

ADDENDUM 5

Rational Method DWA

Project = DE WILDT PV PROJECT

Analysed by = CAS COETZER

Name of river = DE WILDT PV PROJECT

Description of site = DE WILDT PV PROJECT

Date = 4/25/2016

Area of catchment = 14.68 km²

Dolomitic area = 0.0 %

Mean annual rainfall (MAR) = 664.00 mm

Length of longest watercourse = 3.5 km

Flow of water = Defined water course

Height difference along 10-85 slope = 38.7 m

Rainfall region = Coastal

Area distribution = Rural: 100 %, Urban: 0 %, Lakes: 0 %

Catchment description - Urban area (%)

Lawns		Residential	and industry	Business	
Sandy, flat (<2%)	0	Houses	0	City centre	0
Sandy, steep (>7%)	0	Flats	0	Suburban	0

Heavy soil, flat (<2%)	0	Light industry	0	Streets	0
Heavy soil, steep (>7%)	0	Heavy industry	0	Maximum flood	0

Catchment description - Rural area (%)

Surface slopes		Permeability		Vegetation	
Lakes and pans	0	Very permeable	0	Thick bush & forests	0
Flat area	0	Permeable	0	Light bush & cultivated land	0
Hilly	85	Semi-permeable	95	Grasslands	95
Steep areas	15	Impermeable	5	Bare	5

Average slope = 0.01474 m/m

Time of concentration = 53.0 min

Run-off factor

Rural - C1 = 0.554

Urban - C2 = 0.000

Lakes - C3 = 0.000

Combined - C = 0.554

The HRU, Report 2/78, Depth-Duration-Frequency diagram was used to determine the point rainfall.

Return Period (years)	Time of concentration (hours)	Point rainfall (mm)	ARF (%)	Average intensity (mm/h)	Factor Ft	Runoff coefficient (%)	Peak flow (m ³ /s)
1:2	0.88	19.9	98.8	22.2	0.75	41.5	37.66
1:5	0.88	27.1	98.4	30.2	0.80	44.3	54.47
1:10	0.88	34.2	98.0	38.0	0.85	47.0	72.93
1:20	0.88	42.3	97.5	46.7	0.90	49.8	94.88
1:50	0.88	54.9	96.8	60.3	0.95	52.6	129.21
1:100	0.88	67.6	96.1	73.6	1.00	55.4	166.12

Run-off coefficient percentage includes adjustment saturation factors (Ft) for steep and impermeable catchments

Alternative Rational Method

Project = DE WILDT PV PROJECT

Analysed by = CAS COETZER
 Name of river = DE WILDT PV PROJECT
 Description of site = DE WILDT PV PROJECT
 Date = 4/25/2016
 Area of catchment = 14.68 km²
 Dolomitic area = 0.0 %
 Length of longest watercourse = 3.5 km
 Flow of water = Defined water course
 Height difference along 10-85 slope = 38.7 m
 Area distribution = Rural: 100 %, Urban: 0 %, Lakes: 0 %

Catchment description - Urban area (%)

Lawns	Residential and industry	Business
Sandy, flat (<2%) 0	Houses 0	City centre 0
Sandy, steep (>7%) 0	Flats 0	Suburban 0
Heavy soil, flat (<2%) 0	Light industry 0	Streets 0
Heavy soil, steep (>7%) 0	Heavy industry 0	Maximum flood 0

Catchment description - Rural area (%)

Surface slopes	Permeability	Vegetation
Lakes and pans 0	Very permeable 0	Thick bush & forests 0
Flat area 0	Permeable 0	Light bush & cultivated land 0
Hilly 85	Semi-permeable 95	Grasslands 95
Steep areas 15	Impermeable 5	Bare 5

Days on which thunder was heard = 60 days/year
 Weather Services station number = 512613
 Weather Services station location = HARTEBEESSPOORT DAM

Mean annual precipitation (MAP)	= 664 mm						
Duration	2	5	10	20	50	100	200
1 day	58	82	100	120	150	175	203
2 days	73	105	129	156	196	230	268
3 days	82	117	145	176	220	259	301
7 days	105	152	188	227	284	332	384

The modified recalibrated Hershfield relationship was used to determine point rainfall.

Average slope	= 0.01474 m/m
Time of concentration	= 53.0 min
Run-off factor	
Rural - C1	= 0.554
Urban - C2	= 0.000
Lakes - C3	= 0.000
Combined - C	= 0.554

Return period (years)	Time of concentration (hours)	Point rainfall (mm)	ARF (%)	Average intensity (mm/h)	Factor Ft	Runoff coefficient (%)	Peak flow (m ³ /s)
1:2	0.88	27.38	97.8	30.35	0.75	41.5	51.37
1:5	0.88	46.19	97.8	51.19	0.80	44.3	92.44
1:10	0.88	60.42	97.8	66.96	0.85	47.0	128.47
1:20	0.88	74.65	97.8	82.73	0.90	49.8	168.06
1:50	0.88	93.46	97.8	103.58	0.95	52.6	222.10
1:100	0.88	107.68	97.8	119.35	1.00	55.4	269.38
1:200	0.88	121.91	97.8	135.12	1.00	55.4	304.97

Run-off coefficient percentage includes adjustment saturation factors (Ft) for steep and impermeable catchments

Standard Design Flood method

Project name	= DE WILDT PV PROJECT
Analysed by	= CAS COETZER
Name of river	= DE WILDT PV PROJECT
Description of site	= DE WILDT PV PROJECT
Date	= 4/25/2016
Catchment characteristics:	
Area of catchment	= 14.68 km ²
Length of longest watercourse	= 3.5 km
1085 height difference	= 38.7 m
Average slope	= 0.0147 m/m
Drainage basin characteristics:	
Drainage basin number	= 1
Mean annual daily max rain	= 56 mm
Days on which thunder was heard	= 30 days
Runoff coefficient C2	= 10 %
Runoff coefficient C100	= 40 %
Basin mean annual precipitation	= 550 mm
Basin mean annual evaporation	= 1800 mm
Basin evaporation index MAE/MAP	= 3.27

RAINFALL DATA

The rainfall data in the table below are derived from two sources. The daily rainfall is from the Department of Water Affairs's publication TR102 for the representative site. The modified Hershfield equation is used for durations up to four hours. Linear interpolation is used for values between 4 hours and one day.

Weather Services station ex TR102 = 546204 @ STRUAN

Point mean annual precipitation = 550 mm

Dur:	RP =2	5	10	20	50	100	200
.25 h	15	25	33	41	51	59	67
.50 h	20	33	43	53	67	77	87
1 h	24	41	53	66	82	95	107
2 h	29	48	63	78	98	113	127
4 h	33	56	73	90	113	130	148
1 day	56	80	99	119	150	177	206
2 days	71	105	132	161	205	243	286
3 days	80	117	146	177	224	263	308
7 days	102	154	196	242	310	369	435

CAUTION. The time of concentration is less than one hour.

Runoff coefficients C2 = 10 % C100 = 40 %

Return period (years)	Time of concentration (hours)	Point precipitation (mm)	ARF (%)	Catchment precipitation (mm)	Runoff coefficient (%)	Peak flow (m ³ /s)
1:2	0.88	23.3	97.8	22.8	10.0	10.52
1:5	0.88	39.2	97.8	38.4	20.8	36.95
1:10	0.88	51.3	97.8	50.2	26.5	61.49
1:20	0.88	63.4	97.8	62.0	31.1	89.27
1:50	0.88	79.4	97.8	77.7	36.4	130.73
1:100	0.88	91.5	97.8	89.5	40.0	165.55
1:200	0.88	103.6	97.8	101.3	43.2	202.51

ADDENDUM 5

HERBST ALGORITHM DEVELOPED AT THE
DEPARTMENT OF WATER AFFAIRS

$$Q_T = C_{\text{HERBST}} A^{0.46} P^{0.93} (1 + (K_T * C_V) / 100)$$

With:

C_{HERBST} = Coefficient of variation

A = Catchment area in km^2

P = Mean annual precipitation

K_T = Frequency factor

For this situation:

Coefficient of variation = 153.6

Catchment area = 14.68 km^2

Mean annual precipitatio = 664 mm

Frequency factor = 4.3

Recurrence interval = 100 year

Q_T : 146 m^3/s

ADDENDUM 6

HRU ALGORITHM DEVELOPED AT THE
THE WITS UNIVERSITY

$$Q_T = 0,0377 K_T P A^{0,8} (S^{0,5} / (L L_C))^{0,2}$$

With:

K_T = Constant dependant on veld zone and T

A = Catchment area in km^2

P = Mean annual precipitation mm

S = Slope of the longest water course in m/m

L = Length of the longest stream in km

L_C = Distance to the centroid of the catchment in km

For this situation:

$$K_T = 1.200$$

$$A = 14.68 \text{ km}^2$$

$$P = 664 \text{ mm}$$

$$S = 0.0140$$

$$L = 3.5 \text{ km}$$

$$L_c = 5 \text{ km}$$

$$\text{Recurrence interval} = 100 \text{ year}$$

$$Q_T = 95 \text{ m}^3/\text{s}$$

ADDENDUM 5

TEN NOORT STEPHENSON ALGORITHM DEVELOPED AT WITS UNIVERSITY

$$Q_T = (a_3P + b_3)T^{-b_2}A^{b_1}$$

With:

T = Recurrence interval in years

A = Catchment area in km²

P = Mean annual precipitation mm

b₁ = Coefficient dependant on veld zone, region and P

b₂ = Coefficient dependant on veld zone, region and P

b₃ = Coefficient dependant on veld zone, region and P

a₃ = Coefficient dependant on veld zone, region and P

For this situation:

T = 100 year

A = 14.68 km²

P = 664 mm

a₃ = 0.0012

b₁ = 0.69

b₂ = 0.49

b₃ = -0.18

Q_T = 60 m³/s

ADDENDUM 5
UNIT HYDROGRAPH METHOD

$$Q = Q_p \times d_e \times F_m = \text{Peak flood in m}^3/\text{s}$$

With:

$$Q_p = \text{Peak flood of the 1 hour synthetic hydrograph} \\ = K_U \times (A / T_L)$$

$$A = \text{Catchment area in km}^2$$

$$T_L = C_T [L \times L_C / (S^{0.5})]^{0.36} = \text{Basin lag in hour}$$

$$C_T = \text{Constant depending on the sone}$$

$$L = \text{Length of the longest stream in km}$$

$$L_C = \text{Distance to catchment centroid in km}$$

$$S = \text{Average slope along longest stream}$$

$$P = \text{Mean annual precipitation}$$

$$K_U = \text{Constant depending on the sone}$$

$d_e = \text{Percentage of storm run-off} \times d_g$

$d_g = d \times a$ in mm

$d =$ Design rainfall depth in mm

$a =$ Area reduction factor

$i =$ Rainfall intensity mm/hour

and,

$F_m = \text{Highest ordinate obtained with the S-curve transformation}$

For this situation:

A	=	14.68 km ²
T _L	=	2.11 hour
C _T	=	0.35
L	=	3.5 km
L _C	=	5 km
S	=	0.014 m/m
P	=	664 mm
K _U	=	0.277
d _g	=	90.0 mm
d	=	28.8 mm
a	=	1
i	=	100.0 mm/hour
F _m	=	1

$$T = 100 \text{ year}$$

$$Q_p = 1.9229 \text{ m}^3/\text{s}$$

$$d_e = 28.8 \text{ mm}$$

$$Q = 55.38 \text{ m}^3/\text{s}$$