

POWERING POSSIBILITY

MARAPONG EXT 7 HOUSING PROJECT

ENGINEERING SERVICES REPORT

Prepared By



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REFERENCES:

Ref. 1	: GCS_RP_10_2016_Geotechnical Study Report Marapong Housing Extension project DRAFT v01 By Geotechnical Consult Services.
Ref. 2	: Agreement for the Marapong-Boikarabelo Effluent Transfer (MBET) Project
Ref. 3	: Lephalale Local Municipality Section 78(3) For Water And Sanitation Services Feasibility Study.

1 INTRODUCTION

1.1 Appointment

Dries De Ridder Town Planner on behalf of Exxaro contracted Tswella Engineering Projects (Pty) Ltd (TEP) as the Civil and Electrical Engineers to prepare an Engineering Services Scheme Report for the Marapong Ext 7 Housing Project for the purposes of township approval. The development falls within the jurisdiction of Lephalale Local Municipality.

1.2 Scope of Works

The scope of work entails the following:

- Engineering inputs to the Township Layout Plan
- Inputs to the Geotechnical and Stormwater Reports
- Investigations on Existing Bulk Engineering Services
- Preparation of Engineering Services Scheme Report for the proposed Marapong Ext 7 Housing Development

1.3 Key Assumptions

This report is submitted with the understanding that the bulk water and bulk sewer information provided by the Lephalale Local Municipality is correct.

1.4 Objective of this Report

The objective of this report is to identify the availability of bulk services required for the development of the proposed Marapong Ext 7 and to outline the preliminary design (level of service and design standards) of the internal engineering services.

1.5 Purpose of Report

This short report serves as the Engineering Services Report for the proposed development on a portion of the Remainder And Portion 1 Of The Farm Nelsonskop 464 LQ. The objectives of this Engineering Services Report is to provide information on the availability of Bulk Services for the full development as well as conceptual internal Engineering Services designs for Marapong Ext 7, to assist the local,

provincial and national authorities in their assessment in considering development approvals for this proposed development.

2 **PROJECT DETAILS**

2.1 Project Name

The project is referred to as Marapong Ext. 7 Housing Development and consists of approximately 718 residential stands measuring 320m² on average.

2.2 Locality

The site is located approximately 13 km due west of Lephalale, directly north of the Matimba Power station. The site is bounded on the south by the Provincial Road D2816 to Marapong, to the east by Marapong Township and north by the Remainder of the Farm Nelsonskop 464 LQ.

The location of the Marapong Ext 7 site is shown on Fig 2.1 below.



Fig 2.1: Locality of Marapong Ext. 7 Site

3 SITE DESCRIPTION

3.1 Zoning

The site is currently zoned as agriculture and partly Residential 4.

3.2 Servitudes

The following servitudes affect the site:

- An overhead power line near the eastern boundary.
- A sewer pump station at the bottom centre of the site
- A sewer pipeline running in-between Areas 2 and 3
- A balancing dam at the bottom centre of the development for treated effluent from Matimba Power Station
- A conveyor belt traversing the site in-between Areas 3 and 4 •

3.3 **Proposed Development**

3.3.1 Town Planning

The proposed development plan for Marapong housing extension is shown on the drawing in Annexure A. This development will be phased and will comprise the 4 areas depicted as AREA 1 to AREA 4 covering a total of 220.58 hectares.

The township layout under consideration is for AREAs 1 and 2, now being referred to as Marapong Ext 7 covering a total of 86.8 hectares.

The proposed site layout for Marapong Ext 7 is included in Annexure B.

The Land use for Marapong Ext 7 is given in the table below.

USE ZONE	LAND USE	TOTAL NUMBER	AREA (Ha)	% OF WHOLE
Residential 1	Residential - Approx. 320m ² stands	718 Stands	24.57	11.14
Business 1	Business - Coverage - 70%, storeys - 3	8 Stands	13.25	8.02
RSA	Crèche and Police Station - Coverage 80%, storeys-3, FAR-2.4	1 Stand	1.07	0.48
Educational	Coverage-60%, storeys-3, FAR-1.8	2 Stands	2.72	1.23
Special	Stormwater Dams	2 Stands	8.73	3.96
POS	Parks	8 Stands	16.01	7.26
Streets	Streets		28.72	15.06
TOTAL		739 Stands	95.07	43.10

The proposed land use for the future AREAs 3 and 4 are given in the table below

USE ZONE	LAND USE	TOTAL NUMBER	AREA (Ha)	% OF WHOLE DEVELOPMENT
Residential 4	Residential Flats - Approx. 1 unit per 125m ²	7904 Units	98.80	44.79
Residential 4 Sensitive areas	Parks/Gardens		15.50	7.03
Cemetery	Cemetery		5.01	2.27
Roads	Roads		6.20	2.81
TOTAL			125.51	56.9

3.3.2 Phasing of Development

The proposed development is going to be implemented in four (4) phases and the proposed phasing plan is depicted on drawing in Annexure C.

The time frames for completing the phases are as follows:-

- Phase 1 2019
- Phase 2 2020
- Phase 3 2021
- Phase 4 2022

3.4 Topography

The site is located in a relatively flat area, sloping at approximately 1% towards the north east of the site.

3.5 Geology

A geotechnical investigation was undertaken by Geotechnical Consult Services in May 2016 (Report No GCS-RP/010/2016).

The geology of the site consists of the following:-

3.5.1 Regional Geology

The site is underlain by a sandstone member of the Swartrand Formation (Ps) for most of the area and in the north-eastern corner by the Clarens Formation (Tre) which is a mostly massive, well sorted, fine grained sandstone separated by the Daarby Fault. (Refer to Figure 4). The area is covered by a blanket of unconsolidated sand ranging from fine clayey sand reddish to fine grained yellowish sand up to 3m thick.

The Daarby Fault, which connects the Eenzaamheid and Zoetfontein Faults has a maximum throw of 300m and a plunges at 55° in a nor theastern direction in the vicinity of the site. The fault is however not active.

3.5.2 Engineering Geology

In general the sandstone of the Swartrand Formation is stable and no major engineering geological risks are expected, otherwise the Matimba Power station would not have been constructed at its current location.

3.5.3 Ground Water and Hydrology

The Karoo sediments are not a good aguifer and shallow groundwater occurrences is rare, Pans do occur where localized depressions occur and calcrete developed.

Information obtained from Exxaro's Geohydrologist indicated that the water level of the monitoring borehole located at the stormwater control dam is 10m below surface. This borehole is located close to the drainage and it can be regarded as the minimum water level for the study area.

4 LEVEL OF SERVICE AND DESIGN STANDARDS

4.1 Standards & Guidelines

All the services for the sub divisions in Nelsonskop Farm Housing Development will be designed in accordance with the "Guidelines for Human Settlement Planning and Design" as compiled under patronage of the Department of Housing in collaboration with the CSIR, the New Red Book, as amended in 2000. Cognizance will further be taken of the principles contained in the :

- "Guidelines for the Provision of Engineering Services in Residential Townships" published by the Department of Community Development in 1983 (Blue Book), and
- 2. "Guidelines on the Planning and Design of Township Roads and Stormwater Design" of the SA institution of Civil Engineers, and
- 3. The Municipality's and Department of Housing's requirements.
- 4. CIBSE TM46:2008-Energy Benchmarks
- 5. NRS 034 (all parts)
- 6. Eskom Electrification Planning Guidelines

4.2 Water Supply

4.2.1 Proposed Design Standards

The internal services will be designed in accordance with the "Guidelines for Human Settlement Planning and Design" as compiled under the patronage of the Department of Housing in Collaboration with the CSIR (the "Red Book"), as amended in 2000.

The requirements of Lephalale Local Municipality will also be adhered to.

The following standards are proposed:

Average Daily Demand

- Residential (zone 1) : 600
- Residential (zone 4) :
- o Business/Municipal :
- Educational
- o Community Facilities:
- Parks (>2Ha<10Ha) :
- o (>10Ha)
- o Cemetery
 - (>2Ha<10Ha) :

600l/day/erf

12 500l/day

600l/day/ residential unit (minimum) 400l/day/100m² of GFA 400l/day/100m² of GFA 400l/day/100m² of GFA 12 500l/day 10 000l/day

Instantaneous demand peak factor	:	4
Peak flow residual head	:	24m (minimum)
Fire Risk	:	Low risk
Fire hydrant delivery volume	:	15 litre/s at peak flow
Peak flow + fire residual head	:	15m (minimum)
Pipe material	:	uPVC class 12
Pipe size	:	90mm dia Min
Pipe cover	:	700mm minimum
Trenches	:	Widths to SABS 1200, class B bedding, Back-filling to 90% mod AASHTO, Back-filling to 93% mod AASHTO in road reserves
Valves	:	Waterworks type, Cast Iron, clockwise closing, opposite splay pegs, 'Davis & Deale' type plastic box – lid colour as specified.
Hydrant spacing	:	240m on 90mm dia min

4.2.2 Water Demand

Based on the proposed development outlined in Section 3.2 of this report, average annual daily demand (AADD) of the proposed development is estimated in the tables below.

	a water Demand for War			1	
USE ZONE	LAND USE	QTY	Unit	Unit Water	Total Water
				Demand	Demand
				(kł/day)	(kł/day)
Residential 1	Residential - Approx. 320m ² stands	718	stand	0.6	430.800
Business 1	Business - Coverage - 70%, storeys - 3, FAR- 1.4	129 850	m²	0.004	519.400
RSA	Crèche and Police Station - Coverage 80%, storeys-3, FAR- 2.4	20 544	m ²	0.004	82.176
Educational	Coverage-60%, storeys-3, FAR-1.8	29 376	m²	0.004	117.504
Parks	Parks	16Ha	Sum	10.00	10.00
	TOTAL	•		•	1 159.88

Average Annual Water Demand for Marapong Ext 7

USE ZONE	LAND USE	QTY	Unit	Unit Water Demand (kℓ/day)	Total Water Demand (kℓ/day)
Residential 4	Residential Flats - Approx. 1 unit per 125m ²	7904	unit	0.6	4 742.40
Cemetery	Cemetery	5	Ha/Sum	12.5	12.50
Res 4 Sensitive Areas	Parks/Gardens	15.5	Ha/Sum	10.0	12.50
	SUB-TOTAL				4 767.40

Average Annual Water Demand for Future AREAs 3 and 4

The estimated water demand for the full development is 5 927kl/day (5.9Ml/day).

4.2.3 Internal Services

It is proposed for this development that the highest level of service be provided with house connections. All stands will be connected to a comprehensive piped water network to be installed in the street reserves.

The internal water reticulation for Marapong Ext 7 will be designed for an estimated peak flow of 54t/s and for 144t/s including fire demand .

A preliminary layout of the internal water reticulation system for Marapong Ext. 7 is detailed on drawing included as Annexure D. The pipe sizes, based on initial calculations, range between 90mm and 450mm in diameter. The bigger diameters being the distribution mains from the reservoir complex and the smaller diameters reticulating inside the proposed development.

4.3 Sanitation

4.3.1 Design Standards

The internal sewer network will be designed according to the same standard as listed in section 4.1, as well as the following:

Daily flow	:	540 litres/day/erf (Minimum)
Sewer Return Factor	:	0.90 of Water Demand
Peak factor	:	Sliding scale as per Red Book – 2,5 maximum
Pipe material	:	Solid Wall Class 34 uPVC. SABS approved
Pipe size	:	150mm minimum
Pipe cover	:	0,6m minimum at head, 1,0m generally
		1,2m under streets
Infiltration Factor	:	15%

Pipe slopes and minimum velocity	:	To attain minimum flow velocity of 0,7m/s as soon as possible starting with minimum 1:80 slope at head.
Trenches	:	Widths to SABS 1200 LB, class B bedding, back-filling to 90% mod AASHTO, back-filling to 93% mod AASHTO in road reserves. The selected granular material must extend at least over the full trench width and at least 100mm over the pipe
Manhole Spacing	:	90m maximum
Manhole Sizes	:	0m – 2,5m deep : 1,05m dia Chamber, no shaft 2,51m – 3,5m deep : 1,25 dia Chamber, shaft Deeper than 3,5m : 1,5m dia Chamber, shaft

4.3.2 Design Sewer Flows

Based on the proposed development outlined in Section 3.2 of this report the estimated daily flow from the development is as follows:-

- Marapong Ext 7 1 035 kł /day (1.04 M ł/day) with an estimated peak flow of 34l/s.
- Future AREAs 3 and 4 4 268 kl/day (4.27 Ml/day) with an estimated peak flow of 142 l/s.

4.3.3 Internal Services

All stands in the proposed development will be connected to a gravitational pipe network draining into the proposed bulk sewer pipeline.

A preliminary layout of the internal sewer reticulation system for Marapong Ext. 7 is detailed on Drawing included as **Annexure E**. The pipe sizes are estimated to range between 160mm and 250mm in diameter.

4.4 Internal Roads

Structural Design

The structural design is based on the recommendations contained in the Draft TRH 4, Structural Design of Inter-Urban and Rural Road Pavements and UTG 2.

Design Standards

See Table 4.4.1 below

Typical Road Cross Sections

Refer to Annexure F

TABLE 4.4.1

Marapong Ext. 7 : Roads & Streets : Design Standards

Road Class Function	3 District Distributor		4 stributor b	5a Residential Access Collector	5b Residential Access Loop	5c Residential Access Cul-de-sac
Max Dwelling Units Serviced	n.a.	1500	500	200	120	60
Desirable Maximum Length	n.a.	4km	2km	500m	500m	150m
Road Reserve Width	20m, 25m, 30m, 36m	25m	20m	15m	13m	13m
Residential Access	Nil	Nil	Yes	Yes	Yes	Yes
Design Speed	60km/h	50km/h	50km/h	40km/h	30km/h	20km/h
Min Stopping Distance	85km	60m	60m	45m	32m	20m
Intersection Sigh Distance	95m	90m	60m	45m	35m	n.a.
Minimum Centre Line Radius	150m	90m	50m	15m(30m if angle<60°)	12,5m(30m if angle<60°)	12,5m(30m if angle<60°)
Minimum Kerb Size	12m	12m	12m	10m	10m	10m
Minimum Splay Size	5mx20m	5,5m	5,5m	5,5m	5,5m	5,5m
Intersection Spacing Adjacent	90m	90m	50m	50m	40m	n.a.
Intersection Spacing Opposite	90m	90m	25m	25m	20m	n.a.
Favoured Max Gradient	7,00%	7,00%	7,00%	10,00%	12,00%	12,00%
Max Grade / Grade Length	10% 100m	10%/100m	12%/100m	12%/70m	16%/50m	12%/50m
Min Grade	0,30%	0,30%	0,30%	0,30%	0,30%	0,30%
Pavement :						
Roadway Width	8m, 7,4m	7m	6m	5m	4,5m	4,5m
Surface Type	30mm Asphalt	30mm Asphalt	25mm Asphalt	20mm Asphalt	20mm Asphalt	20mm Asphalt

	0.00%	0.00/	0.00/	0.00/	0.00/	0.00/
Cross Fall	3.0%	3.0%	3.0%	3.0%	3.0%	3.0%
Kerbs	Fig.7	Fig.7	Fig.8	Fig.8	Fig.8	Fig.8
Typical Base	150mmG2 100%Mod	125mm G4	98%Mod	200mmG5 95%Mod	200mmG5 95%Mod	200mmG5 95%Mod
Typical Subbase	150mmC3 96%Mod	150mm C4	96%Mod	150mm Rip & Recompact	150mm Rip & Recompact	150mm Rip & Recompact
Typical Subgrade	150mmG7 93%Mod	150mm G7	98%Mod			
Sidewalk (Gravel)	1m wide@ 4% slope	1m wide@ 4% slope		1m wide@ 4% slope	1m wide@ 4% slope	1m wide@ 4% slope
Stormwater						
Recurrence Interval	1:10 years	1:5 y	ears	1:2years	1:2 years	1:2 years
Encroachment : Major	max depth 150mm	max deptl	h 150mm	n.a.	n.a.	n.a.
Encroachment : Minor	No encroachment	40% of road width free		20% of road width free	max depth of 10mm at crown. No overtopping	Max depth of 10mm at crown. No overtopping
Roadside Channels	Min gradient 0,4%	Min gradient 0,4%		Min gradient of 0,4%	Min gradient 0,4%	Min gradient 0,4%
	Max velocity 3m/s	Max velo	city 3m/s	Max velocity 3m/s	Max velocity 3m/s	Max velocity 3m/s

4.5 **Stormwater**

5.5.1 Internal Services

Stormwater runoff within the development will be managed by a conventional drainage system consisting of open side channels next to streets in conjunction with roadways. The street design will also allow for stormwater management for bigger floods.

The runoff shall be safely discharged into the natural stormwater channels.

5.5.2 Design Standards

The internal stormwater system will be designed based on runoffs calculated by using the Rational Method.

Standards:

Recurrence Internal	:	According to road classification in Table 4.4.1
Major Channels	:	Open lined with concrete to handle 1:10 year storm
Minor Channels	:	2,0m wide open lined to handle 1:2 year storm
Lined Side Drains	:	25MPa concrete on 100mm sub-base

A preliminary roads and stormwater layout is included in Annexure G.

4.6 Street lighting

Street lighting will be provided in all applicable streets according to City of Lephalale standards and per the details listed below.

These guidelines would be applied at the discretion of the designer given the actual layout and design requirements, and based on available funding levels. Reduced funding may imply reduced service levels and vice versa.

Table 4.6.1: Street Lighting

Description	Eskom	Poles	Light fittings
Fully Subsidised	Eskom does not include street lighting in their standard network design and will not install streetlights if Council does not take it over or pay a service fee	 Fittings mounted on the network poles. 8m mounting height (m/h) for bus routes 5.5m m/h for residential roads 	 250W HPS for bus routes, 100W HPS for residential roads

4.7 **Electrical Services**

The electrical network will be overhead, with the service connections to the houses both overhead and underground. Due to the density of this development the partial installation of underground medium voltage cables will be investigated and if financially viable, will be done.

The network will consist of wooden poles, medium voltage ACSR Hare and Fox conductor, distribution class 11kV/415V transformers, and Low Voltage Aerial Bundled Conductor (35mm² and 70mm² sizes) incorporating streetlight conductors. The electrical network will be designed in accordance with the ESKOM Distribution Standards.

5 AVAILABILITY OF BULK SERVICES

5.1 Water

5.1.1 Bulk Water Services Provider

The bulk water supply to the proposed development will be provided by Exxaro from its bulk water network.

5.1.2 Bulk Water Demand

The average daily water demand for the whole development is calculated as 5925kl/day with a peak flow of 274l/s.

5.1.3 Existing Bulk Water Infrastructure

The town of Lephalale is currently getting its bulk water supply from Zeeland WTW. Zeeland WTW has just been upgraded from 20MI/d to 40MI/d.

However, Marapong township within which Marapong Ext. 7 lies, gets its bulk water supply from the 1.6M ℓ /d Matimba Water Treatment Works located at Matimba Power Station through the 3.5ML and 8M ℓ Marapong Reservoirs in Marapong (refer to the bulk water supply schematic layout for Marapong included in **Annexure H**. However, Matimba WTW can no longer meet the water demand of the growing township of Marapong and hence cannot support the additional **1.2** M ℓ /day water demand from the proposed development of Marapong Ext. 7 or the 6M ℓ /day demand for the full development.

Furthermore, the two reservoirs in Marapong do not have adequate capacity for the areas under their command. The proposed Marapong cannot therefore be used to supply the proposed new Marapong Housing Extension Development.

5.1.4 Proposed Bulk Water Infrastructure

It has been established from the available reports that the upgraded 40MI/d Zeeland WTW has additional capacity to support future developments in Marapong (which include Marapong Ext 7.) for the 20 year horizon. However, some bulk water pipeline upgrades to Marapong. The proposed bulk pipeline upgrade starts from Zeeland WTW (800mm in dia) and runs generally in the northerly direction to supply the proposed industrial development next to Matimba Power Station from where it will be pumped to Marapong Reservoirs via a 315mm diameter pipeline. The 315mm portion of the bulk pipeline traverses along the southern boundary of the proposed Marapong Housing Extension Development on its way to Marapong. The construction of this bulk pipe line is earmarked to commence in February 2017.

This proposed bulk pipe line is shown on the layout included in Annexure I..

5.1.5 Storage For Marapong Development

It is clear from above that a dedicated storage will be required for the proposed development. A 2 day storage of 12Ml capacity is therefore being proposed to supply

the proposed Nelsonskop Farm Housing Development. This will be in the form of two (2) 6MI reservoirs to be located at the highest elevation of the proposed development. Due to the flat nature of the terrain, an elevated water tank will be built to boost pressure to the proposed development. Initial indications are that a 1.2Mℓ elevated tank will be required. The storage infrastructure is shown on drawing in **Annexure J**.

5.1.6 Marapong Development Bulk Supply Pipeline

It is proposed that supply to the 12Ml storage facility for the proposed development be taped off the 350mm diameter bulk water pumping main discussed under Section 5.1.4 above. The tap off will be in the form of a 315mm diameter uPVC pipeline to convey a peak flow of $104\ell/s$ which is equivalent to 1.5 x AADD. This internal bulk pipeline is shown on drawing in **Annexure J**.

5.2 Sewerage

4.2.1 Expected Sewage Flow

The average daily sewage flow from the whole development is calculated as 5303kl/day with a peak flow of 176l/s (refer to section 4.3.2 of this report).

5.2.1 Bulk Sewer Services Provider

The bulk waste water provisions to the proposed Marapong Ext. 7 development will be provided by Lephalale Local Municipality who are overally responsible for bulk waste water functions.

5.2.2 Existing Bulk Sewer Infrastructure

Marapong Township currently discharges to two Waste Water Treatment Plants (WWTPs), Nelsonskop (2.4ML/d) and Zongezien (0.5Ml/d). This is achieved through pumping via a number of booster pump stations located around Marapong. Also discharging to Nelsonskop WWTP is Matimba Power Station.

Zongezien WWTP is currently being upgraded to 16MI/d and will be able to accommodate the 5.30Mt/day sewage generation from the Nelsonskop Farm Housing Development.

The locations of the existing WWTPs are included on the schematic layout included in **Annexure K**.

5.2.3 Proposed Bulk Sewer Infrastructure

The Development of Marapong Ext. 7 currently does not have bulk sewer pipelines servicing it according to the available reports. A bulk sewer pipeline will therefore be required to support this development. The proposed bulk sewer pipeline which is approximately 4.6km long is shown on the layouts included in **Annexure E**.

The bulk sewer pipeline will be designed for a peak flow of 176l/s from the full development and it is estimated that this bulk sewer pipeline will vary in size ranging from 400mm to 500mm in diameter and laid at an average slope of 0.5%. The indications from the preliminary modelling are that the bulk sewer pipeline will be

2.5m deep when it gets to Zongezien WWTP. A lifting pump will probably be required at the WWTW and its assumed at this stage that this will be the municipality's responsibility.

5.3 Access

5.3.1 Existing Road Access

Marapong Ext. 7 can be accessed via the Provincial Road D2816 (to Marapong), which runs along the southern boundary of the development as shown on the layout included in **Annexure L**.

5.3.2 Future Road Access

A second access road is earmarked in future for the proposed Marapong Ext.7 development in the form of a tarred road, which is going to come off the Provincial Road D2001 to Lephalale and will run along the northern boundary (also shown on layout included in **Annexure L**).

The future access road will be constructed as part of phase 1 to prevent congestion on the existing Road D2816 to Marapong. More accurate information regarding the extent of phase 1 construction will be guided by the Traffic Impact Studies which will be conducted in due course.

5.4 Bulk Stormwater Infrastructure

Non-perennial stormwater channels transects almost diagonally from the south western corner of the site to the north eastern corner of the site. The site is relatively flat. These channels have been established to have additional capacity to convey stormwater that's going to be generated from the proposed site.

The existing stormwater channels are shown on the layout included in **Annexure M**.

5.5 Electricity

5.5.1 Power Demand Calculation Assumptions

Eskom Electrification Planning Guideline, the CSIR Red Book and NRS 034 give an average power consumption of 2.7kVA per household for low income households, and an average annual energy consumption of 165kWh/sq.m¹ is assumed for the Commercial centres, assuming a power factor of 0.9 and load factor of 0.35.

¹ CIBSE TM 46: Energy Benchmarks

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Phases 1 -4(Marapong Ext 7) Demand Estimate

Land Use	Stand Area (m ²) and Classification as per CSIR Red Book Volume 2	Number of Stands	Average ADMD (kVA)	Total ADMD (kVA)
Residential	320sq.m	7944	2.7kVA ²	21 448.8
Residential 4	>320sq.m	4	6kVA	12
Business 1	1.66ha	8	60W ³ /sq.m	7950.0
RSA	1.07ha	1	40W/sq.m ⁴	428.0
Educational	2.72ha	2	40W/sq.m⁵	1088.0
Total for Mar	30 926.8			

Total demand for the full development is 30.926MVA. Allowing for 25% spare capacity, the total load that needs to be catered for is

38.66MVA for the full development.

Currently the area where the development will take place has 22kV lines which traverse the area.

Phases 1 to 4 will be developed from 2019 and completed in 2022.

1. BULK POWER SUPPLY OPTIONS

Given the amount of power that will be required, 3 power supply options have been put forward. These options will need to be discussed with Eskom, to determine which the preferred power supply option for the development is.

a. Extend the existing 22kV Lines to cover the whole development

This option entails extending the existing 22kV lines to cover the whole development.

This option however has the following disadvantages:

- This option will result in a network with under-voltage problems due to the long lines and might necessitate the need for compensation in the form of capacitor banks:
- Possibility of line overloading. Existing lines might need to be uprated, and • this might mean rebuilding the lines completely.
- Power System studies need to be done to assess the technical feasibility of • this option before it is costed.

b. Extend the existing 132/22kV substations and build new lines to cover the whole development and uprate the 132/22kV transformers.

This option entails extending the existing 132/22kV substations and build new 22kV lines to cover the whole development.

This option however has the following disadvantages:

CSIR Human Settlements Planning Guideline Vol 2

³ CIBSE TM 46: Energy benchmarks

⁴ CIBSE TM 46; Energy Benchmarks

⁵ CIBSE TM 46: Energy Benchmarks

C:\Admin\PROJECTS\EXXARO\NELSONSKOP FARM TOWNSHIP DEVELOPMENT\WORKING DOCUMENTS\REPORTS\OUTLINE SERVICES REORT\DRAFT Rev B - 21 OCTOBER 2016\MARAPONG EXT. 7 ENGINEERING SERVICES REORT-FINAL DRAFT Rev C 10-NOVEMBER 2016.doc 18

- This option will require a full Environmental Impact Assessment as the construction activities are considered as listed activities;
- The area around Lephalale has of late been involved in land disputes due to the high demand for land for building infrastructure such as Transmission and Distribution lines. Land issues might delay the implementation of this option.
- New servitude will be required for the new 22kV lines.

However this option will result in less system losses than Option a.

c. Extend the existing 132kV line network into the new development and create a new 132/22kV substation and build new 22kV lines to cover the whole development.

This option however has the following disadvantages:

- This option will require a full Environmental Impact Assessment as the construction activities are considered as listed activities:
- The area around Lephalale has of late been involved in land disputes due to the high demand for land for building infrastructure such as Transmission and Distribution lines. Land issues might delay the implementation of this option.
- New servitude will be required for the new 132kV and 22kV lines.

5.5.2 Way Forward

The purpose of this report is to propose solutions that can be looked at further before a quotation can be submitted. A meeting was held with Eskom to discuss the 3 options. Eskom will carry out network studies to confirm the technical feasibility of each of the 3 options as well as select the least life cycle cost option of providing power to the proposed development.

The 22kV lines which traverse residential stands will require to be rerouted to run along road servitudes. It is proposed that overhead line be used for the diversion rather than underground cables. Underground cables are prone to cable theft, especially in an area that is going to densely populated.

Eskom will do the costing for the technically feasible options as well as the design development for the selected power supply option.

Tswella Projects have commenced discussions with Eskom, and Eskom has made the following requests:

- The developer needs to complete an application for after confirming the • number of properties. An application fee needs to accompany the application and the application fee is dependent on the electrical load. It is thus important to have a correct estimate of the number of properties
- Eskom has given 2 options: either Eskom constructs the lines and substations, • or the developer opts for the "Self Build option" in which the developer constructs the Electrical infrastructure, which is handed over to Eskom after construction. Eskom is currently facing funding challenges, so if time is of the essence, the "Self build" option is recommended.

The conceptual drawing layouts of a 132/22kV substation are attached to Annexure Ν.

6 PROJECT COST ESTIMATES

6.1 Civil Engineering Services Cost Estimates

The Civil Engineering services costs are estimated at **R151million excluding VAT** and contingencies for the bulk infrastructure for the full development as well as well as internal Engineering services for Marapong Ext 7.

The detailed BOQ for the civil engineering infrastructure is included in **Annexure O**.

6.2 Electrical Engineering Services Costs

The electrical engineering services costs are estimated at **R378milion excluding VAT and Contingencies**.

These costs cover the bulk infrastructure and the internal reticulation infrastructure including street lighting for the full Development.

The detailed BOQ for the electrical infrastructure is included in **Annexure P.**

7 CONCLUSIONS

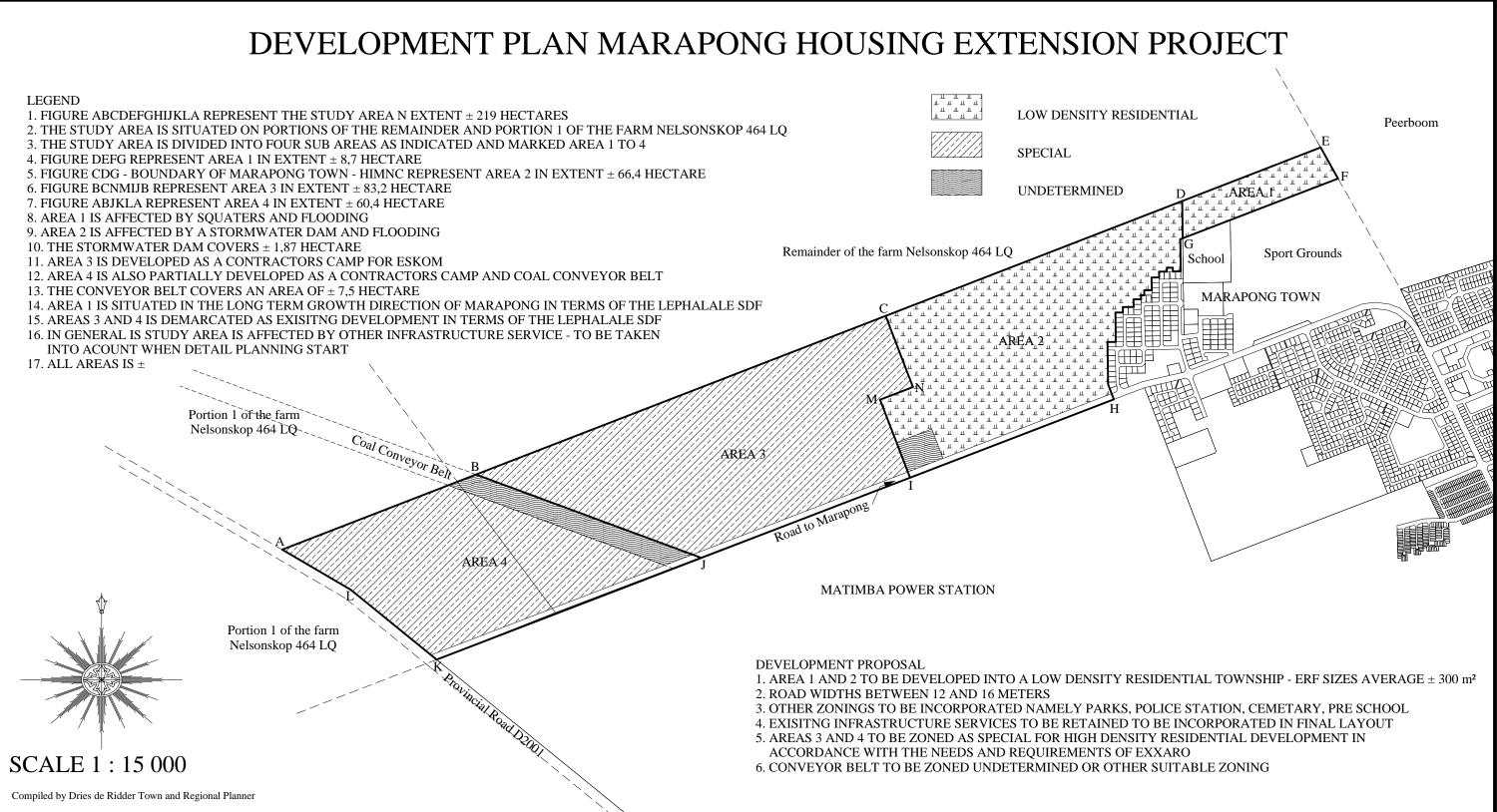
The Municipality is in the process of upgrading the water supply to Marapong and the bulk water pipeline will have sufficient capacity to support the proposed development. Construction of this bulk pipeline is earmarked to commence early next year and will be completed well before the first phase of the development is complete.

Lephalale Municipality is also commencing works on the Zongezien WWTW in February and the first phase of the upgrades will be completed well ahead of the proposed development. The sewage flows from the proposed development can be accommodated by the Zongezien WWTW once the upgrading works are completed.

The development traffic can be accommodated on the existing road network together with the proposed future access. However, upgrades to the intersections will be required and these shall be done in line with the Traffic Impact Study to be undertaken.

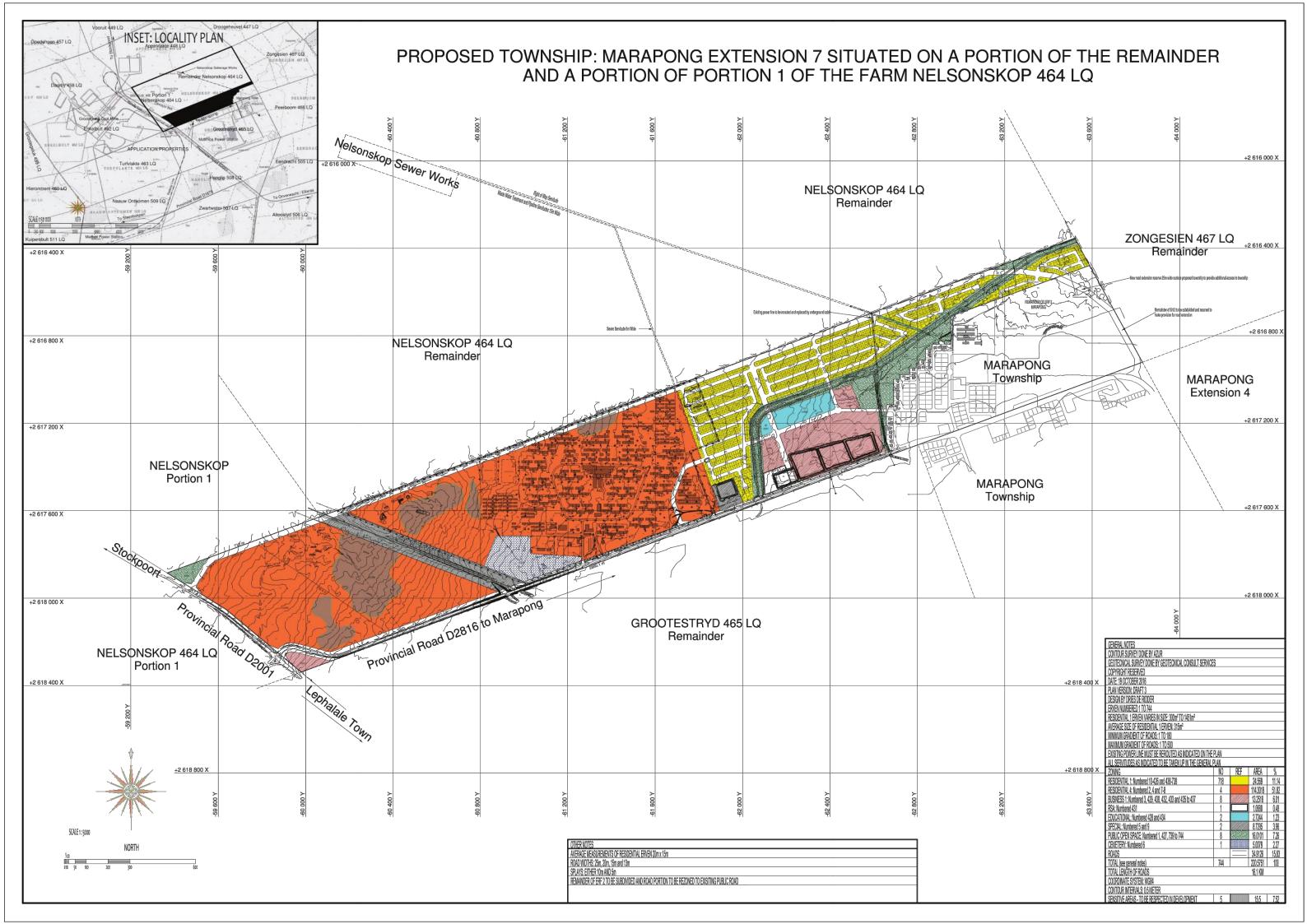
ANNEXURE A

PROPOSED DEVELOPMENT PLAN FOR MARAPONG HOUSING EXTENSION



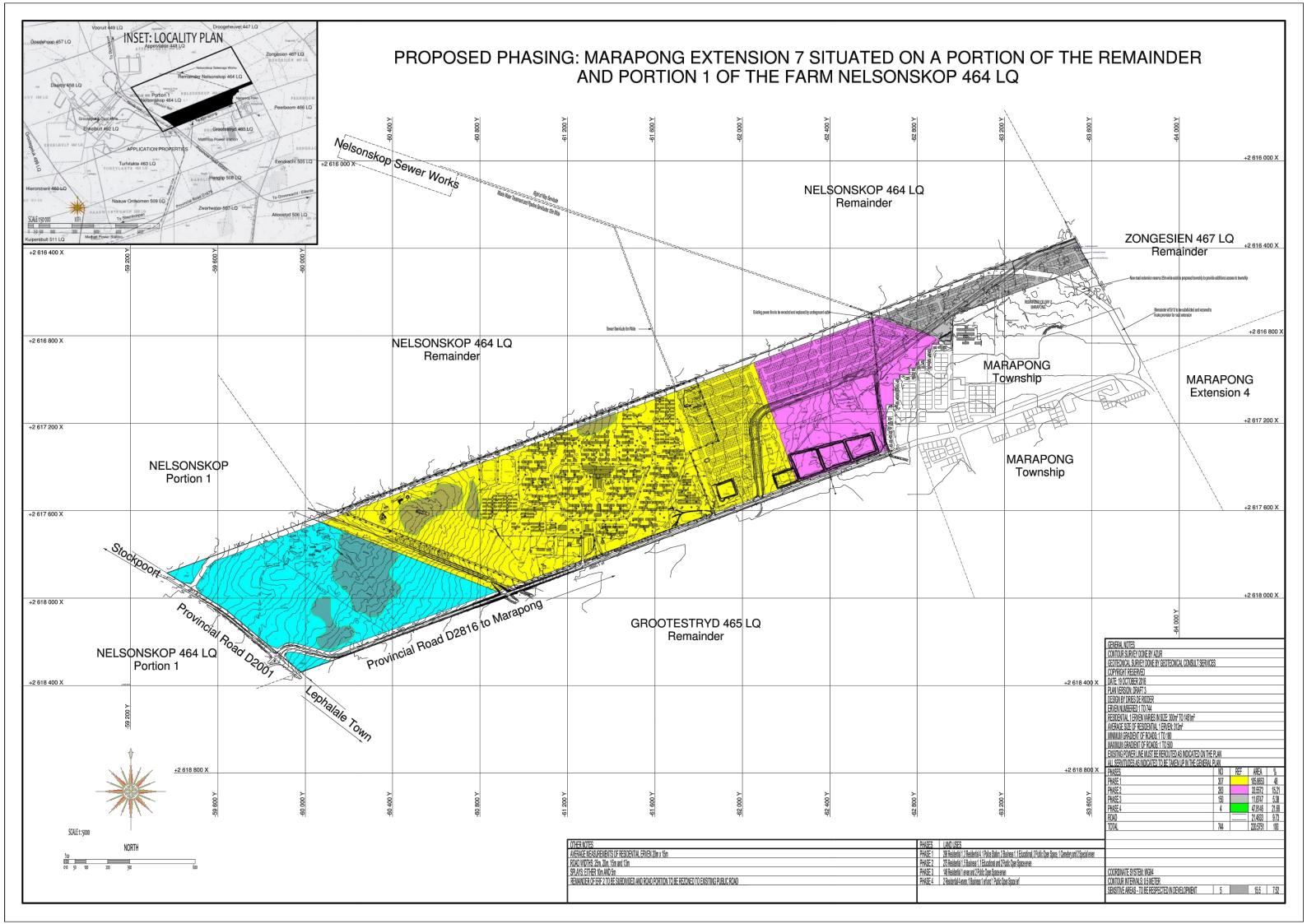
ANNEXURE B

MARAPONG EXT. 7 PROPOSED LAYOUT PLAN



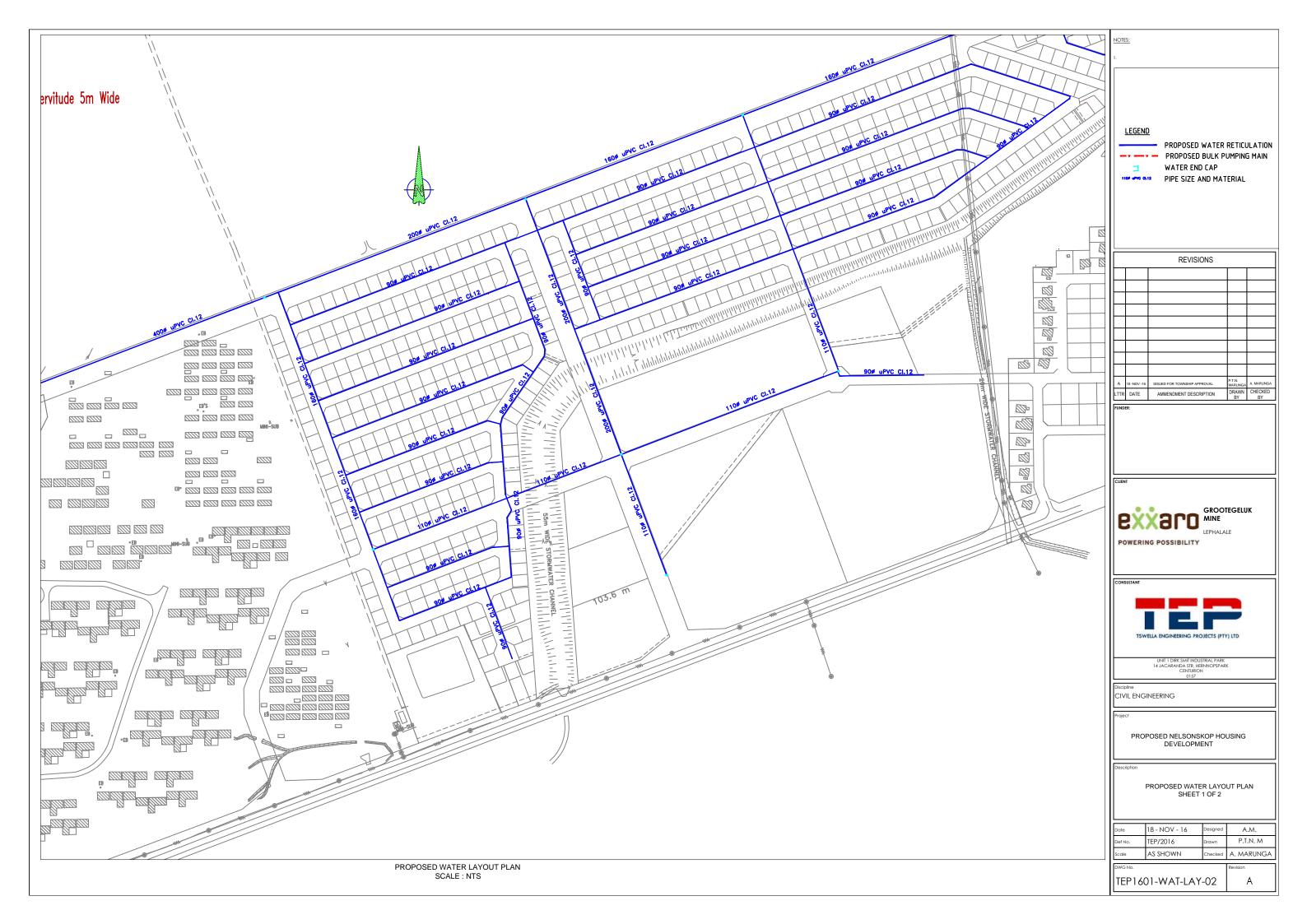
ANNEXURE C

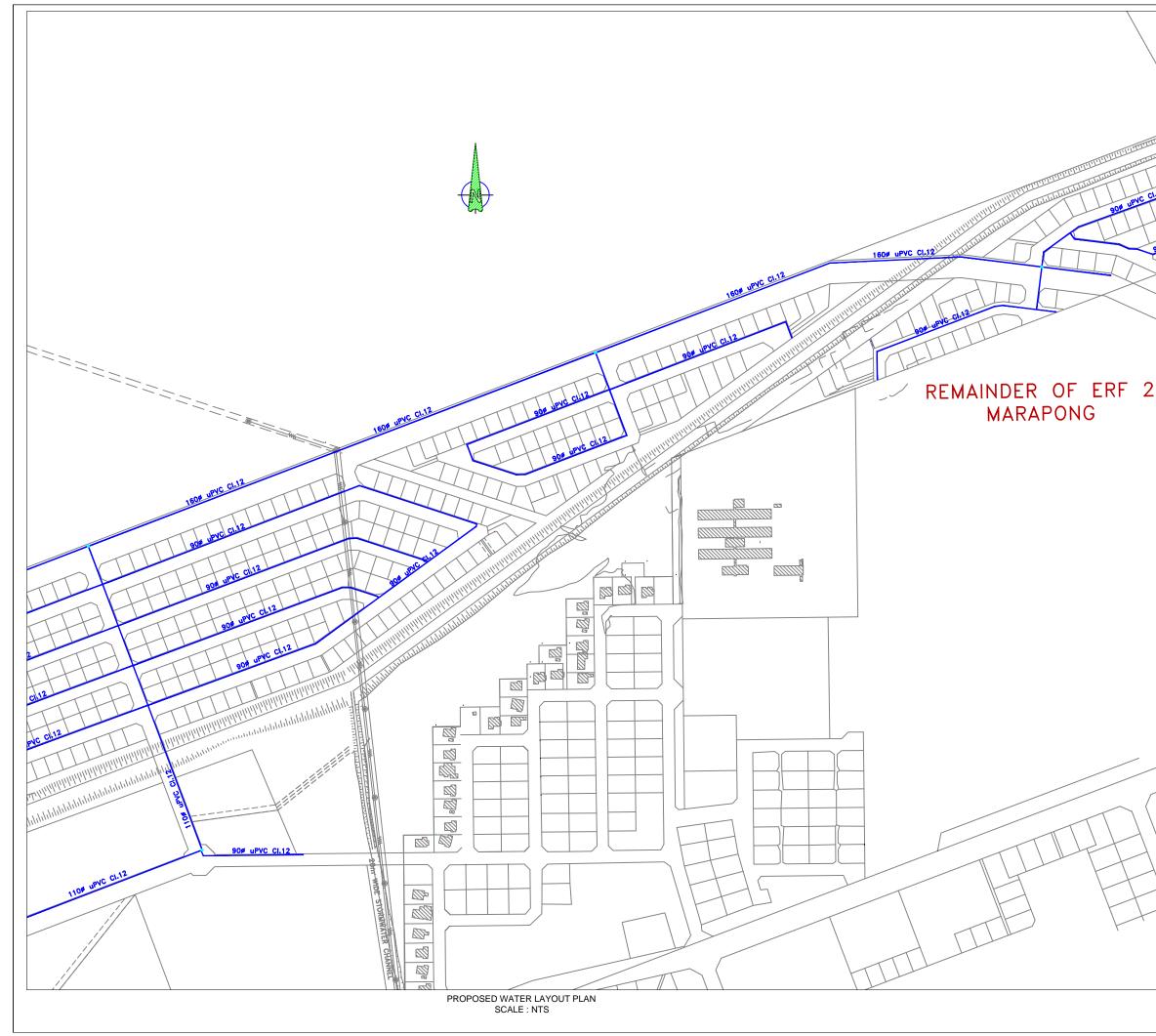
PROPOSED PHASING PLAN FOR MARAPONG HOUSING EXTENSION



ANNEXURE D

PRELIMINARY INTERNAL WATER RETICULATION LAYOUTS





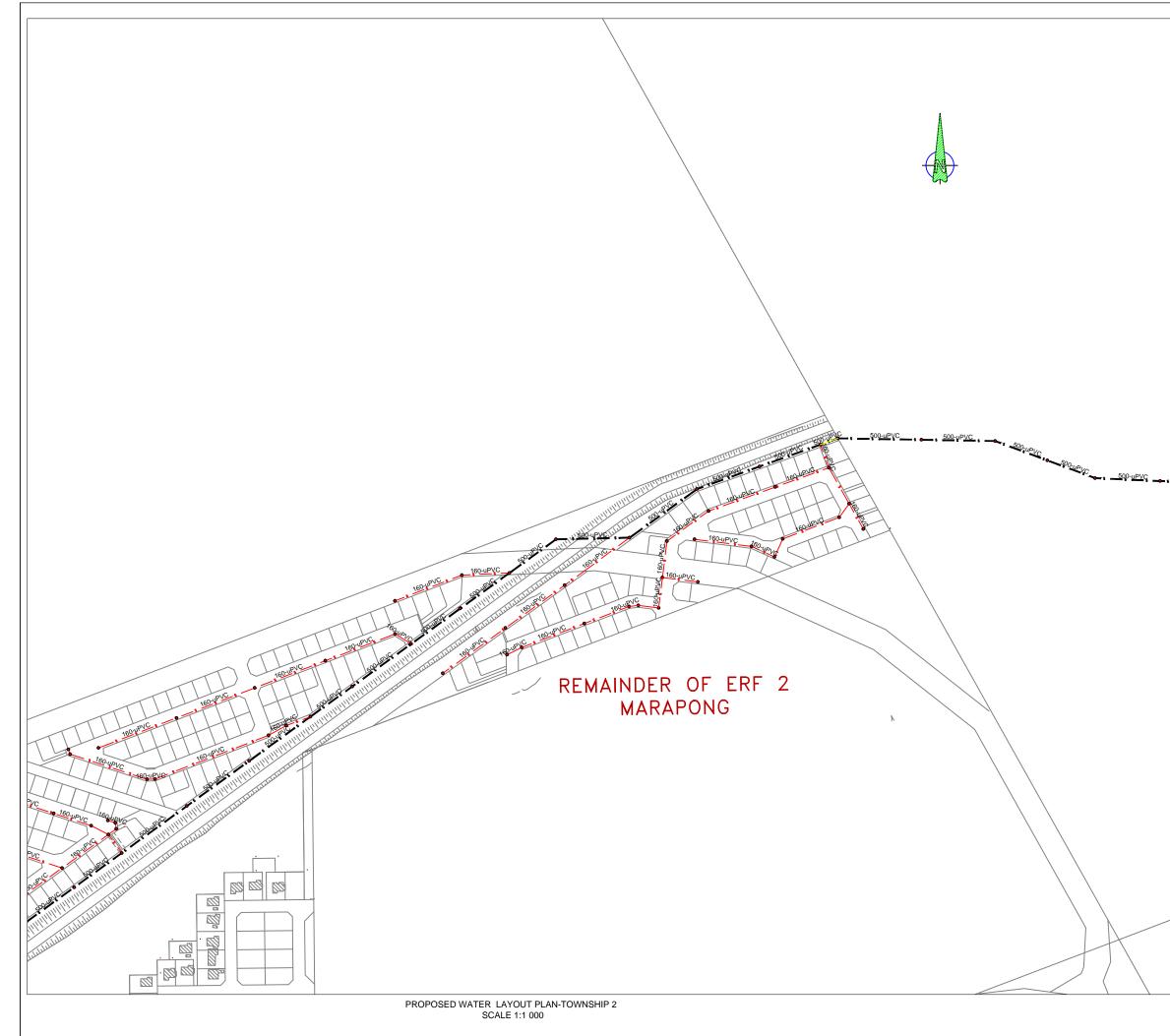
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ANNEXURE E

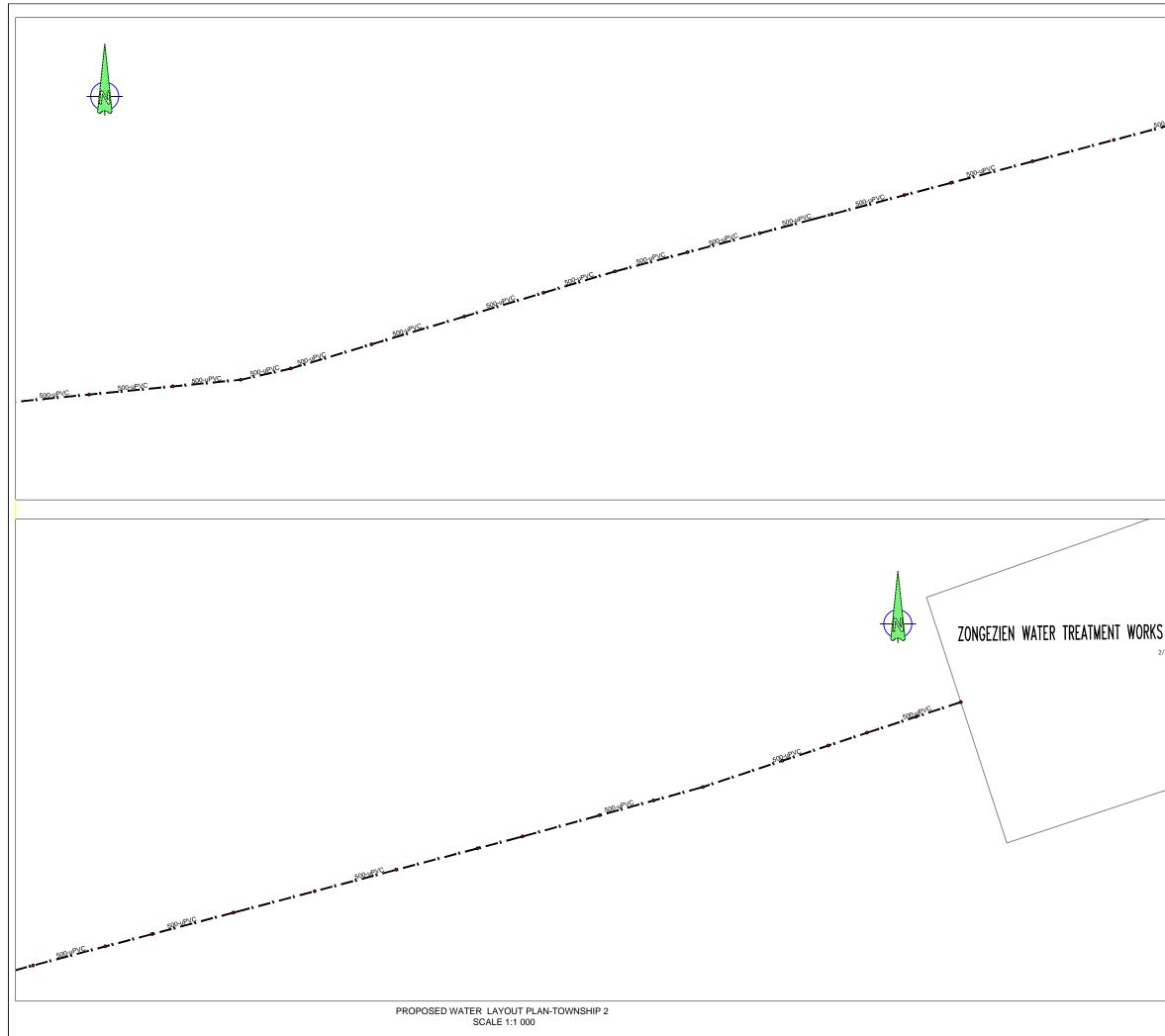
PRELIMINARY INTERNAL SEWER RETICULATION LAYOUTS



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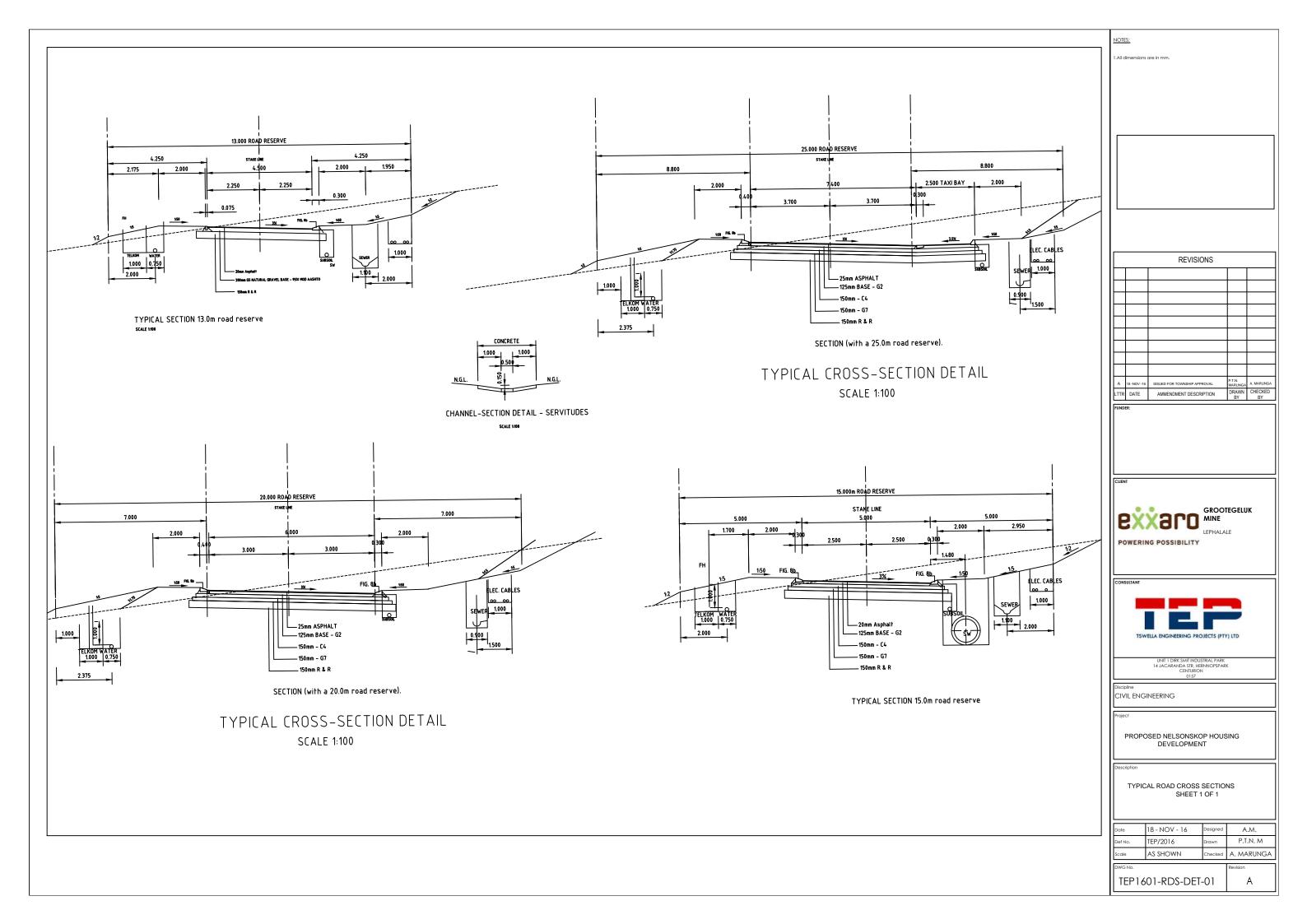
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ANNEXURE F

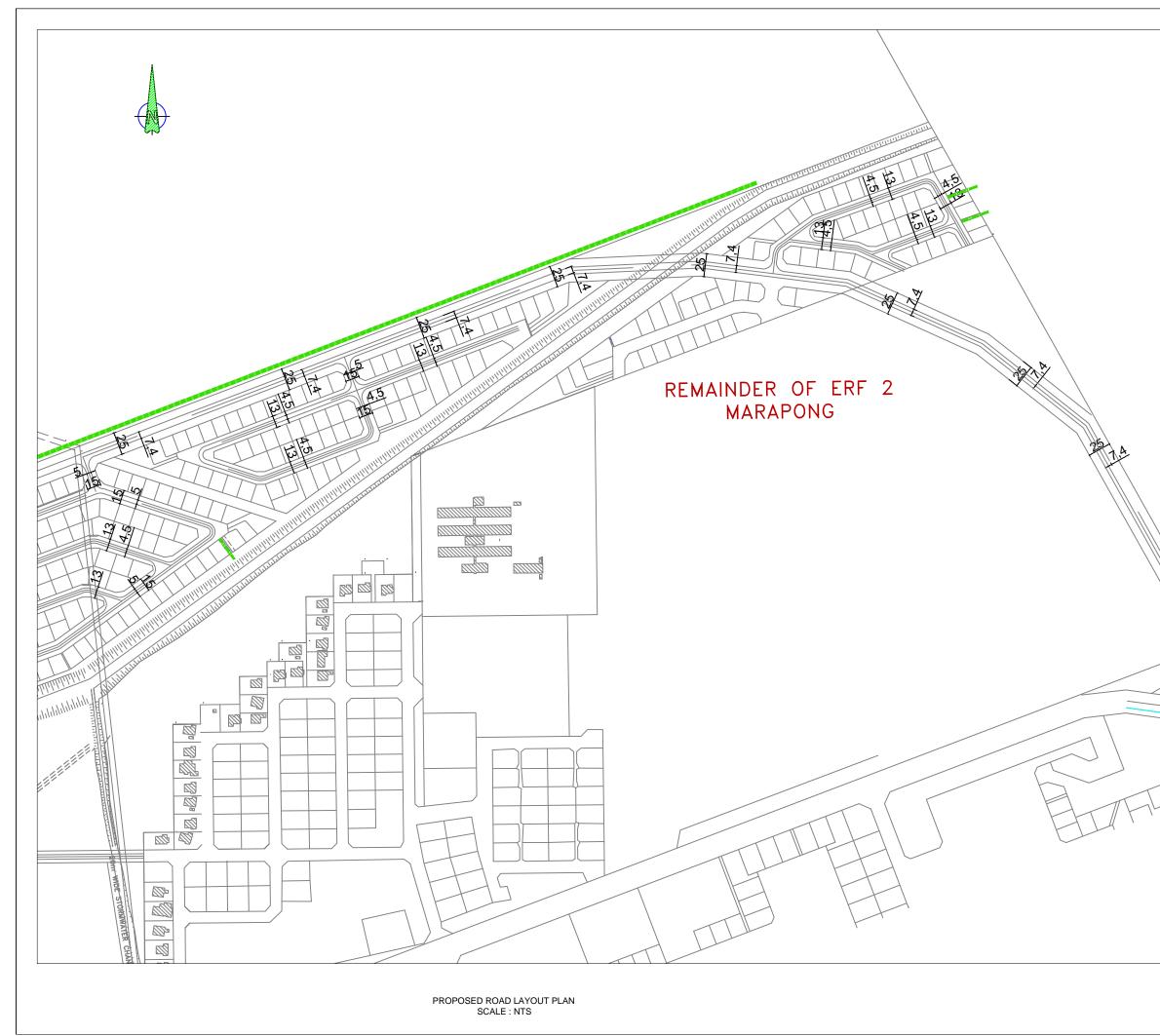
TYPICAL INTERNAL ROAD CROSS SECTIONS



ANNEXURE G

ROADS LAYOUT

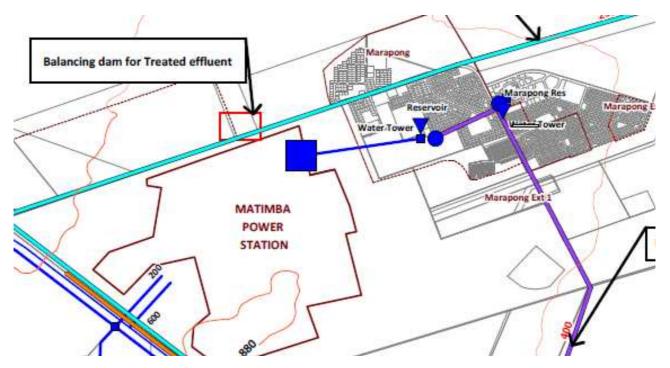




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ANNEXURE H

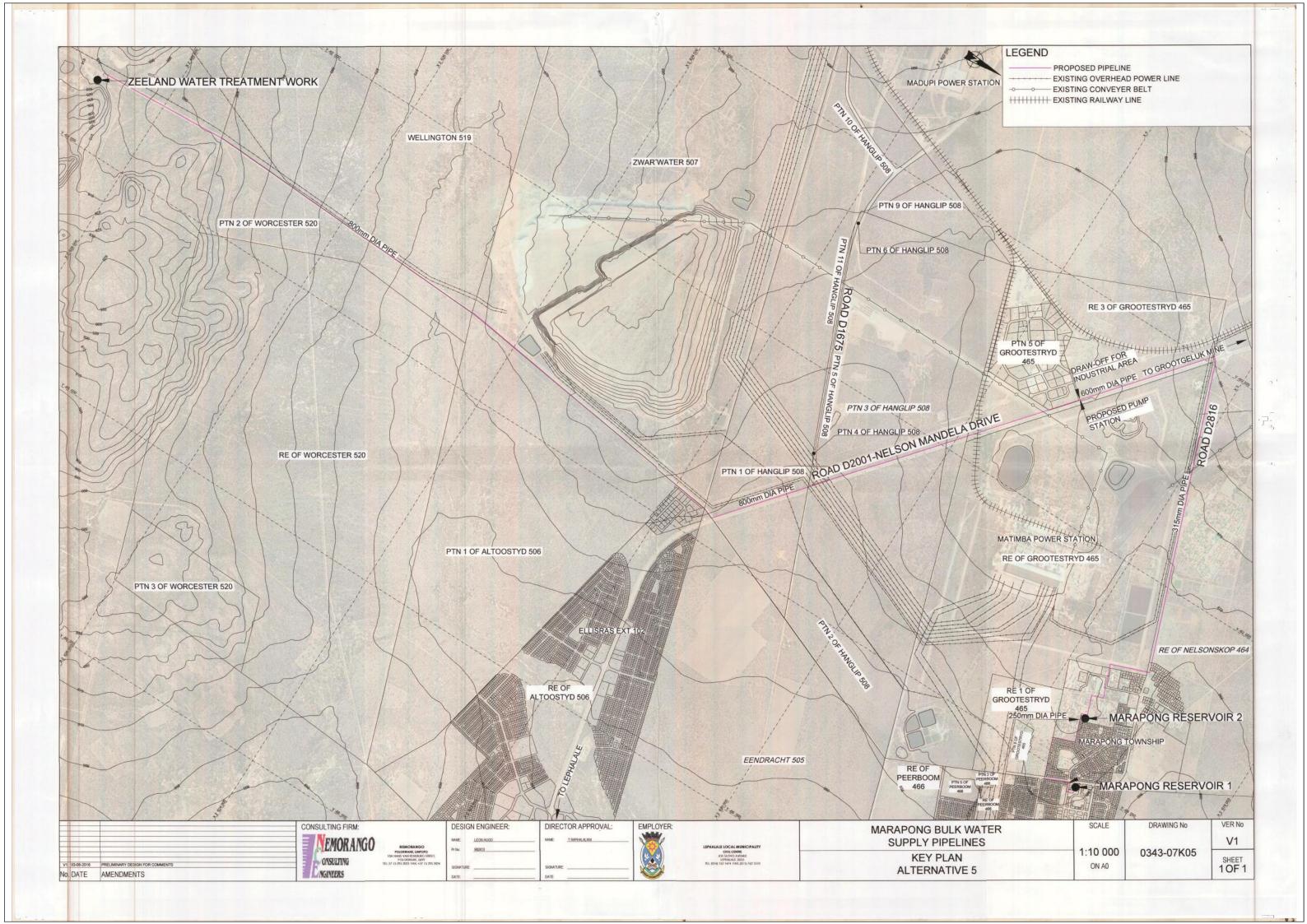
SCHEMATIC LAYOUT OF EXISTING BULK WATER SUPPLY FOR MARAPONG TOWNSHIP



Existing Bulk Water Infrastructure for Marapong

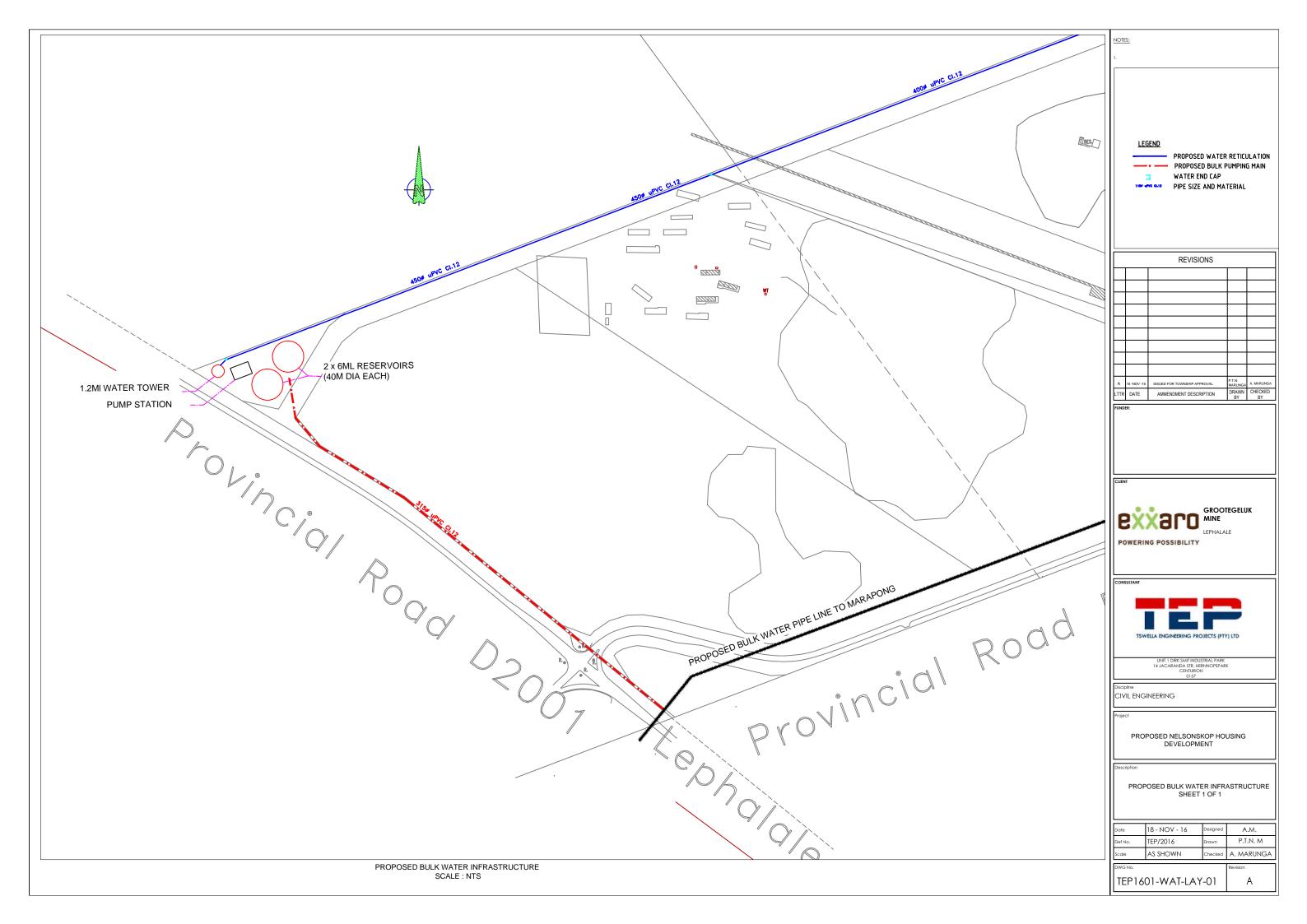
ANNEXURE I

PROPOSED BULK WATER SUPPLY PIPELINE TO MARAPONG



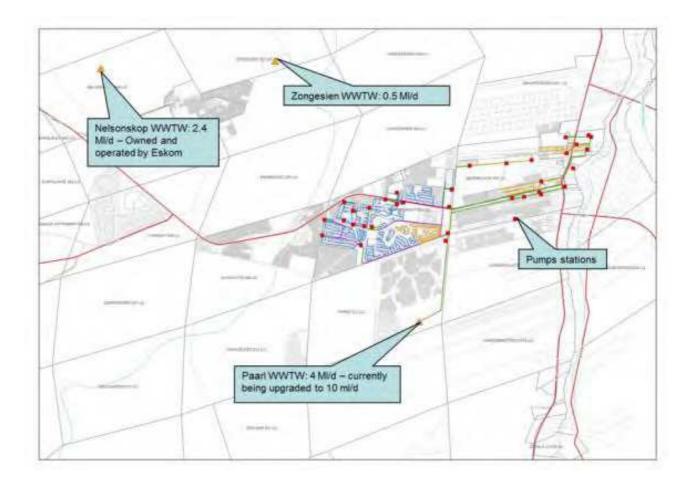
ANNEXURE J

PROPOSED BULK WATER INFRASTRUCTURE FOR MARAPONG EXT 7



ANNEXURE K

EXISTING BULK SEWER INFRASTRUCTURE SCHEMATIC LAYOUT



ANNEXURE L

EXISTING AND FUTURE ROAD ACCESSES TO MARAPONG EXT 7



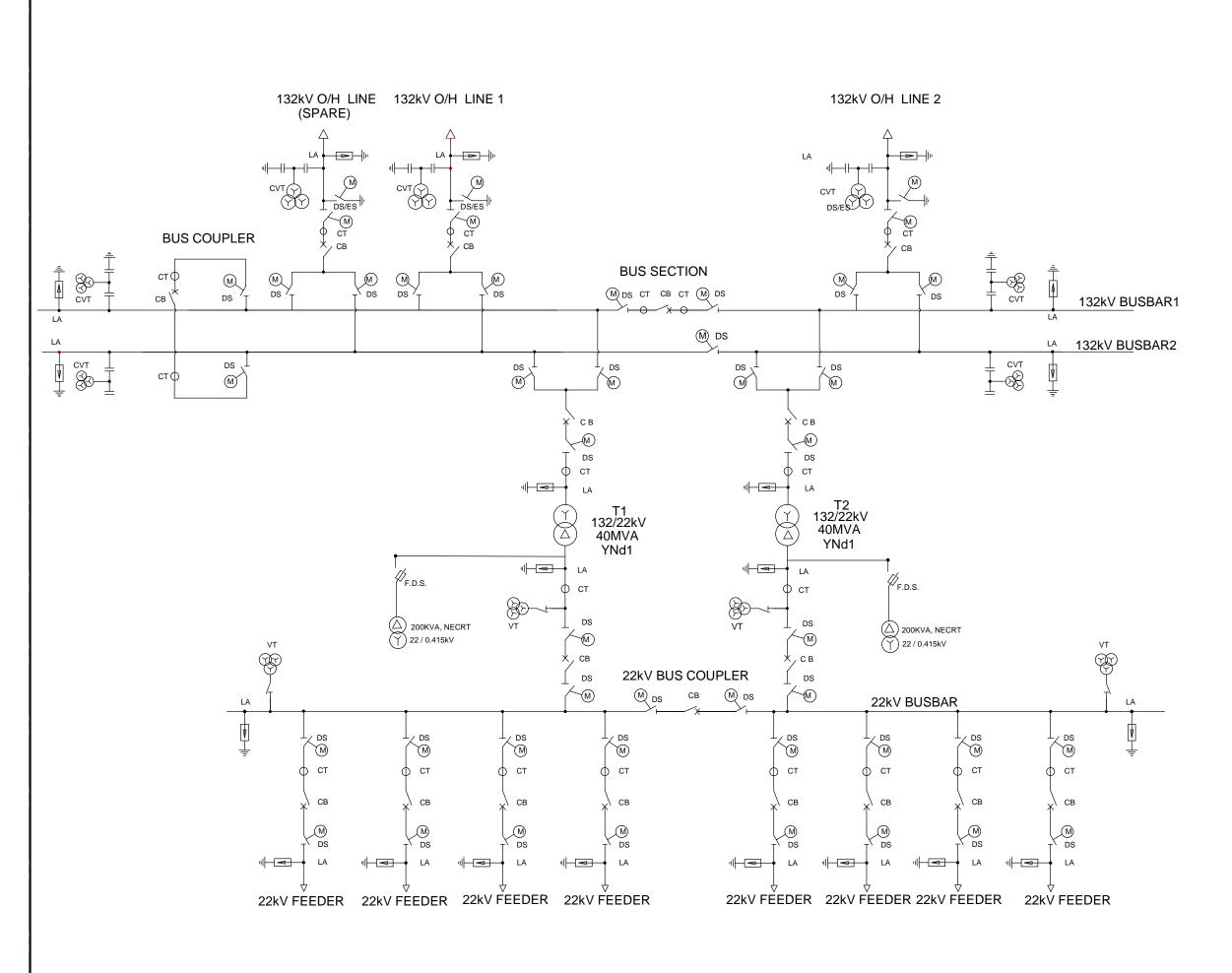
ANNEXURE M

EXISTING NATURAL STORMWATER CHANNELS



ANNEXURE N

CONCEPTUAL 132/22kV SUBSTATION LAYOUTS



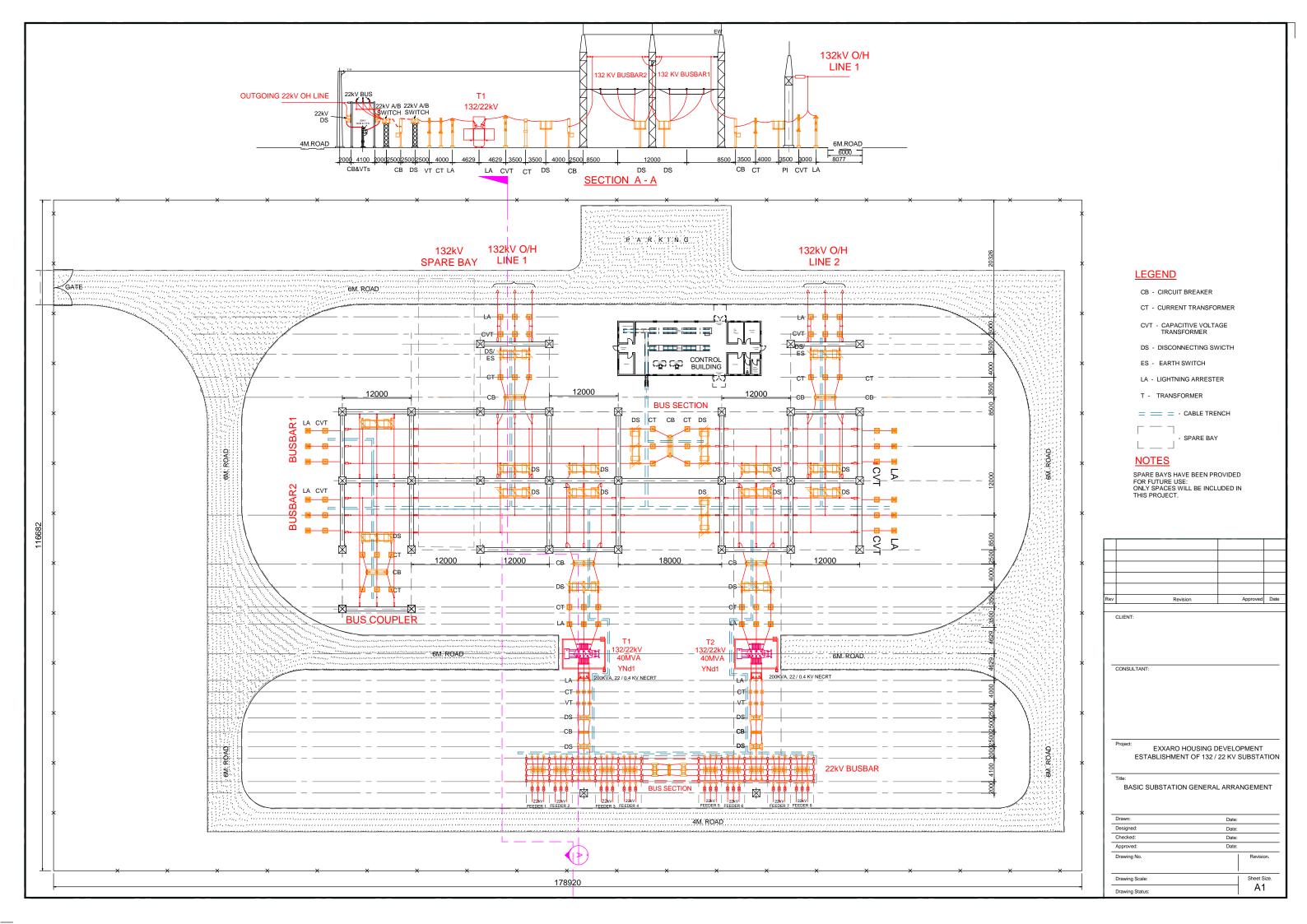
LEGEND

- CB CIRCUIT BREAKER
- CT CURRENT TRANSFORMER
- CVT CAPACITIVE VOLTAGE TRANSFORMER
- DS DISCONNECTING SWICTH
- LA LIGHTNING ARRESTER
- ES EARTH SWITCH
- T TRANSFORMER
- M MOTORISED

<u>NOTES</u>

SPARE BAYS HAVE BEEN PROVIDED FOR FUTURE USE: ONLY SPACES WILL BE INCLUDED IN THIS PROJECT.

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ANNEXURE O

CIVIL ENGINEERING SERVICES BILL OF QUANTITIES

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Item Description	Unit	QTY	Ra	ate	Ar	nount
Water Reticulation						
90mm dia uPVC Class 12 Z-lock	m	6900	R	649.82	R	4 483 736.39
110mm dia uPVC Class 12 Z-lock	m	1140	R	706.32	R	805 207.86
160Ø Class 12 uPVC Z-lock	m	1710	R	902.68	R	1 543 577.05
200Ø Class 12 uPVC Z-lock	m	670	R	1 163.78	R	779 734.79
400mm dia uPVC Class 12 Z-lock	m	1770	R	3 956.86	R	7 003 647.74
450Ø Class 12 uPVC Z-lock	m	1160	R	4 510.82	R	5 232 555.80
Erf connections	no.	718	R	931.03	R	668 477.11
					R	20 516 936.75
Internal Sewer						
160 mm dia uPVC class 34 sewer pipe	m	1948	R	1 164.62	R	2 268 682.95
200 mm dia uPVC class 34 sewer pipe	m	311		1 343.15	R	417 719.43
250Ø uPVC class 34 sewer pipe	m	410		1 674.74	R	686 645.37
Erf connections	no	718		1 488.44	R	1 068 703.18
					R	4 441 750.94
Bulk Water Mains						
315mm dia uPVC Class 12 Z-lock	m	658	R	2 583.60	R	1 700 008.06
		050		2 303.00	R	1 700 008.06
Bulk Sewer						
400 mm dia uPVC class 34 sewer pipe	m	606	R	4 566.71	R	2 767 424.82
500 mm dia uPVC class 34 sewer pipe	m	4014	R	6 530.39	R	26 212 993.13
					R	28 980 417.95
Reservoirs						
Reservoirs	MI	12	R	1 300 000.00	R	15 600 000.00
Water Tower	MI	1.2	R	2 500 000.00	R	3 000 000.00
Pump Station	Sum	1.0	R	5 500 000.00	R	5 500 000.00
					R	24 100 000.00
Internal Roads						
13m road reserves	m	3650	R	2 295.39	R	8 378 156.00
15m road reserves	m	3374	R	3 858.77	R	13 019 473.78
20m road reserves (6m carriageway)	m	634	R	4 914.66	R	3 115 891.97
25m road reserve (7.4m carriageway)	m	4895	R	8 190.90	R	40 094 467.70
25m road reserve (6.0m carriageway)	m	587	R	4 914.66	R	2 884 903.13
					R	67 492 892.58
Internal Stormwater		+				
V-drain 2.5m wide,0.15 deep	m	200	R	1 274.10	R	254 819.76
Stormwater Cut-off trench	m	4000	R	783.84	R	3 135 360.00
					R	3 390 179.76
Cable Ducts	-	+	-			
uPVC pipe 1*110mm dia Telkom	m	550	R	506.33	R	278 480.65
uPVC pipe 1*110mm dia Eskom	m	550	_	506.33	R	278 480.65
Approximate tenter			Ë		R	556 961.30

ANNEXURE P

ELECTRICAL ENGINEERING SERVICES BILL OF QUANTITIES

<u>Mara</u>	pong Ext 7 Development Electrical B	<u>oQ</u>			
ID	Item Decsription	Quantity	Unit	Rate	Total
	1 132kV Infrastructure				
	132/22kV Substation with				
	2x40MVA 132/22kV				
	transformers, 2x132kV feeder				
	bays, 6x22kV feeders, control				
а	room etc	1	Sum	R 45 000 000.00	R 45 000 000.00
	2x132kV feeder bays at				
	Groootgeluk and Matimba				
b	Substations	2	No	R 1 000 000.00	R 2 000 000.00
с	132kV overhead lines	8	km	R 2 000 000.00	R 16 000 000.00
	2 22kV Infrastructure				
а	22kV Overhead lines	60	km	R 300 000.00	R 18 000 000.00
	200kVA 22/0.4kV Pole				
b	mounted substations	150	No	R 300 000.00	R 45 000 000.00
	3 Low Voltage Infrastructure				
а	400V overhead lines	120	km	R 200 000.00	R 24 000 000.00
	Internal electfification to				
b	individual prlperties	1	Sum	R 25 000 000.00	R 25 000 000.00
с	LV connections to properties	8120	No	R 25 000.00	R 203 000 000.00
					R 378 000 000.00