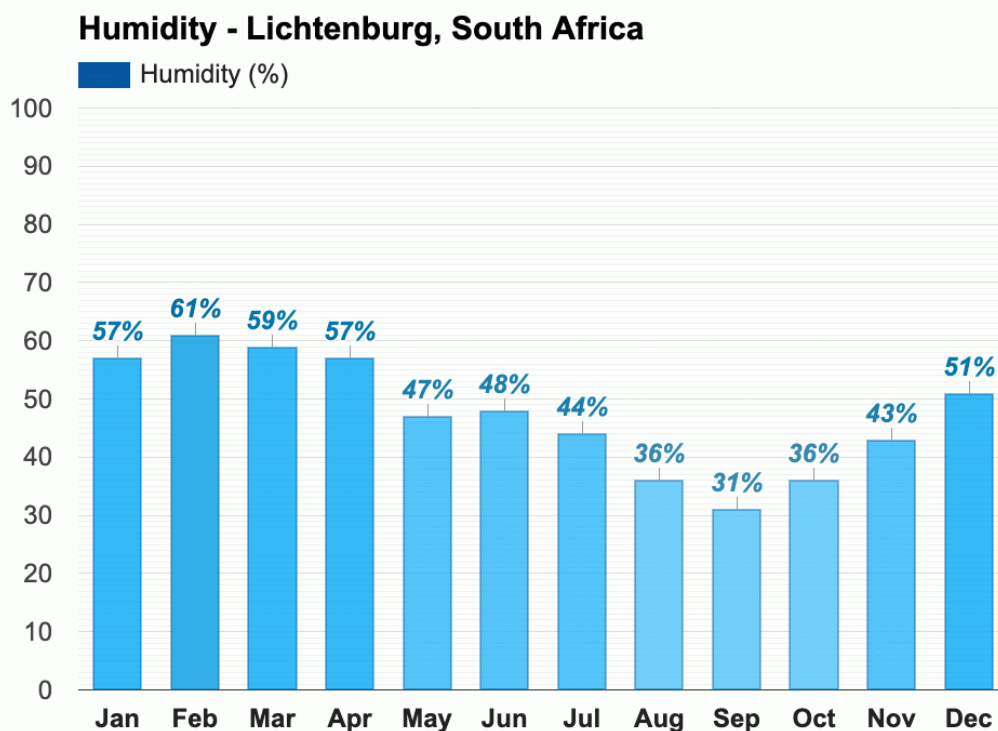


Figure 8:4 Average Relative Humidity – Lichtenburg, South Africa

8.5 Design Rainfall

Design Rainfall Estimation⁶ software was used to obtain the rainfall data (tabulated below in **Table 8:1**) required for the run-off calculations.

Table 8:1 Hillardia PV Design Rainfall Data

| Return Period | | 2yr | 5yr | 10yr | 20yr | 50yr | 100yr | 200yr |
|---------------|-----|---------------------|-------|-------|-------|-------|-------|-------|
| Duration | | Rainfall Depth (mm) | | | | | | |
| 5 | min | 9.20 | 12.60 | 14.90 | 17.10 | 20.10 | 22.50 | 24.90 |
| 10 | min | 13.70 | 18.70 | 22.10 | 25.50 | 29.90 | 33.40 | 36.90 |
| 15 | min | 17.30 | 23.60 | 27.80 | 32.10 | 37.70 | 42.10 | 46.50 |
| 30 | min | 21.90 | 29.80 | 35.30 | 40.60 | 47.70 | 53.30 | 58.90 |
| 45 | min | 25.20 | 34.30 | 40.50 | 46.60 | 54.80 | 61.10 | 67.60 |
| 60 | min | 27.70 | 37.80 | 44.60 | 51.40 | 60.50 | 67.40 | 74.60 |
| 90 | min | 31.90 | 43.40 | 51.20 | 59.10 | 69.40 | 77.40 | 85.60 |
| 120 | min | 35.10 | 44.80 | 56.50 | 65.10 | 76.50 | 85.40 | 94.40 |

⁵ Weather Atlas

⁶ Design Rainfall Estimation in South Africa Version 3 developed by MJ Gorven, JC Smithers and RE Schulze

| Return Period | | 2yr | 5yr | 10yr | 20yr | 50yr | 100yr | 200yr |
|---------------|------|---------------------|--------|--------|--------|--------|--------|--------|
| Duration | | Rainfall Depth (mm) | | | | | | |
| 240 | min | 41.00 | 55.80 | 65.90 | 76.00 | 89.30 | 99.60 | 110.20 |
| 360 | min | 44.90 | 61.10 | 72.20 | 83.20 | 97.80 | 109.00 | 120.60 |
| 480 | min | 47.80 | 65.10 | 77.00 | 88.70 | 104.20 | 116.30 | 128.60 |
| 600 | min | 50.30 | 68.40 | 80.90 | 93.20 | 109.50 | 122.20 | 135.10 |
| 720 | min | 52.40 | 71.30 | 84.20 | 97.10 | 114.10 | 127.20 | 140.70 |
| 960 | min | 55.80 | 76.00 | 89.80 | 103.50 | 121.60 | 135.70 | 150.00 |
| 1200 | min | 58.70 | 79.90 | 94.40 | 108.80 | 127.80 | 142.60 | 157.70 |
| 1440 | min | 61.10 | 83.20 | 98.30 | 113.30 | 133.10 | 148.50 | 164.20 |
| 1 | day | 50.80 | 69.20 | 81.70 | 94.20 | 110.70 | 123.50 | 136.50 |
| 2 | days | 62.50 | 85.10 | 100.50 | 115.80 | 136.10 | 151.80 | 167.90 |
| 3 | days | 70.50 | 96.00 | 113.40 | 130.70 | 153.60 | 171.40 | 189.50 |
| 4 | days | 76.40 | 104.00 | 122.90 | 141.70 | 166.50 | 185.70 | 205.40 |
| 5 | days | 81.40 | 110.80 | 130.90 | 150.80 | 177.20 | 197.70 | 218.60 |
| 6 | days | 85.60 | 116.60 | 137.70 | 158.70 | 186.50 | 208.10 | 230.10 |
| 7 | days | 89.40 | 121.70 | 143.80 | 165.70 | 194.80 | 217.30 | 240.20 |

9 SURFACE HYDROLOGY

9.1 Drainage of Catchment

9.1.1 Primary Catchment

The site falls within the 'Vaal River' drainage catchment (Primary Catchment 'C') which covers an area of ±192 000 km² and extends from the northern border of Lesotho, the Mpumalanga escarpment towards the east, Johannesburg CBD to the north and Douglas to the west as shown in **Figure 9:1** below.

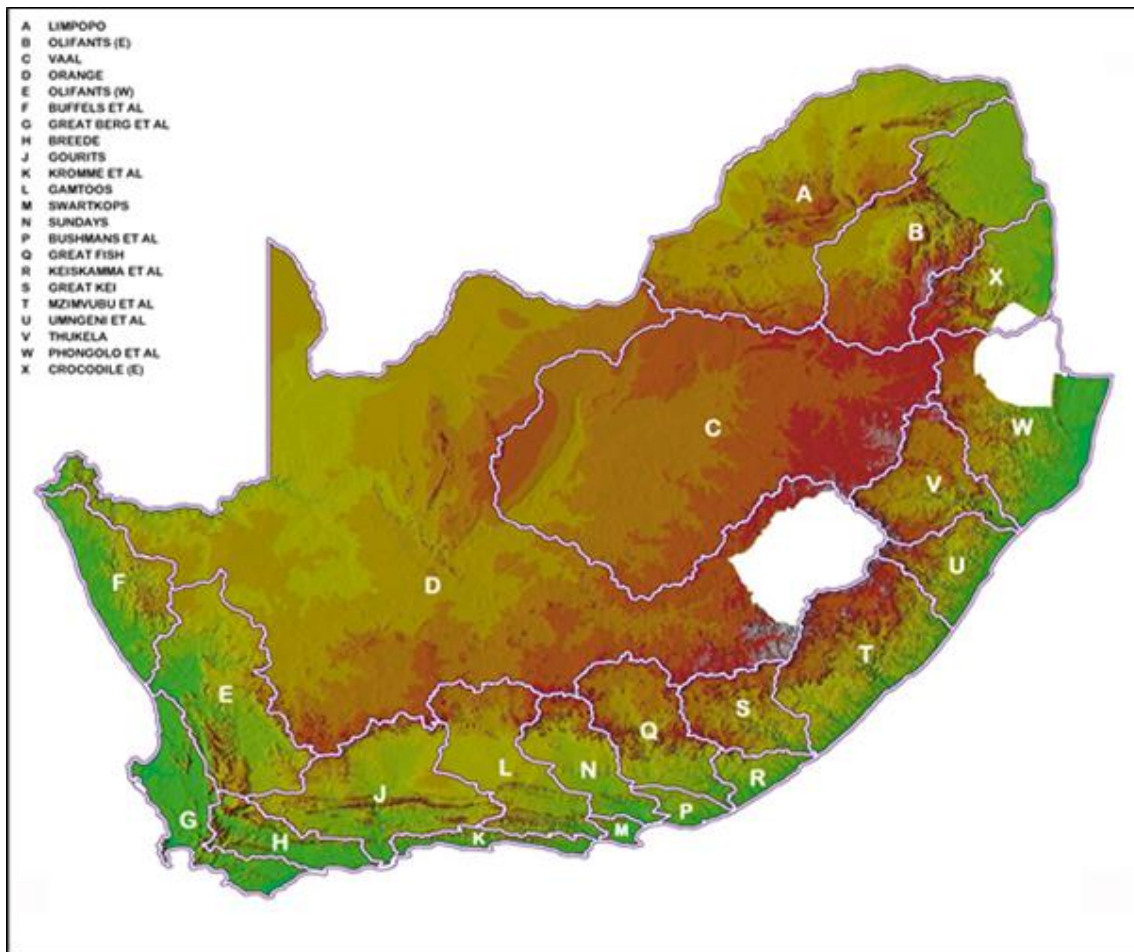


Figure 9:1 Department of Water and Sanitation (DWS) – Primary Catchments

9.1.2 Quaternary Catchment

The proposed facility is located in Quaternary Catchment C31A. This quaternary catchment forms part of the upper reaches of the Harts River, which then flows into the lower reaches of the Vaal River.

10 STORMWATER MANAGEMENT

10.1 Impact of Development⁷

Development is defined as the process of modification or evolution which historically involves the improvement / construction of buildings and civil infrastructure. A new development leads to an alteration in the hydraulic properties of the subjected area, changing surface run-off properties into pervious or impervious layers and subsequently increasing the surface run-off and altering inundation areas. Common historical stormwater infrastructure and surfaces are constructed to manage the run-off more efficiently, resulting in shorter catchment response times and increased peak flows.

As a result of the proposed development, stormwater management is key to reducing the negative impacts and ultimately keeping the receiving environment in its natural state. The management is achieved with adequate mitigation measures, per the applicable stormwater drainage standards and policies, to ensure the development can be accommodated within the receiving environment.

⁷ Guidelines for Human Settlement Planning and Design compiled by CSIR Building and Construction Technology

10.2 The Purpose of Stormwater Management⁸

The purpose of stormwater management is based on several aspects: health and safety, quality of life, and water conservation. These aspects are briefly described below:

- Directing and discharging the stormwater allows the public to protect their health, welfare, and safety. It also provides for the protection of property from flood hazards.
- Enhance the quality of life in communities that are affected.
- To grasp the opportunity to conserve water for beneficial public uses.
- To safeguard the natural environment.
- The balance of economic development and the necessity for a sustainable environment; and
- Optimum stormwater management methodologies are adopted so that the primary beneficiaries pay as per their possible gains.

10.3 Stormwater Management Policies & Design Guidelines

Urban Stormwater Management policies require that the post-development run-off from an area for storms of similar recurrence intervals may not exceed the run-off generated under the pre-development condition. For rural developments, the emphasis should focus more on the detrimental effect to the immediate environment concerning the control of water velocity and erosion rather than minor increases between the pre and post-development flow volumes.

This study area falls within Ditsobotla Local Municipality and greater Ngaka Modiri Molema District Municipality, and, to our knowledge, specific policies, design guidelines, and standards are not available. Therefore, we recommend that the stormwater drainage system refers to the "Red Book⁹" and the "Drainage Manual¹⁰".

10.4 Stormwater Management Philosophy

The Stormwater Management Philosophy for the proposed development urges the developer, the professional teams, and contractors to achieve the following:

- Always maintain adequate ground cover in all areas to reduce the risk of erosion by wind, water and all forms of traffic.
- Prevent concentration of stormwater flow at any point where the ground is susceptible to erosion. Where unavoidable, adequate protection of the ground must be provided.
- Reduce concentrated stormwater flows as much as possible by providing effective attenuation measures.
- Ensure the development does not increase the stormwater flow rate above what the natural ground can safely accommodate.
- Ensure that all stormwater control structures are constructed safely and aesthetically pleasing in keeping with the overall development.
- Prevent pollution of waterways and water features.
- Contain soil erosion by constructing protective works to trap sediment at appropriate locations. This protection applies particularly during construction; and
- Avoid situations where natural or artificial slopes may become saturated and unstable during and after construction.

⁸ *Guidelines for Human Settlement Planning and Design compiled by CSIR Building and Construction Technology*

⁹ *Guidelines for Human Settlement Planning and Design compiled by CSIR Building and Construction Technology*

¹⁰ *Drainage Manual 6th Edition, Published by The South African National Roads Agency SOC Ltd, 2013*

10.5 Stormwater Management Drainage System

Today, stormwater drainage systems can be seen as dual systems incorporating minor and major systems.

The minor stormwater drainage system caters for frequent storm events. Storms of a minor nature usually include stormwater run-off with frequent return periods such as 2yr, 5yr & / or 10 years.

The major stormwater drainage system caters for severe, infrequent storm events. Supported by the minor drainage system. Storms of a major nature include less frequent return periods such as 20yr and more.

11 PRE-DEVELOPMENT RUN-OFF CHARACTERISTICS

11.1 Catchment Description

The development falls within three (3) main catchment areas that will affect the proposed development site. The catchment sizes range from small (3.5km²) to large (47.1 km²) and flat (<1%). As mentioned above in **Section 9**, the catchments all fall within the C31A quaternary catchment. It shows no evidence of clearly defined watercourses. Overland sheet flow occurs in multiple directions through the respective catchments.

The land use is predominantly rural grasslands. Soils were classed under the SCS hydrological soil group C, with a moderately high stormflow potential (slow infiltration rates, shallow soil depths and restricted permeability).

The site is located safely away from any streams, rivers or floodplains and, therefore will not be impacted by a flood line.

11.2 Site Topography

All three developments combined naturally and uniformly slope in two directions, namely north-west and south-west, separated by a minor ridgeline. As mentioned above, no defined drainage lines run through the proposed developments; however, larger drainage lines, namely the Harts River, can be found within the Quaternary catchment outside the proposed developable area, running through Lichtenburg. (Refer to **Figure 11:1** below).

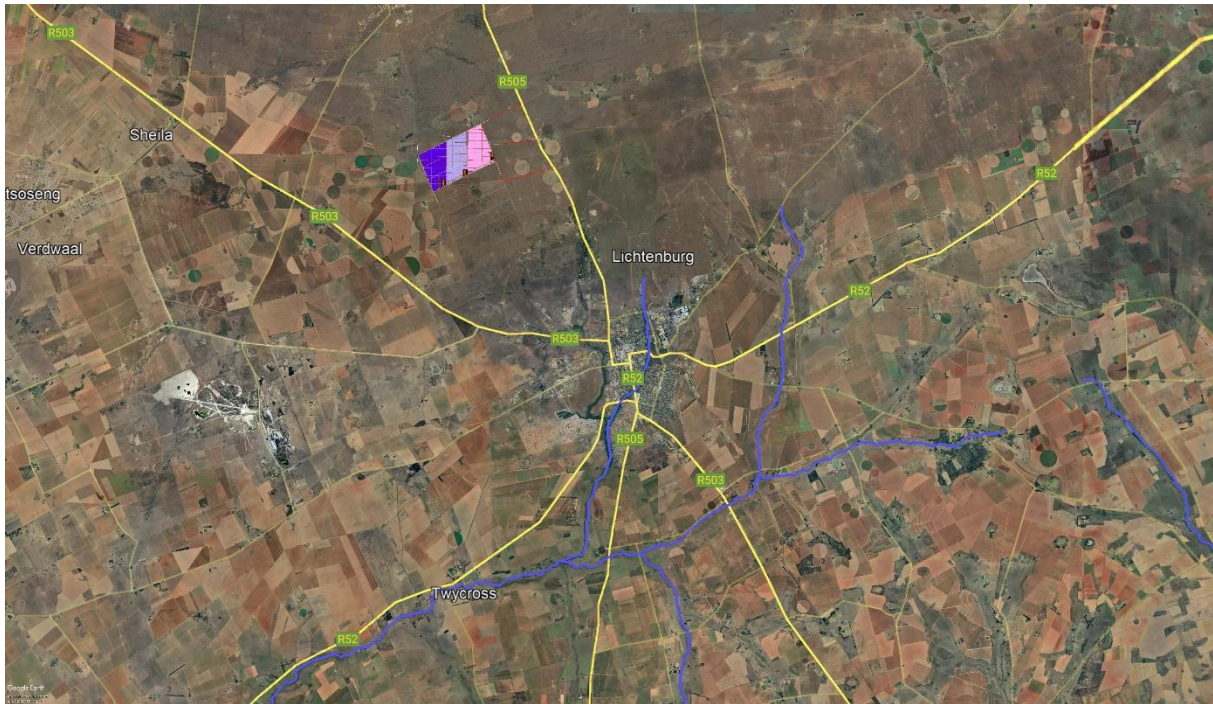


Figure 11:1 Larger Drainage Lines (Blue) outside the Site Parameters (Hillardia PV in Blue)

Please note that detailed contour data was not available for the broader study area. Therefore, the National Geo-Spatial Information (NGI) 's 25m DEM was sourced to provide terrain data for this area.

Contours were generated from the Digital Elevation Model (DEM) at 2.5m intervals using ESRI's 3D Analyst Extension for ArcGIS. Therefore, we recommend that an updated, detailed SWMP be completed once a more accurate Digital Terrain Model (DTM) of the site is available

From **Figure 11:3** below, we confirm a natural slope of $\pm 1:617$ or 0.162% with the following percentages:

- Wetlands & Pans – 80%
- Flat Areas (3% to 10% slope) – 20%
- Hilly Areas (10% to 30% slope) – 0%
- Steep Areas (>30% slope) – 0%

11.3 Site Vegetation

We confirm that the majority of the site covering is made up of short to medium grass with scattered small shrubs and trees.



Figure 11:2 Hillardia PV - Current Site Vegetation

Regarding **Figure 11:2** above, being the typical ground cover on the site, the following percentage splits are applicable: -

- Thick Bush & Plantations – 2.5%
- Light Bush & Farmlands – 30%
- Grasslands – 62.5%
- No Vegetation – 5%

11.4 Geotechnical Conditions

Concerning **Section 7 – Geotechnical Study** above we have assumed the soil conditions to be as follows: -

- Very Permeable – 20%
- Permeable – 70%
- Semi-permeable – 0%
- Impermeable – 10%

11.5 Hardstand Areas

We confirm that the property currently has no areas of hardstand: -

- Hardstand Areas – 0%

11.6 Run-Off Coefficient

Based on *Table 3C.1* of the *Drainage Manual – 6th Edition*¹¹, the following run-off coefficients have been assigned for this calculation: -

Table 11:1 Pre-Development Run-Off Coefficient

| UN-DEVELOPED COMPONENT: Run-off Percentages | | | |
|--|-------------|--------------|--------------|
| Surface Slope - Wetlands & Pans | 0.03 | 80.0% | 0.024 |
| Surface Slope - Flat Areas (3-10%) | 0.08 | 20.0% | 0.016 |
| Surface Slope - Hilly Areas (10-30%) | 0.16 | 0.0% | 0.000 |
| Surface Slope - Steep Areas (>30%) | 0.26 | 0.0% | 0.000 |
| Soil - Very Permeable | 0.04 | 20.0% | 0.008 |
| Soil - Permeable | 0.08 | 70.0% | 0.056 |
| Soil – Semi-Permeable | 0.16 | 0.0% | 0.000 |
| Soil - Impermeable | 0.28 | 10.0% | 0.026 |
| Vegetation - Thick Bush / Plantations | 0.04 | 2.5% | 0.001 |
| Vegetation - Light Bush / Farmlands | 0.11 | 30.0% | 0.033 |
| Vegetation - Grasslands | 0.21 | 62.5% | 0.131 |
| Vegetation - No Vegetation | 0.28 | 5.0% | 0.014 |
| | | | 0.309 |

Based on the preceding, we calculated a **PRE-DEVELOPMENT Run-Off Coefficient** of **0.309**.

It should also be noted that no 'Area Reduction Factor' has been applied as we believe the drainage catchment areas are too small.

¹¹ *Drainage Manual 6th Edition, Published by The South African National Roads Agency SOC Ltd, 2013*



Figure 11:3 Hillardia PV Development Area- Pre-Development Overland Flow

12 POST-DEVELOPMENT RUN-OFF CHARACTERISTICS

12.1 Site Development Plan (SDP)

Concerning the SDP, we confirm this proposed PV Plant layout will consist of a series of PV Panels in a structured pattern along with an access road, internal roads, substation, laydown areas, auxiliary buildings, and external access roads etc. The total development area envisaged will cover a combined area of ±600 ha, whereas Hillardia PV will only cover 207ha.

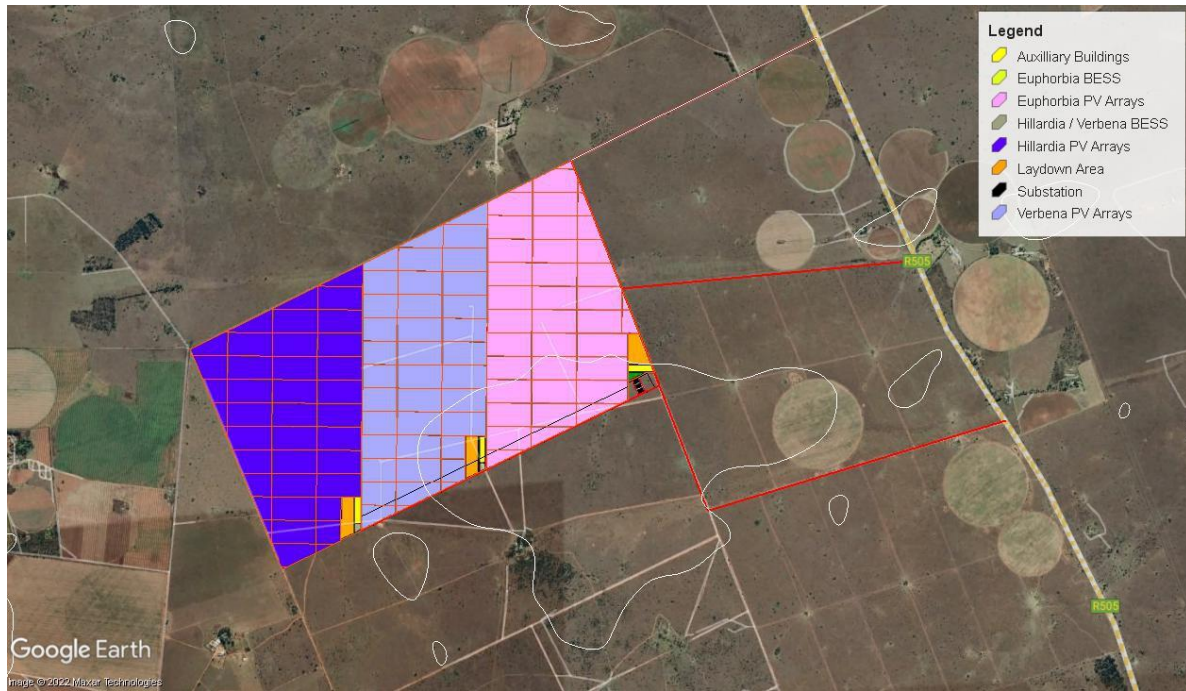


Figure 12:1 Hillardia PV SDP (Indicated in blue)

12.2 Site Vegetation

The intention is to clear the site void of large bushes and shrubs. To rejuvenate the area over time to its original state in **Figure 11:2** above.

The following percentage splits are applicable: -

- Thick Bush & Plantations – 2.5%
- Light Bush & Farmlands – 30%
- Grasslands – 62.5%
- No Vegetation – 2.5%

12.3 Geotechnical Conditions

Concerning **Section 7 – Geotechnical Study**, we have assumed the percentages used in the 'pre-development' run-off coefficient to remain unchanged for the 'post-development' as there would be little or no effect from the facility on the existing ground conditions.

The following percentages have will be used: -

- Very Permeable – 20%
- Permeable – 70%
- Semi-permeable – 0%

- Impermeable – 10%

12.4 Developed Components

Once developed, we confirm that the property will have no significant impervious surfaces in the form of surfaced roads or surface covering other than the natural ground cover. However, gravel roads will be constructed across the site to provide access to the PV panels for maintenance purposes. Gravel roads will have frequent discharge points to reduce stormwater concentrations and ultimately minimise the development impact.

We have therefore assumed a slight increase in the area of imperviousness.

- Gravel Roads – 2%
- PV Facility – 93%
- Grasslands – 0%
- No Vegetation – 5%

12.5 Run-Off Coefficient

Based on *Table 3C.1 of the Drainage Manual – 6th Edition¹²*, the following run-off coefficients percentages have been assigned for this calculation: -

Table 12:1 Post-Development Run-Off Coefficient

| UN-DEVELOPED COMPONENT: Run-off Percentages | | | |
|--|-------------|--------------|--------------|
| Surface Slope - Wetlands & Pans | 0.03 | 80.0% | 0.024 |
| Surface Slope - Flat Areas (3-10%) | 0.08 | 20.0% | 0.016 |
| Surface Slope - Hilly Areas (10-30%) | 0.16 | 0.0% | 0.000 |
| Surface Slope - Steep Areas (>30%) | 0.26 | 0.0% | 0.000 |
| <hr/> | | | |
| Soil - Very Permeable | 0.04 | 20.0% | 0.008 |
| Soil - Permeable | 0.08 | 70.0% | 0.056 |
| Soil – Semi-Permeable | 0.16 | 0.0% | 0.000 |
| Soil - Impermeable | 0.28 | 10.0% | 0.026 |
| <hr/> | | | |
| Vegetation - Thick Bush / Plantations | 0.04 | 2.5% | 0.001 |
| Vegetation - Light Bush / Farmlands | 0.11 | 30.0% | 0.033 |
| Vegetation - Grasslands | 0.21 | 62.5% | 0.131 |
| Vegetation - No Vegetation | 0.28 | 5.0% | 0.014 |
| | | | 0.309 |
| DEVELOPED COMPONENT: Run-off Percentages | | | |
| Surface Slope - Wetlands & Pans | 0.03 | 80.0% | 0.024 |
| Surface Slope - Flat Areas (3-10%) | 0.08 | 20.0% | 0.016 |
| Surface Slope - Hilly Areas (10-30%) | 0.16 | 0.0% | 0.000 |
| Surface Slope - Steep Areas (>30%) | 0.26 | 0.0% | 0.000 |
| <hr/> | | | |
| Soil - Very Permeable | 0.04 | 20.0% | 0.008 |
| Soil - Permeable | 0.08 | 70.0% | 0.056 |
| Soil – Semi-Permeable | 0.16 | 0.0% | 0.000 |
| Soil - Impermeable | 0.28 | 10.0% | 0.026 |
| <hr/> | | | |
| Gravel Roads | 0.50 | 2.0% | 0.010 |

¹² *Drainage Manual 6th Edition, Published by The South African National Roads Agency SOC Ltd, 2013*

| | | | | | | | | |
|---|-------------|--------------|--------------|--------------|--------------|--------------|--------------|--------------|
| PV Facility | | 0.22 | 93.0% | 0.205 | | | | |
| Vegetation - Grass | | 0.21 | 0.0% | 0.000 | | | | |
| Vegetation - No Vegetation | | 0.28 | 5.0% | 0.014 | | | | |
| | | | | 0.359 | | | | |
| RUN-OFF COEFFICIENT: WITH DOLOMITE | | | | | | | | |
| Description | % | Q2 | Q5 | Q10 | Q25 | Q50 | Q100 | Q200 |
| UN-DEVELOPED | 2.0 | 0.003 | 0.003 | 0.003 | 0.004 | 0.005 | 0.006 | 0.006 |
| DEVELOPED | 98.0 | 0.351 | 0.351 | 0.351 | 0.351 | 0.351 | 0.351 | 0.351 |
| TOTAL Run-Off Coefficient | | 0.354 | 0.354 | 0.355 | 0.355 | 0.356 | 0.357 | 0.358 |

Based on the preceding, we calculated a **POST-DEVELOPMENT Run-Off Coefficient** for the development area for the following return periods;

- Q2 **0.354**
- Q5 **0.354**
- Q10 **0.355**
- Q25 **0.355**
- Q50 **0.356**
- Q100 **0.357**
- Q200 **0.358**

The above coefficients were then factored into the existing catchments with the appropriate **POST-DEVELOPMENT Run-Off Coefficients** for each return period used in the modelling below.

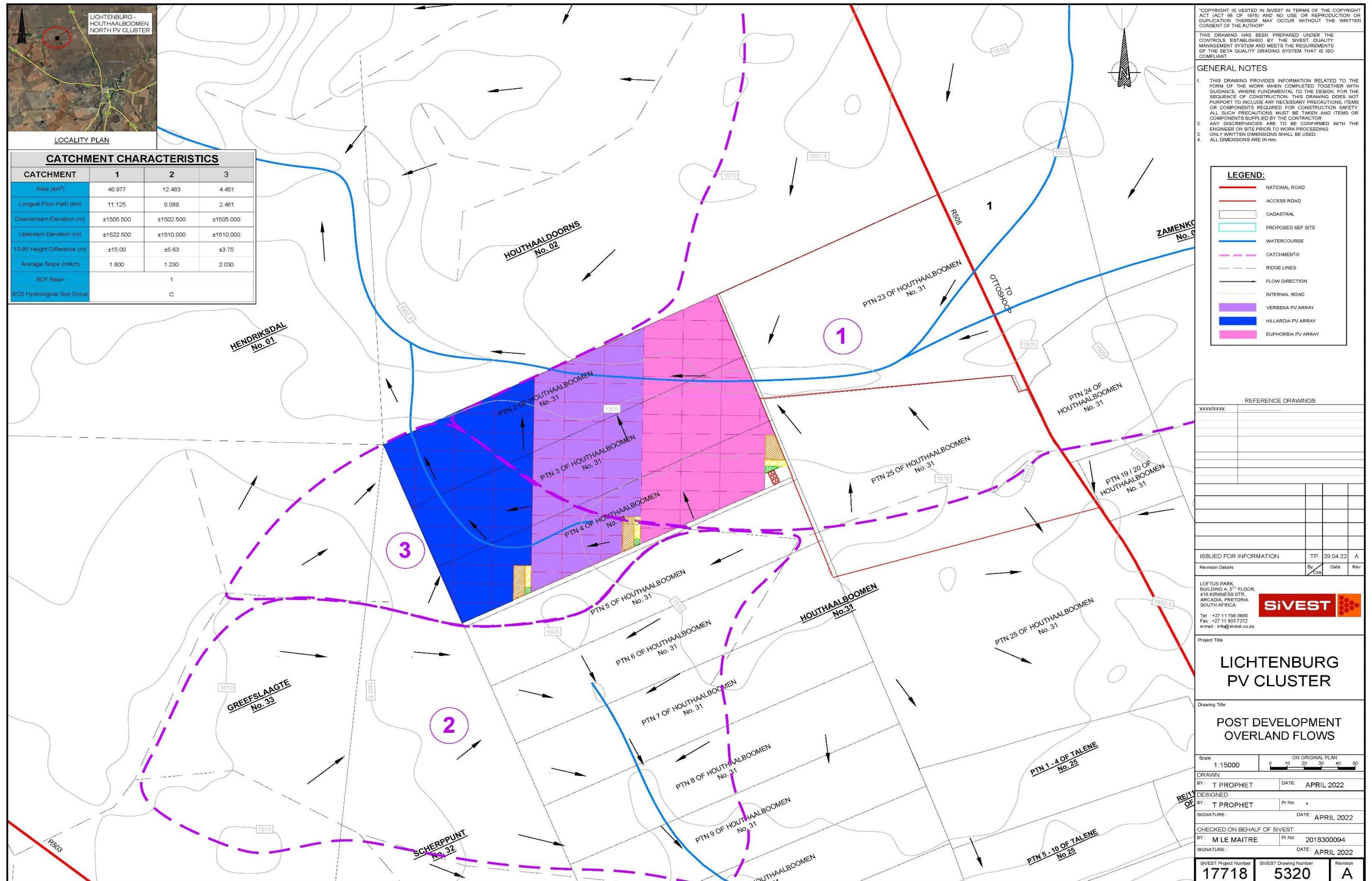


Figure 12:2 Hillardia PV (Indicated in blue above) – Post-Development Overland Flow

13 SURFACE MODELLING

13.1 Modelling Selection

EMPIRICAL and STATISTICAL METHODS were not considered for this project as not enough hydrological records of the area, along with observed events were available. Therefore, a deterministic method has thus been selected to determine the results.

This method comprises mainly manual, graphic and computer-generated spreadsheets. Therefore, we believe our selection of the 'UNIT HYDROGRAPH METHOD' (HRU 1972) and the 'RATIONAL METHOD' is appropriate because the site does not have a varying degree of post-development land change and does not have any existing permanent dams and sub-catchments. Computerised spreadsheets have been used to assist with iterations and to eliminate manual calculation errors.

As noted in **Section 11**, the proposed site is affected by three (3) main catchments: Catchments No. 1, 2 & 3. Section 13.2 below modelled the surface run-off for each catchment for Pre and Post Development conditions.

13.2 Surface Run-Off Modelling Results

13.2.1 CATCHMENT N° 1

Table 13:1 Pre-Development Modelling Results

| Return Storm Period | Unit Hydrograph Method | Rational Method 'Kerby' | Rational Method 'Empirical' | SDF Method | SCS Method |
|---------------------|------------------------|-------------------------|-----------------------------|----------------------|----------------------|
| | (m ³ / s) | (m ³ / s) | (m ³ / s) | (m ³ / s) | (m ³ / s) |
| 1: 2 year | 4,69 | 9,80 | 13,90 | 19,96 | 23,68 |
| 1: 5 year | 24,75 | 19,40 | 25,79 | 43,92 | 42,17 |
| 1: 10 year | 35,32 | 29,41 | 36,80 | 67,50 | 56,22 |
| 1: 25 year | 51,71 | 45,60 | 53,89 | 91,08 | 71,16 |
| 1: 50 year | 75,57 | 70,34 | 78,75 | 122,12 | 91,94 |
| 1: 100 year | 106,43 | 102,79 | 109,33 | 154,66 | 108,71 |

Table 13:2 Post-Development Modelling Results

| Return Storm Period | Unit Hydrograph Method | Rational Method 'Kerby' | Rational Method 'Empirical' | SDF Method | SCS Method |
|---------------------|------------------------|-------------------------|-----------------------------|----------------------|----------------------|
| | (m ³ / s) | (m ³ / s) | (m ³ / s) | (m ³ / s) | (m ³ / s) |
| 1 : 2 year | 5,28 | 11,03 | 15,65 | 19,96 | 23,68 |
| 1 : 5 year | 27,40 | 21,47 | 28,55 | 43,92 | 42,17 |
| 1 : 10 year | 38,54 | 32,10 | 40,17 | 67,50 | 56,22 |
| 1 : 25 year | 55,52 | 48,95 | 57,85 | 91,08 | 71,16 |
| 1 : 50 year | 78,89 | 73,43 | 82,21 | 122,12 | 91,94 |
| 1 : 100 year | 108,87 | 105,13 | 111,83 | 154,66 | 108,71 |

13.2.2 CATCHMENT N° 2

Table 13:3 Pre-Development Modelling Results

| Return Storm Period | Unit Hydrograph Method | Rational Method 'Kerby' | Rational Method 'Empirical' | SDF Method | SCS Method |
|---------------------|------------------------|-------------------------|-----------------------------|----------------------|----------------------|
| | (m ³ / s) | (m ³ / s) | (m ³ / s) | (m ³ / s) | (m ³ / s) |
| 1 : 2 year | 1,92 | 3,03 | 4,75 | 7,20 | 8,91 |
| 1 : 5 year | 9,36 | 6,00 | 8,81 | 15,84 | 15,87 |
| 1 : 10 year | 13,35 | 9,10 | 12,58 | 24,34 | 21,16 |
| 1 : 25 year | 19,55 | 14,11 | 18,42 | 32,84 | 26,78 |
| 1 : 50 year | 28,57 | 21,77 | 26,91 | 44,04 | 34,60 |
| 1 : 100 year | 39,66 | 31,81 | 37,36 | 55,77 | 40,91 |

Table 13:4 Post-Development Modelling Results

| Return Storm Period | Unit Hydrograph Method | Rational Method 'Kerby' | Rational Method 'Empirical' | SDF Method | SCS Method |
|---------------------|------------------------|-------------------------|-----------------------------|----------------------|----------------------|
| | (m ³ / s) | (m ³ / s) | (m ³ / s) | (m ³ / s) | (m ³ / s) |
| 1 : 2 year | 1,92 | 3,03 | 4,75 | 7,20 | 8,91 |
| 1 : 5 year | 9,36 | 6,00 | 8,81 | 15,84 | 15,87 |
| 1 : 10 year | 13,35 | 9,10 | 12,58 | 24,34 | 21,16 |
| 1 : 25 year | 19,55 | 14,11 | 18,42 | 32,84 | 26,78 |
| 1 : 50 year | 28,57 | 21,77 | 26,91 | 44,04 | 34,60 |
| 1 : 100 year | 39,66 | 31,81 | 37,36 | 55,77 | 40,91 |

13.2.3 CATCHMENT N° 3

Table 13:5 Pre-Development Modelling Results

| Return Storm Period | Unit Hydrograph Method | Rational Method 'Kerby' | Rational Method 'Empirical' | SDF Method | SCS Method |
|---------------------|------------------------|-------------------------|-----------------------------|----------------------|----------------------|
| | (m ³ / s) | (m ³ / s) | (m ³ / s) | (m ³ / s) | (m ³ / s) |
| 1 : 2 year | 1,46 | 1,67 | 3,39 | 5,22 | 5,31 |
| 1 : 5 year | 5,74 | 3,30 | 6,29 | 11,49 | 9,46 |
| 1 : 10 year | 8,19 | 5,00 | 8,98 | 17,66 | 12,61 |
| 1 : 25 year | 11,99 | 7,76 | 13,15 | 23,83 | 15,96 |
| 1 : 50 year | 17,52 | 11,97 | 19,22 | 31,95 | 20,62 |
| 1 : 100 year | 24,32 | 17,49 | 26,68 | 40,46 | 24,38 |

Table 13:6 Post-Development Modelling Results

| Return Storm Period | Unit Hydrograph Method | Rational Method 'Kerby' | Rational Method 'Empirical' | SDF Method | SCS Method |
|---------------------|------------------------|-------------------------|-----------------------------|----------------------|----------------------|
| | (m ³ / s) | (m ³ / s) | (m ³ / s) | (m ³ / s) | (m ³ / s) |
| 1 : 2 year | 2,61 | 2,98 | 6,06 | 5,22 | 5,31 |
| 1 : 5 year | 9,57 | 5,51 | 10,50 | 11,49 | 9,46 |

| Return Storm Period | Unit Hydrograph Method | Rational Method 'Kerby' | Rational Method 'Empirical' | SDF Method | SCS Method |
|---------------------|------------------------|-------------------------|-----------------------------|----------------------|----------------------|
| | (m ³ / s) | (m ³ / s) | (m ³ / s) | (m ³ / s) | (m ³ / s) |
| 1 : 10 year | 12,86 | 7,86 | 14,11 | 17,66 | 12,61 |
| 1 : 25 year | 17,49 | 11,32 | 19,20 | 23,83 | 15,96 |
| 1 : 50 year | 22,33 | 15,25 | 24,50 | 31,95 | 20,62 |
| 1 : 100 year | 27,79 | 19,99 | 30,49 | 40,46 | 24,38 |

The results above indicate the proposed development will have a minor increase between the Pre and Post-Development flows. Therefore, we believe implementing adequate stormwater management guidelines can accommodate the proposed development without negatively impacting the downstream catchment.

14 STORMWATER MANAGEMENT & GUIDELINES

The buildings / structures within the development will be required to control stormwater run-off per the stormwater management philosophy and policies of the local authority / municipality. The following guidelines are intended to assist in the design of the major and minor stormwater infrastructure and to ensure that the objectives of this SWMP are met during the planning, design, construction, and operational phases of the development.

14.1 Buildings

Any building will inevitably result in some degree of flow concentration or deflection around the building. The developer / owner shall ensure that all stormwater flow paths are protected against erosion. Discharge from the buildings must be attenuated back to the pre-development state.

Any inlet to a piped system shall be fitted with a screen / grating to prevent debris and refuse from entering the stormwater system. This must be installed immediately on the installation of the infrastructure. The onus is on the owner / developer to maintain the state of the screen / grating to ensure smooth flow.

No building works, earthworks, walls or fences may obstruct or encroach on a watercourse inside or outside the site without approved plans that do not compromise the objectives of the SWMP.

14.2 Roof Drainage

Building designs must ensure that rainfall run-off from roofing and other areas, not subjected to excessive pollution, can be efficiently captured for re-use for on-site irrigation and non-potable water uses.

Where storage for re-use and ground conditions permit, rainwater run-off should connect to detention areas to maximise groundwater recharge. These detention areas must be designed to attenuate run-off, specifically, the peak flows experienced in the reaches of a watercourse-

14.3 Parking and Paved Areas

Parking or paved areas should be designed to attenuate stormwater run-off to an acceptable degree by allowing ponding or infiltration. Stormwater from such areas must be discharged and controlled as overland sheet flow or larger attenuation facilities.

14.4 Roads

Roads should be designed and graded to avoid the concentration of flow along and off the road. Regular side drains discharge points along roads for overland flow to continue as sheet flow towards drainage lines per pre-development conditions. Where flow concentration is unavoidable, measures to incorporate the road into the major stormwater system should be taken, providing appropriately designed attenuation storage facilities at suitable points.

Culverts must be designed to ensure that the capacity of the culvert does not exceed the pre-development stormwater flow at that point, and attenuation storage should be provided on the upstream side of the road crossing.

Outlet and culvert discharge points into the natural watercourse must be designed to dissipate flow energy, and any unlined downstream channel must be adequately protected against soil erosion.

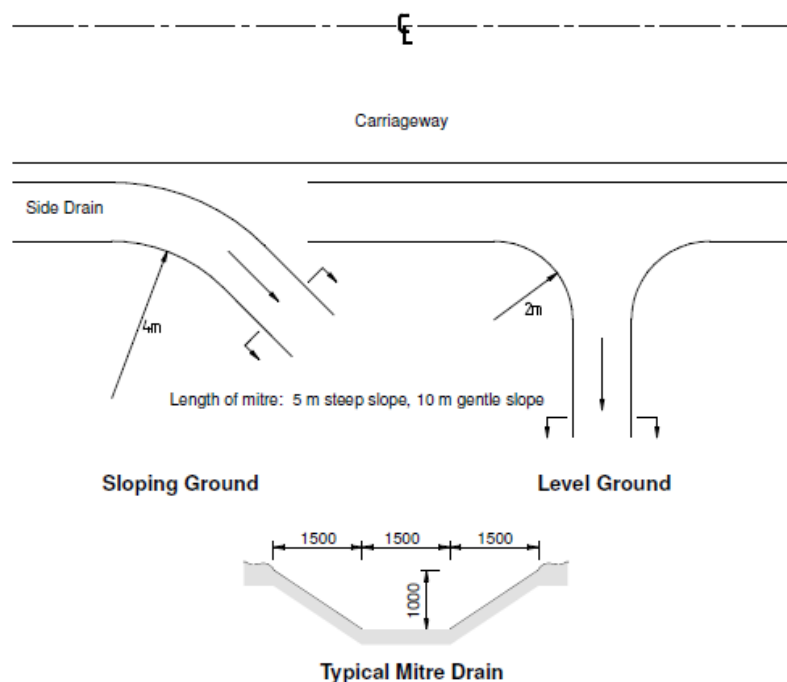


Figure 14:1 Typical Stormwater Mitre Drain / Channel

14.5 Subsurface Disposal of Stormwater

Any construction providing for the subsurface disposal of stormwater should be designed to ensure that such disposal does not cause slope instability or areas of concentrated saturation or inundation. Infiltration structures should be integrated into the terrain to be unobtrusive and in keeping with the natural surroundings.

14.6 Channels

Channels may be constructed to convey stormwater directly to a natural watercourse where deemed necessary and unavoidable. The channels must be suitably lined to prevent erosion and scour and provide maximum possible energy dissipation of the flow. Such linings will vary from vegetated earthen to stone pitching or reinforced concrete.

14.7 Energy Dissipation

Measures should be taken to dissipate flow energy wherever concentrated stormwater flow is discharged onto the natural ground.

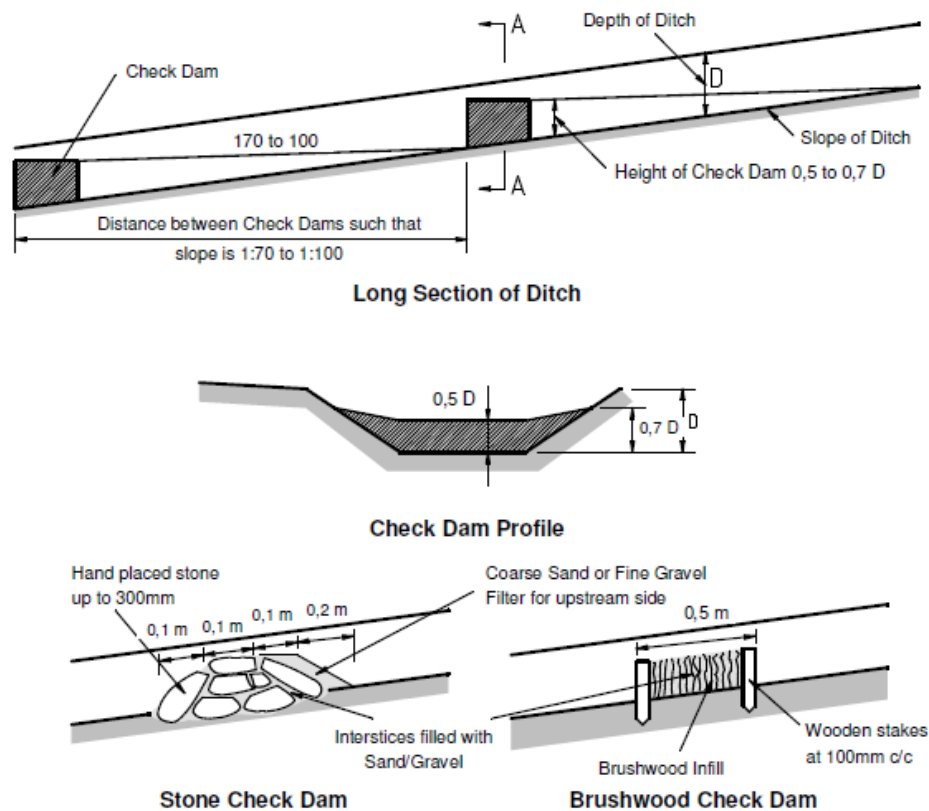


Figure 14:2 Typical Erosion Control

14.8 Open Trenches

Open trenches should not be left open and unprotected for extended periods and should be progressively backfilled as construction proceeds. Excavated material to be used as a backfill must be placed close to the trench on the upstream side to avoid loose material from washing away.

14.9 Stockpiles

Material is to be stockpiled away from drainage paths. Loose material such as stone, sand or gravel must be covered or kept damp to minimise dust. Temporary silt screens are to be positioned immediately downstream of stockpiles to intercept loose material which may be washed away.

14.10 Photovoltaic Panels

For the orientation of panels, we recommend the drainage pattern, flow concentration, drainage area and velocities be considered at the detail design stage. Rows perpendicular to the contours may result in higher run-off concentrations; therefore, mitigation measures are to be included to optimise orientation and keep the run-off as sheet flow across the entire site.

PV panels shall be designed and constructed in such a manner to allow for vegetative growth and maintenance beneath and between the panels. Although not applicable to this development, if any of the PV modules / strings are greater than 3m high, from the lowest vertical clearance of the panels to

above the ground. In that case, non-vegetative control measures will be required to prevent / control erosion and scour along the drip line or otherwise provide energy dissipation from the water running off the panels.

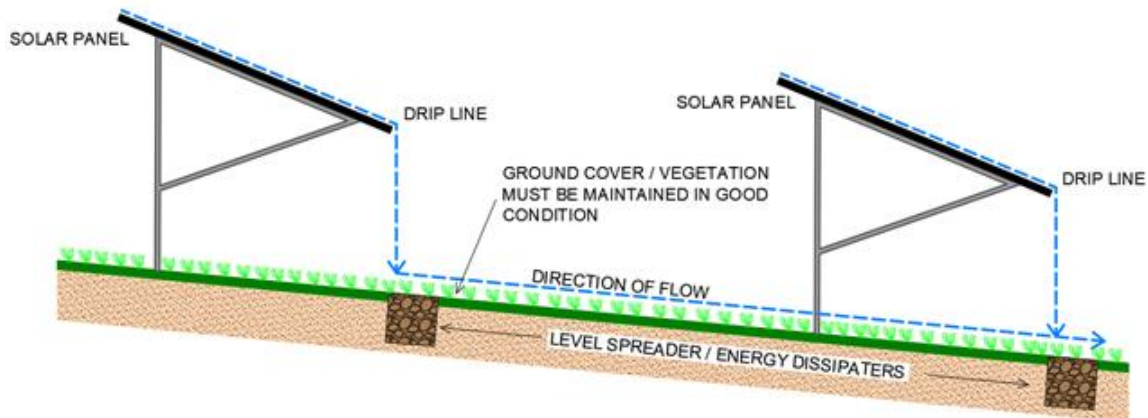


Figure 14:3 Stormwater control of Fixed Tilt PV showing drip line (Conceptual)

14.11 Stormwater Pollution Control

The stormwater systems should be free from any materials that could harm the water systems' fauna, flora, and aquatic life.

Sites which generate "dirty" (Grey or Black) water must have measures in place that separates the clean and "dirty" water. Depending on the nature of the "dirty" water, the water must either be discharged into the wastewater system or contained on-site for treatment by a package treatment plant. It is essential that the wastewater system does not flood and overflow into the stormwater systems. Designers must ensure sufficient capacity for the wastewater system to receive this "dirty" water.

Any site that is required to store substances that could be regarded as hazardous in terms of water pollution must take measures to ensure spillages of such chemicals can be adequately contained and prevent contamination of the water resources within the development area.

15 STORMWATER MANAGEMENT POLICY

The following rules are to be observed by the owner, developer, professional team, contractors, and sub-contractors:

- The Environmental Management Program (EMPr), as per the EIA and approved by the competent authority, will manage stormwater run-off during construction. All construction activities within the development must comply with the EMPr. This SWMP document is supplementary to the EMPr. The control measures set out herein are not considered all-encompassing as the contractor will also have to adapt site-specific control measures.
- Before the commencement of any construction activities, the contractor must compile and submit his EMPr for approval by the competent authority. The approved plan must include measures to control and prevent erosion during and after construction.
- Existing flood lines / wetlands / stormwater attenuation areas should be protected from encroachment by the development.
- Development designs must include measures for attenuating the increased concentration of stormwater run-off. The post-development peak flows can be attenuated to pre-development conditions if adequate stormwater mitigation measures are not implemented.
- On-site stormwater control systems, such as swales, berms and attenuation ponds, must be constructed before any other construction commences. These systems are to be

monitored and appropriately adjusted as construction progresses to ensure complete stormwater, erosion and pollution control.

- All formed embankments must be adequately stabilised.
- This development is located in an area where dolomite is present, and therefore NO stormwater must be allowed to pond for extended periods.
- An approved landscaping and re-vegetation plan must be implemented immediately after building works have reached a stage where newly established ground cover is not at risk from the construction works.
- The contractor must show that all the provisions, regulations and guidelines in this document have been considered.
- In the event of a failure to adequately implement the approved SCP, the contractor shall be responsible for all consequential damage at his own cost. The developer is therefore advised to ensure that all members of the professional team and contractors are competent to undertake the development work and are adequately insured.
- Appropriate designed dolomitic attenuation / detention facilities will be located at appropriately selected sites based on geotechnical, environmental and topographical conditions, including wetland conservation.
- Where conditions permit, open ditches, drains and channels will be used instead of pipes. On steeper slopes, where high flow velocities are anticipated, appropriate linings for all channels must be provided to withstand erosion. Such linings will vary from vegetated earthen to stone pitching and reinforced concrete.
- Flow velocities must be reduced wherever possible to reduce the erosion potential in channels and points of flow concentration (typically at outlets).
- Silt, trash and oil traps must be strategically provided to ensure water quality is not compromised and to prevent blockages in the drainage systems.
- Areas within the proposed development that are bound on stormwater attenuation areas, near road crossings, watercourse confluences and water features might be subject to flooding. In these situations, all development should take place above the outfall levels with an appropriate freeboard allowance.
- Potential future development in these sub-catchments should be considered and any stormwater attenuation requirements should be identified for areas flowing into the development area. Likewise, consideration must be given to the stormwater flowing out of the development, which may impact the downstream areas and watercourses. Appropriate measures must be taken to ensure any upstream development does not result in an increased flood damage risk downstream; and
- All-natural and unlined channels should be inspected for adequate binding of soil by sustainable ground cover. Stone pitching should be used to reinforce channel inverters on steep slopes.
- The development should protect existing wetlands and stormwater attenuation areas from encroachment.

16 CONCLUSION & IMPACT STATEMENT

- In conclusion;
 - The Surface Modeling (**Section 13**) reveals that the proposed development / infrastructure will have a minimal impact on the stormwater quality and quantities of post-development stormwater flow (operational phase).
 - The highest impact will, in all likelihood, occur during the construction phase, and these impacts must be strictly managed under the advisement of the guidelines set out in this document.
 - The need for formal stormwater interventions can be minimised if the development is designed to maintain the existing drainage patterns. Overland flow via poorly-defined drainage paths will be the primary form of conveyance.

- The Civil Engineers must prepare a detailed stormwater management plan describing and illustrating the proposed stormwater and erosion control measures during the detailed design phase.
 - A comprehensive geotechnical study is completed before the detailed design stage of this development.
 - The guidelines described in **Section 14 – STORMWATER MANAGEMENT & GUIDELINES** be incorporated into the detailed design of the development.
 - The policy described in **Section 15 – STORMWATER MANAGEMENT POLICY** be implemented.
- Impact Statement;
 - Concerning this report, associated assessment and the findings made within, it is SiVEST's opinion that the Hillardia PV and associated grid infrastructure will have a nominal impact on the existing stormwater catchment. The project is therefore deemed acceptable from a stormwater perspective, provided the recommendations and mitigation measures in this report are implemented. Hence, Environmental Authorisations (EAs) should be granted for the EIA applications.
 - This document should also be read in conjunction with the EMPr. The developer, owner, and professional team shall ensure that the requirements and conditions set out in the EMPr are adhered to.

17 REFERENCES

Climate Data for Cities Worldwide (Lichtenburg) - www.en.climate-data.org

Council for Scientific and Industrial Research (CSIR) - *Guidelines for Human Settlement Planning and Design compiled by CSIR Building and Construction Technology (2003)*

Lauren M. Cook and Richard H. McCuen – *Hydrologic Response of Solar Farms (May 2013)*

MJ Gorven, JC Smithers and RE Schulze - *Design Rainfall Estimation in South Africa (Version 3)*

South African National Roads Agency – *Drainage Manual 6th Edition (2013)*

Van Der Walt, J – *Heritage Baseline Report: For the Houthaalboomen North PV Cluster (Euphorbia PV, Hillardia PV AND Verbena PV), Lichtenburg, North-West Province. Beyond Heritage (2022)*

Weather Atlas (Lichtenburg) – www.weather-atlas.com

Western Cape Provincial Administration – *Gravel Roads Manual Rev 0*

APPENDIX A: SPECIALIST CURRICULUM VITAE

CURRICULUM VITAE

Merchandt Le Maitre

| | |
|----------------------------|--|
| Name | Merchandt Le Maitre |
| Profession | Civil Engineer |
| Name of Firm | SiVEST SA (Pty) Ltd |
| Present Appointment | Divisional Manager: Civil Engineering Division |
| Years with Firm | 17 Years |
| Date of Birth | 25 September 1982, Johannesburg, South Africa |
| ID Number | 820925 5037 086 |
| Nationality | South African |



Education

- University of Johannesburg (2006)
- University of South Africa (2016)

Professional Qualifications

- N Dip: Civil Engineering
- B Tech: Civil Engineering (Water)
- Pr.Tech.Eng. (Reg. No. 2018300094)

Membership in Professional Societies

- Engineering Council of South Africa (ECSA) – Pr Tech Eng; (Reg N° 2018300094)
- South African Institute of Civil Engineers (SAICE)
- South African Wind Energy Associations (SAWEA)

Employment Record

| | |
|-----------------------|--|
| Nov 2020 – present | SiVEST SA (PTY) LTD: Divisional Manager |
| May 2004 – Oct 2020 | SiVEST SA (PTY) LTD: Senior Civil Engineering Technician |
| Jan 2004 – April 2004 | Con Roux Zambia - Junior Foreman |
| Dec 2002 – Dec 2003 | Neda Engineering - Vacation Work |

Language Proficiency

| LANGUAGE | SPEAK | READ | WRITE |
|-----------|--------|--------|--------|
| English | Fluent | Fluent | Fluent |
| Afrikaans | Fluent | Fluent | Fluent |

Years of Working Experience: 17

Countries of Work Experience

- South Africa
- Swaziland
- Zambia
- Kenya
- Namibia

Fields of Expertise

- Bulk Services Studies
- Feasibility Studies
- Service Reports
- Infrastructure Design
- Contract Documentation & Procurement
- Contract Administration
- Procurement and Construction Monitoring

Overview

Merchandt joined SiVEST as a student Civil Engineering Technician in 2004 to which he received a company bursary to complete his studies and join the company permanently thereafter. Since joining permanently he has been actively involved in numerous township projects and associated infrastructure projects.

A summary of the experience in each field is indicated below:

Roads & Stormwater

Design, Implement & Contract Administration:

- Provincial Road Intersections (Class 2 Roads)
- Municipal Roads (Class 3-5 Roads)
- Residential & Industrial Township services
- Bulk Stormwater Infrastructure

Hydrology

- Attenuation Reports
- Flood Inundation Assessments / Floodline Reports
- Stormwater Management Reports
- Stormwater Assessments / Investigations
- Roof Gutter & Down Pipe Design / Assessments / Reports

Water & Sanitation

Design, Implement & Contract Administration:

- Water supply lines including Bulk Water
- Water pump stations
- Sanitation networks including Outfall Sewers
- Sewer pump stations
- Farm Irrigation Network

Renewable Energy

- Transportation Impact Assessments
- Water Demand Assessments
- Glint & Glare Assessments
- Stormwater Management Reports

- Preliminary Engineering Reports & Designs

Projects Experience (by Sector)

TOWNSHIP SERVICES

- Tijger Valley Extension 10, 20, 21, 22, 23, 27, 38-44, 72, 105-113, 19, 62, 103, 104, 34, 35, 36, 123 etc. - Design, Procurement, Contract Administration and Monitoring.
- Derdepoort Extension 181- Design, Procurement, Contract Administration and Monitoring.
- Project Springbok, Sasolburg - Design, Procurement, Contract Administration and Monitoring.
- Arcadia Extension 11 - Design, Procurement, Contract Administration and Monitoring.
- Lakeside Erf 181- Design, Procurement, Contract Administration and Monitoring.
- Longmeadow Extension 10, 11 & 12 - Design, Procurement, Contract Administration and Monitoring.
- Bushwillow Estate - Design, Procurement, Contract Administration and Monitoring.
- Forum Homini – Draughting Monitoring of Dam Spillway construction & sewer reticulation.
- Longmeadow Extension 7, 8, 9, 10, 11, 12 – Township services and design of earth retaining wall.
- Lakeside Erf 181 – Design and supervision of Township Services including Attenuation facilities.
- Mbabane Kingdom Hall – Bulk earthworks and road Design, Procurement, Contract Administration and Monitoring.
- Kungwini Bulk Water – Draughting and supervision of a Steel Bulk Water Supply Pipe.
- Mooikloof Booster Station – Design and supervision of a water booster pump facility..
- PTN 2 of 148 Athol – Compiling and analysis Stormwater Assessment.
- Mooibosch Development – Compiling of Services reports and Floodline Determination.
- Hazeldean Extension 39 – Design and supervision of Township Services.
- Hazeldean Retirement – Design of Township Services.
- Kungwini Collector Sewer – Design of Collector Sewer.
- Maroeladal Extension 9 – Design and compilation of Services Report.
- Hazeldean Oukraal – Design of Township Services
- Hazeldean Business Park – Design and compilation of Services Reports.
- Erf 181 Derdepoort – Design and compilation of Services Reports and preliminary design of Provincial Intersection.
- Erf 92 Edenburg – Floodline Determination and design and compilation of the Services reports.
- Longmeadow Extension 12 Stormwater – Design of Stormwater Reticulation.
- Astral Foods - Design, Procurement, Contract Administration and Monitoring of civil services.
- Eastgate Solar Roof – Glint & Glare Assessment
- Cotton Gin Mpumalanga – Design & Procure all services

ROADS & INTERSECTION DESIGN

- D631 Intersection – Design, Wayleave Approval, Procurement, Contract Administration and Monitoring.
- D36 Intersection & Road Widening - Design, Wayleave Approval, Procurement.
- K34 Intersection – Design, Wayleave Approval, Procurement, Contract Administration and Monitoring.
- K101 Intersection – Design, Wayleave Approval.
- Justice Mahomed, University, Walton Jameson Rd Intersection – Design, Wayleave Approval.
- Cedar Road West – Design, Wayleave Approval, Procurement, Contract Administration and Monitoring.
- Brikor – Design of New Intersection.
- New Zealand Embassy – Design of Intersection.
- East Point Game - Design, Wayleave Approval, Procurement, Contract Administration and Monitoring.

HYDROLOGY AND STORMWATER

- Hazeldean Floodline - Data collection, Flood determination and compilation.

CURRICULUM VITAE

Merchandt Le Maitre

- Gautrain Railway Stormwater Management – Design and compile stormwater management and attenuation facilities.
- Stormwater Modelling for Project Springbok – Attenuation of hazardous material in stormwater system.
- Sappi Ngodwana Floodline – Data collection, Flood determination and compilation. This floodline included cognisance of the Ngodwana dam.
- Irene Mall Stormwater Management - Accommodation of the Post Development stormwater flow through an existing township / suburb.
- Loftus Park Stormwater Management – Accommodation of the Post Development stormwater flow through an existing township / suburb.
- Pienaars River Floodline Modelling – Modelling of the river through two future Class 1 & 3 road bridge structures.
- Renewable Energy Stormwater Management – A number of Management Plans for the Renewable Energy sector has been completed.
- Longmeadow Extension 10 (Pick & Pay) – Design and compilation of Stormwater Management report.
- Erf 4173 Peter Place – Floodline Determination.
- Irene Mall Township – Design of Township Services and Stormwater Management.
- Mitsubishi McCarthy Midrand – Design and compilation of Stormwater Management report.
- Isago @ N12 – Floodline Determination.
- Innoland – Floodline Determination.
- Lot 204 Edenburg – Floodline Determination
- Erf 90 Douglasdale – Floodline Determination.
- PTN 35 Houtkoppen – Floodline Determination.
- Erf 4173 Peter Place – Floodline Determination.
- Hyde Close Floodline – Floodline Determination.
- Chartwell Floodline – Floodline Determination
- Hyundai East Rand – Roof Gutter & Down Pipe design
- Oilifants River – Floodline Determination

WATER TRANSFER / RETICULATION AND SANITATION COLLECTORS / OUTFALLS

- Bojanala Platinum District Municipality – Water & Sanitation Bulk Master Planning.
- Hazeldean Development – Bulk Water Supply & Collector Sewer Design, Procurement, Contract Administration and Monitoring.
- Mamba Kingdom – Bulk Water Analysis.
- Lesedi Local Municipality Bulk Water - Design, Wayleave Approval, Procurement, Contract Administration and Monitoring.
- NEF Tomato Paste Project – Design of Farm Irrigation Network

RENEWABLE ENERGY

- Dyansons Klip 5 – Stormwater Management Report
- De Aar Solar – Stormwater Management Report
- Droogfontein Solar – Stormwater Management Report
- Mierdam Solar – Stormwater Management Report
- Prieska– Stormwater Management Report
- Hoekplaas – Stormwater Management Report
- Noupoort WEF – Stormwater Management Report
- Copperton PV – Stormwater Management Report
- Klipgats PV – Stormwater Management Report
- Tooverberg Wind Energy Facility – Transportation Impact Assessment & Water Demand Assessment
- Umsobomvu Solar Energy - Transportation Impact Assessment
- Prieska Solar Energy - Transportation Impact Assessment Amendment
- Droogfontein Solar Energy - Transportation Impact Assessment Amendment

CURRICULUM VITAE

Merchandt Le Maitre

- Loeriesfontein Solar Energy - Transportation Impact Assessment Amendment
- Koeris WEF - Transportation Impact Assessment Amendment
- East Gate Shopping Centre - Glint & Glare Assessment
- Oya Energy - Glint & Glare Assessment
- Yemaya – Glint & Glare Assessment
- Beaufort West WEF – Preliminary Engineering Design
- Heuweltjies WEF – Transportation Study
- Kraaltjies WEF – Transportation Study
- Koup 1 & 2 – Transportation Study
- Grootegeluk Solar Project – Transportation Study
- Renewstable Swakopmund – Glint & Glare Assessment
- Several projects are Confidential as they are not yet in the public domain and hence have not been included in the list above.

OTHER

- Project Springbok – Design of Services and Railway Siding.
- Phalaborwa Mining Company – Preliminary Design of Bulk Water feed and Railway Line.
- Kansanshi Copper Mine, Zambia – Junior Site Foreman.
- Final QC for Sasol Secunda.
- NDT testing – MMC Nelspruit, Global Forest Products Sabie.
- Boiler inspections and preliminary design – MMC Nelspruit, Global Forest Products, TSB Malelane.

Computer Skills

- AutoCAD Civil 3D
- AutoCAD Storm and Sanitary Analysis
- Microsoft Office
- Microsoft Project
- TechnoCAD
 - Surfmate
 - Roadmate
 - Pipemate
 - Watermate
- AutoTURN (Vehicle Turning Simulation Software)
- RiverCAD
- HecRAS
 - 1D Flood Modelling
 - 2D Flood Modelling

A handwritten signature in black ink, appearing to read "W. Maitre".



SIVEST Civil Engineering Division

Building A, Loftus Park
417 Kirkness Street, ARCADIA
Pretoria, 0007
Gauteng
South Africa

Tel + 27 11 798 0600
Fax +27 11 803 7272

Email info@sivest.co.za
www.sivest.co.za

Contact Person: Merchandt Le Maitre
Email: merchandtm@sivest.co.za