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Geotechnical Instrumentation

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# Rapid Land Release Program: NHBRC Phase 1 Geotechnical Investigation for Portions 79, 91, 96, 321 and 322 of the farm Hekpoort 504 JQ, Mogale City LM: Final Report

Client: Glad Africa & GDHS

Reference: 19-0866.08R03

Dated: 16 October 2019

GCS Geotechnical (Pty) Ltd

63 Wessels Street Rivonia Cell: +27 (0)82 567 1561 ninow@gcs-sa.biz www.gcs-sa.biz

**GCS Geotechnical** 

Reference:	19-0866.08R03	Date: 16 October 2019

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<b>Reference:</b>	19-0866.08R03	Date: 16 October 2019

## DOCUMENT ISSUE STATUS

Report Issue	Final			
GCS Reference Number	GCS Ref - 19-0866.08R03			
Client Reference	Glad Africa			
Title	RLRP: NHBRC Phase 1 Geotechnical Investigation for Hekpoort, Mogale City LM: Final Report			
	Name	Signature	Date	
Author	Nino Welland	AGWL	16 October 2019	
Document Reviewer	Nino Welland	AGM	16 October 2019	
			1	

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#### Reference: 19-0866.08R03

Date: 11 October 2019

#### **EXECUTIVE SUMMARY**

This report presents the findings of a NHBRC Phase 1 geotechnical investigation for the proposed Agri-village development on portions 79, 91, 96, 321 and 322 of the farm Hekpoort 504JQ, Mogale City LM, and presents the conclusions and recommendations for excavations, foundations and earthworks.

The most important consideration in relation to the proposed development is the presence of potentially collapsible aeolian sand and potentially expansive topsoil and residuum. Groundwater supply may also be a constraint in terms of irrigation.

Based on the 1:250 000 Geological Map titled "2628 West-Rand (1986)", the site can be seen to be underlain by shale of the Silverton Shale Formation and intruded by numerous east-west oriented diabase dykes. The soil cover has been described as colluvium derived from the adjacent mountain ranges.

The materials on site generally classify as <u>SOFT</u> excavation (SABS 1200 D) to between 1.1 and 2.8 m at an average depth of 2.0 m. Below this depth, intermediate to hard excavation is to be anticipated.

The site has provisionally been classified as C2/H2-H3 in Hekpoort A and H1-H3 in Hekpoort B according to NHBRC guidelines. The following foundation options are proposed for the various site classes on site:

Hekpoort A:

NHBRC Site Class	Foundation Options
C2	• Stiffened strip footings
	• RC raft with articulation joints or solid lightly reinforced masonry
	• Compaction of in situ soils beneath individual footings
	• Soil raft
С2-Н2	• RC raft
	Piled construction
	• Soil raft
С2/Н2-Н3	• RC raft
	Piled construction
	Soil raft

#### Hekpoort B:

NHBRC	Foundation Options
Site Class	
H1	Modified normal
	• Soil raft
H2	• <i>RC raft with articulation joints or solid lightly reinforced masonry</i>
	Piled construction
	• Split construction
	• Soil raft
H3	• RC raft
	Piled construction
	• Soil raft

Finally, the ground conditions described in this report refer specifically to those encountered at the test positions advanced on site. It is therefore possible that conditions at variance with those discussed above may be encountered elsewhere on the site. In this regard it is critical that the NHBRC Phase 2 geotechnical investigation be commissioned and completed to assist the subsidy variation process.

Reference: 19	0866.08R03 Date: 11 October 2019
Commonoiale	Definitions and Abbreviations
<u>Commerciai:</u>	
GCS Geotechnical	GCS Geotechnical (Pty.) Ltd.
<u>Technical:</u>	
СН	Chainage (metres)
mbgl	metres below ground level
masl	metres above sea level
NGL	Natural Ground Level
FL	Foundation Level
BH	Borehole
SPT	Standard Penetration Test
Ν	SPT N value (blows per 300 mm)
TLB	Tractor-mounted Loader Backhoe
TP	Test Pit
DCP	Dynamic Cone Penetrometer
EABC	Estimated Allowable Bearing Capacity
G1-G10	Standard classification of natural road building materials (TRH 14)
CBR	California Bearing Ratio
MDD	Maximum Dry Density (kg/m3)
MADD	Modified AASHTO Dry Density
OMC	Optimum moisture Content (%)
PI	Plasticity Index
LL	Liquid Limit
LS	Linear Shrinkage
RMR	Rock Mass Rating
GSI	Geological Strength Index
mi	Hoek-Brown Constant (origin & texture dependent)
RQD	Rock Quality Designation (%)
FF	Fracture frequency
UCS	Unconfined Compressive Strength (MPa)
C (c')	Cohesion (kPa) – total stress and (effective stress)
$\Phi (\Phi')$	Friction Angle (degrees) – total stress and (effective stress)
Kv	Modulus of Subgrade Reaction (MN/mm or kPa/mm)
CFA	Continuous Flight Auger (pile type)
DCI	Driven Cast In situ (pile type)
Cv	Coefficient of Consolidation (m2/yr)
Mv	Modulus of Compressibility (m2/MN)
MC1	Moisture Content Before Test (%)
MC2	Moisture Content After Test (%)
ρ	Dry Density (kg/m3)
VSR	Very soft rock
SR	Soft rock
MHR	Medium hard rock
HR	Hard rock
VHR	Very hard rock

Reference: 19-0866.08R03

Date: 11 October 2019

## 1. INTRODUCTION & TERMS OF REFERENCE

At the request of Nivendra Moodley of Glad Africa (and on behalf of the Gauteng Department of Human Settlements), *GCS Geotechnical* (hereafter referred to as GCS) was asked to provide a proposal and cost estimate quotation for the undertaking of a Phase 1 NHBRC geotechnical investigation for the proposed Agri-village development to be located on erven 79, 91, 96, 321 and 322 of the farm Hekpoort 504 JQ, Mogale City LM.

## 2. AVAILABLE INFORMATION

The following information was drawn upon for the purposes of the investigation:

- The 1:250 000 Geological Map titled *"2628 West Rand"* as compiled by the South African Geological Survey, 1986,
- Google Earth Imagery,
- SABS 1200 D Earthworks, and
- Report titled "*RLRP: Desk Study Geotechnical Report for Hekpoort*", referenced 19-0866.08R01, written by GCS in 2019.
- Report titled "*RLRP: Preliminary Geotechnical Report for Hekpoort*", referenced 19-0866.08R02, written by GCS in 2019.

The table below shows the available published physiographical information on the site.

Parameter	Value	Reference
Development	NHBRC Phase 1 Housing Dev.	Glad Africa & GDHS
Site coordinates	25°52'59.33"S / 27°36'59.85"E	
Weinerts N-value	2-5	Weinert (1974)
Climatic Region	Moderate	TRH 2 (1978)
Rainfall	600-650 mm	2526 Johannesburg (1999) 1:500 000
		scale
Temperature	0.1 °C - 27.5°C	after DWAF (1986)
Evaporation	1670 mm	After DWAF (1986)
Water Balance	Deficit	Schulze (1985)
Weathering Type	Slight disintegration, moderate	Fookes et al (1971)
	decomposition with frost action &	
	very slight weathering.	
Geology	Shale of the Silverton Shale	Geological Map Series: West Rand-
	Formation with diabase dykes.	2628 (1986) 1:250 000 scale
Soil Cover	Narrowly graded fersiallitic sands	Brink (1985)
	and loams, aeolian sand, mainly	
	red.	
Origin	Transported and residual soils	Brink (1985)
Topography	1:61 or 1.6% to NE	Garmap SA Topo & Rec 2012.1
Drainage	Not well defined	Garmap SA Topo & Rec 2012.1
Drainage Region	Quaternary Catchment: A21	DWAF (1999)
Hydrogeology	D3: Intergranular & fractured /	1:500 000 scale
	0.5-21	
Groundwater depth	35 mbgl	DWAF-WRC (1995) & pers., comm.
Erodibility Index	16-20 – Low	WRC (1992)
Seismic Intensity	VI (MMS)	Fernandez et al (1972)
Liquefaction	Likely (100-200 cm/s2)	Welland (2002)

## Table 2: Summary of Available Desk Study Information

## **3. SITE DESCRIPTION**

The site is located to the north (Hekpoort A) and south (Hekpoort B) of the R560 and east of the intersection with the R563 in the Mogale City LM.

The total site area is approximately 74.4 Ha in size.

Topographically, the site is fairly flat at 1:61 or 1.6% towards the north-west and drains into the Magalies River. The site is currently used for farming and cattle grazing and sparsely vegetated with thorn bush.

No services were identified on the site except for numerous boreholes and a reticulation pipeline along the eastern boundary of Hekpoort A.

No drainage paths were noted on site.

The far northern perimeter of Hekpoort A was not accessible due to a cut-off trench.

## 4. **GEOLOGY**

Based on the 1:250 000 Geological Map titled "2628 West-Rand (1986)", the site can be seen to be underlain by shale of the Silverton Formation, Pretoria Group.

Further, more detailed mapping at 1:50 000 shows the shale bedrock to be intruded by eastwest trending diabase dykes. The surficial deposits comprise transported soils (scree, colluvium, talus) derived from the erosion of the two low mountain ranges to the north and south.

## 5. FIELDWORK

TLB-excavated test pits were conducted on site, in order to ascertain and better understand the general engineering properties and parameters of the subsurface materials.

## 5.1 **TLB-Excavated Trial Pits**

Fourteen test pits were excavated over the 74 Ha site (roughly 50% of the required minimum according to guidelines of GFSH-2 of 2002), in order to better understand the engineering properties of the subsurface soil / rock conditions.

The results of the test pits indicated refusal depths ranging between 1.1 m and 2.8 m below existing ground level, refusing at an average depth of 2.0 m. Typically the ground conditions comprised a thin veneer of topsoil and hillwash or colluvium with some evidence of aeolian sand. These layers have been ferriginised and cemented to varying degrees. A summary of the soil profiles across the site are given below in Table 6.1a to 6.1b.

TP No.	Topsoil	Aeolian	Ferricrete
11	0-0.5	0.5-1.2	1.2-2.1
12		0-1.6	
13		0-1.6	
14	0-0.3		0.3-1.8
Ave	0.4	1.3	1.2

#### Table 5.1a: Summary of Soil Layers in Test Pits Hekpoort A (North)

#### Table 5.1b: Summary of Soil Layers in Test Pits Hekpoort B (South)

TP No.	Topsoil	PM/Talus	Residuum	Ferricrete	CWR Saprolite	VSR-SR Shale
1	0-0.6	0.8-1.5		0.6-0.8		
2	0-0.6			0.6-1.2		
3	0-0.5		0.5-1.7		1.7-2.1	2.1-2.5
4	0-0.6			0.6-1.7	1.7-2.2	
5	0-0.5	0.5-1.1				
6		0-0.5	0.5-1.6		1.6-2.6	
7		0-0.5	0.5-1.3		1.3-2.1	
8	0-0.6			0.6-1.8	1.8-2.1	
9	0-0.3			0.3-1.2	1.2-1.8	1.8-1.9
10	0-0.3			0.3-2.0	2.0-2.8	
Ave	0.5	0.6	1.0	1.0	0.6	2.0+

Depth			FARC	Ky	F	0
From (m)	To (m)	Description	(kPa)	(kPa/mm)	(MPa)	(kPa)
Topsoil						
0	0.4	Dry to slightly moist, red brown to grey brown, STIFF to VERY STIFF, shattered, clayey sandy SILT to SILT/CLAY with roots.	N/A	N/A	N/A	N/A
Aeolian	-					-
0.4	1.7	Slightly moist, red brown to red orange brown, MEDIUM DESNE or STIFF, silty clayey fine SAND with roots.	100-150	<25	<3	18-72
Ferricret	e					
0.8	2.0	Slightly moist, red brown to yellow grey brown speckled black, MEDIUM DENSE or STIFF, bioturbated and voided in places, sandy CLAY/SILT with scattered ferricrete nodules.	100-150	25-40	15-25	72+-

EABC = estimated allowable bearing capacity (ignoring collapse potential)

 $\mathbf{K}\mathbf{v} = \mathbf{modulus} \ \mathbf{of} \ \mathbf{subgrade} \ \mathbf{reaction}$ 

E = elastic modulus

## Table 5.1d: Summary of Soil Profile Hekpoort B (South)

Depth			FARC	Ky	F	C
From	То	Description	(kPa)	(kPa/mm)	L (MPa)	(kPa)
( <b>m</b> )	( <b>m</b> )		(M u)		(1011 a)	(IXI a)
Topsoil						
0	0.5	Dry to slightly moist, red brown to grey brown, FIRM to STIFF, shattered, clayey sandy SILT to SILT/CLAY with roots.	N/A	N/A	N/A	N/A
Pebble M	larker/Tal	us				
0.3	0.9	MEDIUM DENSE, COBBLE GRAVEL and BOULDERS (up to 800 mm) of hard rock shale or quartzite in a matrix of slightly moist, red brown speckled black, FIRM to STIFF, sandy clayey SILT with ferricrete nodules.	100-150	50-80	3-10	18-72
Residuun	n					
0.5	1.1	Dry to slightly moist, grey brown, FIRM to STIFF, SILT/CLAY.	100-150	25-40	15-25	72+-
Ferricret	е					
0.5	1.5	Slightly moist to moist, red brown speckled black, FIRM, CLAY/SILT with scattered ferricrete nodules.	40-80	80-100	25+	18-72
Complete	ly Weathe	ered Saprolite				•
1.6	2.2	Slightly moist, yellow brown speckled black, FIRM to STIFF, SILT/CLAY to VERY SOFT ROCK.	300+	80-100+	25+	150+
Very Soft	Rock She	ale or dolerite				
0.8	2.0+	Highly weathered, grey speckled black, friable or jointed, VERY SOFT ROCK to MEDIUM HARD ROCK DOLERITE or SHALE	500+	150+	50+	400+
	EADC and	imated allowable bearing conseity (ignoring collary		•		

EABC = estimated allowable bearing capacity (ignoring collapse potentia

Kv = modulus of subgrade reaction

E = elastic modulus

## 6. **GROUNDWATER**

No groundwater seepage occurred on site in any of the test pits, although during summer months and during times of prolonged or heavy rainfall, it may be assumed that a perched groundwater table may be present at relatively shallow depths over the site.

The site resides in the A21 Tertiary drainage region in an area that appears to be fair in terms of borehole yield (0.5 to 2.0 l/hr) and this increases towards the north at the river to 2.0 to 5.0 l/hr. The groundwater level has been reported at between 10 and 25 mbgl (Barnard, 2000) but may have been compromised since 2000.

Personal communications with the local farmers indicated that groundwater has been encountered in a number of boreholes at about 35 mbgl but that the yield has been compromised by over-utilisation of the groundwater aquifers in the Tarlton area and subsequently reducing the yield to less than 5,000 l/hr.

## 7. LABORATORY TESTING

Laboratory tests were scheduled and completed on soil samples recovered from the site. The following tests were carried out:

- Four foundation indicator tests (particle size distribution, hydrometer, moisture content, and Atterberg Limits).
- Collapse potential tests.
- Chemical tests (pH and conductivity).

The detailed laboratory test results are provided in Appendix B, while summaries of these results are presented below as in Tables 7a to 7d:

тр	Depth	Depth (m-m) LL PI GM PE* CBR* (%)	БІ	CM	DE*	CBR*	Classifications		
Ir	( <b>m</b> - <b>m</b> )		TRH14	PRA	USCS				
Topsoil									
1	0-0.8	37	17	0.74	М	10	G9+	A.6	CL-OL
2	0-0.5	50	27	0.24	Н	4	G10	A.7.6	CH-OH
14	0-0.3	27	13	0.40	М	11	G8	A.6	CL-OL
Aeolian									
11	1.0	41	13	0.78	L	14	G7+	A.7.6	SC
Ferricre	te								
14	0.3-1.8	44	22	0.78	М	8	G9	A.7.6	CL-OL
Residuu	т								
2	1.5	53	28	0.56	М	5	G10	A.7.6	CH-OH
3	1.7-2.1	60	34	0.28	VH	3	G10	A.7.6	CH-OH

 Table 7a: Summary of Foundation Indicators

\*CBR estimated from PI-GM relationship.

\*PE - Potential Expansiveness; L=low; M=medium; H=high; VH=very high

	Table 7b: Summary of Collapse Potential Test Results						
ТР	Depth	MCb	MCa	Yd	e0	СР	Settlement
	( <b>m</b> )	(%)	(%)	(kg/m3)		(%)	(mm)
Aeolian							
11	1.0	21.83	24.61	1288	1.02	10.7	125-150
Ferruginis	ed Hillwash						
14	0.3-1.8	15.25	22.29	1192	1.21	6.0	50-75

## Table 7c: Summary of Corrosivity Test Results

ТР	Depth	pH	EC	Res	Corrosivity
	( <b>m-m</b> )		(µS/cm)	(Ohm/cm)	
Topsoil					
1	0-0.8	5.6	281	3559	Very
2	0-0.5	5.3	466	2146	Very
14	0-0.3	6.3	367	2725	Very
Aeolian					
11	1.0	6.8	326	3067	Very
Ferricrete					
14	0.3-1.8	6.8	360	2778	Very
Residuum					
2	1.5	6.2	360	2778	Very

## Table 7d: Materials Classification and Recommended Usage

Material Description	Classification		Anticipated Recommended Usage
Topsoil	PI = GM = Classification:	13-27 0.24-0.74 A.6; G8-10; CL-CH	G8-G10 (subgrade to selected layers)
Aeolian	PI = GM = Classification:	13 0.78 A.7.6; G7+; SC	G7+ (upper selected subgrade)
Ferricrete	PI = GM = Classification:	22 0.78 A.7.6; G9; CL-OL	G9 (lower selected subgrade)
Residuum	PI = GM = Classification:	28-34 0.28-0.56 A.7.6; G10; CH-OH	G10 (subgrade only)

## 8. DEVELOPMENT RECOMMENDATIONS

## 8.1 Materials Usage

The soils include hillwash, pebble marker/talus and residuum. These layers are underlain by completely to highly weathered shale or dolerite.

Based on visual and tactile means, together with the limited laboratory results, the materials on site may be assumed to be used as follows:

## • Topsoil

This layer is assumed to qualify as G8-10 and is to be cut and carted off site or stockpiled for landscaping.

## •Aeolian

This layer is assumed to qualify as G7+ (and therefore could be re-used as selected layer).

## •*Ferricrete*

This layer qualifies as G9 and can potentially only be used as subgrade or low-grade fill.

## • Residuum

This layer is generally too deep and will only be encountered in service trenches. The material if fine-grained and rates as G10 and should be carted to spoil.

## 8.2 NHBRC Classification

Prior to obtaining laboratory results in order to quantify the effects of soil movement beneath the site, *GCS Geotechnical* has classified the site based on general experience in similar geological environments.

The site is underlain by potentially expansive topsoil and residuum (H1-H3) with limited potentially collapsible hillwash (C2) in Hekpoort A to the north, which is underlain by a localised pebble marker/talus, saprolite and weathered bedrock in places. These assumptions coupled with the layer thickness have led to the suggestion that this site can be represented by NHBRC classification: **C2/H2-H3 for Hekpoort A to the north and H1-H3 for Hekpoort B to the south.** This signifies a cumulative potential collapse of up to 175 mm and potential ground heave of between 7 and 85 mm.

## 8.3 Foundations

The NHBRC Site Classification based on test pit logs excavated over the site can be mitigated by the following foundation options:

Table 8.3a: Summary of foundation Options for Hekpoort A

NHBRC	Foundation Options
Site Class	
<i>C2</i>	• Stiffened strip footings
	• RC raft with articulation joints or solid lightly
	reinforced masonry
	• Compaction of in situ soils beneath individual footings
	• Soil raft
С2-Н2	• RC raft
	Piled construction
	• Soil raft
С2/Н2-Н3	• RC raft
	Piled construction
	• Soil raft

NHBRC	Foundation Options
Site Class	
H1	Modified normal
	• Soil raft
H2	• <i>RC</i> raft with articulation joints or solid lightly reinforced masonry
	<i>Piled construction</i>
	Split construction
	• Soil raft
H3	• RC raft
	Piled construction
	• Soil raft

#### 8.4 Excavatability & Earthworks

All materials on site classify as <u>SOFT</u> excavation (SABS 1200 D) to depths ranging between 1.1 m and 2.8 m with an average depth of around 2.0 m. Below this depth, intermediate to hard excavation is to be anticipated due to weathered granite bedrock which has been identified along the western and eastern boundaries.

#### 8.5 Drainage

For the promotion of a stable site, with no soil movement-related issues (settlement and/or heave), it is extremely important that adequate drainage, both surface and subsurface, be constructed so that no water ingress into the subsurface soils in and around foundation bases is possible. Drainage should be such that any rainfall is diverted to the nearest stormwater drainage system. Areas of potential pooling or damming of rainfall on site should be carefully designed and sloped so as the remove this water away from the foundations.

## 9. CONCLUSIONS & RECOMMENDATIONS

## <u>General</u>

- This report presents the findings of a NHBRC Phase 1 geotechnical investigation for the proposed Agri-village development on portions 79, 91, 96, 321 and 322 of the farm Hekpoort 504 JQ and provides the conclusions and recommendations for excavations, foundations and earthworks.
- The most important consideration in relation to the proposed development is the presence of potentially expansive transported soils and residuum and isolated potentially collapsible aeolian sand.

## Geology & Ground Conditions

• Based on the 1:250 000 Geological Map titled *"2628 West-Rand* (1986)", the site can be seen to be underlain by shale of the Silverton Formation with diabase dykes.

## **Excavatability**

All materials on site classify as <u>SOFT</u> excavation (SABS 1200 D) to depths ranging between 1.1 m and 2.8 m with an average depth of around 2.0 m. Below this depth, intermediate to hard excavation is to be anticipated.

## **Foundations**

The site has provisionally been classified as C2/H2-H3 in Hekpoort A to the north and H1-H3 in Hekpoort B to the south according to NHBRC guidelines. The following foundation recommendations are proposed for the site:

## Hekpoort A:

NHBRC Site Class	Foundation Options
<i>C2</i>	• Stiffened strip footings
	• <i>RC raft with articulation joints or solid lightly reinforced masonry</i>
	• Compaction of in situ soils beneath individual footings
	• Soil raft
С2-Н2	• RC raft
	Piled construction
	• Soil raft
C2/H2-H3	• RC raft
	Piled construction
	• Soil raft

## Hekpoort B:

NHBRC Site Class	Foundation Options
H1	<ul> <li>Modified normal</li> <li>Soil raft</li> </ul>
H2	<ul> <li>RC raft with articulation joints or solid lightly reinforced masonry</li> <li>Piled construction</li> <li>Split construction</li> <li>Soil raft</li> </ul>
НЗ	<ul> <li>RC raft</li> <li>Piled construction</li> <li>Soil raft</li> </ul>

## Further Investigations

AAM

N Welland: Pr.Eng. / Pr.Sci.Nat

Dale Franklin

16 October 2019

For GCS Geotechnical (Pty) Ltd

ninow@gcs-sa.biz www.gcs-sa.biz

# **APPENDIX A TLB-Excavated Trial Pit Profiles**





DOCE GCS Geotechnical















DOCE GCS Geotechnical





G	Gla Hel	d Africa <poort -="" a<="" th=""><th>HOLE No: TP12 Sheet 1 of 1</th></poort>	HOLE No: TP12 Sheet 1 of 1
Geotechni	ical Engineering		JOB NUMBER: 19.0866.08
Scale 1:10	0.00 1.60 1) 2) 3) 4)	Slightly moist, red brown, medium dense, pinho <u>SAND</u> . Alluvial Deposits.	olled, silty clayey fine
CONTRACTOR : MACHINE : DRILLED BY : PROFILED BY :	N/A CAT 428E - Dale Franklin	INCLINATION : - E DIAM : N/A DATE : - DATE : 18/09/2019	LEVATION : N/A X-COORD : 27 36 32.66"E Y-COORD : 25 52 30.68"S
TYPE SET BY :	Dale Franklin	DATE : 16/10/2019 10:32	HOLE No: TP12
SETUP FILE :	STANDARD.SET	TEXT :866.08HekpoortTPLogs.TXT	

Glae Hek	d Africa poort - A	HOLE No: TP13 Sheet 1 of 1
Geotechnical Engineering		JOB NUMBER: 19.0866.08
Scale 1.10 1.60 1) 2) 3) 4)	Slightly moist, red brown, medium dense, pinho SAND, Alluvial Deposits.	olled, silty clayey fine
CONTRACTOR : N/A MACHINE : CAT 428E DRILLED BY : - PROFILED BY : Dale Franklin	INCLINATION : - E DIAM : N/A DATE : - DATE : 18/09/2019	LEVATION : N/A X-COORD : 27 36 37.27"E Y-COORD : 25 52 38.88"S
TYPE SET BY : Dale Franklin SETUP FILE : STANDARD SET	DATE : 16/10/2019	HOLE No: TP13



Glad Africa Hekpoort - A

GCS

LEGEND Sheet 1 of 1

JOB NUMBER: 19.0866.08

		BOULDERS	{SA01}
	000	GRAVEL	{SA02}
	0 0 0 0	GRAVELLY	{SA03}
		SAND	{SA04}
		SANDY	{SA05}
		SILT	{SA06}
		SILTY	{SA07}
		CLAY	{SA08}
		CLAYEY	{SA09}
		SHALE/siltstone	{SA12}
		QUARTZITE	{SA15}
		DOLERITE	{SA18}{SA42}
	•••	FERRICRETE NODULES	{SA24}
		SCATTERED FERRICRETE NODULES	{SA25}
		PARTIALLY CEMENTED	{SA30}
Name		UNDISTURBED SAMPLE	{SA37}
Name 🔶		DISTURBED SAMPLE	{SA38}
	۶ ,	ROOTS	{SA40}
	$P_{a}$	COBBLES	{SA58}
CONTRACTOR	:	INCLINATION :	ELEVATION :
DRILLED BY	:	DIAM : DATE :	Y-COORD :
PROFILED BY	:	DATE :	LEGEND
I YPE SET BY SETUP FILE	: Dale Frankl : STANDARL	in DATE : 16/10/2019 10:32 D.SET TEXT :866.08HekpoortTPLogs.TXT	SUMMARY OF SYMBOLS

DOCE GCS Geotechnical

# **APPENDIX B Laboratory Test Results**



TEL: (011) 674 1325 FAX: (011) 674 4513 e mail: lab@geopractica.co.za

## COLLAPSE POTENTIAL at 200 kPa

Client	GCS GEOTECHNICAL ENGINEERING		
Location	HEKPOORT. I	P 11 @ 1,0m	
Date	08 OCTOBER 2019	Test No	3107
Job No	19281	Checked By	EB

	Sample Height (mm) 20	20	Sample Diameter (	(mm)	64	Sample	Specific Gravity	y	2.603
--	-----------------------	----	-------------------	------	----	--------	------------------	---	-------

NMC

Sample Preparation

Effective Time Consolidation Voids Strain Stress Reading Ratio (%) (kPa) (mins) 1067 1.020 0.00 10 60 90 1068 1.019 10 0.05 1.015 130 1072 0.25 33 190 1079 65 1.008 0.60 127 310 1105 0.982 1.90 200 1750 1126 0.961 2.95 200 3190 1339 0.745 13.60 498 3430 1437 0.646 18.50 993 3670 1510 0.573 22.15 1868 5110 1575 0.507 25.40 743 5230 25.25 1572 0.510 118 5350 1560 0.522 24.65 10 23.80 5470 1543 0.539

## **Moisture Content Calculations**

Mass wet sample plus ring before test (gms)	313.00
Mass wet sample plus ring after test (gms)	315.30
Mass dry sample plus ring (gms)	294.90
Mass ring (gms)	212.00
Moisture content before test (%)	21.83
Moisture content after test (%)	24.61

#### **Other Data**

Initial Dry Density (kg/m3)	1288
Initial Void Ratio	1.02

Programe Data Revision No 2 (19/03/2001)

#### **VOIDS RATIO v EFFECTIVE STRESS**



## STRAIN v EFFECTIVE STRESS







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## FOUNDATION INDICATOR

Client	GCS GEOTECHNICAL ENGINEERING			
Location	HEKPOORT IP 2	2 @ 0,0 - 0,5m		
Date	07 OCTOBER 2019	Test No	3103	
Job No	19281	Checked By	EB	

#### SIEVE ANALYSIS

Values are expressed as a percentage of total sample

Sieve	Total
Size	Passing
(mm)	(%)
75.00	100.00
53.00	100.00
37.50	100.00
26.50	100.00
19.00	100.00
9.50	100.00
4.75	99.92
2.00	99.70
0.425	93.89

#### HYDROMETER ANALYSIS

Values are expressed as a percentage of total sample

Sieve	Total
Size	Passing
(mm)	(%)
0.0847	82.64
0.0609	79.13
0.0437	75.61
0.0314	72.09
0.0045	43.96
0 0014	22.86

#### **ESTIMATED COMPOSITION (As BS 1377)**

Clay (<0.002)	26.92
0.002 < Silt < 0.06	52.04
0.06 < Sand < 2.0	20.75
Gravel > 2.0	0.30
% less than 0.075	81.22



#### **ATTERBERG LIMITS & OTHER VALUES**

Liquid Limit	50	
Plastic Limit	23	
Plastic Index	27	
Linear Shrinkage	13	
Grading Modulus	0.24	
Moisture Content	8.59	
PI on Whole Sample	26	
PRA Classification	A.7.6	
Unified Classification	See Plasticity Chart	







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## FOUNDATION INDICATOR

Client	GCS GEOTECHNICAL ENGINEERING		
Location	HEKPOORT IP 1	@ 0,0 - 0,8m	
Date	07 OCTOBER 2019	Test No	3102
Job No	19281	Checked By	EB

#### SIEVE ANALYSIS

Values are expressed as a percentage of total sample

Sieve	Total
Size	Passing
(mm)	(%)
75.00	100.00
53.00	100.00
37.50	100.00
26.50	100.00
19.00	99.05
9.50	98.20
4.75	96.92
2.00	89.63
0.425	79.84

#### HYDROMETER ANALYSIS

Values are expressed as a percentage of total sample

Sieve	Total
Size	Passing
(mm)	(%)
0.0908	56.82
0.0660	50.84
0.0476	46.35
0.0343	41.87
0.0048	22.43
0 0014	11 96

#### **ESTIMATED COMPOSITION (As BS 1377)**

Clay (<0.002)	13.74
0.002 < Silt < 0.06	35.63
0.06 < Sand < 2.0	40.26
Gravel > 2.0	10.37
% less than 0.075	53.01



#### **ATTERBERG LIMITS & OTHER VALUES**

Liquid Limit	37	
Plastic Limit	19	
Plastic Index	17	
Linear Shrinkage	9	
Grading Modulus	0.74	
Moisture Content 10.94		
PI on Whole Sample	14	
PRA Classification	A.6	
Unified Classification	See Plastici	ty Chart







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## FOUNDATION INDICATOR

Client	GCS GEOTECHNICAL ENGINEERING		
Location	HEKPOORT IP 14 @ 0,0 - 0,3m		
Date	07 OCTOBER 2019	Test No	3108
Job No	19281	Checked By	EB

#### SIEVE ANALYSIS

Values are expressed as a percentage of total sample

Sieve	Total
Size	Passing
(mm)	(%)
75.00	100.00
53.00	100.00
37.50	100.00
26.50	100.00
19.00	100.00
9.50	100.00
4.75	99.92
2.00	99.70
0.425	93.89

#### HYDROMETER ANALYSIS

Values are expressed as a percentage of total sample

Sieve	Total
Size	Passing
(mm)	(%)
0.0908	66.82
0.0660	59.79
0.0476	54.51
0.0343	49.23
0.0048	24.62
0 0014	10 55

#### **ESTIMATED COMPOSITION (As BS 1377)**

Clay (<0.002)	12.85
0.002 < Silt < 0.06	45.21
0.06 < Sand < 2.0	41.65
Gravel > 2.0	0.30
% less than 0.075	62.34



#### **ATTERBERG LIMITS & OTHER VALUES**

Liquid Limit	27	
Plastic Limit	14	
Plastic Index	13	
Linear Shrinkage	7	
Grading Modulus	0.40	
Moisture Content	8.59	
PI on Whole Sample	Sample 12	
PRA Classification	A.6	
Unified Classification	See Plasticity Chart	







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## FOUNDATION INDICATOR

Client	GCS GEOTECHNICAL ENGINEERING		
Location	HEKPOORT IP 2 @ 1,5m		
Date	07 OCTOBER 2019	Test No	3104
Job No	19281	Checked By	EB

#### SIEVE ANALYSIS

Values are expressed as a percentage of total sample

Sieve	Total
Size	Passing
(mm)	(%)
75.00	100.00
53.00	100.00
37.50	100.00
26.50	100.00
19.00	98.59
9.50	92.83
4.75	89.77
2.00	86.81
0.425	83.34

#### HYDROMETER ANALYSIS

Values are expressed as a percentage of total sample

Sieve	Total
Size	Passing
(mm)	(%)
0.0847	73.36
0.0609	70.23
0.0437	67.11
0.0314	63.99
0.0045	39.02
0.0014	20.29

#### **ESTIMATED COMPOSITION (As BS 1377)**

Clay (<0.002)	23.89
0.002 < Silt < 0.06	46.19
0.06 < Sand < 2.0	16.73
Gravel > 2.0	13.19
% less than 0.075	72.09



#### **ATTERBERG LIMITS & OTHER VALUES**

Liquid Limit	53	
Plastic Limit	24	
Plastic Index	28	
Linear Shrinkage	14	
Grading Modulus	0.56	
Moisture Content	23.10	
PI on Whole Sample	24	
PRA Classification	A.7.6	
Unified Classification	See Plasticity Chart	







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## FOUNDATION INDICATOR

Client	GCS GEOTECHNICAL ENGINEERING		
Location	HEKPOORT IP 3	3 @ 1,7 - 2,1m	
Date	07 OCTOBER 2019	Test No	3105
Job No	19281	Checked By	EB

#### SIEVE ANALYSIS

Values are expressed as a percentage of total sample

Sieve	Total
Size	Passing
(mm)	(%)
75.00	100.00
53.00	100.00
37.50	100.00
26.50	100.00
19.00	100.00
9.50	99.95
4.75	99.26
2.00	96.63
0.425	93.25

#### HYDROMETER ANALYSIS

Values are expressed as a percentage of total sample

Sieve	Total
Size	Passing
(mm)	(%)
0.0847	82.08
0.0609	78.58
0.0437	75.09
0.0314	71.60
0.0045	47.15
0.0014	29.69

#### **ESTIMATED COMPOSITION (As BS 1377)**

Clay (<0.002)	33.25
0.002 < Silt < 0.06	45.16
0.06 < Sand < 2.0	18.22
Gravel > 2.0	3.37
% less than 0.075	80.66



#### **ATTERBERG LIMITS & OTHER VALUES**

Liquid Limit	60	
Plastic Limit	25	
Plastic Index	34	
Linear Shrinkage	17	
Grading Modulus	0.28	
Moisture Content	26.01	
PI on Whole Sample	32	
PRA Classification	A.7.6	
Unified Classification	See Plasticity Chart	







SOIL and MATERIAL TESTING P.O. BOX 227 MARAISBURG 1700 TEL: (011) 674 1325 FAX: (011) 674 4513 e mail: satisfied@geopractica.co.za

## SOIL pH and CONDUCTIVITY TEST RESULT

Client	GCS GEOTECHNICAL ENGINEERING			
Location	HEKPOORT.			
Date	OCTOBER 2019 Test No			
Job No	19281		MM	

Sample Description	рН	Electrical Conductivity EC (µS/cm)	Total Dissolved Salts TDS (ppm)	Resistivity R (Ohms/cm)
TP 1 @ 0.0 - 0.8	5.6	281	141	3559
TP 2 @ 0.0 - 0.5	5.3	466	234	2146
TP 2 @ 1.5	6.2	360	180	2778
TP 3 @ 1.7 - 2.1	5.8	381	191	2625
TP 11 @ 1.0	6.8	326	163	3067
TP 14 @ 0.0 - 0.3	6.3	367	184	2725
TP 14 @ 0.3 - 1.8	6.8	360	180	2778
				ERR
				ERR
				ERR

рН	Degree of Acidity
<4	Extremely Acidic
4.0 - 5.4	Strongly Acidic
5.5 - 6.4	Moderately Acidic
6.5 - 7.0	Slightly Acidic
7.1 - 7.4	Slightly Alkaline
7.5 - 8.4	Moderately Alkaline
>8.4	Strongly Alkaline

Resistivity (Ohmn/cm)	Degree of Corrosivity
0 - 2 000	Extremely Corrosive
2 000 - 4 000	Very Corrosive
4 000 - 5 000	Corrosive
5 000 - 6 000	Mildly Corrosive
>10 000	Not Generally Corrosive

Programed Data Revision No 1 (28/02/2001)



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## FOUNDATION INDICATOR

Client	GCS GEOTECHNICAL ENGINEERING		
Location	HEKPOORT IP 11 @ 1,0m		
Date	07 OCTOBER 2019	Test No	3106
Job No	19281	Checked By	EB

#### SIEVE ANALYSIS

Values are expressed as a percentage of total sample

Sieve	Total
Size	Passing
(mm)	(%)
75.00	100.00
53.00	100.00
37.50	100.00
26.50	100.00
19.00	100.00
9.50	100.00
4.75	98.30
2.00	91.35
0.425	78.09

#### HYDROMETER ANALYSIS

Values are expressed as a percentage of total sample

-	
Sieve	Total
Size	Passing
(mm)	(%)
0.0921	52.65
0.0669	46.80
0.0482	42.41
0.0348	38.02
0.0048	19.01
0.0014	8.77

#### **ESTIMATED COMPOSITION (As BS 1377)**

Clay (<0.002)	10.44
0.002 < Silt < 0.06	34.73
0.06 < Sand < 2.0	46.18
Gravel > 2.0	8.65
% less than 0.075	48.68



#### **ATTERBERG LIMITS & OTHER VALUES**

Liquid Limit	41	
Plastic Limit	28	
Plastic Index	13	
Linear Shrinkage	7	
Grading Modulus	0.78	
Moisture Content	18.09	
PI on Whole Sample	10	
PRA Classification	A.7.6	
Unified Classification	SC	







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## FOUNDATION INDICATOR

Client	GCS GEOTECHNICAL ENGINEERING			
Location	HEKPOORT IP 1	- POORT IP 14 @ 0,3 - 1,8m		
Date	07 OCTOBER 2019	Test No	3109	
Job No	19281	Checked By	EB	

#### SIEVE ANALYSIS

Values are expressed as a percentage of total sample

Sieve	Total
Size	Passing
(mm)	(%)
75.00	100.00
53.00	100.00
37.50	100.00
26.50	100.00
19.00	100.00
9.50	98.22
4.75	90.95
2.00	82.95
0.425	76.40

#### HYDROMETER ANALYSIS

Values are expressed as a percentage of total sample

Sieve	Total
Size	Passing
(mm)	(%)
0.0867	62.95
0.0628	58.66
0.0454	54.37
0.0328	50.08
0.0047	27.19
0 0014	14 31

#### **ESTIMATED COMPOSITION (As BS 1377)**

Clay (<0.002)	16.60
0.002 < Silt < 0.06	41.38
0.06 < Sand < 2.0	24.97
Gravel > 2.0	17.05
% less than 0.075	60.85



#### **ATTERBERG LIMITS & OTHER VALUES**

Liquid Limit	44	
Plastic Limit	22	
Plastic Index	22	
Linear Shrinkage	11	
Grading Modulus	0.78	
Moisture Content	16.37	
PI on Whole Sample	17	
PRA Classification	A.7.6	
Unified Classification	See Plasticity Chart	







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## **COLLAPSE POTENTIAL at 200 kPa**

Client	GCS GEOTECHNICAL ENGINEERING			
Location	HEKPOORT IF	9 14 @ 0,3 - 1,8m		
Date	08 OCTOBER 2019	Test No	3110	
Job No	19281	Checked By	EB	

Sample Height (mm)	20	Sample Diameter (mm)	64	Sample Specific Gravity	2.633
--------------------	----	----------------------	----	-------------------------	-------

Sample Preparation

NMC

Effective	Time	Consolidation	Voids	Strain
Stress		Reading	Ratio	(%)
(kPa)	(mins)	5		× /
10	60	807	1.209	0.00
10	90	808	1.208	0.05
33	130	812	1.203	0.25
65	190	818	1.197	0.55
127	310	831	1.182	1.20
200	1750	842	1.170	1.75
200	3190	962	1.038	7.75
498	3430	1053	0.937	12.30
993	3670	1130	0.852	16.15
1868	5110	1194	0.781	19.35
743	5230	1181	0.796	18.70
118	5350	1165	0.813	17.90
10	5470	1149	0.831	17.10

## **Moisture Content Calculations**

Mass wet sample plus ring before test (gms)	299.80
Mass wet sample plus ring after test (gms)	305.20
Mass dry sample plus ring (gms)	288.10
Mass ring (gms)	211.40
Moisture content before test (%)	15.25
Moisture content after test (%)	22.29

## **Other Data**

Initial Dry Density (kg/m3)	1192
Initial Void Ratio	1.21

Programe Data Revision No 2 (19/03/2001)

#### **VOIDS RATIO v EFFECTIVE STRESS**



## STRAIN v EFFECTIVE STRESS

Test No: 3110



FIGURE 1 Site Plan

# **RAPID LAND RELEASE PROGRAM - HEKPOORT NHBRC CLASSIFICATIONS**



LEGEND						
•	Test Pits					
Rivers	and Streams					
$\sim$	Perennial					
	Hekpoort Sit	e Bound	lary			
NHBR	с					
	H2 - H3					
	C2/H2 - H3					
	C2 - H2					
	H1 - Heave 7.5 - 15 mm					
	H2 - Heave 15 - 30 mm					
	H3 - Heave > 30 mm					
	C2 - Collapsible Sand > 10 mm					
			R513			
	NORTH WI	EST	R104			
	and an and	2560	1			
		R <sup>3</sup>	and a second			
	R24	· · · · · · · · · · · · · · · · · · ·				
R	R509	GAUTEN				
500		1	P28~			
Data Sources: FSR Baseman 2019						
Data suplied by Specialist (N. Welland)						
	SCALE:	1:8 000				
FIGURE NO.:	-	MAP NUMBER	R: 19-0866-16			
DRAWN BY:	GIS CONSULTANT	REVIEWED B	Y: GIS SPECIALIST			
PROJECTION:	GEOGRAPHIC	DATE:	26 SEPTEMBER 2019			
CLIENT:	GLAD AFRICA					
63 Wessel Road Woodmead PO Box 2597 Rivonia 2128 South Africa						
Water & Environmental Consultants         Tel: +27 (0) 11 803 5726           Fax: +27 (0) 101 803 5745         E-mail: jhb@gcs-sa.biz           www.secs-sa.biz         www.secs-sa.biz						
	-		-			

# FIGURE 2 Geological Plan

# **RAPID LAND RELEASE PROGRAM - HEKPOORT: GEOLOGY**



## LEGEND **Rivers and Streams** Non-Perennial N Perennial Hekpoort Lithology Surface deposits (undifferentiated) di Diabase; hybrid diabase (····); diabase dyke (\_•··) Vsi Slate, shale, hornfels, graphitic (>>) with andalusite (+>>); quartzite (+>>) Quartzite, minor hornfels (.....) Vdq Quartzite Andesite, basalt, tuff, agglomerate (\*\*); shale (==); chert (\_\_\_) Shale, slate, with andalusite (----); ferruginous quartzite (----); Vt quartzite; slate ( ) NORTH WEST Data Sources: Council for Geoscience 1:250 000 Geological Series; 2526 0.5 2 Kilometers SCALE: 1:60 000 FIGURE NO .: MAP NUMBER: 19-0866-04 N NAIDOO GIS CONSULTANT C BOTHA GIS SPECIALIST DRAWN BY: REVIEWED BY: DATUM: WGS84 PROJECTION: GEOGRAPHIC 12 SEPTEMBER 2019 DATE: PROJECT: RAPID LAND RELEASE PROGRAM GLAD AFRICA CLIENT: 53 Wessel Road Woodmea PO Box 2597 Rivonia 2128 South Africa G Tel: +27 (0) 11 803 5726 Fax: +27 (0) 11 803 5745 E-mail: jhb@gcs-sa.biz www.gcs-sa.biz