APPENDIX C (iv)

Geotechnical Report

ANNEXURE I





YIHLA ENVIRONMENTAL AND CONSULTING

ENGINEERS

A GEO-TECHNICAL INVESTIGATION REPORT FOR THE PROPOSED CONSTRUCTION OF 1100 HOUSES AT SIYABUSWA, WITHIN DR JS MOROKA LOCAL MUNICIPALITY OF NKANGALA DISTRICT MPUMALANGA

PREPARED BY: YINHLA ENVIRONMETAL AND CONSULTING ENGINEERS

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DATED: JULY 2014

PROJECT TEAM

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1. INTRODUCTION

1.1. Preamble

In June 2014, Golden-Goal, appointed Yinhla Environmental Consultants to undertake a geotechnical investigation on portions; 42, 43, 47 of the farm Valschfontein 33 JS, in Siyabuswa Township, Dr. J.S. Moroka Local Municipality of Nkangala District, Mpumalanga Province of South Africa. Yinhla Environmental Consulting was also advised that the proposed development will include the construction of 1100 houses. This report discusses the findings and constraints of the investigation carried out to assess the geotechnical conditions and constraints of the site with regards to the proposed development.

1.2. Objectives

The objectives of the investigation were to complete a geotechnical survey of the site which would:

- Identify the soil/rock profile to a depth of approximately 3.0m or refusal of excavation
- Determine the engineering parameters of the near surface soils
- Recommend specific foundations for the structures
- Assess the suitability of the near surface soils for use in earthworks
- Comment on any geotechnical problems that may impact upon the construction
- Classify the site according to the site classifications recommended by the NHBRC Home Building Manuals.

2. FACTUAL REPORT

2.1. Programme of Works

Literary Review

Prior to the commencement of a field work, a literary review was conducted on the data obtained from previous investigations carried out by other consultants. A 1:250 000 geological map was consulted to determine the local geology.

Field Work

On the 26th of June 2014, a total of 4 trial pits were hand excavated across the site using picks and shovels, to refusal of excavation. The locations of trial pits were positioned on the field by a field geologist by taping and pacing from known points, and were later marked using a hand-held Garmin GPS with an accuracy of approximately 5m.

Each pit was internally profiled by the engineering geologist according to the "Guidelines for soil and rock logging in South Africa, 2nd Impression 2002", sampled where necessary and subsequently loosely backfilled. Detailed soil profiles appear in Appendix of the report, while the locations of the trial holes pits are indicated on the site plan which appears in Appendix 2.

Laboratory Testing

From various soil samples recovered during the field work, the following laboratory tests were undertaken and results are attached as Appendix to this report.

V	Foundation Indicator Test	4
\checkmark	CBR	3
\checkmark	Grading analysis	3

The report was finally prepared using the data obtained from the entire source mentioned above.

2.2. Site Description

2.2.1. Location

The study area falls in terms of current political dispensation, within the jurisdiction of Dr. J.S. Moroka Local Municipality of Nkangala District refer to Locality Map 1.

The study area is located on the following coordinate:

Latitude: 25º 6' 167"S

Longitude:

29º 2' 22" E

2.3. Local Geology

The regional geology of the area is described in terms of Geological Sheet 2530 Barberton, 1: 250 000. The study area is underlined by Biotete-Trondhjemite Gneiss (Goudplaats-Houtriver Gneiss Suite, Paleoarchean Granitoid Intrusion).

2.4. Topography, Climate and Hydrology

The study area is located on a gentle to steep slope towards the westerly direction. The area exhibits an average slope of between 1.5° and 2° west. No prominent regional topographical features occur within the boundaries of the study area.

Climatic conditions in the area support variety of agricultural activities. Temperatures range between 18°C and 28°C with an average of 25.5°C . Generally summer have a high number of sunshine hours with occasional afternoon thunderstorms. The general average rainfall of the area ranges between 450mm to 800mm.

The study area is located in the Olifants Water Catchments Management Area. The site is drained by means of surface run off; with storm water collecting towards the southern western direction.

2.5. Field Observation

Several soil horizons were encountered across the site and their descriptions has been discussed below.

Fill Materials

On all the trial pits, a horizon of hill wash which results as surface run-off forms the upper most soil horizon. This hill wash is described as generally greyish brown, soft and fine sandy silt. The average thickness ranges from 0-0.3m

Residuals

Immediately below the fill soils are horizons of residual materials with different alterations. The materials have generally been described as red brown mottled yellow with depth, slightly fissured clayey silt with yellowish stained weathering runnels and occasionally semi-ferruginesed ferricrete concretions. Its consistency varies from soft to firm grading to stiff. Its thickness was not determined as maximum allowance of excavation or trial pit termination on very stiff horizon was reached respectively.

Laboratory Results

Summaries of the laboratory results have been shown below, whilst the detailed results are presented in Appendix of this report

Table 1: Summary of laboratory test results								
TP No.	Depth (m)	Material	PI (%)	PI (ws)	GM	MC (%)	Activity	
(A1 & A2)	0.6-1.1	Residual	8	8	1.48	9.6	Medium	
(C2 & C1)	0.5-0.7	Residual	8	8	1.00	9.6	Medium	
(E1 & E2)	0.7-1.0	Residual	8	8	1.24	12.6	Medium	
(G1 & G2)	1.2-1.3	Residual	3	3	1.44	8.8	Medium	

3. INTERPRETATIVE REPORT

3.1. Discussion of Results

Fill Materials

Due to the nature of its source and its sporadic distribution across the site, no laboratory testing was undertaken. Its consistency has also been described as loose and as such it is not recommended for founding medium for the proposed development.

Residuals

Laboratory tests were conducted within residuals show that these materials exhibit a medium heave potential. It should be mentioned that the expansive potential of material is instigated by change in its moisture content where an increase in the moisture content will result in expansion of the material and a reduction will result in shrinkage of similar material. This effectively implies that if the current moisture content or present moisture during construction is kept constant an insignificant movement can be anticipated, hence less damage/distress to structures.

However, due to insignificant expansive and compressible potentials this horizon is recommended as a founding medium for the proposed development.

3.2. Watertable

It should be mentioned that this fieldwork was undertaken during winter season just after a week of rains and this is evident by the amount of moisture that is present within the soils. However no water seepage was encountered in any of the trial pits. The depth of the water table is unknown.

3.3. Excavatibility of the ground

As it was mentioned that there are no double storey structures on the proposed development, the trial pits were hand-excavated using picks and shovels. The average depth of excavation up to refusal is 1.42m

3.4. Geotechnical Classification

The site has been classified according to the "Geotechnical Classification for Urban Development" after Watermeyer and Tromp (1992) and the Joint Structural Division. The following classes designated across the site while its site classification reference has been presented in Appendix 4 to this report

✓ Class H (< 7.5mm of heave movement)

3.5. Founding Solution

It is recommended that the development be founded on Stiffened or Concrete Raft, a suitably reinforced concrete raft.

Alternatively these structures should be founded on a Soil Raft and the details are:

✓ An area at least 0.5m larger than the foot print of the structure will need to be excavated to a depth of not less than 0.5m below the current ground level.

3.6. General

As mentioned above that the trial pits were loosely backfilled, where foot prints of the proposed structures are to be placed directly on top of these trial pits, the holes should be identified and properly backfilled.

3.7. Additional investigations

No further investigation will be necessary for the proposed development.

4. CONSTRUCTION MONITORING

4.1. Excavation Inspection

It is recommended that all foundations be inspected by a competent person prior to placing any concrete.

4.2. Control Testing

Regular checks on the quality and compaction of the backfill to the terraces should be made

Report Provision

While every effort is made during the fieldwork phase to identify the various soil horizon, areas subject to perched water Table, areas of poor drainage, areas underlain by hard rock, and to estimate their distributions, it is impossible to guarantee that isolated zones of poorer foundation materials or harder rock have not been overlooked.

For this reason, this investigation has sought to highlight areas of potential foundation, groundwater and excavation problems, as well as to provide prior warning to the developers.

All information and deductions contained within this report are dependent not only on access to the site and previous information, but also on the accuracy of the results received from the accredited laboratories and the proposed site map supplied by the Client.

Proffessional Natural Scientist (Earth Sciences)

M. NGOBELI (Pr.Sci.Nat)

Engineering Geologist

P. Mudau

REFERENCE

Jennings JE et al . "Revised Guide to Soil Profiling for Civil Engineering Purposes in Southern Africa" – Civil Engineer in South Africa, January 1973

Van der Merwe DH. "The prediction of heave from the Plasticity Index and percentage clay fraction of soils" – Civil Engineer in South Africa Vol 6, 1964

Guidelines for Soil and Rock Logging in South Africa – AEG-SA section, SAICE, SAIEG – 2002.

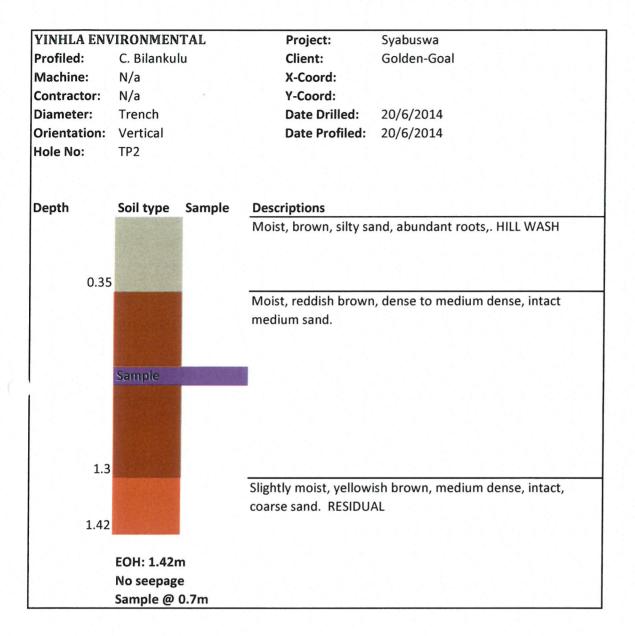
Code of Practice – The safety of Persons Working in Small Diameter Shafts and Test Pits for Civil Engineering Purposes – Geotechnical Division of SAICE – 2003.

SAICE's Guidelines for Urban Engineering Geological Investigations.

Code of Practice – Foundations and Superstructures for Single Storey Residential Buildings of Mansonry Construction – The Joint Structural Division of SAICE and IStructE, First edition 1995.

APPENDIX: 1 SITE LAYOUTS

APPENDIX: 2 SOIL PROFILES



YINHLA ENVIRONMENTAL Siyabuswa Project: Profiled: c. Bilankulu **Client:** Golden-Goal Machine: N/a X-Coord: Contractor: N/a Y-Coord: Diameter: Trench **Date Drilled:** 20/6/2014 Orientation: Vertical **Date Profiled:** 20/6/2014 Hole No: TP1 Depth Soil type Sample **Descriptions** Moist, brown, silty sand, abundant roots,. HILL WASH 0.3 Moist, reddish brown, dense to medium dense, intact medium sand. 1.1 Sample Slightly moist, yellowish brown, medium dense, intact, coarse sand. RESIDUAL 1.3 EOH: 1.3m No seepage Sample @ 1.1m

APPENDIX: 3

LABORATORY TEST RESULTS



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GRAVEL, SOIL AND SAND TEST REPORT Method A1-A5 A7 A8 & Client: Yinhla Engineers & Environmental Date Sampled 26-Jun-14 Doc No: P2793/2(i) Address: P.O Box 2729 Thohovandou Contract: Construction of New Housing Development - Siyabuswa MP Date Tested : 03-Jul-14 Description Sample nr C2 & C1 material delivered to lab sampled by client Sieve Analysis Atterberg Description * Cumulative percentage passing Limits (%) Classification * Grading Modulus inear Shrinkage Plasticity Index COLTO: 1998 US. Highway Group Index Liquid Limit Sample No Unified Soil Depth (m) 0,075 (Unified Soil Classification) G9 **GRADING ANALYSIS** 100 90 CUMULATIVE PERCENT PASSING P2793/2 80 70 60 50 40 30 20 0 0,075 2,00 4,75 Sieve Size **GENERAL*** PERFORMANCE AS GRAVEL WEARING COURSE * Effective size (mm): <0.075 500 Class D Uniformity co-eff.: 234 Slipperv 400 Curvature co-eff.: 0,4 Good - may be dusty 300 Class C Oversize Index: 0 Class A -----Glass-E-----Shrinkage Product: 279 200 Erodible materials Ravels Good SHRINKAGE Grading co-eff.: 11,0 100 CBR RESULTS (%) : Class B: Ravels and corrugates 0 @ 100% comp.: 13 25 45 GRADING COEFFICIENT @ 98% comp.: 12 @ 97% comp. : 12 @ 95% comp.: 11 REMARKS @ 93% comp.: 9 @ 90% comp.: 6 Soil Mortar Analysis Coarse Sand (<2.0>0.425mm): 18,8% Fine Sand (<0.425>0.075mm): 28,2% Material < 0.075mm: 41,5% Please note that test results are only relevant to the sample tested, which were delivered to the lab, and were uncontaminated and fit for testing. Any results may only be reproduced in their entiretywith the written consent of Letaba Lab (Pty) Ltd. and any opinions and interpretations expressed fall outside Letaba Lab's Quality Document. 2014-07-07 Technical signatory (Name): Date Issued: Signature.

TMH 1:1986 LETABA LAB CBR and Modified A.A.S.H.T.O Density test report Method A7, A8 & Client: Yinhla Engineers & Environmental Date tested. 03-Jul-14 Contract: Construction of New Housing Development - Siyabuswa MP Doc no: P2793/2(II) Sample no: **P2793/2** Sample nr C2 & C1 material delivered to lab sampled by client Description: Maximum dry density = 1995 kg/m³ Optimum moisture content = 9,6 % 2000 1990 1980 Dry Density kg/m3 1970 1960 1950 1940 1930 1920 10 12 % Moisture California Bearing Ratio (readings) 25 PROC NRB MOD 20 (KN) 15 10 5 0,0 1.0 2,0 3.0 5,0 6,0 7,0 8,0 California Bearing Ratio 1000.0 CBR @ 2.54 mm - CBR @ 5.08 mm --#--- CBR @ 7.62 mm 100,0 10,0 1,0 91 92 94 101 102 % Compaction % Compaction CBR of 13.344 kN 100 98 97 95 93 90 13 12 12 11 9 6 Briquette Info Mod N.R.B. Proc Dry Density (kg/m³ 2013 1903 Compaction Moisture (%) 9.5 9,5 9.5 Compaction (%) 100,9% 95,4% 90,2% % Swell 0,08 0,15 0,22 Please note that test results are only relevant to the sample lested, which were delivered to the lab, and were uncontaminated and fit for testing. Any results may only be reproduced in their entitiety with the written consent of Letaba Lab (Pty) Ltd, and any opinions and interpretations expressed fall outside Letaba Lab's Quality Document. Date Issued. 2014-07-07 Technical signatory (Name): Signature.



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GRAVEL, SOIL AND SAND TEST REPORT Method A1-A5,A7, A8 & Client: Yinhla Engineers & Environmental Doc No: P2793/1(i) Date Sampled: 26-Jun-14 P.O Box 2729 Thohoyandou Address: Construction of New Housing Development - Siyabuswa MP Contract : Date Tested : 03-Jul-14 Sample nr G1 & G2 material delivered to lab sampled by client Description Sieve Analysis Atterberg Description * Cumulative percentage passing Limits (%) Classification * Grading Modulus COLTO: 1998 inear Shrinkag Plasticity Index US. Highway Liquid Limit Group Index 8 Unified Soil Depth (m) 0,075 Sample (Unified Soil Classification) 26, t Brown Silty/Clayey sand **GRADING ANALYSIS** 100 90 CUMULATIVE PERCENT PASSING 80 70 60 50 40 30 20 10 0,075 0,425 4,75 Sieve Size **GENERAL** * PERFORMANCE AS GRAVEL WEARING COURSE * Effective size (mm): <0.075 500 Class D Uniformity co-eff.: 766 Slippery 400 Curvature co-eff.: 7,4 Good - may be dusty 300 Class C Oversize Index: 0 Class A --- Class-E-Ravels Shrinkage Product: 174 200 Erodible materials Good SHRINKAGE Grading co-eff.: 21,5 100 Class B : Ravels and corrugates CBR RESULTS (%): 0 @ 100% comp.: 15 10 25 40 GRADING CCEFFICIENT @ 98% comp.: 14 @ 97% comp.: 13 @ 95% comp.: 13 REMARKS @ 93% comp.: 12 @ 90% comp.: 10 Soil Mortar Analysis Coarse Sand (<2.0>0.425mm): 23,5% Fine Sand (<0.425>0.075mm): 21,1% Material < 0.075mm: 29,9% Please note that test results are only relevant to the sample tested, which were delivered to the lab, and were uncontaminated and fit for testing. Any results may only be reproduced in their entirety with the written consent of Letaba Lab (Pty) Ltd. and any opinions and interpretations expressed fall outside Letaba Lab's Quality Document. Date Issued: 2014-07-07 Technical signatory (Name): Signaturé

TMH 1:1986 LETABA LAB Method A7, A8 & CBR and Modified A.A.S.H.T.O Density test report Client: Yinhla Engineers & Environmental Date tested. 03-Jul-14 Construction of New Housing Development - Siyabuswa MP P2793/1(ii) Sample no: P2793/1 Description: Sample nr G1 & G2 material delivered to lab sampled by client Maximum dry density = 2040 kg/m³ Optimum moisture content = % 8,8 2045 2040 2035 Density kg/m3 2030 2025 DN 2020 2015 2010 2005 9 11 % Moisture California Bearing Ratio (readings) 25 PROC NRB MOD 20 (KN) 10 0,0 2,0 3,0 6,0 7,0 8,0 Penetration (mm) California Bearing Ratio 1000,0 CBR @ 2.54 mm CBR @ 5.08 mm CBR @ 7.62 mm 100,0 CBR 10,0 1,0 90 91 92 94 95 96 97 98 99 100 101 102 % Compaction % Compaction 100 98 97 95 93 90 CBR of 13.344 kN 15 13 13 12 10 Briquette Info N.R.B. Mod Proc 2053 1948 1837 Dry Density (kg/m³) Compaction Moisture (%, 8,9 8,9 95,5% 90,1% Compaction (%) 100.6% % Swell 0,06 0,14 0,19 Please note that test results are only relevant to the sample tested, which were delivered to the lab, and were uncontaminated and fit for testing. Any results may only be regioduced in the entirety with the written consent of Letaba Lab (Pty) Ltd. and any opinions and interpretations expressed fall oytside Letaba Lab's Quality Document. 2014-07-07 Technical signatory (Name): Date Issued: Signature



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GRAVEL, SOIL AND SAND TEST REPORT Method A1-A5,A7, A8 & Client: Yinhla Engineers & Environmental Date Sampled 26-Jun-14 Doc No: P2793/3(i) P.O Box 2729 Thohoyandou Address: Construction of New Housing Development - Siyabuswa MP Contract: 03-Jul-14 Date Tested: Sample nr E1 & E2 material delivered to lab sampled by client Description Sieve Analysis Atterberg Description * Cumulative percentage passing Limits (%) Classification * Grading Modulus inear Shrinkage COLTO: 1998 Plasticity Index Index US. Highway Liquid Limit Unified Soil Depth (m Sample f 0,075 (Unified Soil Classification) Group I it Brown Clayey sand **GRADING ANALYSIS** 100 90 CUMULATIVE PERCENT PASSING P2793/3 80 70 60 50 40 30 20 10 0 0,075 0,425 2,00 Sieve Size 4,75 **GENERAL*** PERFORMANCE AS GRAVEL WEARING COURSE * Effective size (mm): <0.075 500 Class D Uniformity co-eff.: 417 Slippery 400 Curvature co-eff.: 0,2 Good - may be dusty 300 Class C Oversize Index: 0 Class A --- Class-E---Ravels 200 Erodible materials Shrinkage Product : 241 SHRINKAGE Grading co-eff.: 21,5 100 Class B: Ravels and corrugates CBR RESULTS (%): 0 @ 100% comp.: 11 10 20 25 30 40 45 @ 98% comp.: 9 @ 97% comp.: 9 @ 95% comp.: 7 REMARKS @ 93% comp.: 7 @ 90% comp.: 6 Soil Mortar Analysis Coarse Sand (<2.0>0.425mm): 15,8% Fine Sand (<0.425>0.075mm): 20,8% Material < 0.075mm: 39,4% Please note that test results are only relevant to the sample tested, which were delivered to the lab, and were uncontaminated and fit for testing. Any results may only be reproduced in their entirety with the written consent of Letaba Lab (Pty) Ltd. and any opinions and interpretations expressed fall outside Letaba Lab's Quality Document. Date Issued: 2014-07-07 Technical signatory (Name): Signature;

LETABA LAB Method A7, A8 & CBR and Modified A.A.S.H.T.O Density test report Yinhla Engineers & Environmental Date tested: 03-Jul-14 Construction of New Housing Development - Siyabuswa MP Doc no: P2793/3(ii) Sample no: P2793/3 Description: Sample nr E1 & E2 material delivered to lab sampled by client Maximum dry density = 1882 kg/m³ Optimum moisture content = 12,6 % 1885 1880 1875 Density kg/m3 1870 1865 1860 Dry 1855 1850 1845 1840 1835 12 13 % Moisture California Bearing Ratio (readings) 25 PROC NRB MOD 20 (KN) 15 10 0.0 1,0 2,0 3,0 4,0 5 Penetration (mm) 5,0 6,0 7,0 8.0 9,0 California Bearing Ratio 1000,0 CBR @ 5.08 mm 100,0 CBR 10,0 1.0 92 95 97 98 99 100 101 102 % Compaction % Compaction 100 97 95 93 90 CBR of 13.344 kN 11 9 Briquette Info Mod N.R.B Proc 1897 1797 Dry Density (kg/m²) 1696 Compaction Moisture (%) 12,4 12,4 12,4 100.8% 95.5% Compaction (%) 90,1% % Swell 0,07 0,13 0,21 Please note that test results are only relevant to the sample tested, which were delivered to the lab, and were uncontaminated and fit for testing. Any results may only be reproduced in their entirety with the written consent of Letaba Lab (Pty) Ltd. and any opinions and interpretations expressed fall outside Letaba Lab's Quality Document. 2014-07-07 Technical signatory (Name) Date Issued: Signature.

100

Group Index

Croup Index

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Croup Index

Group Index

Job NoP2793-1.4.xls



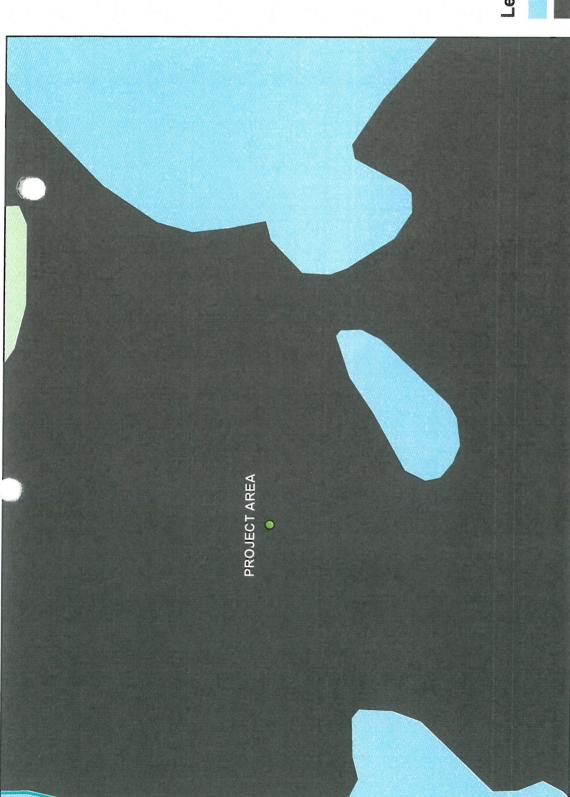
P.O. Box: 795 Faunapark 0787 123 Rivier Street Ladine Polokwane 0699

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GRAVEL, SOIL AND SAND TEST REPORT Method A1-A5.A7. A8 & Client: Yinhla Engineers & Environmental Date Sampled 26-Jun-14 Doc No: P2793/4(i) P.O Box 2729 Thohoyandou Address: 03-Jul-14 Construction of New Housing Development - Siyabuswa MP Date Tested : Contract Description Sample nr A1 & A2 material delivered to lab sampled by client Sieve Analysis Atterberg Description * Classification * Cumulative percentage passing Limits (%) Grading Modulus inear Shrinkage COLTO: 1998 Plasticity Index US. Highway Group Index Liquid Limit Unified Soil Sample No Depth (m) 0,075 (Unified Soil Classification) A-2-4 **GRADING ANALYSIS** 100 90 CUMULATIVE PERCENT PASSING P2793/4 80 70 60 50 40 30 20 10 0 0,075 2,00 4,75 Sieve Size **GENERAL*** PERFORMANCE AS GRAVEL WEARING COURSE * Effective size (mm): <0.075 500 Class D Uniformity co-eff. : 911 Slippery 400 Curvature co-eff.: 0,1 Good - may be dusty 300 Class C Oversize Index: 0 Class A -Class-E Ravels Shrinkage Product : 209 Erodible materials 200 Good SHRINKAGE Grading co-eff.: 24,5 100 Class B : Ravels and corrugates CBR RESULTS (%): 0 25 @ 100% comp.: 16 GRADING CC-FFFICIENT @ 98% comp.: 14 @ 97% comp.: 13 REMARKS @ 95% comp.: 11 @ 93% comp.: 10 @ 90% comp.: 8 Soil Mortar Analysis Coarse Sand (<2.0>0.425mm): 15,6% Fine Sand (<0.425>0.075mm): 20.3% Material < 0.075mm: 32,0% Please note that test results are only relevant to the sample tested, which were delivered to the lab, and were uncontaminated and fit for testing. Any results may only be regigible of their entirety ith the written consent of Letaba Lab (Pty) Ltd, and any opinions and interpretations expressed fall outside Letaba Lab's Quality Document 2014-07-07 Technical signatory (Name): Signature Date Issued:

TMH 1:1986 LETABA LAB Method A7, A8 & CBR and Modified A.A.S.H.T.O Density test report Client: Yinhla Engineers & Environmental 03-Jul-14 Date tested. Contract : Construction of New Housing Development - Siyabuswa MP Doc no: Sample no: P2793/4 Description: Sample nr A1 & A2 material delivered to lab sampled by client Maximum dry density = 2010 kg/m³ Optimum moisture content = 9.6 % 2020 2010 2000 Dry Density kg/m3 1990 1980 1970 1960 1950 1940 1930 10 12 % Moisture California Bearing Ratio (readings) 25 PROC NRB MOD Series4 20 (KN) 15 10 5 0 4,0 5,0 Penetration (mm) 0,0 2,0 3,0 1.0 6.0 7,0 8,0 9.0 California Bearing Ratio 1000.0 - CBR @ 2.54 mm - CBR @ 5.08 mm -*--- CBR @ 7.62 mm 100.0 10,0 90 91 92 93 94 95 99 100 101 102 % Compaction % Compaction 100 98 97 95 93 90 CBR of 13.344 kN 16 14 13 11 10 8 Briquette Info Mod N.R.B. Proc. Dry Density (kg/m² 2024 1920 1815 Compaction Moisture (%) 9.7 9.7 9.7 Compaction (%) 100,7% 95,5% 90,3% 0.06 0.13 0.15 % Swell Please note that test results are only relevant to the sample tested, which were delivered to the lab, and were uncontaminated and fit for testing. Any results may only be reproduced in their entirety with the written consent of Letaba Lab (Pty) Ltd, and any opinions and interpretations expressed fall outside Letaba Lab's Quality Document. 2014-07-07 Technical signatory (Name): Date Issued

APPENDIX: 4 GEOTECHNICAL CLASSIFICATION





Ecca_Grp,_karoo_Spgrp

Lebowa_Granite_Sui,Bushveld_Cplx



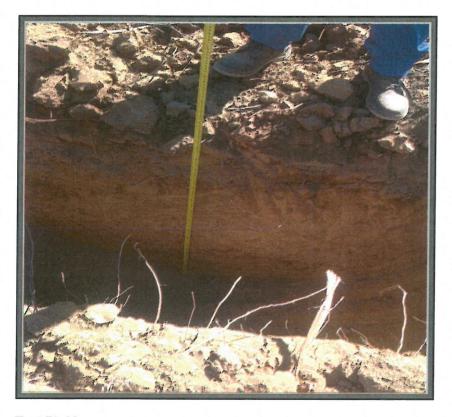
THE MAP SHOWING GEOLOGY AROUND THE PROJECT AREA

2.5 Kilometers	
0	
1.25	
2.5	

APPENDIX: 5 SITE PHOTOGRAPHS



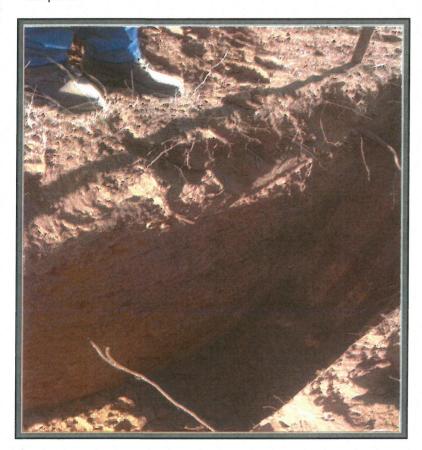
Test Pit 01



Test Pit 02



Test pit 03



Test pit 04