HOTAZEL PHOTOVOLTAIC FACILITY AND ASSOCIATED INFRASTRUCTURE

CONCEPTUAL STORMWATER MANAGEMENT PLAN

NOVEMBER 2018

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1. INTRODUCTION

Knight Piésold Consulting was appointed by ABO Wind Hotazel PV (Pty) Ltd to investigate and compile a Conceptual Stormwater Management Plan for the proposal of a photovoltaic (PV) energy facility and associated infrastructure.

This report should be viewed as a localised high-level study and not as a detailed design report. The objective is purely to demonstrate that stormwater from the new development could be managed and controlled in an optimised and non-destructive manner. The purpose of this study is to prepare a conceptual Stormwater Management Plan (SMP) to support the Environmental Impact Assessment Process of the proposed Hotazel PV facility.

The SMP includes the following:

- Determining the catchment area of the project site;
- Defining the topography, slope gradients and rainfall intensities;
- Estimating expected floods for the catchment;
- Confirming of existing drainage patterns and streams;
- Proposing drainage elements such as side drains, outlets and other mitigation measures to accommodate the resultant stormwater flows.

2. DEFINITIONS AND ASSUMPTIONS

The following assumptions are made on stormwater calculations and are deemed to be adequate for a conceptual investigative report:

- The Rational Method is used for flood calculations, which is widely accepted to be very accurate for areas of this size and
- The recurrence period applied is a 1:50 year design flood.
- There are no watercourses that will affect planning and the design of the solar facility.

3. EXISTING SITE CONDITIONS

3.1. Location

The site is situated approximately 3km south-east of Hotazel. Other towns in proximity of the project include Kuruman, located approximately 52km south-east, and Kathu located approximately 60km south of the project site, see *Figure 3.1*. The site falls within Ward 4 of the Joe Morolong Local Municipality of the John Taolo Gaetsewe District Municipality. A railway line runs along the south-western boundary of the project site, and also traverses the area just west of the project in a north-to-south direction. A formal gravel access road currently provides access from the regional road (R31) to the property. This gravel road then becomes a formal 2 wheel track. The site can be accessed through one of two gates.

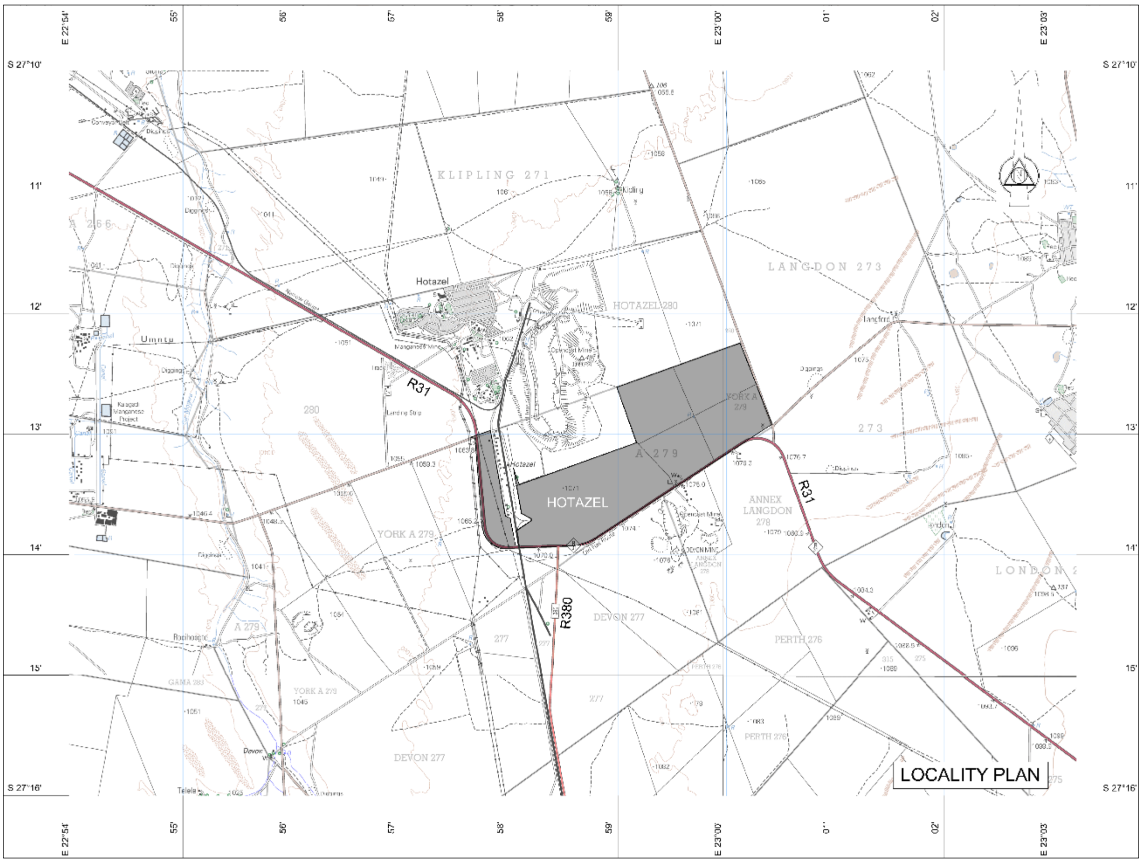

Figure 3.1: Locality Plan

3.2. Topography, Geomorphology and Vegetation (Drainage Characteristics)

The Northern Cape Province is situated in the north-western extent of South Africa. It is South Africa's largest Province. The Remainder of the Farm York A 279 has an almost level topography with a straight shape and the slope gradient of 0.5%. It is currently utilised for limited agricultural purposes and is characterised by Kathu Bushveld vegetation type with a well-developed grass layer and a variable density tree layer.

3.2.1. Drainage Patterns and Runoff Characteristics

The approximate total drainage area of the site is in excess of 636.8 hectares. Although the site does not reflect evidence of any watercourses, the drainage pattern slopes in a north westerly direction, see *Figure 3.2.* The slope gradient for the longest drainage path length within the catchment area is 0.27%.

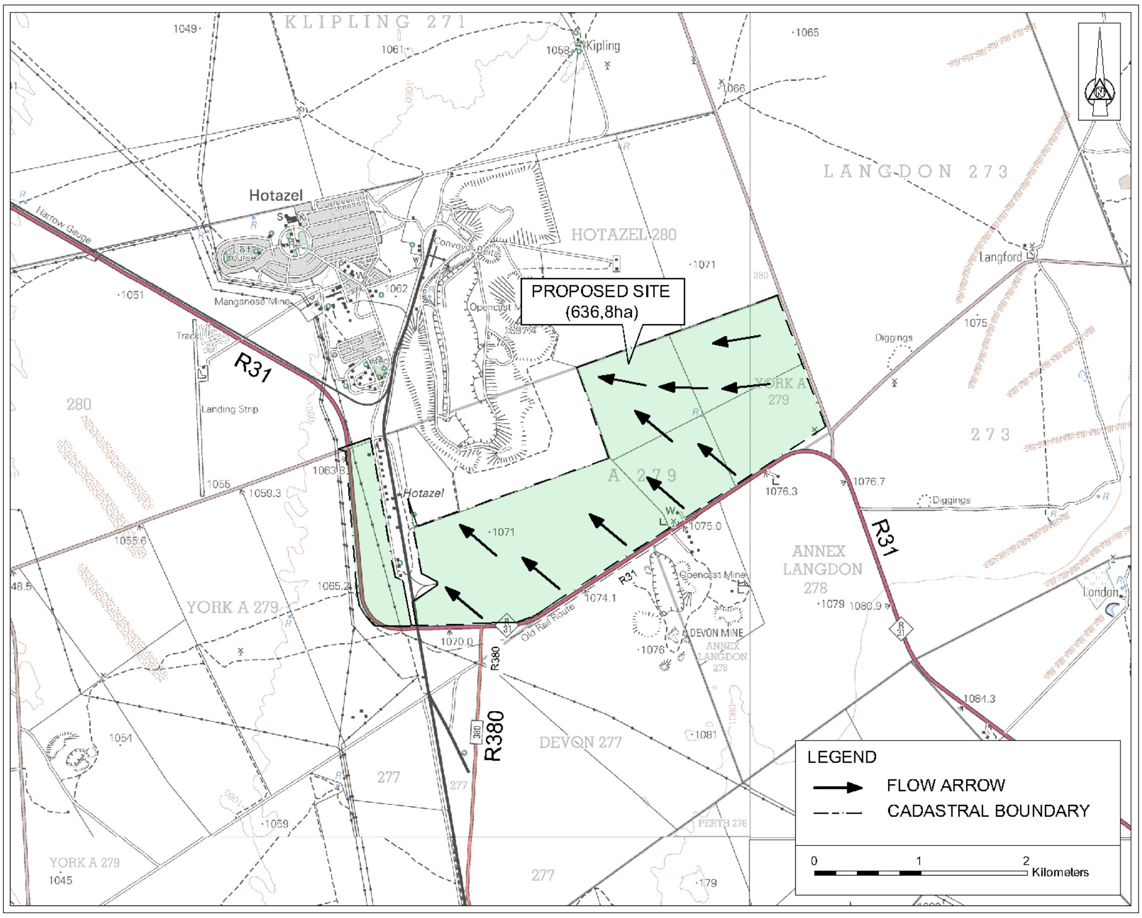

Figure 3.2: Drainage Pattern of Existing Site

It should be noted that, in the absence of detailed topographical survey information, 1:10 000 orthographical maps together with spot height data taken on site were used to establish the drainage patterns. The greater catchment area is 1 830ha, see *Figure 3.3* below. The sparse vegetation, together with the flat gradient and permeable soils yield very low runoff coefficients.

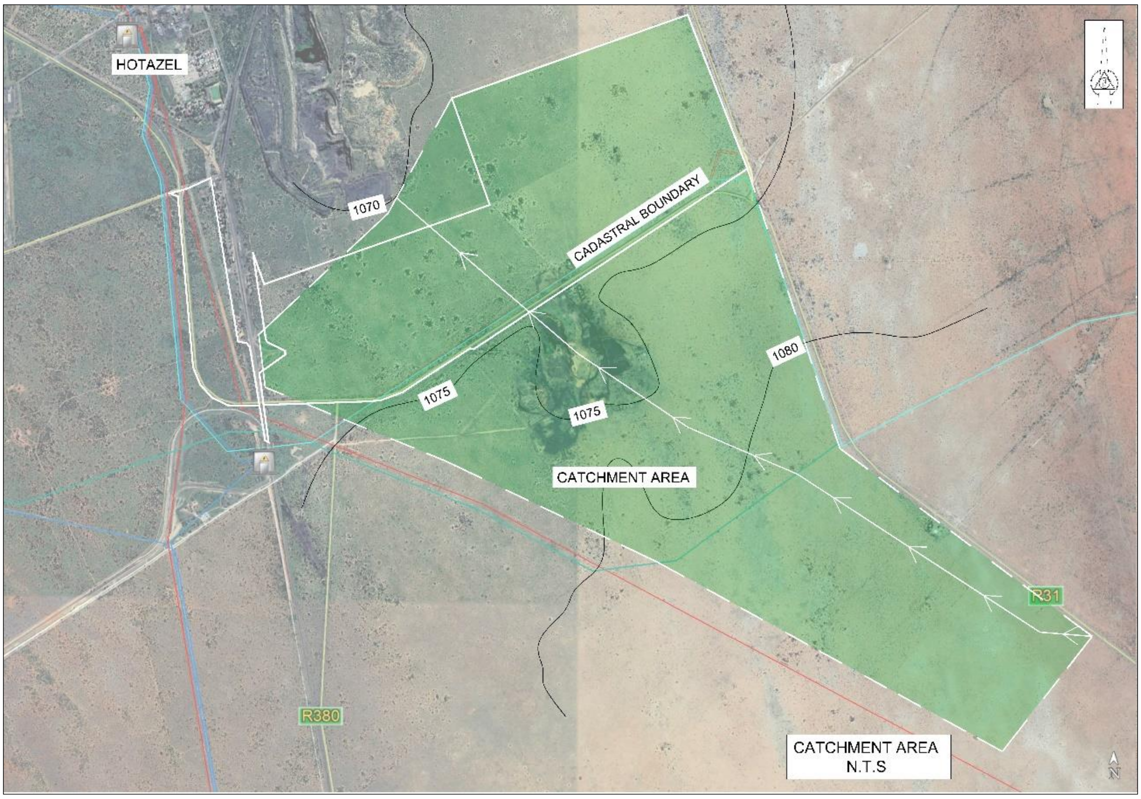

Figure 3.3: Catchment Areas

3.3. Geology and Soils

A number of manganese mining operations occur within close proximity of the project site. The Langdon Devon Manganese Mine is located immediately south of the project site. As a result, numerous waste rock dumps associated with these manganese mines are located within the vicinity of the project site. The manganese mine located directly to the north of the site is no longer in use and is under rehabilitation with reduced dust emission.

The geological map as well as field studies in the region show that the Kalahari sands here are extensively underlain by hardpan calcretes, some of which at least can be assigned to the Mokalanen Formation of the Kalahari Group. Soils in the region are usually red and yellow well drained sandy soil. Lime is generally present in part or most of the landscape and structureless free drained soil may occur. These soils may also have restricted depth, excessive drainage, high erodibility and low natural fertility.

3.4. Climate and Hydrology

The Kalahari region has consistent temperatures with summer and early autumn rainfall. Winters are very dry. The wettest part appears in the east with a mean annual precipitation of 500mm and the driest in the west with 120mm. The MAP for the whole Ecozone is 250mm per annum. Precipitation is the lowest in July, with an average of 2 mm. With an average of 66 mm, the most precipitation occurs in March; see *Figure 3.4* for a graph indicating the average monthly rainfall figures.

The Northern Cape and Namibia boasts the highest solar radiation intensity anywhere in Southern Africa. At an average temperature of 26.1 °C, January is the hottest month of the year. July has the lowest average temperature of the year, which is 11.2 °C; see *Figure 3.5* for average monthly temperatures. The information was gathered from the weather station at Winton, situated approximately 46km from the proposed site.

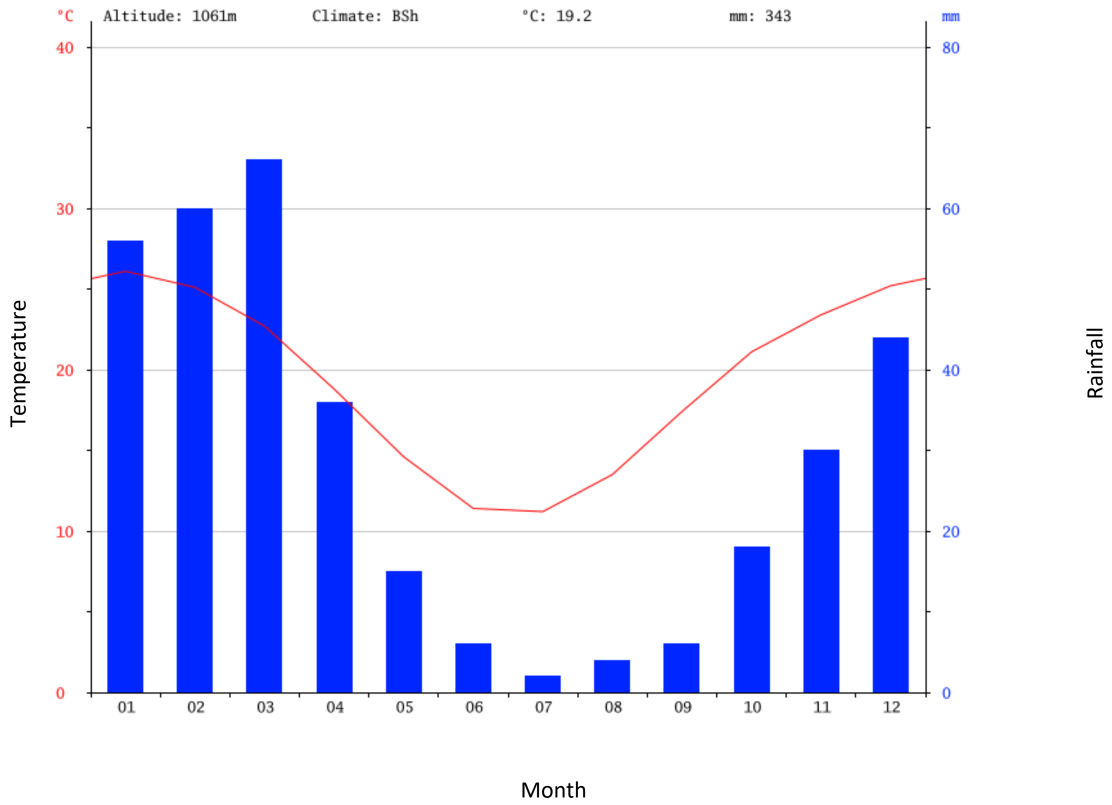

Figure 3.4: Monthly Rainfall for Hotazel

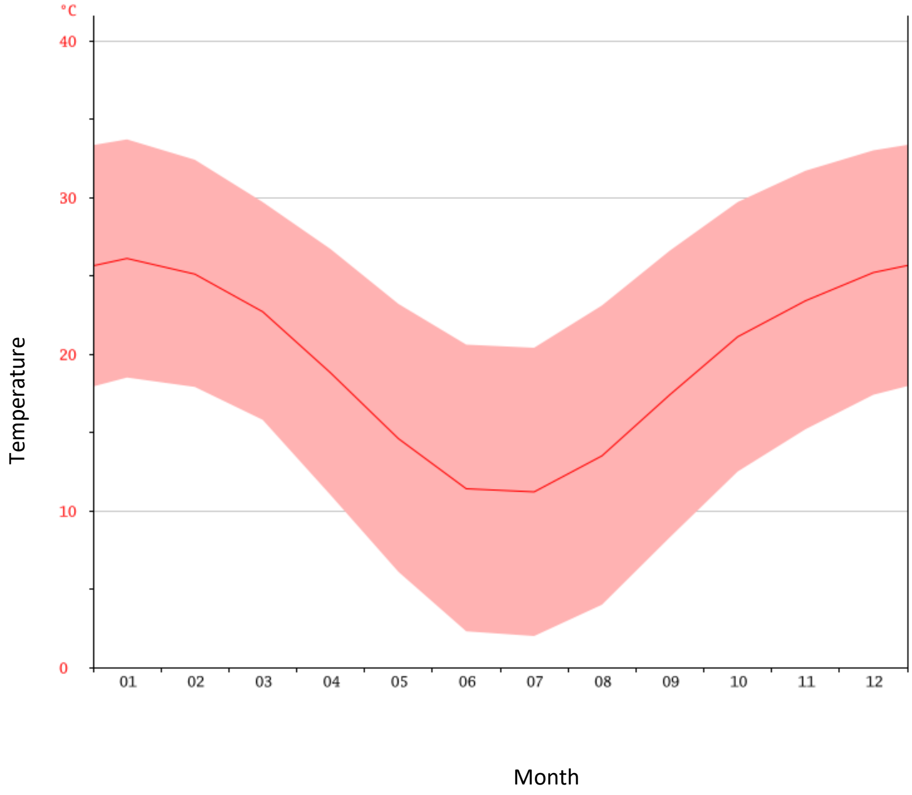

Figure 3.5: Average Monthly Temperatures for Hotazel

4. STORWATER CALCULATIONS

As mentioned previously, the calculations to determine the run off volumes and intensities of the site are based on the Rational Method with a return period of 1:50 years.

4.1. Runoff Coefficient

4.1.1. Pre-development

The pre-development runoff coefficient was calculated by making an allowance for 20% of semi-permeable soil. The site consists mainly of Bushveld with a well-developed grass layer and a variable density tree layer; see run-off coefficient percentages listed in *Table 4.1* below.

Table 4.1: Pre-development Runoff Coefficient Percentages

Permeability	% Applied	Vegetation	% Applied
Very	0	Thick bush & Forest	0
Permeable	80	Light Bush & Cultivated Land	30
Semi-Permeable	20	Grasslands	70
Impermeable	0	Bare	0
TOTAL	*100*	*TOTAL*	*100*

The calculated runoff coefficient based on the above for the pre-development phase is 0.284; refer *Annexure A* for further detail calculations in this regard.

4.1.2. Post-development

The post-development runoff coefficient takes the installation of the panels into account, as well as the vegetation alterations that may occur post construction. An area of 275ha (approximately 43% of the total project site) is required for the development of Hotazel PV. Even though the PV panels are impermeable, they will be mounted on bases that only cover a small surface area. A small percentage of the run-off coefficient was thus allowed for hardened surface. The Hotazel Solar facility will aim to make use of driven/ rammed piles, or ground/ earth screw mounting systems, and only in certain instances resort to concrete foundations, should geotechnical studies necessitate this. Concrete foundations may be used for the tracker at the end of each row.

During the construction phase, vegetation will be lost and this may not fully recover because of the shade that will be created by the panels post construction. A further allowance was made by amending the vegetation area when calculating the post-development peak runoff flows by allowing for 20% bare areas or no vegetation. These percentage figures are reflected in *Table 4.2* below.

Table 4.2: Post-development Runoff Coefficient Percentages

Permeability	% Applied	Vegetation	% Applied
Very	0	Thick bush & Forest	0
Permeable	75	Light Bush & Cultivated Land	30
Semi-Permeable	20	Grasslands	50
Impermeable	5	Bare	20
TOTAL	*100*	*TOTAL*	*100*

The calculated runoff coefficient based on the above for post-development phase is 0.310; refer *Annexure B* for further detail calculations in this regard.

4.2. Time of Concentration

The following formula was used to calculate the time of concentration, which is the time it takes for surface water at the furthest point on the site to reach the lowest area:

$$Tc = \left(\frac{0.87 \times L^{2}}{1000.S}\right)0.385$$

Where Tc = Time of Concentration (hours), L = Length of waterway (km), S = average slope.

4.3. Point Intensity

Point Intensity is based on standard time of concentration, and information was extracted from rain fall intensity depth graphs for the area.

4.4. Runoff

4.4.1. PV Area

The runoff distribution of the respective catchment areas will be dictated by the layout of the larger PV area, as well as the internal roads and channels. Each PV area which is a combination of smaller blocks should preferably be orientated in such a way to minimise the impact on natural drainage patterns. A typical PV panel configuration (subject to the final site development plan) is indicated in the *Figure 4.1,* with the resultant drainage pattern in *Figure 4.2* as follows:

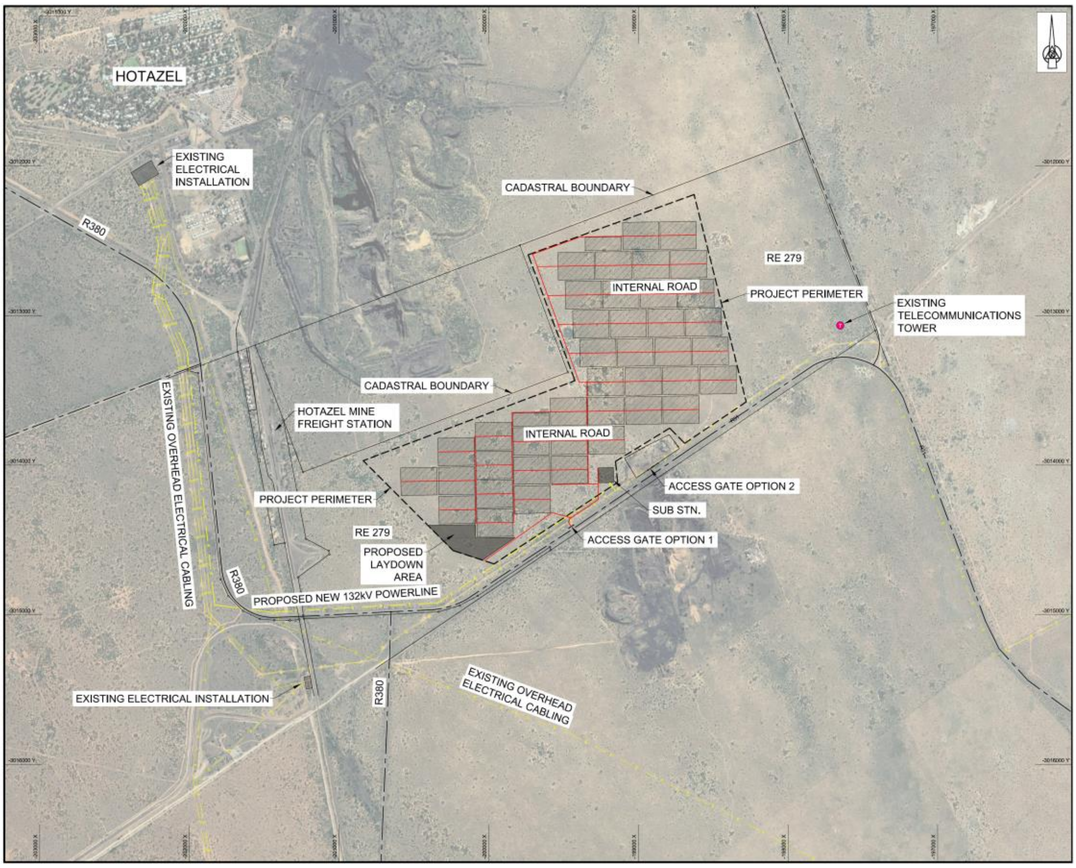

Figure 4.1: Conceptual Layout of PV Panels

Figure 4.2: Drainage pattern for conceptual layout of PV panels

There are no waterbodies or places of ponding visible on the proposed site.

The 1:50 year flood occurrence for pre and post-development runoff for the catchment area is shown below:

Catchment Area = 18.3 km2

Pre-Development C = 0.284

Post-Development C = 0.310

TC = 2.03 hours

Intensity = 28.3 mm/hr

Rational Method Pre-Development Q = $\frac{\text{CIA}}{3.6}$ = **38.97 m^3/s**

Rational Method Post-Development Q = $\frac{\text{CIA}}{3.6}$ = **42.28 m^3/s**

4.4.2. Access Roads

As mentioned previously, the access road intersects with the R31 (see Figure 4.3) and this road does have sufficient drainage. It is recommended that the first 200m of the access road be upgraded to a hardened (bitumen) surface, to prevent damage to the road edge. This upgrade should allow for sufficient drainage, however the remainder of the access road will remain gravel and provision must be made for drainage thereof.

The run-off across the gravel road is viewed to be very limited. The flow velocity and depth at the various outlets will have to be confirmed during the detailed design stage. The average velocity is in the order of 1.0 m/s for the gentle slopes on this site being 0.5%. Such flows will not cause any serious erosion, but appropriate measures should be implemented at outlets and points of concentration caused by drainage channels. This will reduce the risk of erosion damage. Frequent nominal drainage measures, typically piped culverts and/or mitre drains cut by a grader, must be provided at intervals between 200m to 300m as dictated by the site conditions and must be taken care of in the detail design. These could also be in-situ formed drifts where the road alignment is close to the natural ground level.

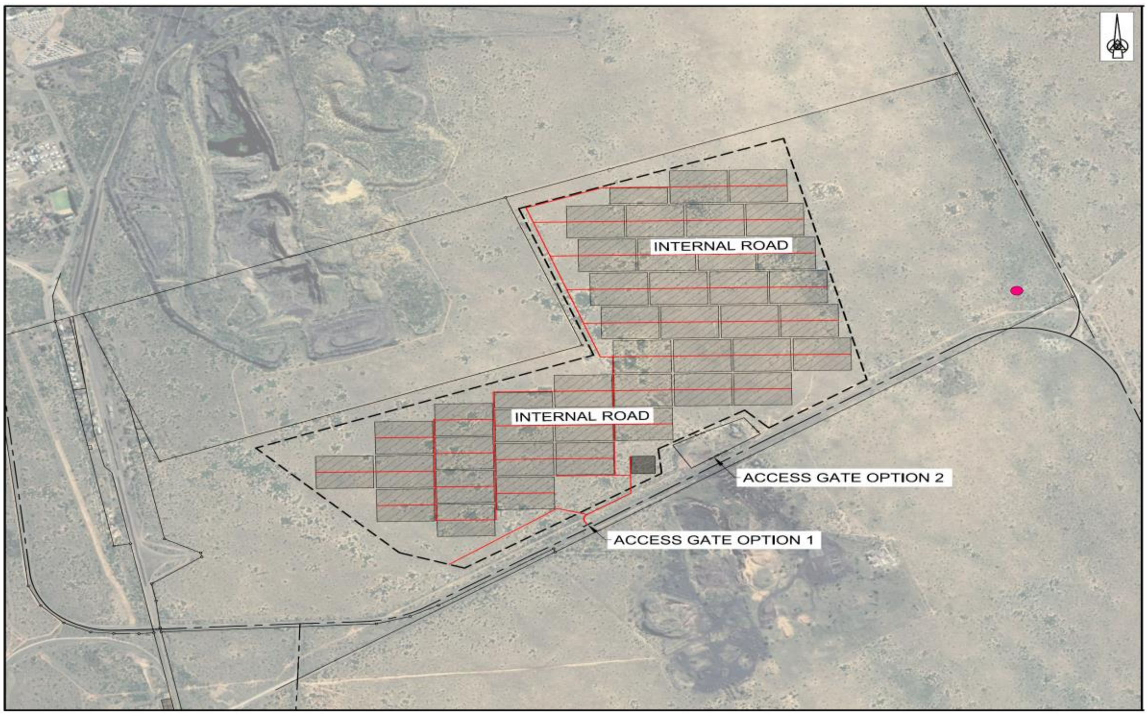

Figure 4.3: Access Roads

5. PROPOSALS FOR STORMWATER MANAGEMENT

The existing drainage patterns and characteristics should be preserved as far as possible. It is therefore suggested that the existing contours and vegetation be retained and that the internal roads are designed and constructed to minimum standards. The runoff calculations indicate that an additional 3.3 m^3/s or roughly an 8.5% increase in peak runoff will have to be accommodated when designing the stormwater management measures.

Drainage structures would include smaller diameter pipes (encased in concrete because of the low fill anticipated) or preferably gravel or concrete drifts. These drifts should have cut-off walls on the down-stream side as a minimum requirement.

5.1. Side Drains

Open drains will be provided along the proposed internal roads or between PV panels. These drains would be gravel drains with concrete or edge beam protection at road crossings, where required.

5.2. Berms

Berms are proposed to prevent external stormwater from entering the PV area and for directing flow to suitable areas of release; see *Figure 5.1* for typical berm details. Cut off drains are proposed on the southern property boundary to reduce runoff from the larger catchment area (see Fig 3.3).

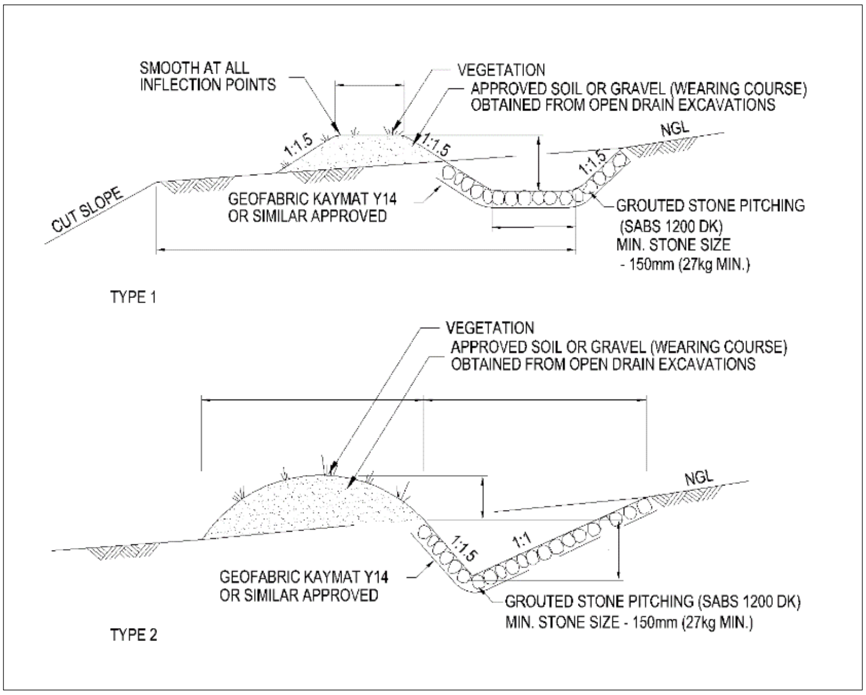

Figure 5.1: Typical Berm Detail

5.3. Outlets

All culverts on the access roads must be provided with concrete outlets with erosion protection. Side drain outlets should be terminated with suitable erosion protection to reduce the velocity and the flow depth.

6. EROSION PROTECTION MEASURES

The volume and velocity of the stormwater runoff must be thoroughly evaluated during the detailed design phase. The following erosion protection measures should be considered:

- Side drains, see *Figure 6.1 and 6.2*
- Inlet and outlet structures, see *Figure 6.3 and 6.4*

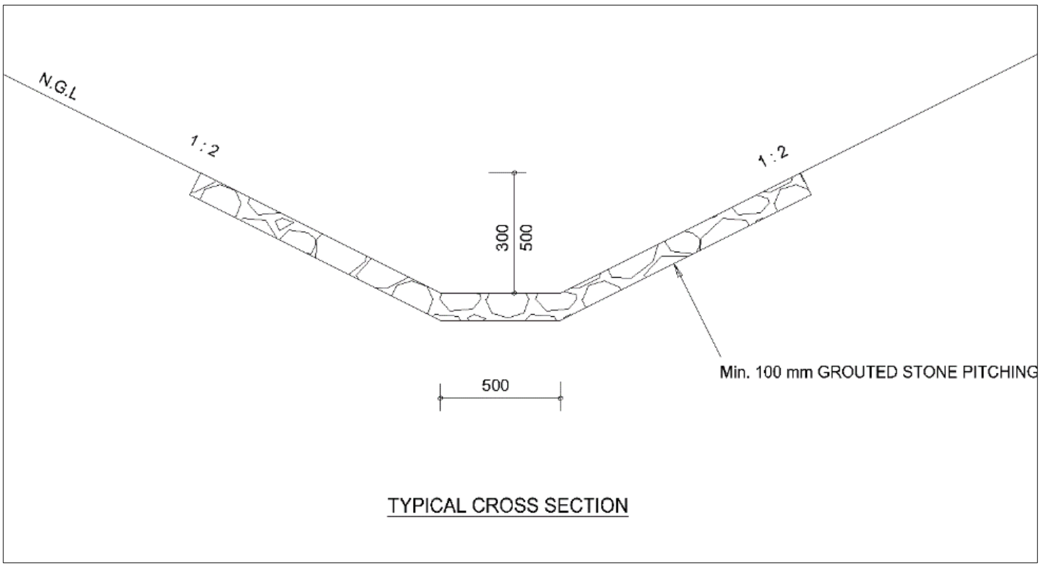

Figure 6.1: Typical Stone Pitched Side Drain

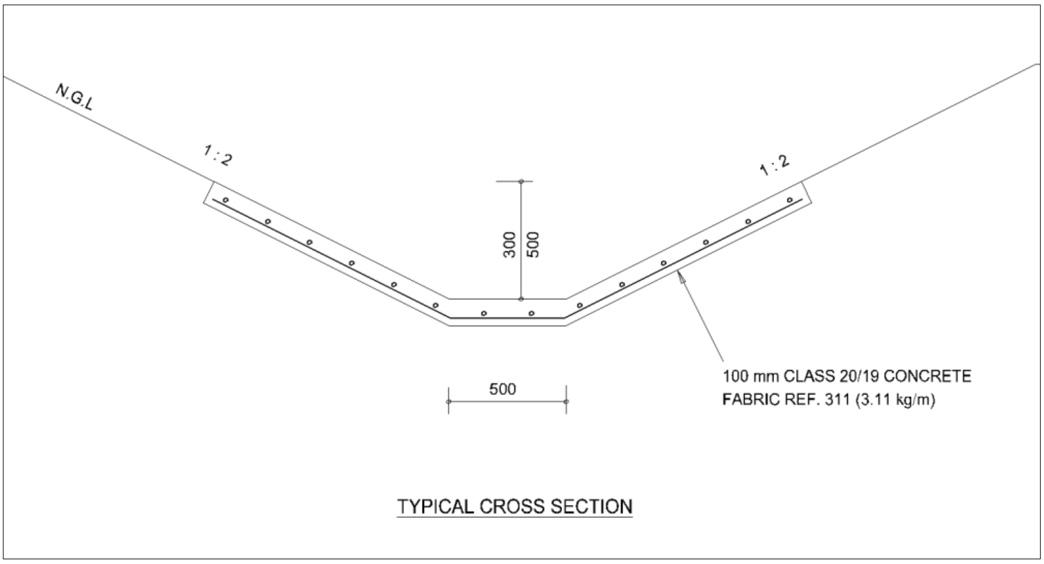

Figure 6.2: Typical Concrete Lined Side Drain

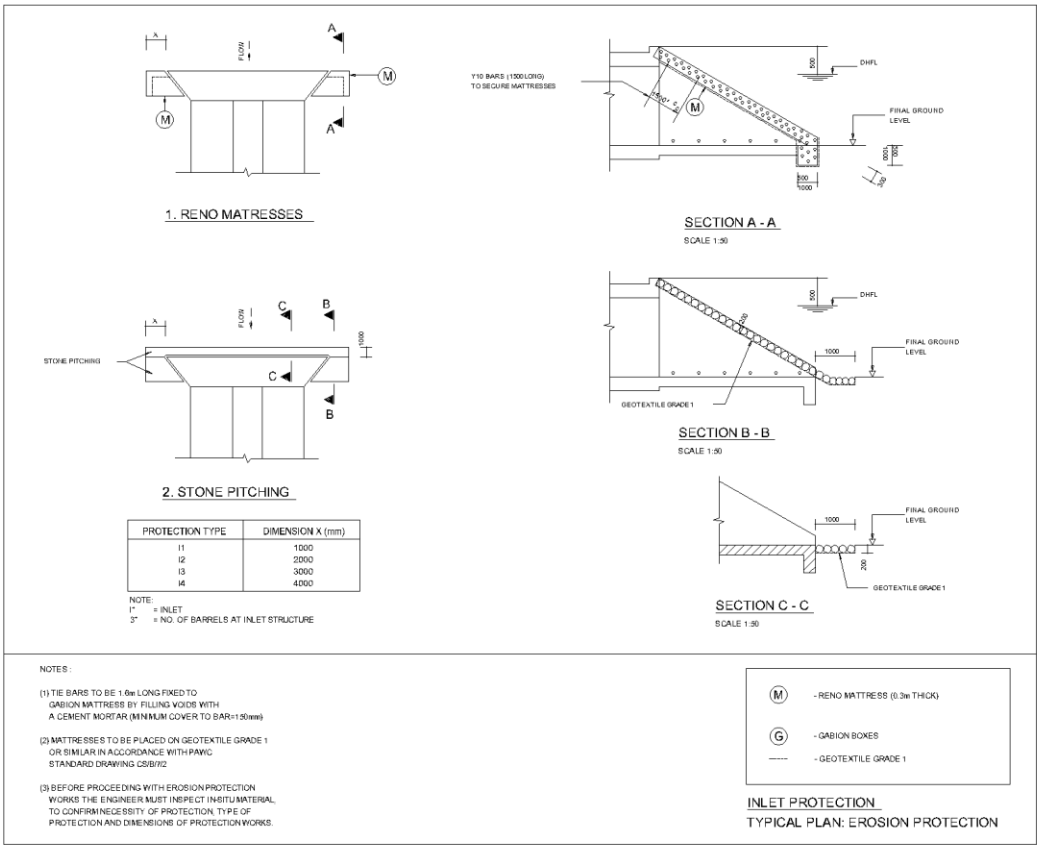

Figure 6.3: Typical inlet erosion protection measures

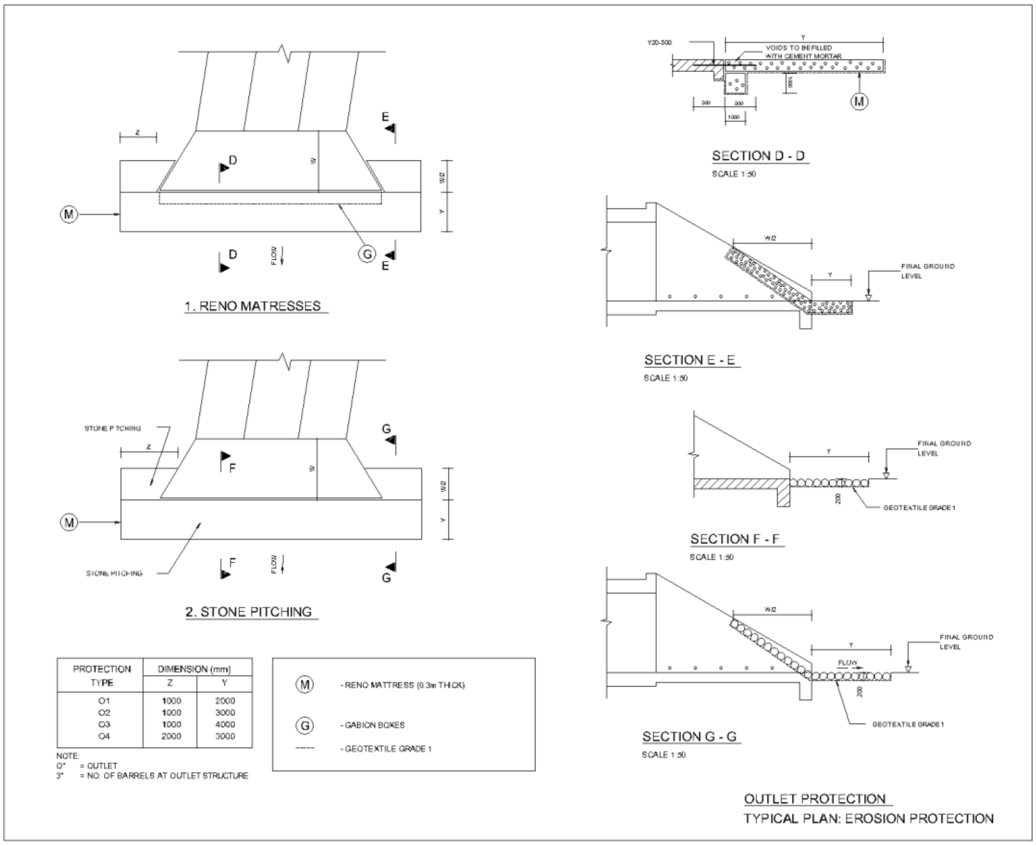

Figure 6.4: Typical outlet erosion protection measures

It is envisaged that in combination with the above the following are also likely to be required:

- Stone masonry walls to reduce the flow velocity in steeper areas;
- Side Drain Outlets with stone pitching to prevent erosion; and
- Temporary berms and straw bales during construction in the vicinity of identified streams to reduce flow and sediment transport during this phase.

During the construction phase, special attention must be given to stormwater so that construction activities do not result in any water ponding, especially in the vicinity of the roads and structures.

7. WASTE WATER MANAGEMENT

Several mines in the area are located to the north-west, south-west and south of the Hotazel Solar site. Venturing closer to these mining areas will expose the PV energy facility to increased dust levels, thus reducing the efficiency of the solar PV modules. After the installation of the panels, the cleaning (washing) of the solar panels is likely to generate small amounts of additional runoff. This process is estimated to occur twice a year, and add approximately 3l/m² of additional runoff to the site, over a period of approximately 2 weeks. This runoff would however be spread throughout the site, and due to the low localised water volumes would cause minimal, if any, erosion on the site and may even help as a form of dust control. The methods used for washing the panels determine the mitigation measures to be applied. This could be in the form of phasing the washing of panels or optimising the methods used. The overall effect on the site is expected to be very low, provided the cleaning water is free from detergents and includes only approved bio-degradable substances.

Rain will also aid in keeping the PV panels clean. The solar module surfaces are installed at a relatively large incline with gaps between modules. This does not allow significant water build up on the modules while also reducing the energy generated by the falling rain droplets.

On large structures or buildings, appropriate guttering could be used around the building to avoid water erosion where roof water would be flowing off the roof. Wherever practically possible, stormwater run-off from the gutter/roofs will be captured and stored in rainwater tanks. If this water cannot be captured, water will be channelled into energy dissipating structures to spread the water and slow it down to reduce risk of erosion. Such a structure could be constructed from precast concrete or loosely packed rock or perforated bags filled with stone.

8. CONCLUSIONS AND RECOMMENDATIONS

The additional stormwater runoff generated from the new facility post-development is almost negligible compared to pre-development. It is therefore envisaged to do limited stormwater management to reduce the impact of the proposed development on the environment.

By implementing earth drains, lined drains and limited erosion protection structures, the stormwater on the site, between the PV panels and for the adjacent access roads may easily be accommodated safely and in a non-destructive way. The development of the site will also be done in accordance with the existing slopes. The contours will be followed closely in order to minimise impacts on the existing drainage patterns.

9. REFERENCES

- Various Municipal Management of Urban Stormwater Impacts Policies
- The Georgia Stormwater Management Manual
- The South African National Roads Agency Limited. (2006). Drainage Manual Fully Revised 5th Edition
- Adamson P.T. (1983). Technical Report TR 102. Southern African Storm Rainfall
- Cape Environmental assessment Practitioners: Hotazel Solar, Final Scoping report, September 2018

10. ANNEXURES

Annexure A: Pre-Development Runoff Calculations

Flood Frequency Analysis: Rational Method

Project	= HOTAZEL
Analysed by	= RdV
Name of river	= N/A
Description of site	= RE OF FARM YORK A 279 : PRE-DEVELOPED
Date	= 2018/10/09
Area of catchment	= 18.3 km²
Dolomitic area	= 0.0 %
Mean annual rainfall (MAR)	= 326.00 mm
Length of longest watercourse	= 7.0 km
Flow of water	= Overland flow
Height difference	= 19.0 m
Value of r for over land flow	= Clean soil (r=0,1)
Rainfall region	= Inland
Area distribution	= Rural: 100 %, Urban: 0 %, Lakes: 0 %

Catchment description - Urban area (%)

Lawns		Residential and industry		Business	
Sandy, flat (<2%)	0	Houses	0	City centre	0
Sandy, steep (>7%)	0	Flats	0	Suburban	0
Heavy soil, flat (<2%)	0	Light industry	0	Streets	0
Heavy soil, steep (>7%)	0	Heavy industry	0	Maximum flood	0

Catchment description - Rural area (%)

Surface slopes		Permeability		Vegetation	
Lakes and pans	0	Very permeable	0	Thick bush & forests	0
Flat area	80	Permeable	80	Light bush & cultivated land	30
Hilly	20	Semi-permeable	20	Grasslands	70
Steep areas	0	Impermeable	0	Bare	0

Average slope	= 0.00271 m/m
Time of concentration	= 2.03 h
Run-off factor	
Rural - C1	= 0.284
Urban - C2	= 0.000
Lakes - C3	= 0.000
Combined - C	= 0.284

The HRU, Report 2/78, Depth-Duration-Frequency diagram was used to determine the point rainfall.

Return Period (years)	Time of concentration (hours)	Point rainfall (mm)	ARF (%)	Average intensity (mm/h)	Factor Ft	Runoff coefficient (%)	Peak flow (m³/s)
1:20	2.03	45.1	98.4	21.9	0.90	25.6	28.41
1:50	2.03	58.7	98.0	28.3	0.95	27.0	38.79
1:100	2.03	72.2	97.5	34.6	1.00	28.4	50.02

Run-off coefficient percentage includes adjustment saturation factors (Ft) for steep and impermeable catchments

Calculated using Utility Programs for Drainage 1.1.0

Annexure B: Post-Development Runoff Calculations

Flood Frequency Analysis: Rational Method

Project	= HOTAZEL
Analysed by	= RdV
Name of river	= N/A
Description of site	= RE OF FARM YORK A 279 : POST-DEVELOPED
Date	= 2018/10/09
Area of catchment	= 18.3 km²
Dolomitic area	= 0.0 %
Mean annual rainfall (MAR)	= 326.00 mm
Length of longest watercourse	= 7.0 km
Flow of water	= Overland flow
Height difference	= 19.0 m
Value of r for over land flow	= Clean soil (r=0,1)
Rainfall region	= Inland
Area distribution	= Rural: 100 %, Urban: 0 %, Lakes: 0 %

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Lawns		Residential and industry		Business	
Sandy, flat (<2%)	0	Houses	0	City centre	0
Sandy, steep (>7%)	0	Flats	0	Suburban	0
Heavy soil, flat (<2%)	0	Light industry	0	Streets	0
Heavy soil, steep (>7%)	0	Heavy industry	0	Maximum flood	0

Catchment description - Rural area (%)

Surface slopes		Permeability		Vegetation	
Lakes and pans	0	Very permeable	0	Thick bush & forests	0
Flat area	80	Permeable	75	Light bush & cultivated land	30
Hilly	20	Semi-permeable	20	Grasslands	50
Steep areas	0	Impermeable	5	Bare	20

Average slope	= 0.00271 m/m
Time of concentration	= 2.03 h
Run-off factor	
Rural - C1	= 0.310
Urban - C2	= 0.000
Lakes - C3	= 0.000
Combined - C	= 0.310

The HRU, Report 2/78, Depth-Duration-Frequency diagram was used to determine the point rainfall.

Return Period (years)	Time of concentration (hours)	Point rainfall (mm)	ARF (%)	Average intensity (mm/h)	Factor Ft	Runoff coefficient (%)	Peak flow (m³/s)
1:20	2.03	45.1	98.4	21.9	0.90	27.9	30.96
1:50	2.03	58.7	98.0	28.3	0.95	29.4	42.28
1:100	2.03	72.2	97.5	34.6	1.00	31.0	54.51

Run-off coefficient percentage includes adjustment saturation factors (Ft) for steep and impermeable catchments

Calculated using Utility Programs for Drainage 1.1.0