

RICHBAY CHEMICAL (PTY) LTD

**PROPOSED NEW CHEMICAL PLANT:
VOSLOORUS, EKURHULENI**

**FEASIBILITY DOLOMITE STABILITY AND
GEOTECHNICAL INVESTIGATION**



PREPARED FOR:



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1. INTRODUCTION

Knight Piésold (KP) was appointed by Mr Martin Klopper of Richbay Chemicals (Pty) Ltd (purchase order number 53246) to conduct a feasibility level dolomite stability and geotechnical investigation of a site in Vosloorus in Ekurhuleni, Gauteng. It is understood that the intended use of the site is predominantly for the manufacture, packaging, storage and distribution of chemical products. Large laydown areas for products, storage tanks and parking facilities encompass most of the intended development. Warehouse structures and administration buildings are also anticipated.

As the site is underlain by dolomite of the Chuniespoort Group, Transvaal Supergroup, a dolomite stability investigation is required, in accordance with SAN 1936 Parts 1 to 4. The stability investigation is aimed at determining the potential for sinkholes and subsidence's to develop. The outcome of the stability investigation will determine the type of development that will be allowed as well as the density of development.

The investigation aims to provide a preliminary assessment of the dolomite stability conditions across the site, which has a total area of approximately 8 ha. The objectives of the investigation were to:

- Establish the site stratigraphy and the engineering properties thereof,
- Establish the potential for sinkholes and subsidence's to develop and, on the basis thereof, zone the site and establish the nature of the development that is permissible,
- Identify potential problem soils,
- Establish the potential for in-situ materials to be used in pavement layers, and
- Provide preliminary foundation recommendations for the proposed development.

This report details the work carried out, analyses the results, and gives our preliminary conclusions and recommendations.

2. AVAILABLE INFORMATION

The desk study assessments were based on published geological maps, satellite imagery (Google Earth ®) and published data of geological formations present on site. The data sources included:

- 1:250 000 scale Geological Series sheet 3318 Cape Town Map.
- Engineering Geology of Southern Africa by ABA Brink.
- Google Earth Imagery ®.

No borehole records of a previous dolomite stability investigation on the site are available from the Council for Geoscience (CGS) dolomite databank, however, additional sinkhole events and boreholes are noted nearby.

3. SITE LOCATION AND DESCRIPTION

The site is located approximately 2,5 km east of Vosloorus in Ekurhuleni Metropolitan Municipality (EMM) as indicated in the locality plan (Figure 1), included at the end of this report. The site lies immediately east of the N3 national highway and falls within the bounds of Portion 86 of the farm Vlakplaats 138 IR.

The Weinert Climatic N-value [1]¹ for the site is less than five. Chemical weathering is the predominant weathering form within this climatic environment and facilitates the breakdown of primary minerals into secondary minerals and clays.

Waterlands Road defines the eastern boundary of the site, and vacant farmland encompasses the immediate remaining boundaries. The entire site is enclosed by a 4 m high concrete wall which is visible from satellite imagery. Access to the site is gained from the main access gate off Waterlands Road in the north-eastern corner of the site.

The general topography of the site slopes gently towards the N3 highway, with sporadic heaps of dumped material occurring in the southern central portion of the site. Regional surficial drainage generally follows the topography and flows westwards from Waterlands Road to the N3 Road Embankment, which channels the storm water to a concrete line catchpit adjacent to the southwest corner of the site. Available GIS databases indicate that a storm water pipeline cuts north-south through the western half of the site. Refer to site photographs included in Appendix A at the end of this report.

No sinkholes or subsidence's were observed on the site at the time of investigation. CGS records indicate that the nearest occurrences of sinkholes are between 380 m and 470 m east of the site (namely EK_S220 and EK_S221) [2]. Visual inspection of these areas was not undertaken; however, satellite imagery suggest that these two sinkholes were possibly related to leaking stormwater pipes to the west of the R103 regional route.

Vegetation across the site predominantly comprises veldt grasses with scattered trees across the site and one small grove in the central portions of the site. It appears that much of the upper soil profile (<0,5 m below current ground level) has been highly disturbed by historic use as laydown areas over most of the site.

Numerous small access roads, lined with aggregate crosscut the site. These provide access to the limited infrastructure, comprised of a workshop, administration building and change room in the centre of the site. All of which are bound with precast walling.

Remnants of a forecourt and possible buried fuel tank are evident in the centre of the site, directly opposite the workshop. It is unclear whether the fuel tank was in fact above ground or remained buried.

To the west of the change house is a smaller building which houses the borehole pump, used to fill a storage tank which feeds the change house groundwater. Satellite imageries suggest the installation of this borehole predates 2003.

¹ References are indicated thus and are listed at the back of the report.

4. METHODOLOGY

The investigation was carried out by performing a gravity survey of the site on a 20 m grid to provide gravimetric anomalies highlighting subsurface conditions for investigation. The gravity survey was carried out by GeoFocus Geophysical Services in mid-September 2021. The results of the survey have been used to determine investigation positions and are contained in Appendix D.

Following the results of the gravity survey, the field investigation comprised the excavation of twelve test pits (TP1 to TP12) and drilling of eleven percussion boreholes (PH1 to PH11) on 16 and 17 September 2021 respectively.

The test pits were excavated to maximum reach or refusal conditions of a CAT 428F TLB. The test pits were excavated by V&S Cathire to investigate the shallow soil conditions. The test pits were profiled in-situ by an engineering geologist and representative samples selected for laboratory testing. Test pits were backfilled on the same day as the percussion drilling concluded.

Representative samples were taken from the soil profiles and have been submitted to Specialised Testing (ST) commercial laboratory in Pretoria, South Africa for geotechnical testing. The following laboratory tests have been requested:

- 12x Particle Size Distribution (PSD) and Atterberg Limits,
- 3x Modified AASHTO maximum dry density and optimum moisture content tests,
- 3x California Bearing Ratio (CBR) tests,
- 3x pH and electric conductivity (Ec) tests,
- 1x Collapse potential (CP) test, and
- 1x Consolidation (Oedometer) test.

The percussion boreholes were drilled by Hennie Erwee Drilling using a Super Rock 1000 percussion drilling rig to a maximum depth of 60 m or until 6 m sound bedrock was encountered. Two drilling rigs were established on the same day to complete drilling in one day. The boreholes investigated gravimetric anomalies to assess the subsurface conditions at the site.

Chip samples were recovered at 1 m intervals from the percussion drilling, and the depths of water strikes, sample and air losses and penetration rates were accurately recorded by the driller during the drilling operation.

The boreholes were logged on site by an engineering geologist in accordance with industry guidelines and standards, which included, but are not limited to the following:

- SANS 1936: 2012: Development of Dolomite Land. Parts 1 to 4.[3].
- SANS 633:2012: Soil profiling and rotary percussion borehole logging on dolomite land in Southern Africa for engineering purposes [4].

Groundwater rest levels were recorded 24 hours after the completion of drilling, after which the boreholes were backfilled and sealed.

The positions of the test pits and boreholes were recorded on site using a hand-held GPS accurate to 3 m. The GPS co-ordinates are provided on the profiles and are presented in WGS84 datum, South African grid, Lo29.

The positions of the test pits and boreholes are shown on Figure 5. The test pit profiles are summarised in TABLE 3 and full profiles are contained in Appendix B. The borehole profiles are summarised in TABLE 4 and the full profiles contained in Appendix C.

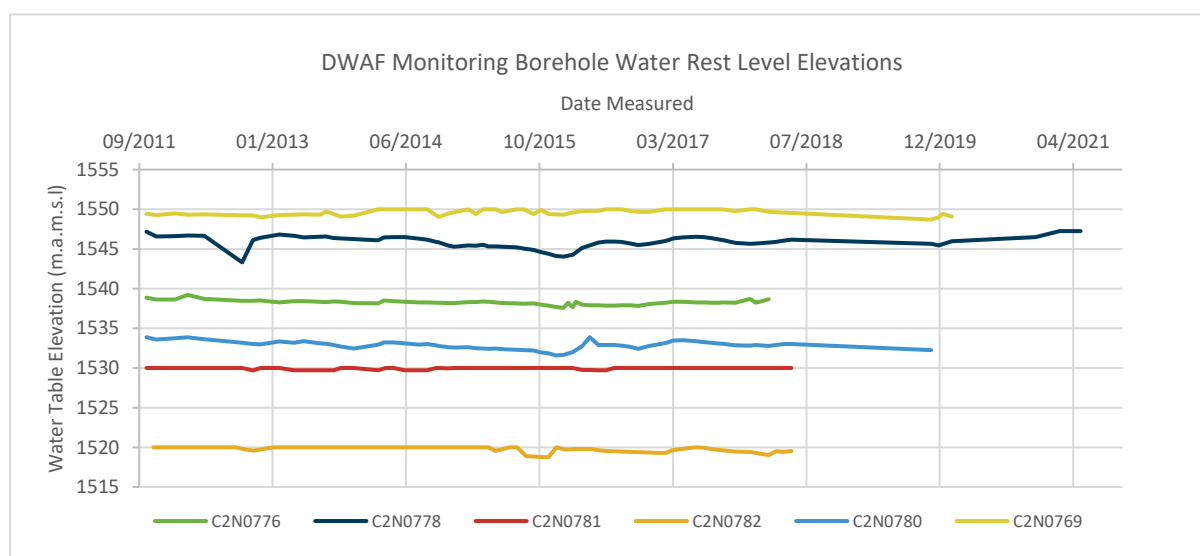
5. REGIONAL GEOLOGY AND GROUNDWATER

According to the published 1:250 000 scale Geological Map, Sheet 2628 East Rand [5], the site is underlain by karstic rocks of the Malmani Subgroup, Chuniespoort Group, Transvaal Supergroup. Regionally, geological contacts with younger dolerite rocks of Post Karoo Intrusive origin, are indicated on the map approximately 760 m east and 1,4 km west of the site. Significant erosion of the dolerite is interpreted given the regional topography and its inferred absence below the site. Locally the dolomite is intensely interfingered with dolerite across most of the site, with the latter being widespread. A complex geological profile is the result of the interrelation of these two dominant lithologies.

The site appears to be located on the eastern bank of a regional drainage feature, draining southwards, sub-parallel to the N3 highway. This feature drains to the Rietspruit River which lies approximately 1,7 km south of the site. An extract of the published geological map of the area is provided in Figure 2.

The Malmani Subgroup consists of chert-rich, alternating with chert-poor dolomite formations. These rocks have a notorious reputation for the development of karst subsurface landscape, associated with highly irregular and voided bedrock profile, as well as heterogenic soil conditions. The soil cover often comprises highly erodible soils, which can readily mobilise downward by means of percolating water to create leached or voided zones, which may result in the formation of sinkholes and subsidences [6].

The site forms part of the Natspruit East dolomite groundwater compartment with many ground water monitoring boreholes, as indicated in Figure 3. The median groundwater level in the compartment is recorded as 15,2 m with the minimum and maximum ad-hoc measurements varying between 12,3 m and 21,4 m below surface [7]. Water rest elevations from the DWAF monitoring boreholes surrounding the site are indicated below.



Water rest levels recorded in monitoring boreholes operated by the Department of Water Affairs (DWAF), indicate that regional water level for boreholes near the site over the last 10-year period is relatively stable. However, slight fluctuations (<1m) do occur, which can possibly be attributed to seasonal drought and possibly nearby abstraction of water for industrial and/or agricultural use.

The static water level (SWL) for this site has been taken as the regional median of 15 m below surface and used in the stability analysis in Section 8 of this report. The investigation was conducted at the turn of seasons from dry to wet. No significant water sources of consequence or in excess of seasonal rainfall, were noted near the site. A summary of the water rest levels as encountered in the boreholes is presented in TABLE 1 and the descriptive statistics of the water rest levels are shown in TABLE 2.

TABLE 1: Summary of Bedrock and Water Rest Elevations

Hole No.	Hole Depth (m)	Water Strike (m)	Water Rest Level (m)	Dolomite Bedrock depth (m)	Ground Elevation (m.a.m.s.l.)	Water table Elevation (m.a.m.s.l.)	Dolomite Bedrock Elevation (m.a.m.s.l.)
PH1	12	-	Dry	3.0	1546.8		1543.8
PH2	25	8.1	9.6	18.5	1543.9	1534.3	1525.4
PH3	22	13.5	11.6	16.0	1546.1	1534.5	1530.1
PH4	20	-	10.9	13.0	1543.2	1532.3	1530.2
PH5	28	11	10.2	18.0	1546.9	1536.7	1528.9
PH6	24	14	11.2	>24	1545.7	1534.5	<1521.7
PH7	20	11	10.6	11.0	1545.0	1534.4	1534
PH8	23	14.9	10.3	18.0	1541.9	1531.6	1523.9
PH9	21	-	12.2	>21	1540.9	1528.7	<1519.9
PH10	26	14.3	10.8	25.0	1547.4	1536.6	1522.4
PH11	21	-	9.9	14.0	1541.4	1531.5	1527.4

TABLE 2: Descriptive Statistics of Site Water Rest Levels

Zone	Water Rest Level (m)	Dolomite Bedrock depth (m)	Water table Elevation (m.a.m.s.l.)	Dolomite Bedrock Elevation (m.a.m.s.l.)
All data	10	Boreholes drilled within the site		
Avg	10.7	16.7	1533.5	1527.8
St Dev	2.4	4.0	2.4	3.6
Min	9.6	11.0	1528.7	1522.4
Max	12.2	25.0	1536.7	1534.0
Variance	0.6	16.2	5.5	12.6
No. used	10	8	10	8

In general, the water strikes encountered in boreholes were notably deeper than the final water rest levels in the boreholes. This possibly indicates that the confined conditions may be present due to the down slope orientation of the dolerite dyke relative to the dolomite. The Original Water Level (OWL) is generally deeper than the water rest level recorded from this drilling campaign by roughly 4,5 m. The water rest levels are generally above dolomite bedrock, except for boreholes PH1 (Dry) and PH7, which is 0,5 m above bedrock. The average water rest level for the site is regarded as 11 m below surface for the purpose of stability analysis.

6. INVESTIGATION RESULTS

6.1 TYPICAL PROFILE

The results of the test pit excavations indicate that the site is covered by fill and underlain by transported and residual soils extending to bedrock. TABLE 3 provides a summary of the test pit logs. The summary of the borehole profiles is included in TABLE 4. A geological cross section of the drilling results is presented in Figure 4. The ground profile is described as follows:

- Most of the site is blanketed by a generally thin fill of variable composition. This varies between 0,1 m and 0,8 m thick with the average thickness of 0,29 m. Typical descriptions for the thin occurrences of fill are greyish brown to black silty fine sand combined with coarse (27mm) dolomite and chert aggregate. Instances of fill thicker than 0,2 m are generally highly variable in description and properties. Particularly thick layer of fill was encountered in test pit TP3 which comprises gravelly silty sand with minor cobbles. Additionally, spoil heaps of stockpiled fill materials are visible from surface in the areas between test pit TP3 and TP6 near the southern boundary of the site.
- One instance of topsoil was encountered below the surface to a maximum depth of 0,4m. The topsoil has presumably been stripped in other areas across the site prior to use as laydown areas. This was noted as a dark brown silty fine gravelly sand of dense consistency with abundant rootlets.
- The transported soil underlying the topsoil comprises hillwash, which is generally overlain by fill across most of the site. The hillwash is typically encountered to a depth of 1,4 m below surface in all test pits confined to the western half of the site with an average thickness of 0,6 m. In the eastern half of the site, hillwash occurs to a shallower depth of 0,75 m on average with a similar thickness and encountered in test pits TP11 and TP8 only. Hillwash on site is described as red brown to dark red brown, slightly fine gravelly sandy silty clay for the occurrences above residual dolerite profiles. Consistencies vary between firm to stiff in the tactile descriptions. In the eastern portions of the site underlain by dolomite, the profile appears sandier from tactile descriptions and comprises clayey silty fine sands of medium dense consistency.
- A pebble marker horizon varies between 0,3 m and 0,8 m thick and is present to a maximum depth of 2,1 m in test pit TP1. The horizon is described as dark reddish brown stained black speckled white, silty sandy fine to coarse gravel with a medium dense to generally dense consistency. The gravel and cobble-sized fragments comprise highly to completely weathered chert.
- Isolated occurrences of pedogenic soils are encountered in test pits TP2 and TP5. Nodular ferricrete has developed at depth within the reworked residual dolerite in the former and within the transported soils (hillwash) in the latter. Slow excavation was noted at the base of TP2 which is situated in the lowest lying portion of the site, and this was due to the only occurrence of honeycomb ferricrete at its base. This may be indicative of seasonal development of a shallow perched ground water table within the surficial soils.
- The residual dolerite soil profile, which occurs below the transported soils, is often reworked to an average thickness of 0,9 m in its upper portions and situated at an average depth of 1,4 m below surface. This reworked portion is underlain by residual dolerite in all instances except for TP1, TP2 and TP4. Grading of the reworked to partially reworked soils typically correlate with a change in colour from dark red brown mottled black yielding slightly sandy silty clay to clayey silt. The latter being reddish orange slightly sandy clayey silt. Consistencies of the reworked portions are typically in the range of firm to stiff with occasional instances of very stiff. Tactile descriptions of the residual

dolerite vary from orange-brown to yellow brown slightly clayey sandy silt to silty sand. Consistencies vary between stiff to very stiff for the former grading and dense to very dense in the latter.

- Dolomite residuum, both fines dominated, and chert dominated were encountered in the test pits across the eastern half of the site, viz. TP7 – TP12. Residual dolomite (fines) was encountered in the test pits TP7 – TP9 and identified as reworked in all cases, with an average thickness of 0,6 m. Descriptions for this are typically dark brown mottled or speckled red brown and black, silty sandy clay to fine gravelly clayey silty sand. The coarser fraction within the reworked horizons mainly consisted of chert. The chert residuum (coarse) was encountered in all test pits on the eastern side of the site and has an average thickness of 1,35 m. Typically, this has been described as red brown speckled or blotched white stained orange, clayey sandy fine to coarse gravel with occasional cobbles. Consistencies typically range between medium dense to dense.
- One instance of a mixed transitional zone was encountered in TP12 near the centre of the site. This comprised a mixture of chert dominated residuum and highly weathered fragments of residual dolerite. In total, two test pits had identified the contact zones between the dolomitic profile and the intrusive dolerite profile near surface, namely TP7 and TP12.
- Refusal of the TLB was encountered in test pits TP5 to TP7, TP10 and TP11. This was mainly on very soft rock to soft rock dolerite in TP5 and TP6, on shallow chert boulders/ bedrock in TP7 and TP11 with dolomite bedrock encountered in the base of TP10.
- The borehole profiles show the bedrock to be highly variable with complex interfingering of the dolerite within the dolomite. Negligible dolomite residuum was interpreted to be present within the soil profile where it extends below the maximum depth of the test pits, except for borehole PH5 between 2 m and 9 m. A 3 m thick layer of residual dolomite (wad) was encountered in borehole PH10 at shallow depth. One zone of air and sample loss (<50% return) was encountered in PH1 between 8 m and 11 m. Outside of these identified zones, no air and sample losses were encountered, also no weathered products of dolomite were encountered at the contacts between differing lithologies.

The conditions across the site are uniform with minor variations observed between eastern and western halves. No groundwater seepage was encountered in any of the test pits. Most of the boreholes encountered water strikes and recorded a static water level at least 24 hours after drilling. The water rest level for the site as recorded in the boreholes has been taken as 11 m below surface and used as such in the stability analysis in subsequent sections.

6.2 GEOPHYSICAL SURVEY

The gravity survey was carried out in mid-September 2021. The site characterised by gravity high features in the extreme eastern and central-western portions which grades to gravity low features surrounding it. The latter gravity high straddles the site with a NW-SE trend and the central broad gravity low integrates multiple smaller lows. A peak-to-peak envelope of the results is roughly 0,3 mGal. Refer to Figure 6 detailing the revised residual gravimetric map and borehole positions. The detailed report by the geophysicist is included in Appendix E.

Borehole PH1 intersected bedrock at shallow depth (6 m) and was used to apply corrections to the gravity data. Similarly, the two prominent gravity low features in the central portions of the site were intersected by PH2 and PH3. The remaining boreholes intersected dolerite of varying thicknesses and at differing elevations, aimed at refining the geotechnical model as well as the gravimetric survey results. A revision of the gravity survey results was carried out by the geophysicist, to account for the

bedrock depths encountered in the boreholes. Overall, although the presence of dolerite influenced the gravity readings to a great degree, a fair reflection of the variances in bedrock is represented by the final gravity results.

6.3 LABORATORY TEST RESULTS

Representative samples were collected from the test pits and submitted to Specialised Testing (ST) Laboratory in Pretoria for testing. The results are summarised in TABLE 5 at the end of the report. A condensed version of the results is presented below and discussed. The detailed laboratory results are included in Appendix D.

6.3.1 HILLWASH

Hillwash has been described in the profiles as sandy to silty clay. The results of testing on hillwash samples from the western portion of the site indicate the following:

- The material consists mainly of sand (ranges from 24% to 38%), silt (ranges from 28% to 31%) and clay (ranges from 32% to 37%).
- The average GM is 0,51.
- The Plasticity Index (PI) ranges between 18% and 19% with a low potential expansiveness [8].
- The unified soil class indicates that hillwash classifies as a clay of low plasticity (CL).
- Consolidation tests on the hillwash predict settlements of 10 mm/m, 15 mm/m and 39 mm/m at 100 kPa, 150 kPa and 250 kPa bearing pressures should foundations be placed on top of the hillwash horizon.
- A collapse potential of 0.4 % was achieved in testing, which indicates that “no problem” with respect to collapsible fabric exists within the hillwash horizon [9].

6.3.2 PEDOGENIC SOILS

One instance of nodular ferricrete was tested for its potential use in layerworks. Grading results indicate it comprises a slightly clayey silty sandy gravel with a PI of 16%, a low potential expansiveness, GM of 1,98 and classifies as a clayey gravel (GC) according to the USCS. Compaction results indicate that crushing of the ferricrete nodules occurs under compaction and strength is then lost, such that the material qualifies as poorer than G9 quality material [10] and can only be used as low-grade general fill. This is not suitable for use in layerworks.

6.3.3 REWORKED RESIDUAL DOLERITE

Reworked residual dolerite has been described in the profiles as clayey sandy silts to gravelly clayey sand. The results of testing on topsoil samples indicate the following:

- The material consists mainly of gravel (ranges from 6% to 32%), sand (ranges from 26% to 29%), silt (ranges from 20% to 38%) and clay (ranges from 19% to 30%).
- The average GM is 0,86 with a range between 0,53 and 1,29 depending on degree of pedogenic cementation. Soils with GM<1 is typically dominated by fines particles and do not compact well.
- The Plasticity Index (PI) ranges between 20% and 22% with liquid limits of between 40% and 44%, which results in a low to medium potential expansiveness.

- The unified soil class indicates that the reworked residual dolerite classifies as a clay of low to intermediate plasticity (CL-CI) and occasionally a clayey sand (SC).
- A slightly acidic pH of 5,7 and electric conductivity of 0,022 S/m indicate that the soils will likely be corrosive towards steel [11] and mildly aggressive towards concrete [12].

6.3.4 RESIDUAL DOLERITE

Residual dolerite has been described in the profiles as a silty sand to sandy silt. The results of testing on topsoil samples indicate the following:

- The material consists mainly of sand (ranges from 29% to 67%), silt (ranges from 23% to 44%) and clay (ranges from 5% to 25%).
- The average GM is 0,67 with a range between 0,33 and 0,98. Soils with $GM < 1$ is typically dominated by fines particles and do not compact well.
- The Plasticity Index (PI) ranges between 8% and 24% with liquid limits of between 31% and 54%, which results in a low to medium potential expansiveness.
- The unified soil class indicates that the fines portion of the residual dolerite classifies as a clay of high plasticity (CH) and occasionally a clayey sand (SC).
- A slightly acidic pH of 5,5 and electric conductivity of 0,022 S/m indicate that the soils will likely be corrosive towards steel and mildly aggressive towards concrete.

6.3.5 REWORKED RESIDUAL DOLOMITE

6.3.5.1 Chert Dominated Residuum

The chert dominated residuum has been described in the profiles as a silty sandy fine to course gravel and cobbles. The results of testing on the samples indicate the following:

- The material consists mainly of gravel (ranges from 72% to 73%), sand (ranges from 13% to 15%), silt (ranges from 8% to 9%) and clay (typically 5%).
- The average GM is 2,35.
- The Plasticity Index (PI) ranges between 12% and 15% with a low potential expansiveness.
- The unified soil class indicates that chert residuum classifies as a clayey gravel (GC).
- Compaction results on this material yields an average Maximum Dry Density (MDD) of 2046 kg/m³ at an Optimum Moisture Content (OMC) of 9%. This horizon qualifies for use as a G6 quality material.
- The above parameters notably include one mixed sample of the pebble marker with the chert residuum below appear to have not significantly influenced the properties.

6.3.5.2 Fines Dominated Residuum

Fines dominated residuum has been described in the profiles as a clayey silty sand to a clayey sandy silt. The results of testing on the samples indicate the following:

- The material consists mainly of gravel (ranges from 17% to 21%), sand (ranges from 36% to 40%), silt (ranges from 17% to 23%) and clay (ranges from 22% to 24%).
- The average GM is 1.0 with a range of 0,95 to 1,06.

- The Plasticity Index (PI) ranges between 13% and 14% with a low potential expansiveness.
- The unified soil class indicates that fines dominated residuum classifies as a clayey sand (SC) and clay of low (CL) plasticity.

7. GEOTECHNICAL ASSESSMENT

7.1 GENERAL

The site has been divided into two main geotechnical zones as indicated in the geotechnical plan (Figure 5) included at the end of this report. This is largely based on the potential for differing soil conditions linked to prevailing lithology of surface soils split east from west.

7.1.1 ZONE A

The typical soil profile encountered within this zone is comprised primarily of transported soils to a depth of 1,5 m on average, which are underlain by reworked and residual dolerite soils to a depth of 3 m below surface. Laboratory results indicate that the reworked residual dolerite soils are likely to pose a medium potential for expansiveness. Therefore, the design of any structure to be founded in this portion of the site will need to cater or mitigate heave in the order of 20 mm to 25 mm. Bearing pressures of the in-situ soil profile can be regarded as follows:

- 50 kPa can be achieved within the hillwash, pebble marker and nodular ferricrete within this zone at an average depth of 0,5 m which will vary in some instances, particularly in the central portions of this zone viz. TP2, TP3 and TP4, where this is achieved slightly deeper.
- 100 kPa can be achieved within the hillwash, pebble marker and nodular ferricrete within this zone at an average depth of 0,8 m which will vary in some instances, particularly in the central portions of this zone viz. TP2, TP3 and TP4, where this is achieved slightly deeper.
- 150 kPa can be achieved within the residual and reworked residual dolerite soils within this zone at an average depth of 1,2 m which will vary in some instances, particularly in the central portions of this zone viz. TP4, where this is achieved at roughly 2 m from surface.

7.1.2 ZONE B

The typical soil profile encountered within this zone is comprised primarily of transported soils to a depth of 0,5 m on average below which chert residuum, reworked and residual dolomite soils are encountered to a depth of 3 m below surface, except for TP10 which encountered shallow dolomite rock. Bearing pressures of the in-situ soil profile can be regarded as follows:

- 50 - 100 kPa can be achieved in the chert residuum and reworked residual dolerite within this zone at an average depth of 0,5 m, which will vary in some instances, particularly in the south-eastern portions of this zone viz. TP8. 100 kPa is achievable at significantly greater depth on account of a thick hillwash layer of lower consistency which extends to 1,6 m depth. Ground improvement may be considered in these areas.
- 150 kPa can be achieved in the chert residuum and reworked residual dolerite within this zone at an average depth of 0,8 m which will vary in some instances, particularly in the central portions of this zone viz. TP8. This bearing pressure is achieved significantly deeper on account of a thick

hillwash layer of lower consistency which extends to 1,6 m depth. Ground improvement may be considered in these areas.

7.2 SEISMICITY

Seismic activity in the vicinity of the site is indicated to be significant to the development. The seismic hazard map of South Africa [13] was consulted in order to obtain the peak ground acceleration of the area of interest. In general, the Gauteng Province ranges are medium to high in terms of seismic activity.

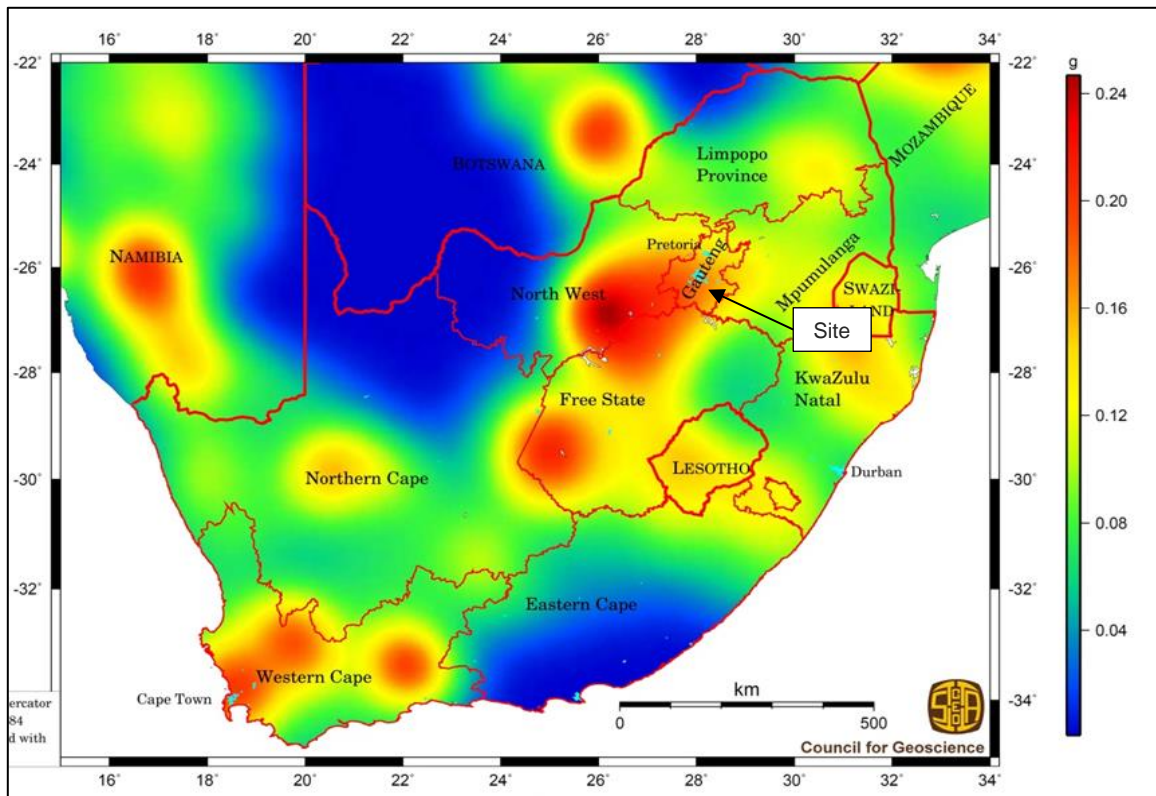


Figure 1: Peak ground acceleration (g) with 10% probability for being exceeded in a 50 year return period.

According to Kijko, the peak ground acceleration for the area is approximately 0,16 m/s², with a 10% probability of being exceeded in a 50-year period. The seismic activity in the area is therefore considered to be medium to high. Appropriate design considerations may be found in SANS 10160-4 [14].

7.3 AREAS SUBJECT TO FLOODING

Although no evidence of ponding water was encountered during the investigation, special attention should be paid to the areas in which cut to fill operations are envisaged. This to ensure good drainage around newly constructed terraces. Both the 50- and 100-year flood lines must be determined for the site by an appropriately qualified and experienced engineer. This information must then be incorporated in the design.

7.4 SLOPE STABILITY

It is likely that the creation of terraces during construction will result in the creation of slopes, both cut and fill. Special advice should be sought with respect to ensuring that these are stable. No indication of the presence of unstable natural slopes was found during the investigation.

7.5 EXCAVATABILITY

The excavation characteristics of the different soil horizons have been evaluated according to the South African Bureau of Standards standardised excavation classification for earthworks [15] and earthworks (small works – SABS 1200DA). In terms of this classification and the in-situ soil/rock consistencies as profiled, the relationships given below are generally applicable. For specific excavation requirements refer to the soil profiles in Appendix B.

- “soft excavation” - very loose/very soft through to dense or stiff.
- “intermediate excavation” - very dense/very stiff through to very soft rock.
- “hard excavation” - soft rock or better.

Across the site, soils are classified as “soft excavation” to an average depth of 2,8 m, below which “intermediate excavation” is expected, and power tools may be required to excavate further. In some instances, hard excavation may be expected in the areas surrounding test pits TP7, TP10 and TP11, where shallow rock is encountered at depths of 2,7 m, 1,9 m and 2,6 m respectively. Sporadic instances of shallower rock may be encountered, particularly near the north-eastern corner of the site within the weathered dolomite profile.

8. DOLOMITE STABILITY ASSESSMENT

8.1 GENERAL

The results of the boreholes were used during this study to assess the general dolomite stability of the site. The dolomite risk characterisation of the boreholes was done in accordance with the method proposed by Buttrick *et. al* [16]. The risk classification of the borehole profiles is summarised in TABLE 4 at the back of this report.

According to the methodology of scenario supposition, the conditions in each borehole must be evaluated in terms of a non-dewatering as well as a dewatering scenario. The method evaluates the stability of an area by investigating the presence of receptacles in the dolomite profile, depth to potential receptacles, maximum sinkhole development space, the nature and mobilisation potential of the blanket material and the presence of mobilising agents.

The factors influencing the stability of the area are briefly defined as follows:

(a) Blanketing Layer

The blanket layer (dolomitic overburden) comprises all the materials occurring between the ground surface and the dolomitic bedrock surface. The term blanket layer is defined here as the component of the dolomitic overburden that overlies the potential receptacles.

(b) Receptacles

Receptacles in the dolomite profile may occur either as small disseminated and interconnected openings in the overburden or as substantial openings (especially where wad is present). Receptacles also occur as substantial openings (cavities) in the bedrock. Both types of opening may be able to receive mobilised (transported) materials from overlying horizons.

(c) Mobilisation and Mobilising Agents

Mobilisation is defined as the movement of dolomite overburden by subsurface erosion. Mobilising agents include ingress of water, ground vibrations, water level drawdown or any process that can include mobilisation of the material in the blanket layer under the force of gravity.

(d) Maximum Potential Development Space

This is a simplified estimation of the maximum size sinkhole that can be expected to develop in a particular profile, provided that the available space is fully exploited by a mobilising agent. The available space depends on the depth below ground surface to the throat of a receptacle or disseminated receptacle and the 'angle of draw' in the various blanket materials.

8.2 HAZARD CLASSIFICATION

(a) Blanketing Layer

The blanketing layer comprises transported soils (viz. fill; hillwash; topsoil; pebble marker), residual dolerite with varying percentages of chert residuum and residual dolomite, dolerite rock and dolomite rock.

The transported soils are confined to the upper 3m, generally not exceeding 2,5 m. Shallow geotechnical investigation test pits provide more detailed information, as described in Section 6.1 above, to include topsoil and hillwash. The penetration rates of the fill/talus horizon are generally between 14 sec/m and 49 sec/m in the medium hard chert fragments and reddish brown clayey sandy silt. This horizon has a high to medium mobilisation potential.

The residual dolerite is encountered in all but two boreholes (namely PH1 and PH10) generally at shallow depths (within 15 m below natural ground level). The residual dolerite often contains friable zones with typical penetration rates of less than 15 sec/m. However, the typical range for the residual dolerite is 26 sec/m to 56 sec/m with medium mobilisation potential. The residual dolerite horizons are described as orange-brown clayey silt/ sandy silt to reddish brown silty clay with varying proportions of very soft to soft rock bands of dolerite, primarily near the base of the olive brown silty sand/sandy silt.

Residual dolomite sporadically also occurs within the residual dolerite and near the dolomite bedrock interface with typical penetration rates of 8sec/m to 18sec/m, with a medium to high mobilisation potential.

Dolerite rock is present in all the boreholes except for PH1 where dolomite was found at shallow depths. The dolerite is in various stages of weathering. The highly weathered dolerite is recorded within 25 m below ground level with typical penetration rates of 23 sec/m to 1 min 24 sec/m but

is most commonly 25 sec/m to 40 sec/m. The highly weathered dolerite is described as light brown to olive brown speckled black, very soft rock to soft rock. The dolerite rock is considered to have a low mobilisation potential.

Slightly weathered hard rock dolerite has penetration rates in excess of 3 min/m with low mobilisation potential. The weathering and interpreted rock strength from the chips do not indicate higher penetration rates, as expected.

Moderately to slightly weathered dolomite rock is encountered in most of the boreholes at varying depths, typically enclosed with dolerite. This dolomite has penetration rates from 34 sec/m to 1 min 54 sec/m, the lower penetration rates are limited to the occurrences of dolomite near the inferred bedrock level. The weathered dolomite rock has low mobilisation potential.

Sound dolomite bedrock is encountered in all but four boreholes, viz. PH4, PH5, PH6 and PH9, described as slightly weathered to unweathered, hard to very hard rock dolomite with penetration rates exceeding 3min/m, with low mobilisation potential.

The two boreholes that were terminated in dolerite and not dolomite bedrock, were terminated with at least 6m of unweathered dolerite rock with penetration rates exceeding 3 min 20 sec/m up to 4 min 20 sec/m. The dolerite rock is described as low mobilisation potential.

The nature of the blanketing layer is predominantly non-dolomitic with extensive dolerite, as residual soils and rock. Dolomite rock is encountered within the profile, with only one occurrence of no dolerite intersected in borehole PH1. Shallow dolomite bedrock was intersected and correlate to a gravity high anomaly in the north-eastern corner where it was drilled.

(b) Receptacles

No receptacles were encountered during the drilling investigation. Only one instance of significant air/sample loss recorded in the shallow portion of the profile.

For the purpose of the risk assessment, it should in any event be assumed that receptacles do occur within the upper portion of the dolomitic bedrock and/or disseminated zones in the residual dolomite, irrespective whether these were encountered by drilling or not.

(c) Mobilisation Agents

In an urbanised/agricultural area it should be assumed that a mobilising agent is always present in the form of leaking wet services, ponding of surface water and ground vibrations. In the area where the water table is deep, a thick portion of the blanketing layer is exposed to mobilisation than the areas with a shallow water table. If the water table is ever allowed to be drawn down substantially, weak and/or erodible horizons in the dolomitic profile could be exposed to erosion and the formation of sinkholes and subsidence's will thus be enhanced.

The site is underlain by dolerite which adds to the protection of any potential weak zones due to its less permeable and erodible nature.

A storm water pipeline has been inferred from public GIS data and may serve as a potential source for concentrated water ingress. This may at times be exacerbated by excessive groundwater abstraction via the long-standing domestic borehole already present on the site.

(d) Maximum Potential Development Space of Sinkhole and Subsidence Formation

Subsidence's usually form where compaction of highly compressible material takes place (often associated with the gradual lowering of the groundwater level), or where the receptacle has limited available space, or where the potential sinkhole formation process is halted due to remedial measures taken in time.

According to the drilling results, the potential for subsidence formation across the site is low due to the relatively thick overlying dolerite profile and absence of weak and erodible zones.

The water table is generally situated within the residual dolerite or dolerite rock horizons and above the bedrock level. The risk potential for sinkhole formation will therefore increase in conjunction with groundwater drawdown. These are indicated for each borehole for water ingress and a groundwater level drawdown scenario in TABLE 4.

(e) Hazard Class

The boreholes drilled were classified in terms of the eight different Inherent Hazard Classification (IHC) classes proposed in the method for dolomite land hazard and risk assessment in South Africa (refer to TABLE 4). Figure 6 shows the Hazard Zonation of the site.

The majority of boreholes that were drilled on site have a low to medium potential for the formation of small to medium sized sinkholes and subsidence's (IHC 2/3). Therefore, an overall zonation of IHC 3 is recommended for the entire site in the drawdown and water ingress scenario, excluding the north-eastern corner of the site. Conditions within north-eastern corner of the site indicate a high potential for small sized sinkholes to develop (IHC 5) for water ingress and the water drawdown scenario. According to SANS 1936-1, the site is classified to have a D3 + FPI area designation when considering C3, C5 or C6 land usage.

9. CONCLUSIONS AND RECOMMENDATIONS

The 8ha site is proposed for commercial development of three blocks, comprising double volume warehouses with a first storey office. Shallow and deep soils investigations were carried out at the site, viz. geotechnical and dolomite stability investigation.

Twelve test pits were excavated, and eleven percussion drilled boreholes were drilled to investigate the site. The site is underlain by dolomite, which has been intruded extensively by dolerite., providing a protective horizon above the dolomite bedrock for the majority of the site.

The test pits indicate thin overlying variable transported soils above the residual dolerite as fill, topsoil, hillwash and a pebble marker horizon.

The following recommendations are made according to the results of the test pit investigation and laboratory testing and it is assumed that these conditions are uniform between the test pits. The following recommendations are provided:

- Special foundation precautionary measures are recommended due to the medium potential for expansiveness and the dolomite classification. Total potential heave for the thick residual dolerite soil profiles encountered varies between 20 mm to 25 mm.

- Founding options for the structures include either stiffened or cellular rafts, or split construction with proper site drainage and plumbing requirements. Since the site must adhere to dolomite requirements in terms of SANS1936-3, drainage and plumbing precautions will in any event be required. Given the dolomite requirement stipulated SANS 1936-3 that 5m loss of support must be accommodated, raft foundation solution is proposed. This will override any small geotechnical problems that might be associated with the site. Structure specific geotechnical conditions are to be confirmed in the design level geotechnical investigation which will include footprint drilling for the relevant structures.
- Old trenches are to be excavated, backfilled and recompacted to limit any differential settlements of foundations at these positions.

As discussed above the site is considered to fall within an IHC3 category, indicating that there is a medium potential for medium subsidence's to occur in the event that concentrated water ingress into the sub-strata takes place after dewatering occurs. The dolomite area designation is thus D3+FPI for this type of development. The FPI portion of the designation requires that design level drilling and test pitting is conducted. D3 measures as outlined in SANS1936-3 should be adopted with respect to the prevention of water ingress into the soil. In this respect care should be taken to landscape the site so that water does not pond and all service trenches must be properly backfilled.

The following measures must be implemented on this site:

- All stormwater should be effectively captured and led off the site.
- A one metre wide concrete apron should be constructed around buildings which is designed to shed stormwater away from the structure.
- No ponding of water should be allowed in this area, both during and after construction.
- All courses in the plinth wall should be reinforced with brickforce. Brickforce should be incorporated in every fourth course thereafter and in at least three courses above all openings such as doors and windows.
- Construction joints, to allow relative movement, should be incorporated at intervals of not more than 5 m in linear walls or at points to be determined by the structural engineer.
- All yard walls, steps and similar structures should be isolated from the main structure.
- Flexibility should be incorporated into wet services where they enter or leave buildings in order to ensure that relative movement does not result in leaking pipes.

It must be borne in mind that the development must fulfil the general precautionary measures prescribed by SANS1936-3 for a D3 site. A Dolomite Risk Management Plan (DRMP) must be created for the development and construction supervision of foundations must be carried out by a competent person (engineer or geo-professional).

Furthermore, it is recommended that a hydrogeologist be approached to determine the sustainable yield of the existing domestic borehole on site, assuming this information does not already exist. This will be required to assess the potential for continued use of the borehole on site, which can later be used in monitoring of the water level below the site.

From a geotechnical perspective, the site is considered to be economically and practically developable provided that the recommendations stated above, are adhered to.

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11. CERTIFICATION

This report was prepared and reviewed by the undersigned.

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TABLE 3:
Summary of Test Pit Profiles

Test Pit No.	Total depth (m)	Refusal	Thickness of Layers m) – (m)											Geotech Zone	
			Transported Soil				Pedogenic Nodular/ *Honeycomb/ **Hardpan Ferricrete	Residual Soil				Bedrock			
			Fill	Topsoil	Hillwash	Pebble Marker		Reworked Residual Dolerite	Reworked Residual Dolomite	Residual Dolerite	Transitional Material	Chert Residuum	Chert/ *Dolomite		Very Soft Rock Dolerite
TP1	3	-	-	0.0 - 0.4	0.4 - 1.4	1.4 - 2.1	-	2.1 - 3.0+	-	-	-	-	-	-	A
TP2	3	NR	0.0 - 0.5	-	0.5 - 1.4	-	2.7 - 3.0+ NR	1.4 - 2.7	-	-	-	-	-	-	A
TP3	3.1	-	0.0 - 0.8	-	0.8 - 1.3	1.3 - 1.6	-	1.6 - 1.9	-	1.9 - 3.1+	-	-	-	-	A
TP4	3.2	-	-	-	0.0 - 0.7	0.7 - 1.3	-	1.3 - 3.2+	-	-	-	-	-	-	A
TP5	3.2	R	0.0 - 0.2	-	1.0 - 1.4	-	0.2 - 1.0	1.4 - 1.6	-	1.6 - 2.7	-	-	-	2.7 - 3.2+ R	A
TP6	2.5	R	0.0 - 0.3	-	0.3 - 0.8	-	-	0.8 - 1.5	-	1.5 - 2.5	-	-	-	2.5+ R	A
TP7	2.7	R	0.0 - 0.15	-	-	-	-	0.7 - 1.8	2.2 - 2.7	1.8 - 2.2	-	0.15 - 0.7	2.7+ R	-	A
TP8	3.2	-	0.0 - 0.15	-	0.15 - 0.8	0.8 - 1.6	-	-	2.2 - 3.2+	-	-	1.6 - 2.2	-	-	B
TP9	3.2	-	0.0 - 0.4	-	-	-	-	-	2.8 - 3.2+	-	-	0.4 - 2.8	-	-	B
TP10	1.9	R	0.0 - 0.1	-	-	-	-	-	-	-	-	0.1 - 1.9	*1.9+ R	-	B
TP11	2.6	R	0.0 - 0.1	-	0.1 - 0.7	-	-	-	-	-	-	0.7 - 2.6	2.6+ R	-	B
TP12	3.1	-	0.0 - 0.15	-	-	-	-	2.1 - 2.9	-	2.9 - 3.1+	0.9 - 2.1	0.15 - 0.9	-	-	B

Notes: NR - Near Refusal
R - Refusal
T- Terminated by geologist

TABLE 4:
Summary of Percussion Borehole Profiles

BH No.	Coordinates (WGS84 Lo29)		Collar Elevation (masl)	Inclination	Borehole Depth (m)	SWL (masl)	BLANKET LAYER DEPTH (m) - (m)							Bedrock		Air and/or Sample Loss	Depth to Dolomite Bedrock	Water Strike	Water Rest Level Depth (m)	Hazard Assessment for Sinkhole and Subsidence Development								Hazard Zone
							Transported Soils		Residual Soils											Dolerite	Weathered dolomite or dolomite	Sinkholes				Subsidence Formation	Inherent Hazard Class (IHC)	
	Fill	Hillwash					Residual Dolerite	Residual Shale	Chert dominated residuum	Fines dominated residuum	Wad	Max Size		Hazard								Water Ingress	Groundwater Drawdown	Water Ingress	Groundwater Drawdown			
												Water Ingress	Groundwater Drawdown	Water Ingress	Groundwater Drawdown					Water Ingress	Groundwater Drawdown							
PH1	76095.86	2916707.48	1546.8	Vertical	12	1535.2	0.0 - 0.5					0.5 - 3.0			3.0 - 12.0	8.0 - 11.0	3	-	Dry	Small	Small	High	High	Medium	5	5	A	
PH2	76279.25	2916772.05	1543.9	Vertical	25	1535	0.0 - 1.0	1.0 - 2.0	2.0 - 7.5			7.5 - 11.0		11.0 - 18.5 23.0 - 25.0	18.5 - 23.0		18.5	8.1	9.6	Medium	Medium	Medium	Medium	Medium	3	3	B	
PH3	76190.88	2916811.75	1546.1	Vertical	22	1534.8			0.0 - 11.0			15.5 - 16.0		11.0 - 15.0	16.0 - 22.0		16	13.5	11.6	Small	Medium	Medium	Medium	Low	2	3	B	
PH4	76273.02	2916880.06	1543.2	Vertical	20	1534.6	0.0 - 1.0		3.0 - 8.0		1.0 - 3.0		8.0 - 9.0 10.0 - 13.0 14.0 - 20.0	9.0 - 10.0 13.0 - 14.0		13	-	10.9	Small	Small	Medium	Medium	Medium	2	2	B		
PH5	76116	2916797.02	1546.9	Vertical	28	1534.9			9.0 - 16.0		1.0 - 2.0	0.0 - 1.0 2.0 - 9.0		16.0 - 18.0 21.5 - 28.0	18.0 - 21.5		18	11	10.2	Small	Medium	Medium	Low to Medium	Medium	2	2/3	B	
PH6	76206.66	2916709.25	1545.7	Vertical	24	1535.2	0.0 - 1.0		3.5 - 11.0 14.0 - 15.0	15.0 - 18.0	1.0 - 3.5		11.0 - 14.0 18.0 - 24.0			>24	14	11.2	Small	Small	Low	Low to Medium	Medium	1	1/2	B		
PH7	76222.32	2916845.85	1545	Vertical	20	1534.7	0.0 - 0.5	0.5 - 1.0	2.0 - 8.0			1.0 - 2.0	8.0 - 11.0 13.0 - 20.0	11.0 - 13.0		11	11	10.6	Small	Small	Medium	Medium	Low	2	2	B		
PH8	76370.48	2916706.89	1541.9	Vertical	23	1535.2	0.0 - 0.3	0.3 - 1.0	1.0 - 16.5				16.5 - 18.0	18.0 - 23.0		18	14.9	10.3	Small	Medium	Medium	Medium	Low	2	3	B		
PH9	76377.64	2916861.72	1540.9	Vertical	21	1534.6		0.0 - 1.0	1.0 - 15.0				15.0 - 21.0			>21	-	12.2	Small	Small	Low	Low	Low	1	1	B		
PH10	76109.91	2916881.86	1547.4	Vertical	26	1534.6	0.0 - 2.0					2.0 - 4.0 14.0 - 16.0	4.0 - 7.0	7.0 - 14.0 16.0 - 25.0	25.0 - 26.0		25	14.3	10.8	Medium	Medium	Medium	Medium	Medium	3	3	B	
PH11	76352.87	2916780.35	1541.4	Vertical	21	1534.9	0.0 - 0.5	0.5 - 2.5	2.5 - 13.5				13.5 - 14.0 16.0 - 18.0	14.0 - 16.0 18.0 - 21.0		14	-	9.9	Small	Medium	Low to Medium	Medium	Low	2	3	B		

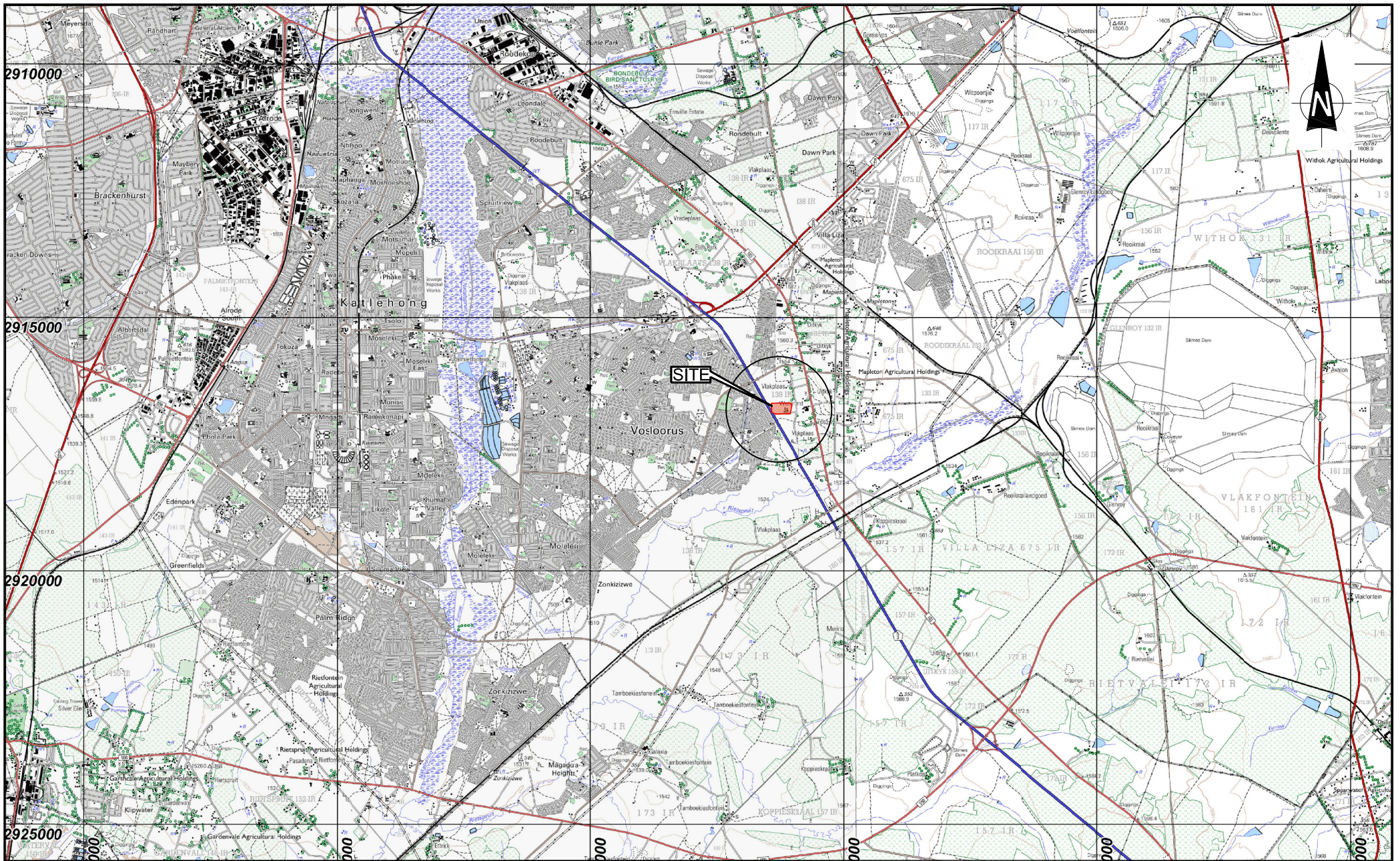
Notes: SWL – Static Water Level
Masl – Meters above sea level

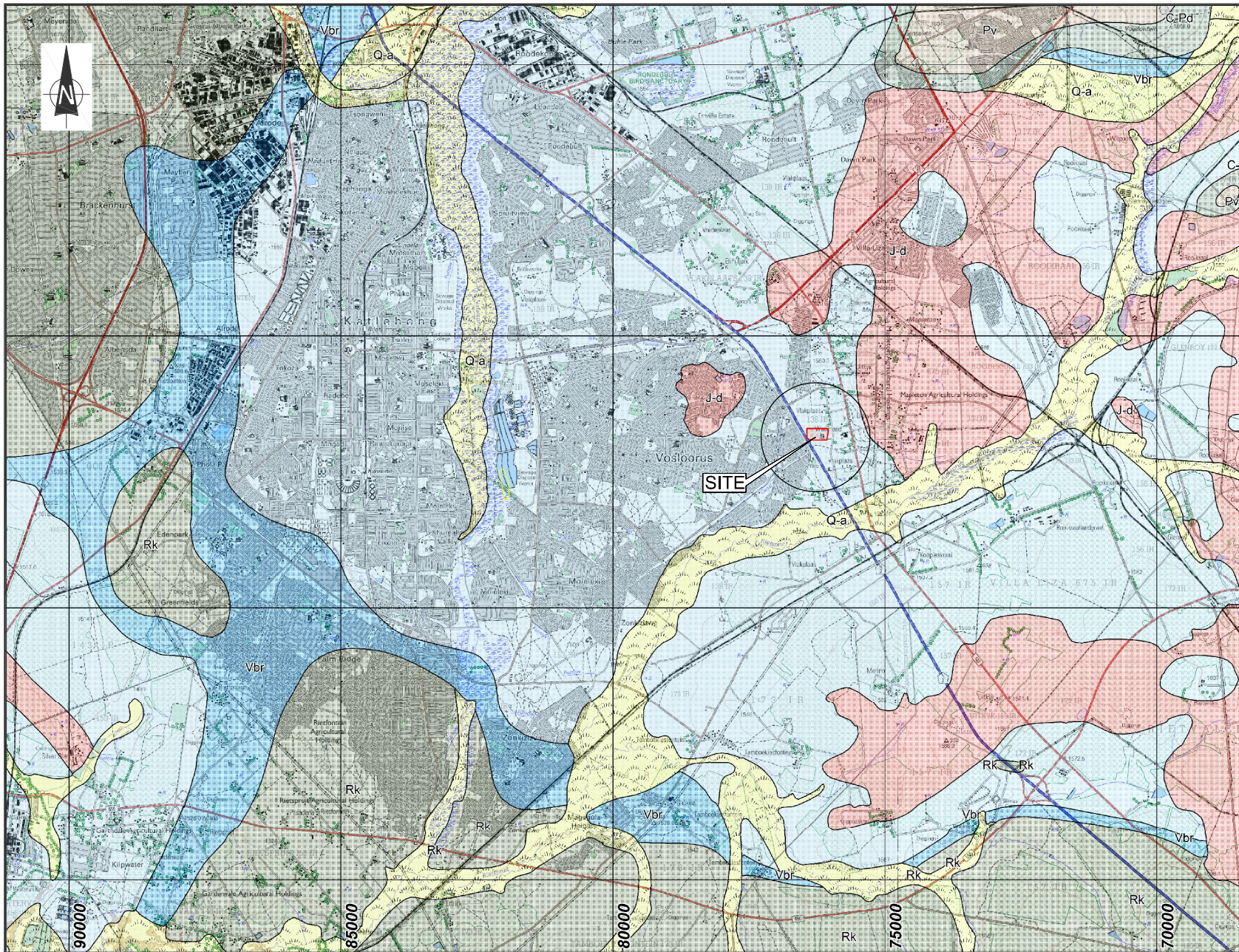
TABLE 5:
Summary of Laboratory Test Results

Sample		Grading %				Atterberg Limits %			GM	PE	USC	pH	Ec (S/m)	CBR (Modified AASHTO) TMH1 A7							Maximum Dry Density (kg/m ³) @ OMC (%)	COLTO Classification	Material Description			
No.	Depth (m-m)	Gravel	Sand	Silt	Clay	LL	PI	LS						% Swell	90%	93%	95%	97%	98%	100%						
TP1/1	2.1 - 3.0	15	28	38	19	42	22	11	0.75	Medium	CL	5.7	0.022													Reworked Residual Dolerite
TP1/2	1.0 - 1.4	2	38	28	32	35	18	9.5	0.51	Low	CL															Hillwash
TP2/1	1.0 - 1.4	8	24	31	37	41	19	9.5	0.51	Low	CL															Hillwash
TP2/2	1.4 - 2.7	58	18	10	14	38	16	8.5	1.98	Low	GC			0.1	13	16	19	21	22	24	1805 @ 14.9	<G9			Nodular Ferricrete	
TP3/1	1.9 - 3.1	5	67	23	5	31	8	4	0.98	Low	SC	5.5	0.022													Residual Dolerite
TP4/1	1.3 - 3.2	32	26	20	22	40	20	10	1.29	Low	SC															Reworked Residual Dolerite
TP5/1	1.6 - 2.7	2	29	44	25	54	24	11	0.33	Medium	CH															Residual Dolerite
TP6/1	0.85 - 1.5	6	29	35	30	44	20	11	0.53	Medium	CI															Reworked Residual Dolerite
TP8/1	2.2 - 3.2	17	36	23	24	29	14	7	0.95	Low	CL															Reworked Residual Dolomite
TP8/2	0.8 - 2.2	72	15	8	5	27	12	6	2.35	Low	GC			0	20	24	26	30	31	35	2079 @ 8.6	G6			Reworked Residual Dolomite	
TP9/1	2.8 - 3.2	21	40	17	22	31	13	6.5	1.06	Low	SC															Reworked Residual Dolomite
TP11/1	0.7 - 2.6	73	13	9	5	31	15	7.5	2.34	Low	GC	5.8	0.011	0.1	14	21	27	33	36	42	2012 @ 9.6	G6			Reworked Residual Dolomite	

Notes:

LL – Liquid Limit	PE – Potential Expansiveness	Ec – Electric Conductivity
PI – Plasticity Index	USC – Unified Soil Classification	CBR – California Bearing Ratio
LS – Linear Shrinkage	MDD – Maximum Dry Density	
GM – Grading Modulus	OMC – Optimum Moisture Content	





- Legend**
- Site Boundary
 - Regional Geology**
 - River Sand (Q25)
 - Alluvium (Q-a)
 - Mokolian Syenite (M4)
 - 9005 Diabase (V-di)
 - 0891 Karoo Dolerite Suite (J-d)
 - 0694 Klapperkop Quartzite Member (Vkp)
 - 0571 Vryheid Formation (Pv)
 - 0284 Hekpoort Formation (Vh)
 - 0282 Timeball Hill Formation (Vti)
 - 0281 Rooihoogte Formation Chert (Vrh)
 - 0273 Black Reef Formation (Vbr)
 - 0236 Alberton Formation (Ral)
 - 0235 Westonaria Formation (Rwe)
 - 0226 Booyens Formation (Rbo)
 - 0095 Malmani Dolomite (Vma)
 - 0075 Dwyka Group Tillite
 - 0030 Klipriviersberg Lava (RK)

REFERENCE MAPS
 1:250 000 SCALE
 GEOLOGICAL SERIES
 SHEETS:2628 EAST RAND
 NOT TO ORIGINAL SCALE



**RICHBAY CHEMICALS NEW PLANT, VOSLOORUS
 GEOTECHNICAL INVESTIGATION**

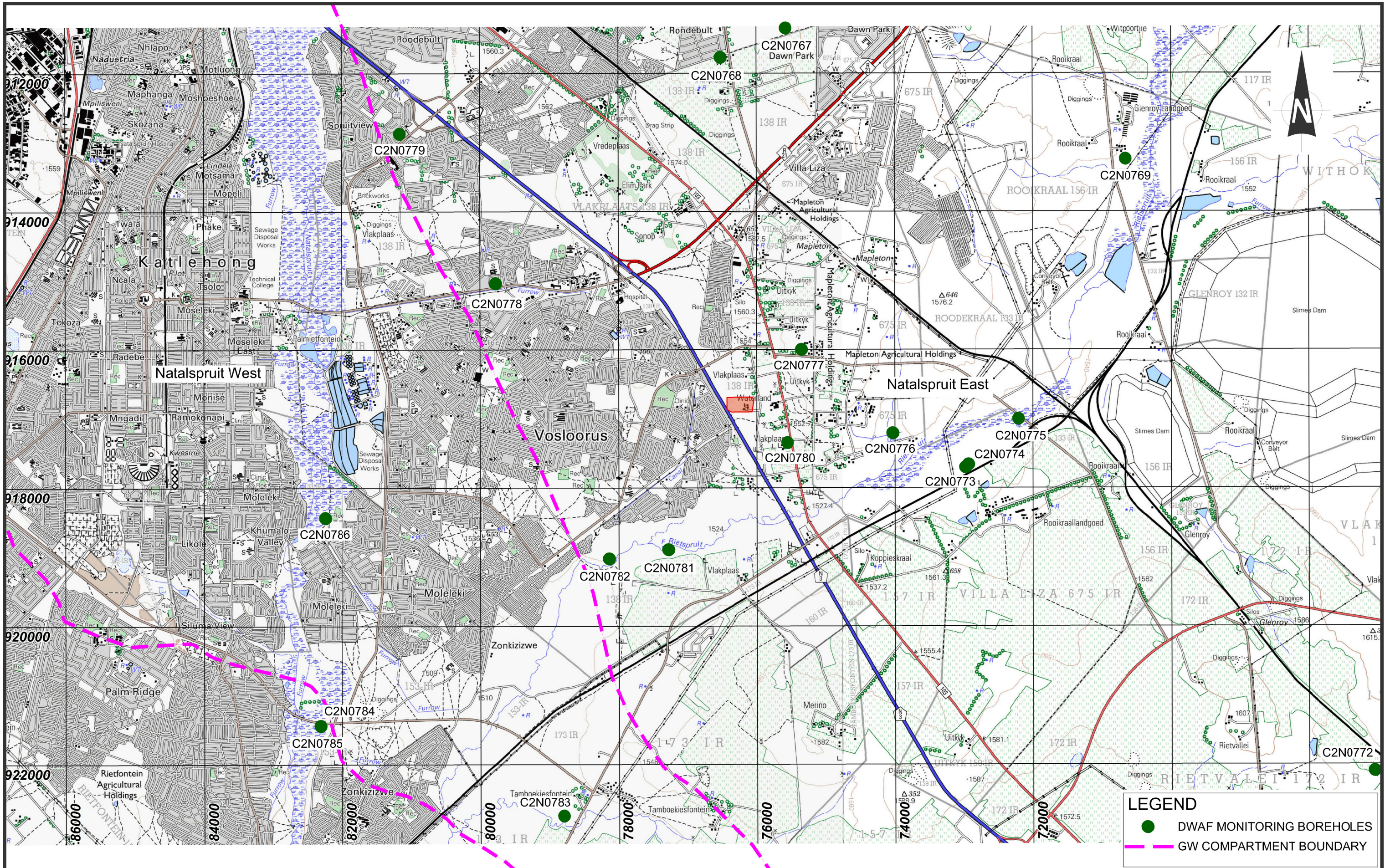
GEOLOGY MAP

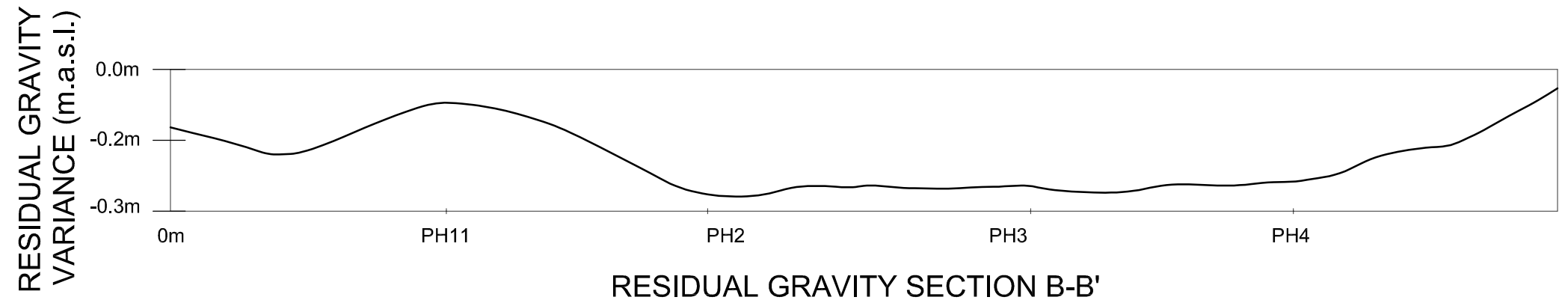
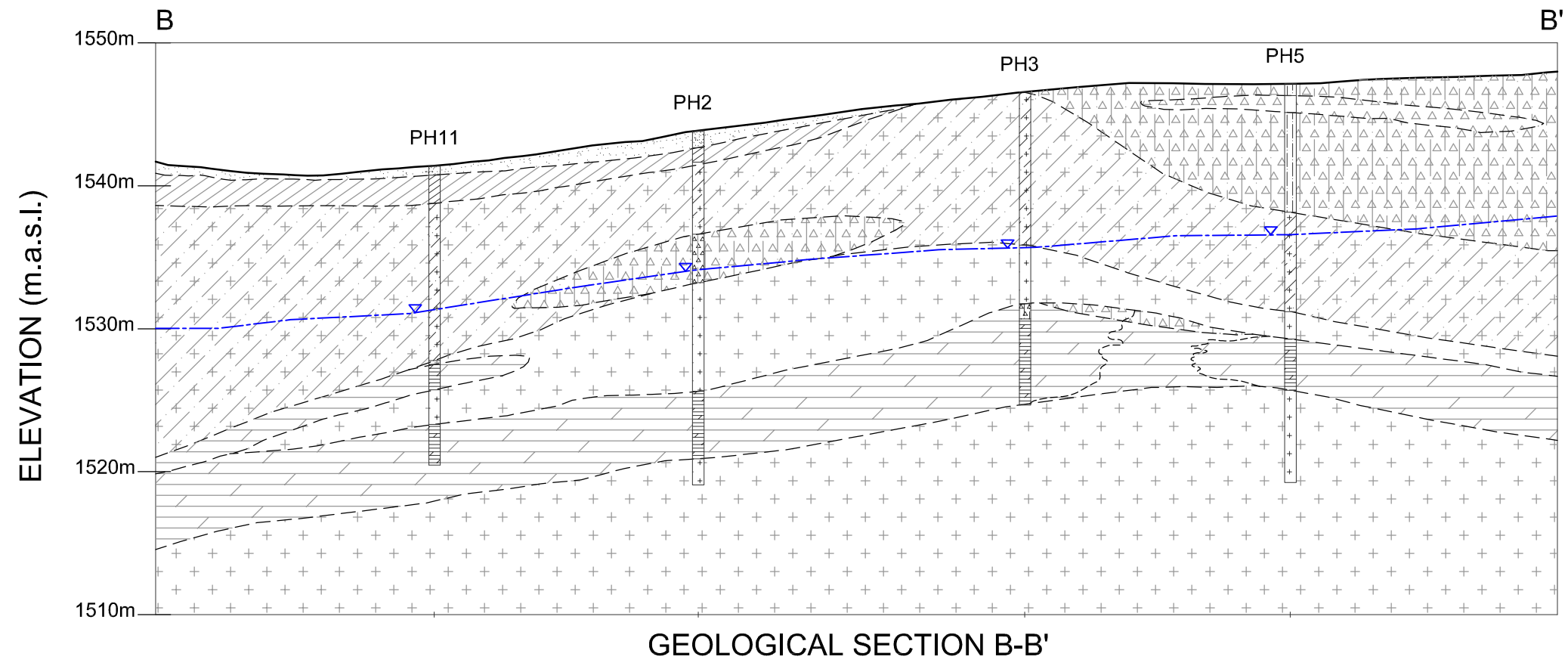
PROJECT NO: 31100288/72

FIGURE NO 2

SCALE N.T.S.

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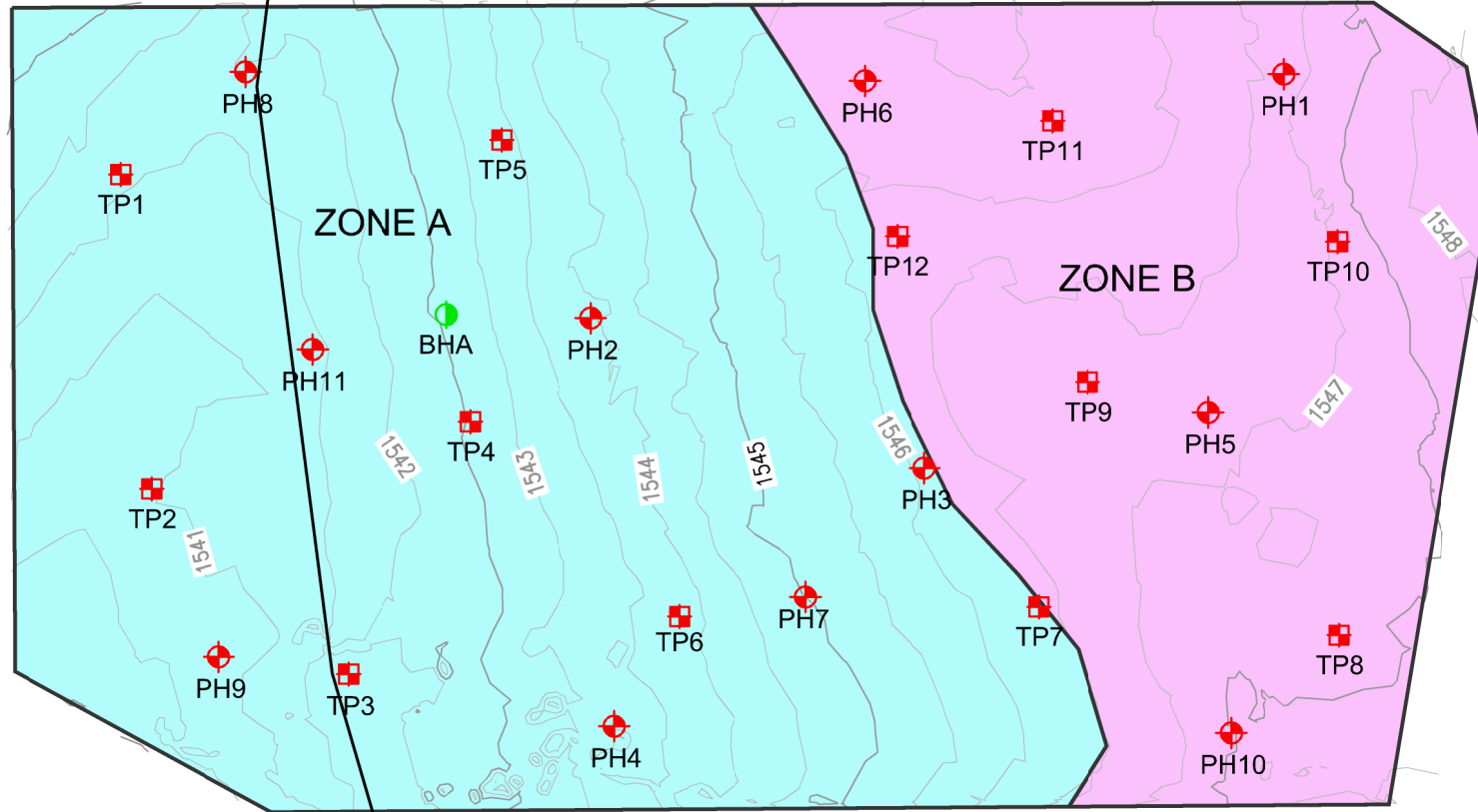




LEGEND:




- GEOLOGICAL BOUNDARY
- - - - - WATER REST LEVEL
- [Pattern: Dotted] TOPSOIL
- [Pattern: Diagonal lines] HILLWASH RESIDUAL DOLERITE
- [Pattern: Crosses] DOLERITE
- [Pattern: Horizontal lines] DOLOMITE
- [Pattern: Triangles] FINES DOMINATED RESIDUUM
- [Pattern: Small triangles] CHERT RESIDUUM

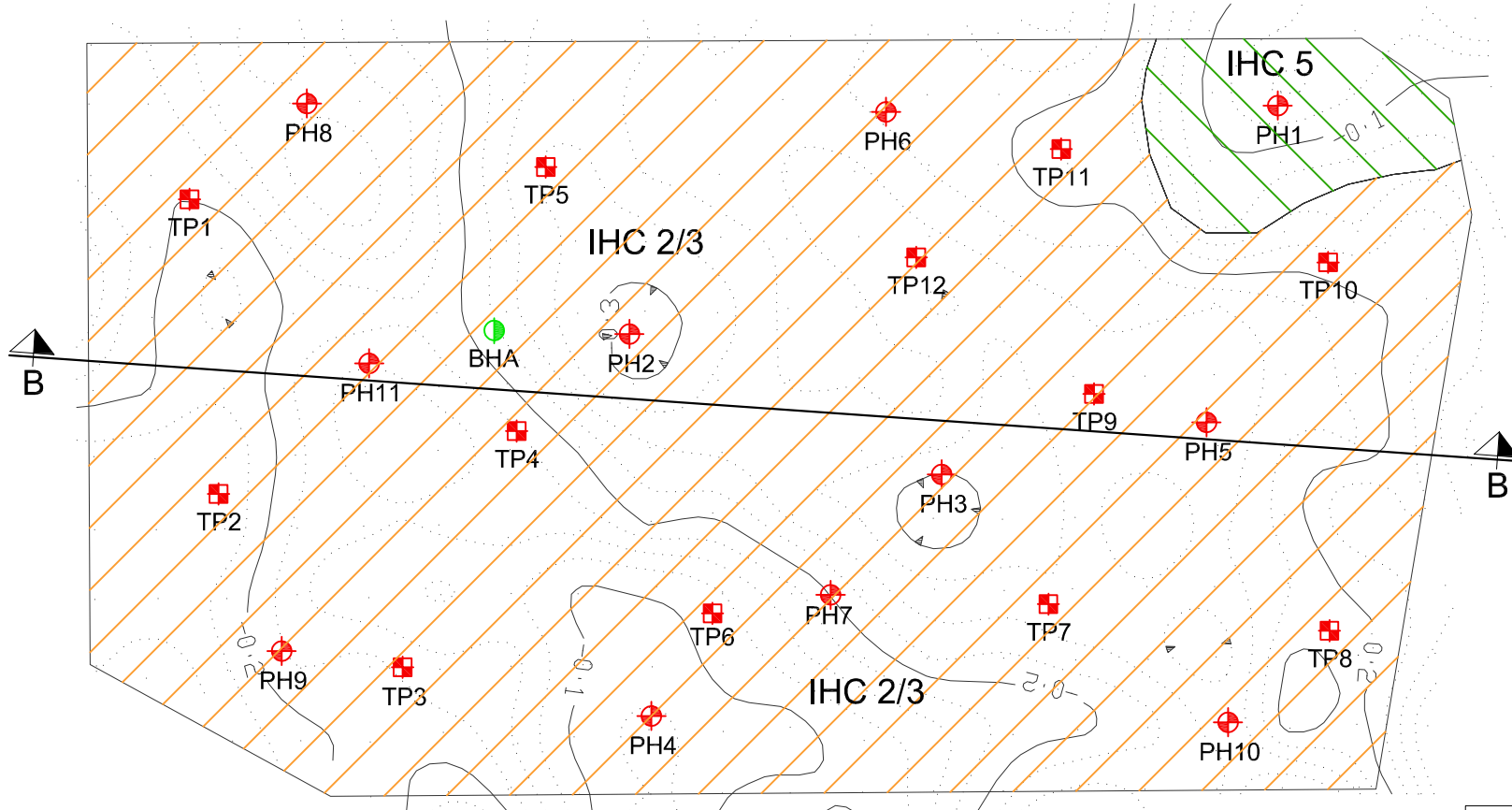
COPYRIGHT: THIS DRAWING AND ALL THE INFORMATION THEREON IS THE PROPERTY OF KNIGHT PIESOLD AND MAY NOT BE COPIED, REPRODUCED OR TRANSMITTED IN PART OR IN FULL WITHOUT THE FIRM'S CONSENT.






STORMWATER / SEWER LINE

LEGEND

-  PERCUSSION BOREHOLES
-  DOMESTIC BOREHOLES
-  TEST PITS



LEGEND

-  PERCUSSION BOREHOLES
-  DOMESTIC BOREHOLES
-  TEST PITS



**RICHBAY CHEMICALS NEW PLANT, VOSLOORUS
GEOTECHNICAL INVESTIGATION**

SITE PLAN SHOWING IHC

PROJECT NO: 31100288/72

FIGURE NO 6

SCALE 1:2000

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APPENDIX A

Site Photographs

SITE PHOTOGRAPHS

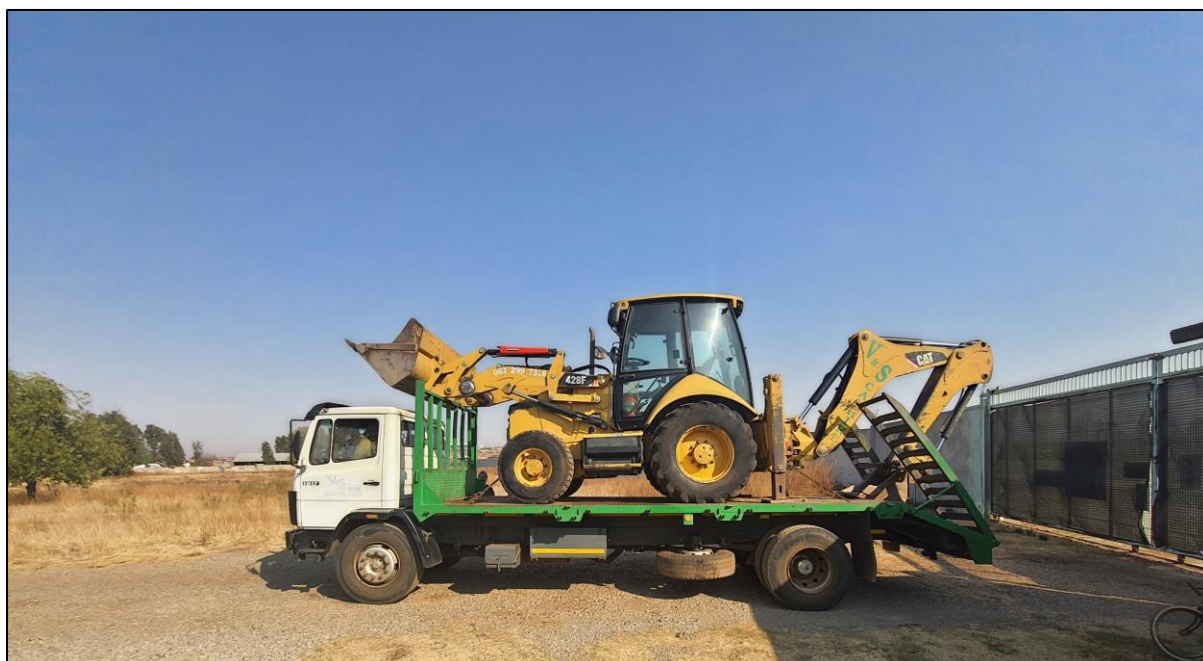


PHOTO 1 – View of TLB equipment used for excavation of test pits.

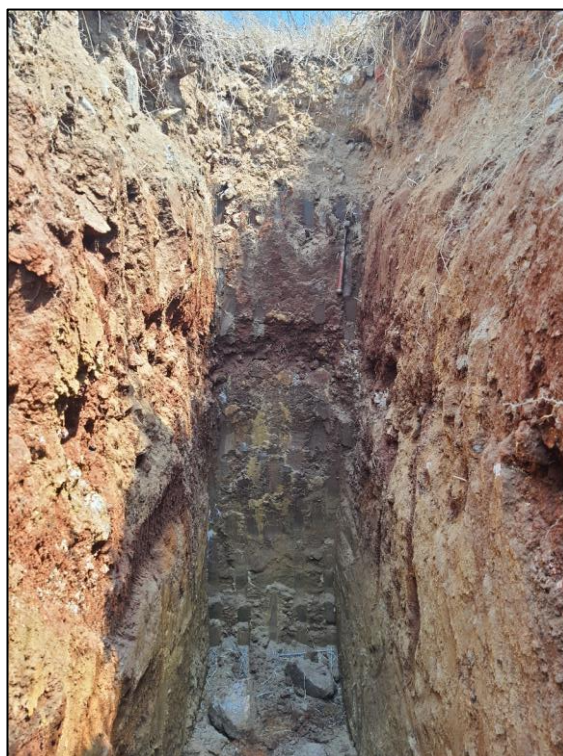


PHOTO 2 – Typical residual dolerite profile encountered on site.



PHOTO 3 – Typical transition between reworked residual (top) and residual dolerite (bottom).

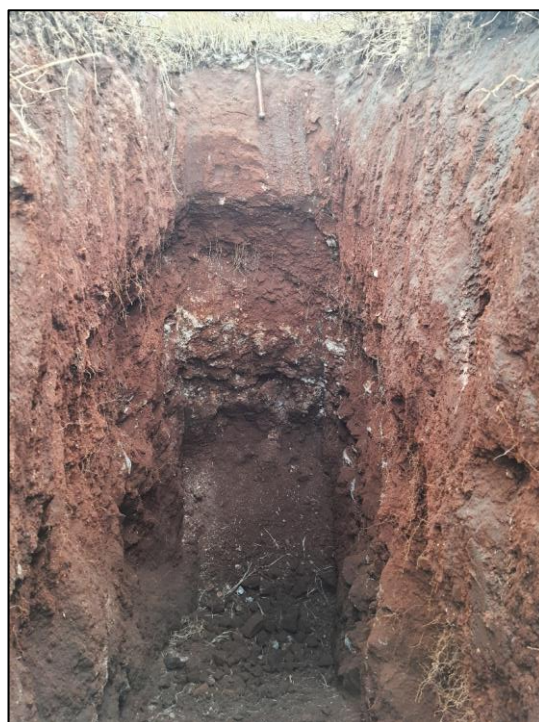


PHOTO 4 – View of typical Dolomitic profile with upper transported soils and lower chert dominated residuum.

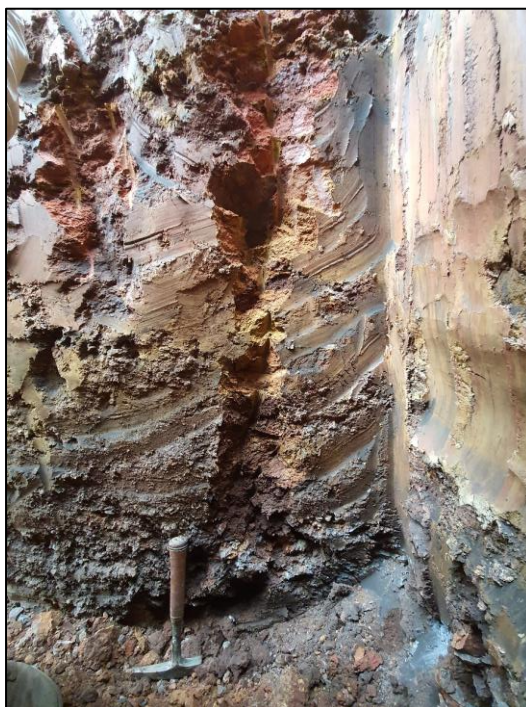


PHOTO 5 – Transition between residual dolerite (upper reddish brown and yellow brown) soils and reworked residual (lower dark brown) dolomite soils



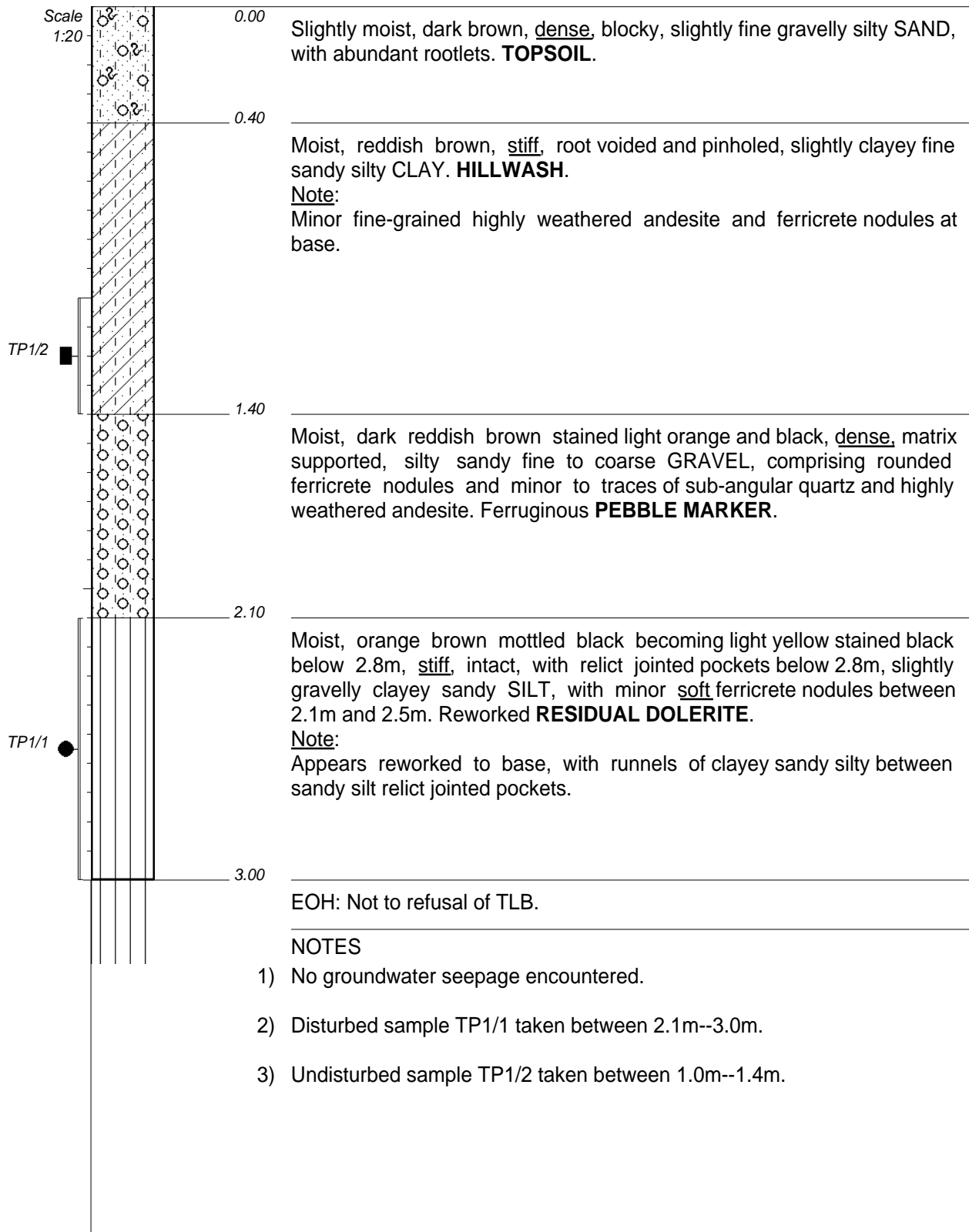
PHOTO 6 – View of western portion of site, looking south from borehole PH8.

APPENDIX B

Test Pit Profiles

**FEASIBILITY GEOTECHNICAL AND
DOLOMITE STABILITY INVESTIGATION**

JOB: 3110028872



CONTRACTOR : yV&S Cathire
MACHINE : CAT428F
DRILLED BY : Amos & Johannes
PROFILED BY : D. Bester (114260)

INCLINATION : Vertical
DIAM : 700mm
DATE : y16/09/2021
DATE : 16 Sep 2021

COORDINATE SYSTEM : WGS84 (Lo29)
X-COORD : 2916734.01
Y-COORD : 76403.66

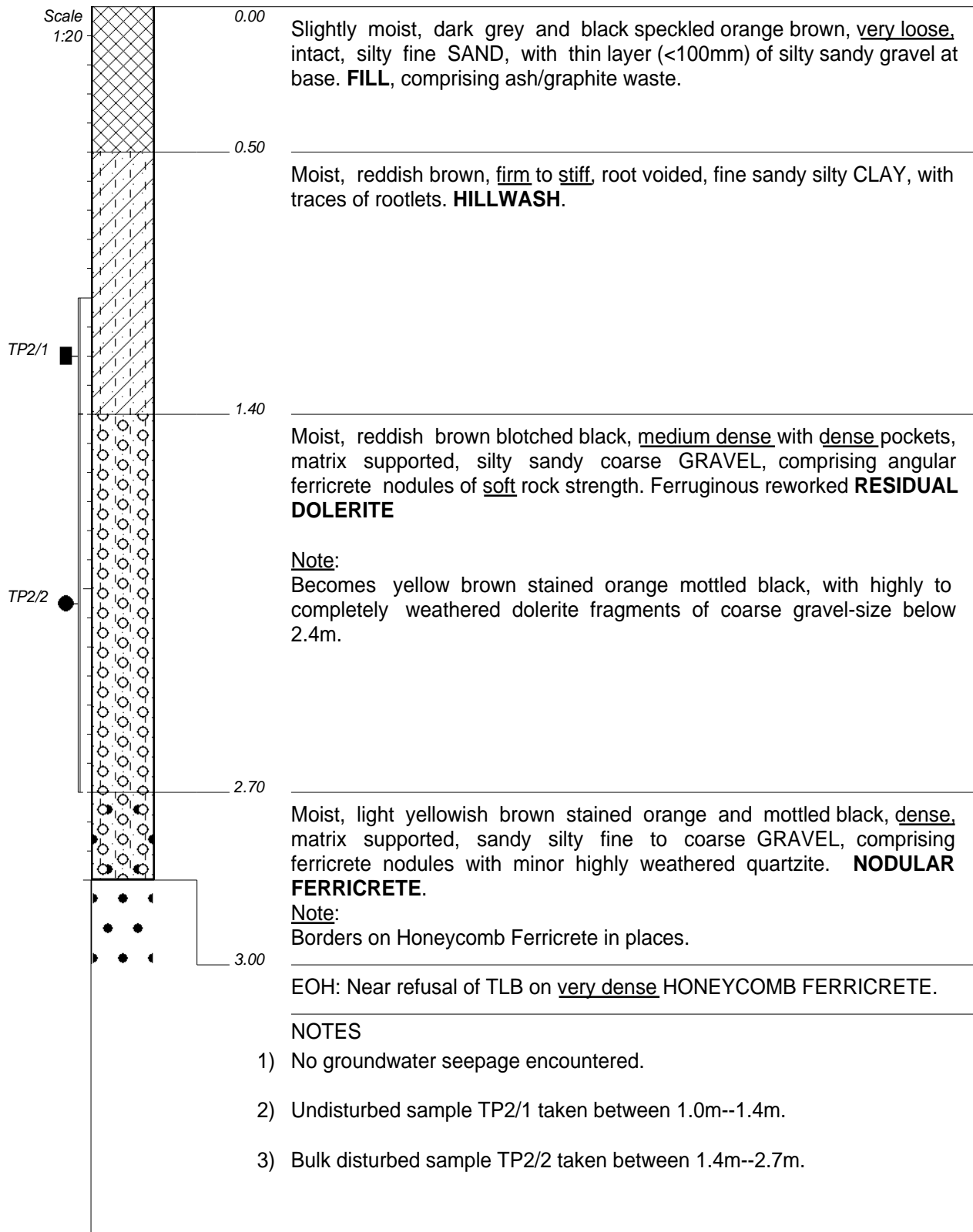
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

DATE : 19/11/2021 12:15
TEXT : C:\WP51\PROFILES\PEITP.TXT

HOLE No: TP1

**FEASIBILITY GEOTECHNICAL AND
DOLOMITE STABILITY INVESTIGATION**

JOB: 3110028872



CONTRACTOR : ÿV&S Cathire
MACHINE : CAT428F
DRILLED BY : Amos & Johannes
PROFILED BY : D. Bester (114260)
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM : 700mm
DATE : ÿ16/09/2021
DATE : 16 Sep 2021

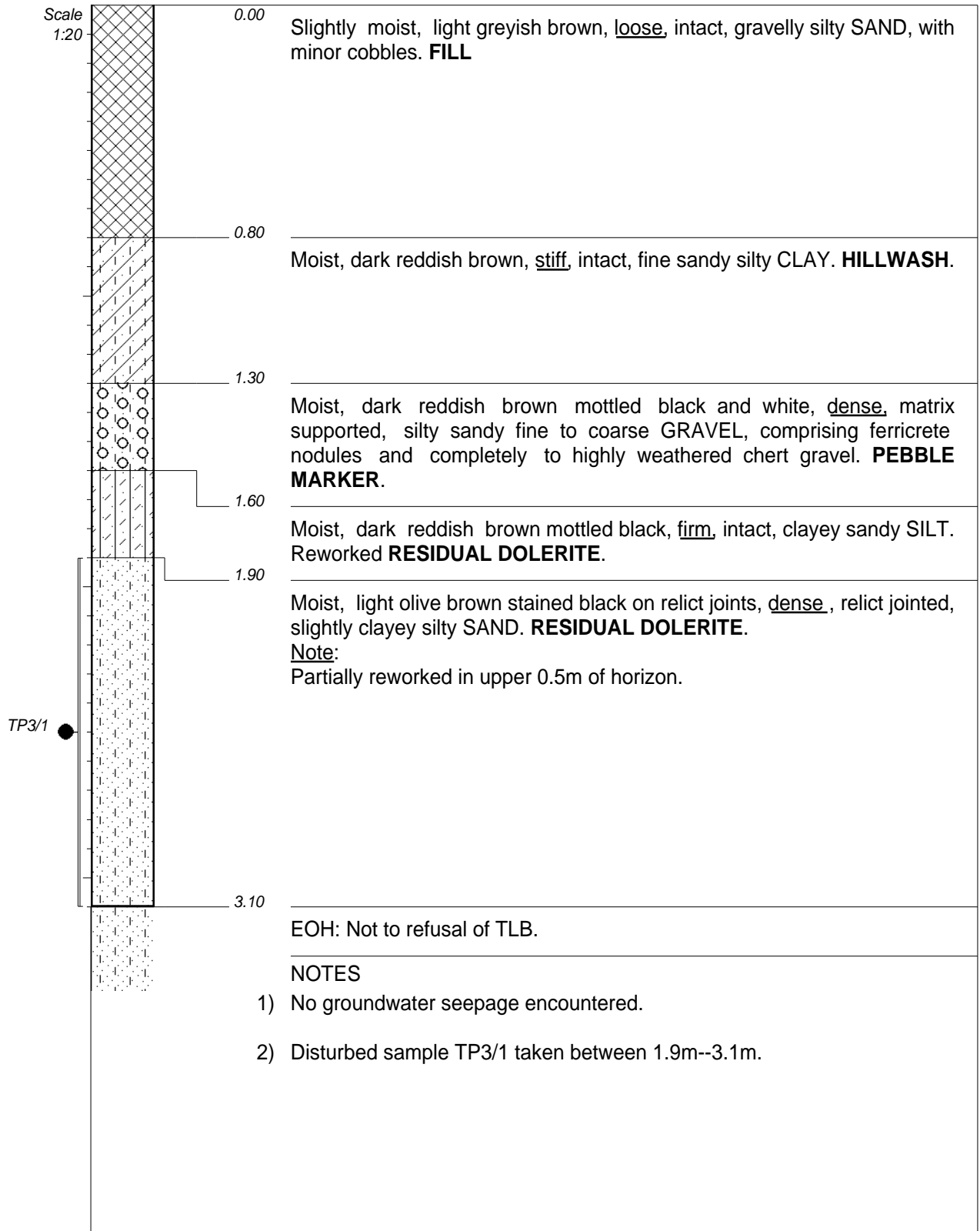
COORDINATE SYSTEM : WGS84 (Lo29)
X-COORD : 2916817.28
Y-COORD : 76395.48

HOLE No: TP2

DATE : 19/11/2021 12:15
TEXT : C:\WP51\PROFILES\PE\TP.TXT

**FEASIBILITY GEOTECHNICAL AND
DOLOMITE STABILITY INVESTIGATION**

JOB: 3110028872



CONTRACTOR : yV&S Cathire
MACHINE : CAT428F
DRILLED BY : Amos & Johannes
PROFILED BY : D. Bester (114260)
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

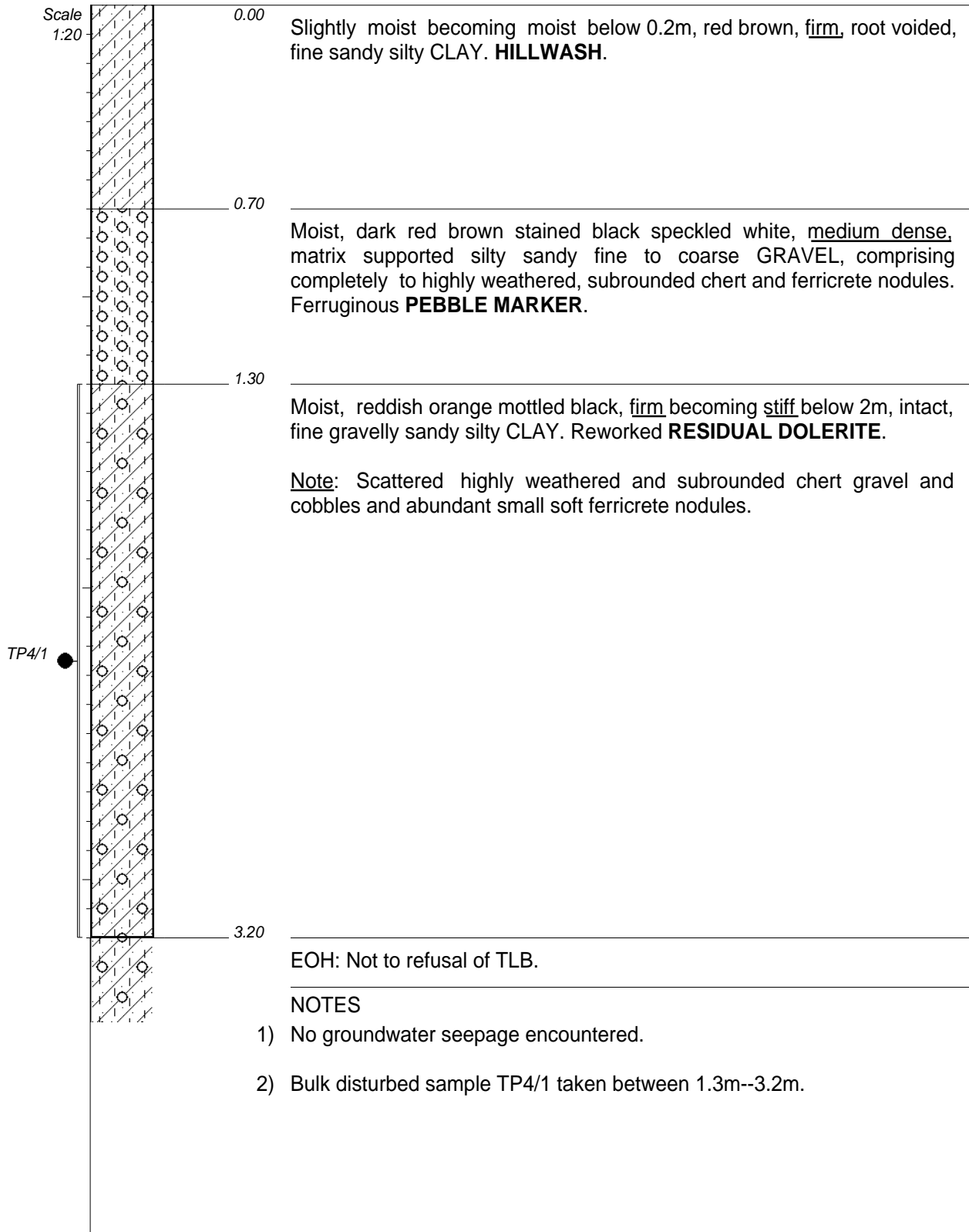
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DIAM : 700mm
DATE : y16/09/2021
DATE : 16 Sep 2021
DATE : 19/11/2021 12:15
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COORDINATE SYSTEM : WGS84 (Lo29)
X-COORD : 2916866.29
Y-COORD : 76343.38

HOLE No: TP3

**FEASIBILITY GEOTECHNICAL AND
DOLOMITE STABILITY INVESTIGATION**

JOB: 3110028872



CONTRACTOR : yV&S Cathire
MACHINE : CAT428F
DRILLED BY : Amos & Johannes
PROFILED BY : D. Bester (114260)

INCLINATION : Vertical
DIAM : 700mm
DATE : y16/09/2021
DATE : 16 Sep 2021

COORDINATE SYSTEM : WGS84 (Lo29)
X-COORD : 2916799.5
Y-COORD : 76311.14

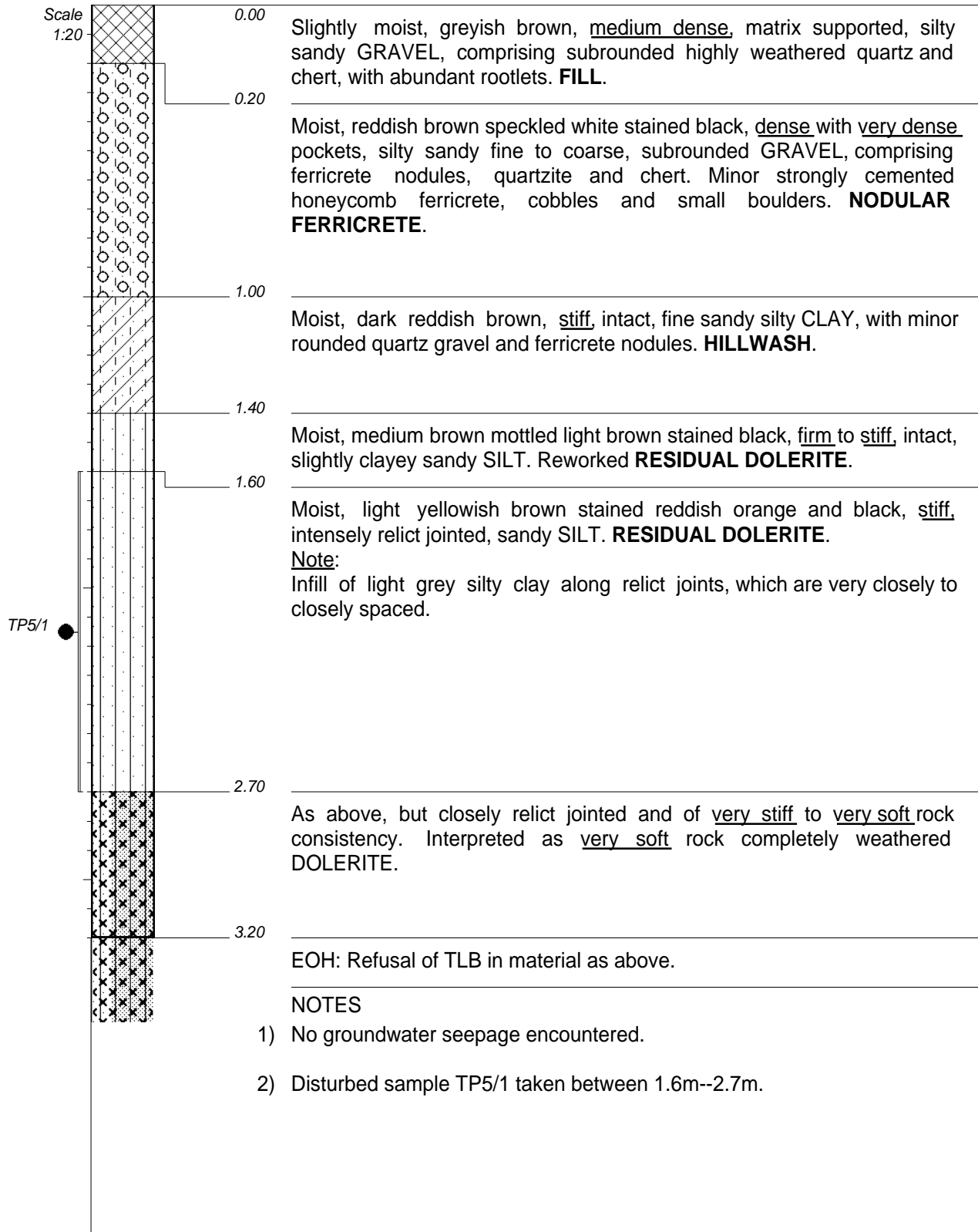
TYPE SET BY : EM
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DATE : 19/11/2021 12:15
TEXT : C:\WP51\PROFILES\PEITP.TXT

HOLE No: TP4

FEASIBILITY GEOTECHNICAL AND
DOLOMITE STABILITY INVESTIGATION

JOB: 3110028872



NOTES

- 1) No groundwater seepage encountered.
- 2) Disturbed sample TP5/1 taken between 1.6m--2.7m.

CONTRACTOR : yV&S Cathire
MACHINE : CAT428F
DRILLED BY : Amos & Johannes
PROFILED BY : D. Bester (114260)
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM : 700mm
DATE : y16/09/2021
DATE : 16 Sep 2021

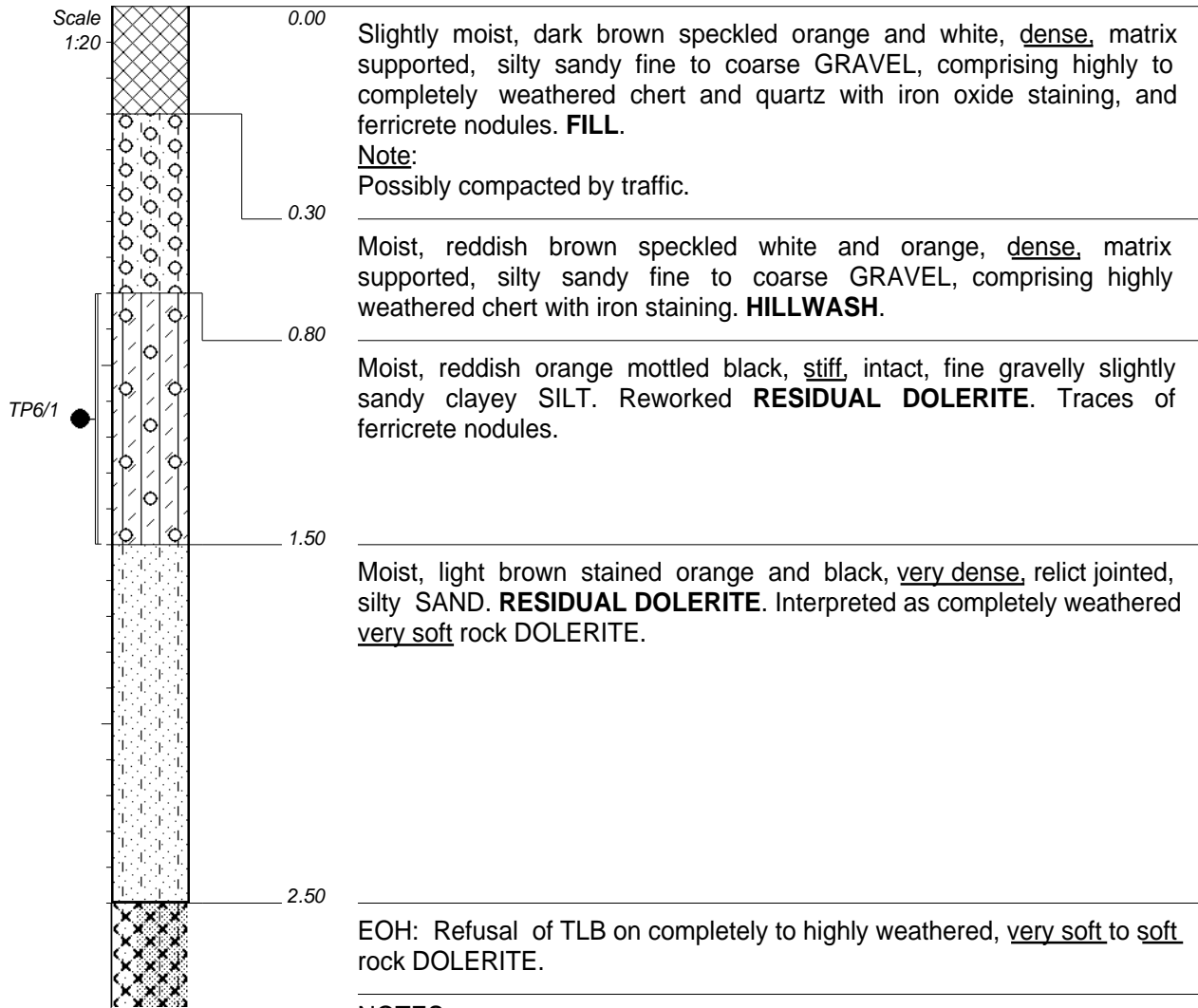
COORDINATE SYSTEM : WGS84 (Lo29)
X-COORD : 2916724.88
Y-COORD : 76302.89

HOLE No: TP5

DATE : 19/11/2021 12:15
TEXT : C:\WP51\PROFILES\PE\TP.TXT

**FEASIBILITY GEOTECHNICAL AND
DOLOMITE STABILITY INVESTIGATION**

JOB: 3110028872



NOTES

- 1) No groundwater seepage encountered.
- 2) Disturbed sample TP6/1 taken between 0.8m--1.5m.

CONTRACTOR : yV&S Cathire
MACHINE : CAT428F
DRILLED BY : Amos & Johannes
PROFILED BY : D. Bester (114260)

INCLINATION : Vertical
DIAM : 700mm
DATE : y16/09/2021
DATE : 16 Sep 2021

COORDINATE SYSTEM : WGS84 (Lo29)
X-COORD : 2916851.03
Y-COORD : 76255.73

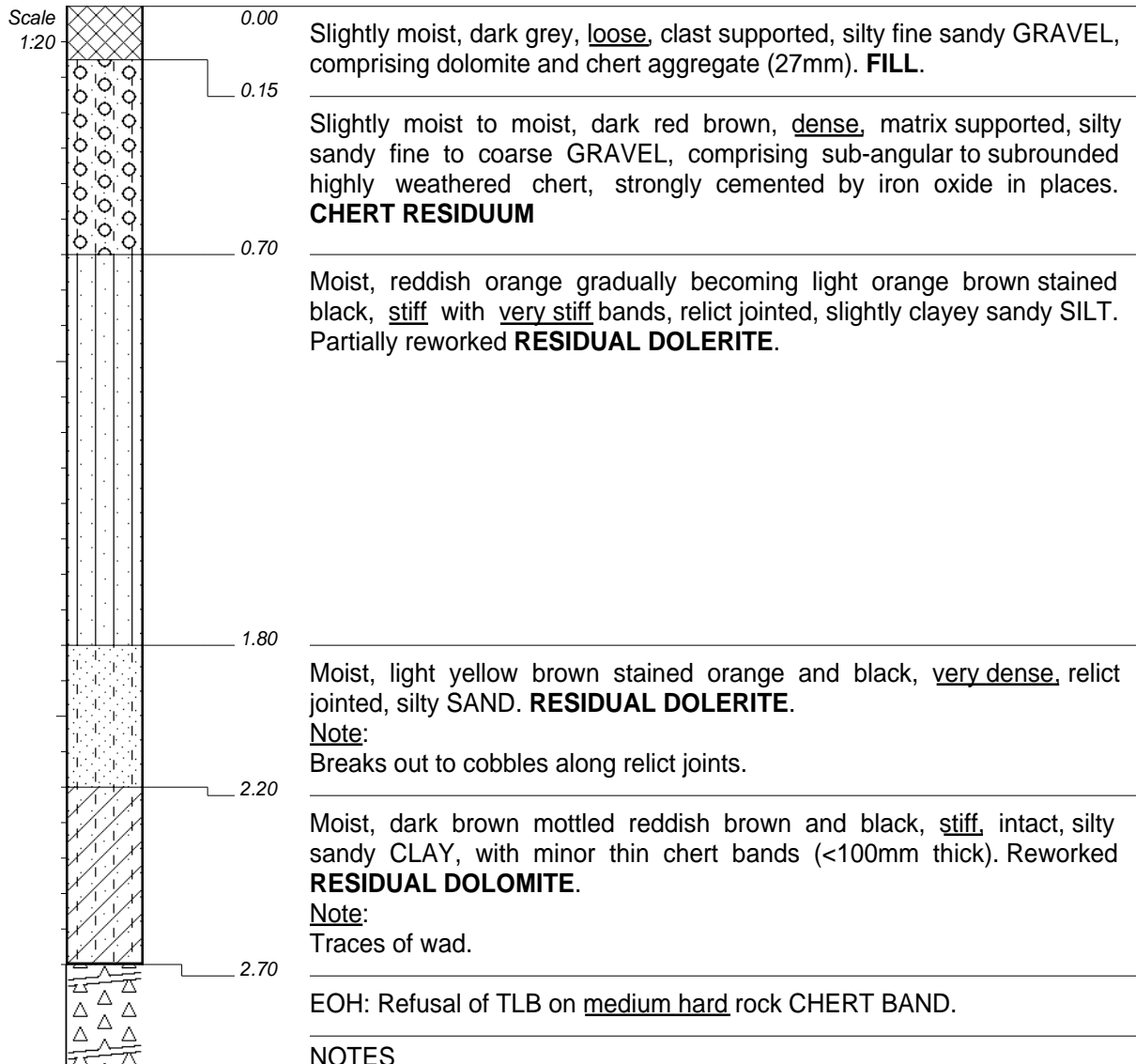
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DATE : 19/11/2021 12:15
TEXT : C:\WP51\PROFILES\PE\TP.TXT

HOLE No: TP6

**FEASIBILITY GEOTECHNICAL AND
DOLOMITE STABILITY INVESTIGATION**

JOB: 3110028872



NOTES

- 1) No groundwater seepage encountered.

CONTRACTOR : yV&S Cathire
MACHINE : CAT428F
DRILLED BY : Amos & Johannes
PROFILED BY : D. Bester (114260)

INCLINATION : Vertical
DIAM : 700mm
DATE : y16/09/2021
DATE : 16 Sep 2021

COORDINATE SYSTEM : WGS84 (Lo29)
X-COORD : 2916848.47
Y-COORD : 76160.72

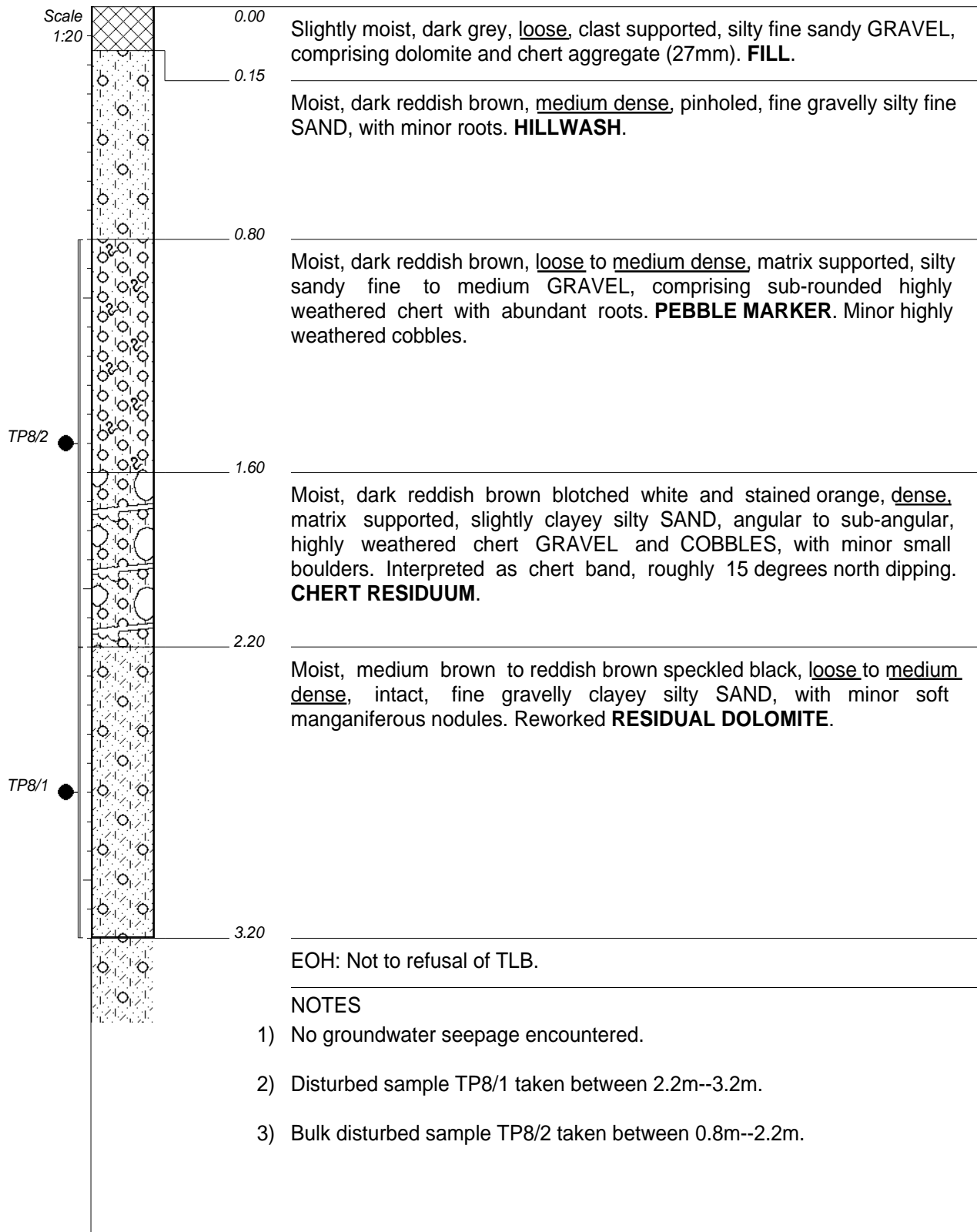
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DATE : 19/11/2021 12:15
TEXT : C:\WP51\PROFILES\PEITP.TXT

HOLE No: TP7

**FEASIBILITY GEOTECHNICAL AND
DOLOMITE STABILITY INVESTIGATION**

JOB: 3110028872



CONTRACTOR : yV&S Cathire
MACHINE : CAT428F
DRILLED BY : Amos & Johannes
PROFILED BY : D. Bester (114260)

INCLINATION : Vertical
DIAM : 700mm
DATE : y16/09/2021
DATE : 16 Sep 2021

COORDINATE SYSTEM : WGS84 (Lo29)
X-COORD : 2916855.98
Y-COORD : 76081.22

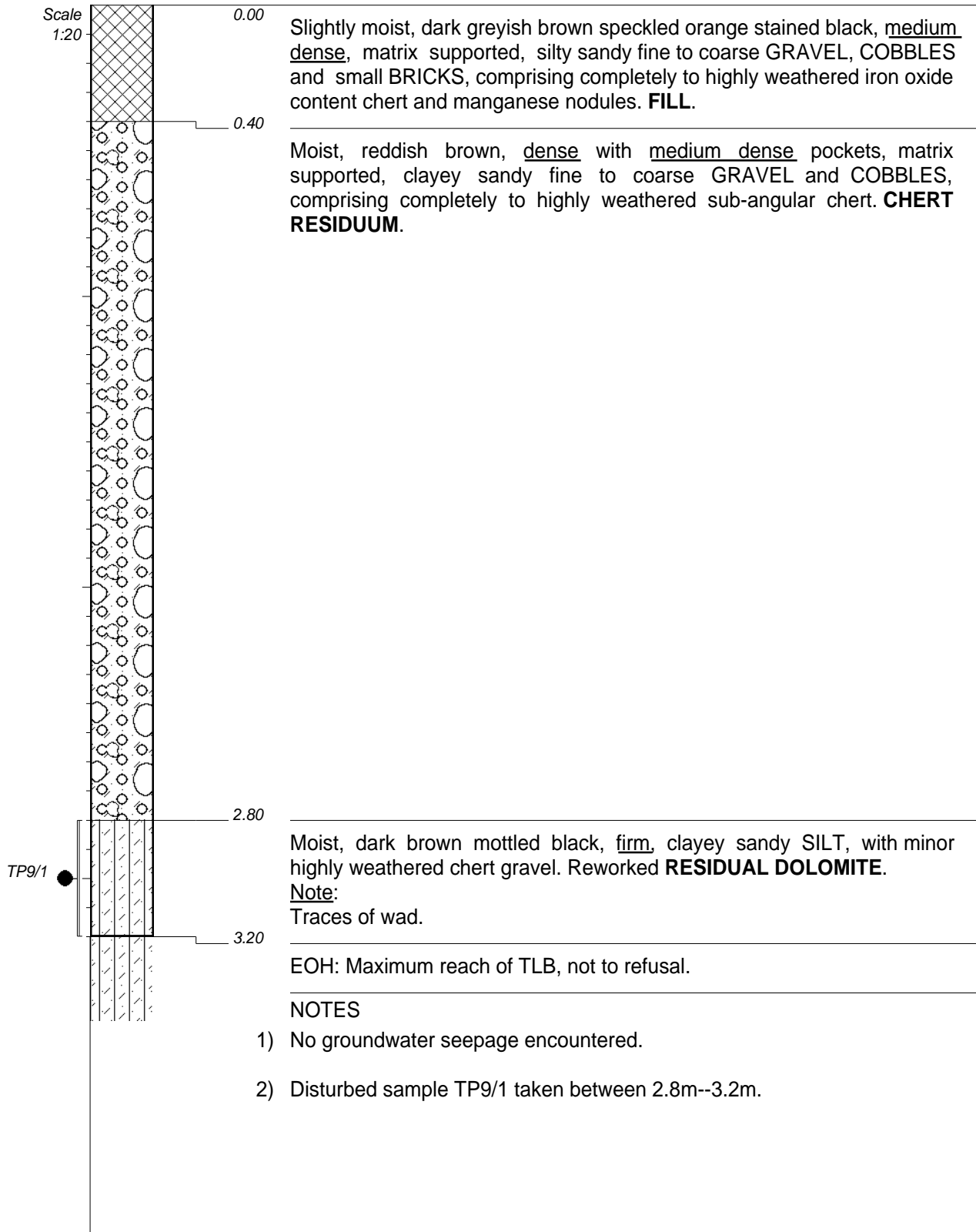
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SETUP FILE : KP8.SET

DATE : 19/11/2021 12:15
TEXT : C:\WP51\PROFILES\PEITP.TXT

HOLE No: TP8

**FEASIBILITY GEOTECHNICAL AND
DOLOMITE STABILITY INVESTIGATION**

JOB: 3110028872



CONTRACTOR : yV&S Cathire
MACHINE : CAT428F
DRILLED BY : Amos & Johannes
PROFILED BY : D. Bester (114260)

INCLINATION : Vertical
DIAM : 700mm
DATE : y16/09/2021
DATE : 16 Sep 2021

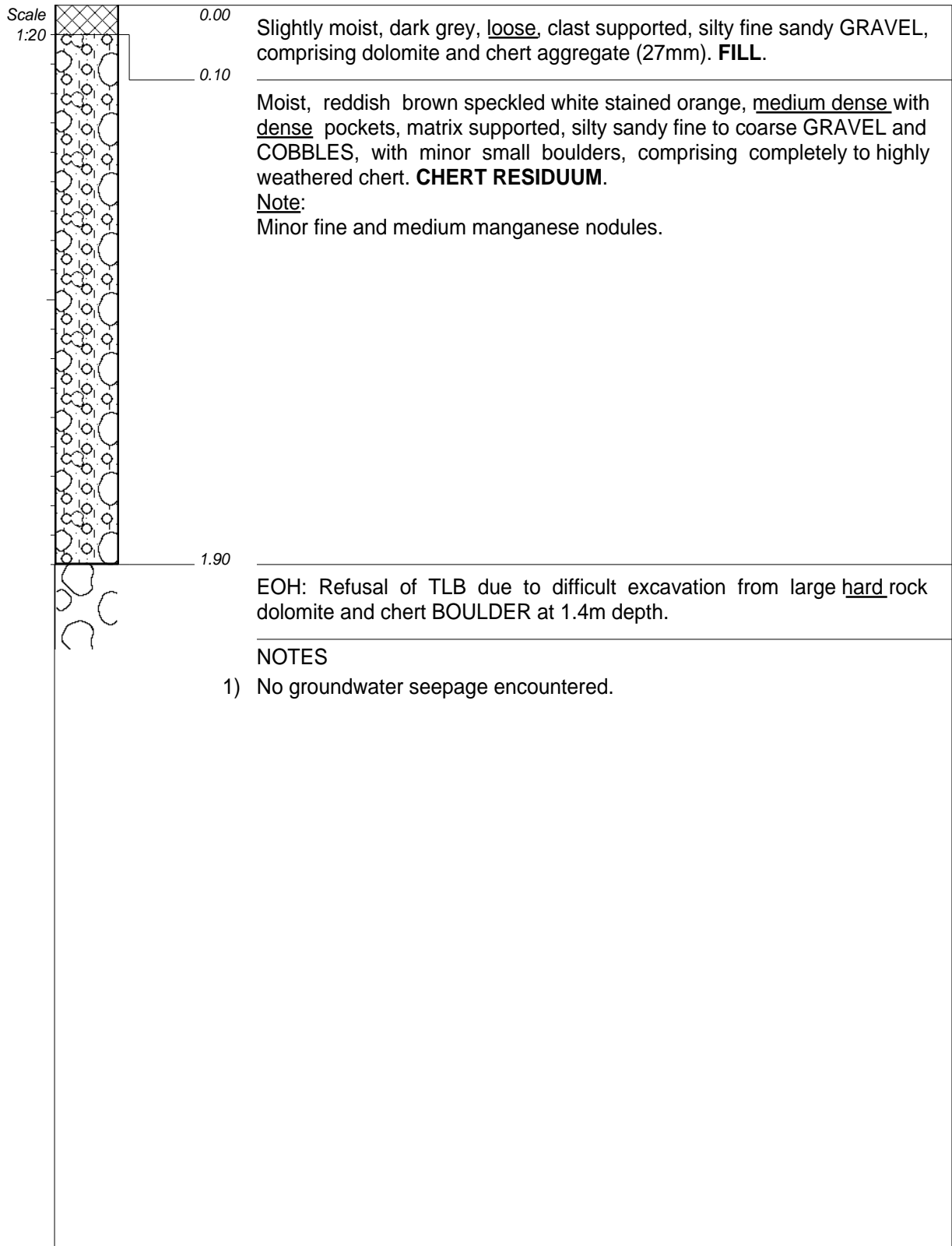
COORDINATE SYSTEM : WGS84 (Lo29)
X-COORD : 2916789
Y-COORD : 76147.79

TYPE SET BY : EM
SETUP FILE : KP8.SET

DATE : 19/11/2021 12:15
TEXT : C:\WP51\PROFILES\PE\TP.TXT

HOLE No: TP9

**FEASIBILITY GEOTECHNICAL AND
DOLOMITE STABILITY INVESTIGATION**

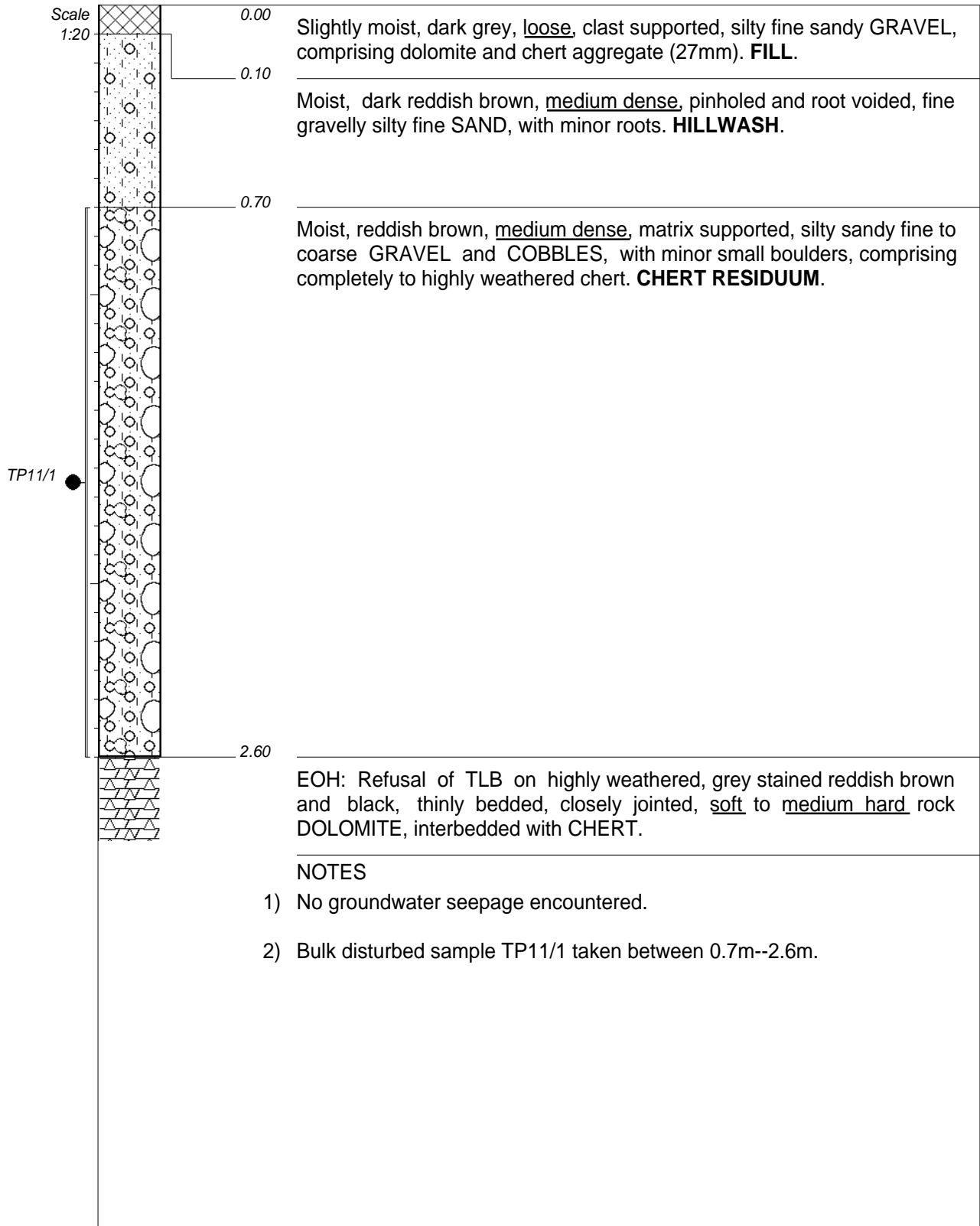


CONTRACTOR : yV&S Cathire
MACHINE : CAT428F
DRILLED BY : Amos & Johannes
PROFILED BY : D. Bester (114260)
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM : 700mm
DATE : y16/09/2021
DATE : 16 Sep 2021
DATE : 19/11/2021 12:15
TEXT : C:\WP51\PROFILES\PEITP.TXT

COORDINATE SYSTEM : WGS84 (Lo29)
X-COORD : 2916751.82
Y-COORD : 76081.63

**FEASIBILITY GEOTECHNICAL AND
DOLOMITE STABILITY INVESTIGATION**

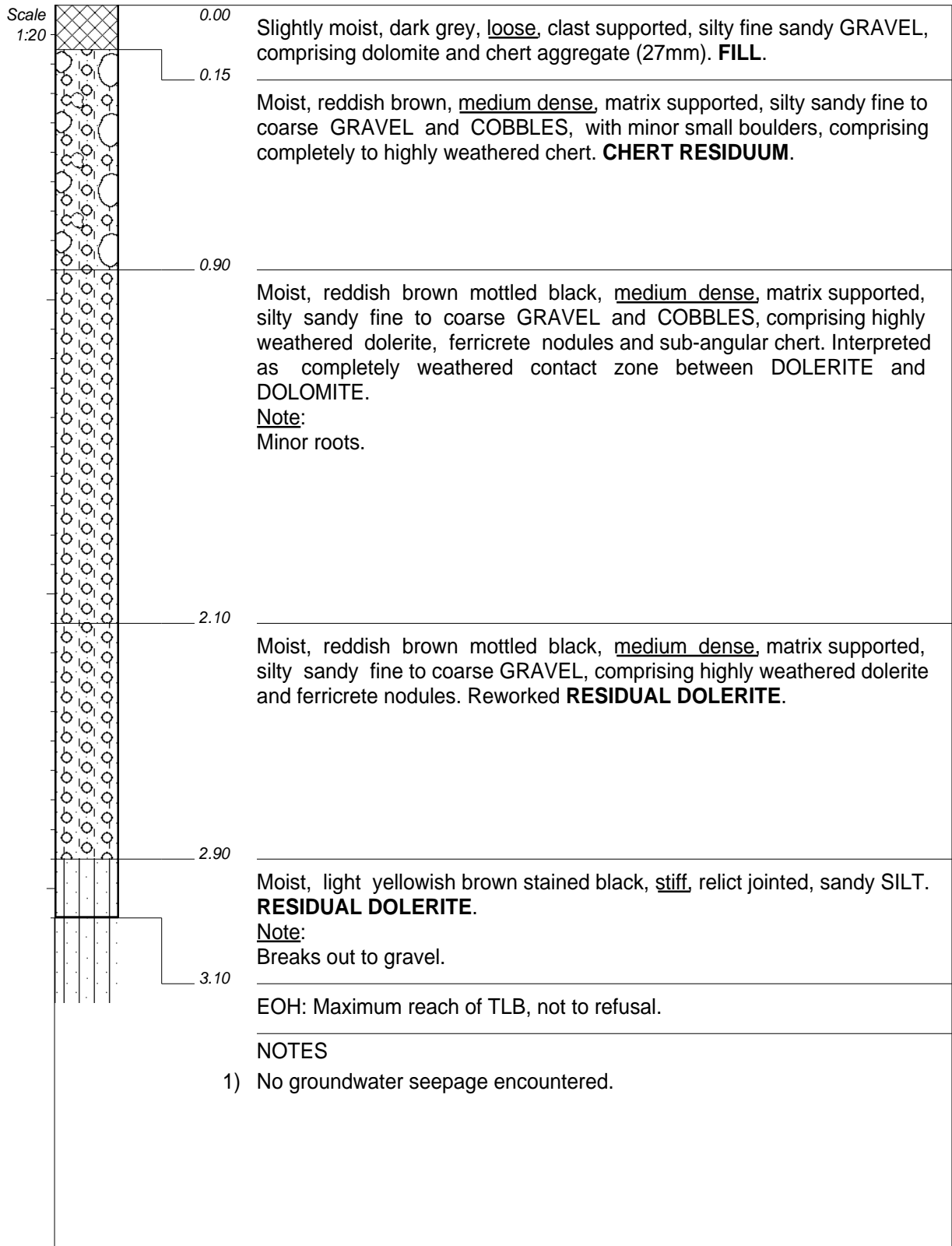


CONTRACTOR : yV&S Cathire
MACHINE : CAT428F
DRILLED BY : Amos & Johannes
PROFILED BY : D. Bester (114260)
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM : 700mm
DATE : y16/09/2021
DATE : 16 Sep 2021
DATE : 19/11/2021 12:15
TEXT : C:\WP51\PROFILES\PE\ITP.TXT

COORDINATE SYSTEM : WGS84 (Lo29)
X-COORD : 2916719.81
Y-COORD : 76157.09

**FEASIBILITY GEOTECHNICAL AND
DOLOMITE STABILITY INVESTIGATION**



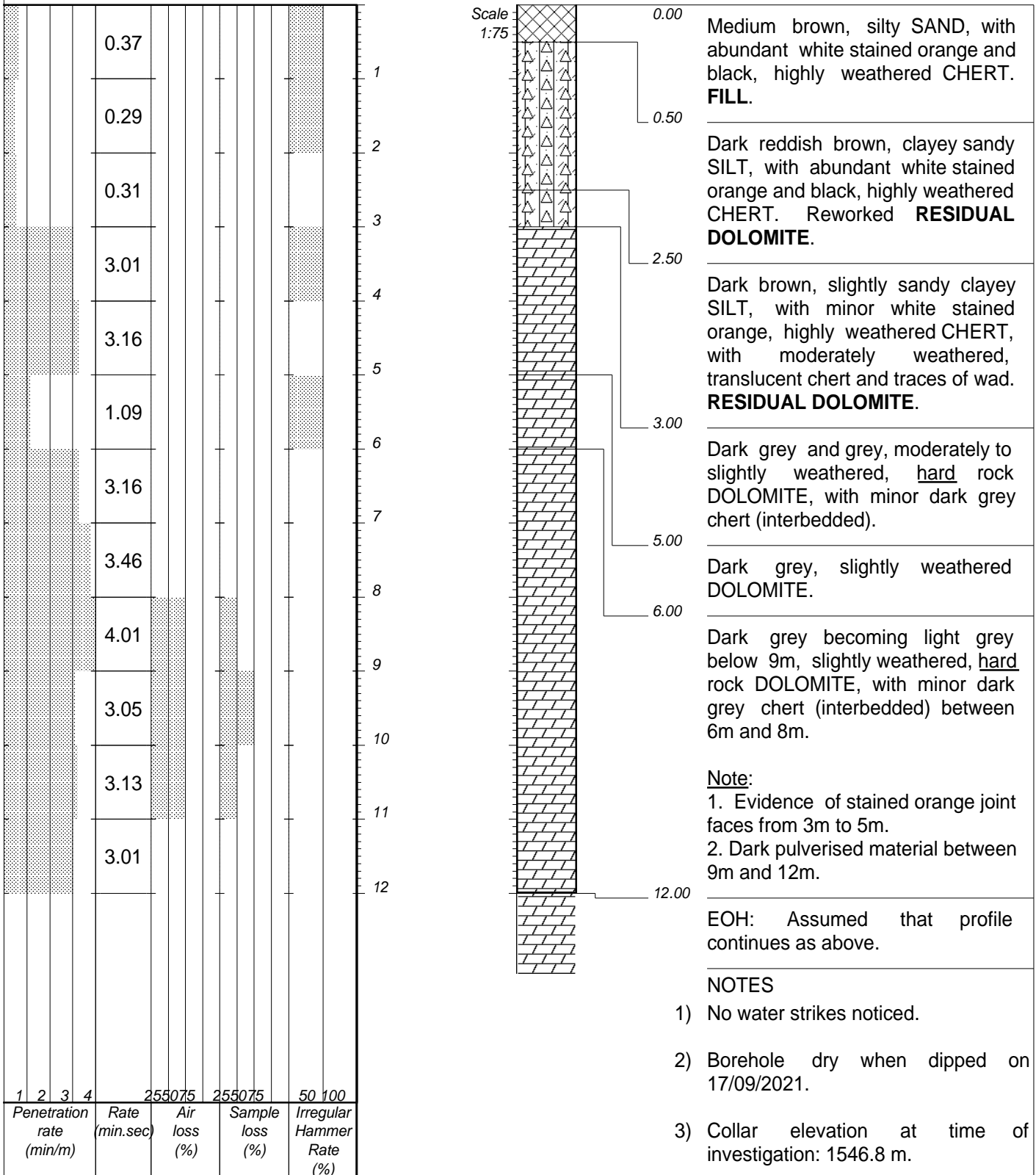
CONTRACTOR : yV&S Cathire
MACHINE : CAT428F
DRILLED BY : Amos & Johannes
PROFILED BY : D. Bester (114260)
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM : 700mm
DATE : y16/09/2021
DATE : 16 Sep 2021
DATE : 19/11/2021 12:15
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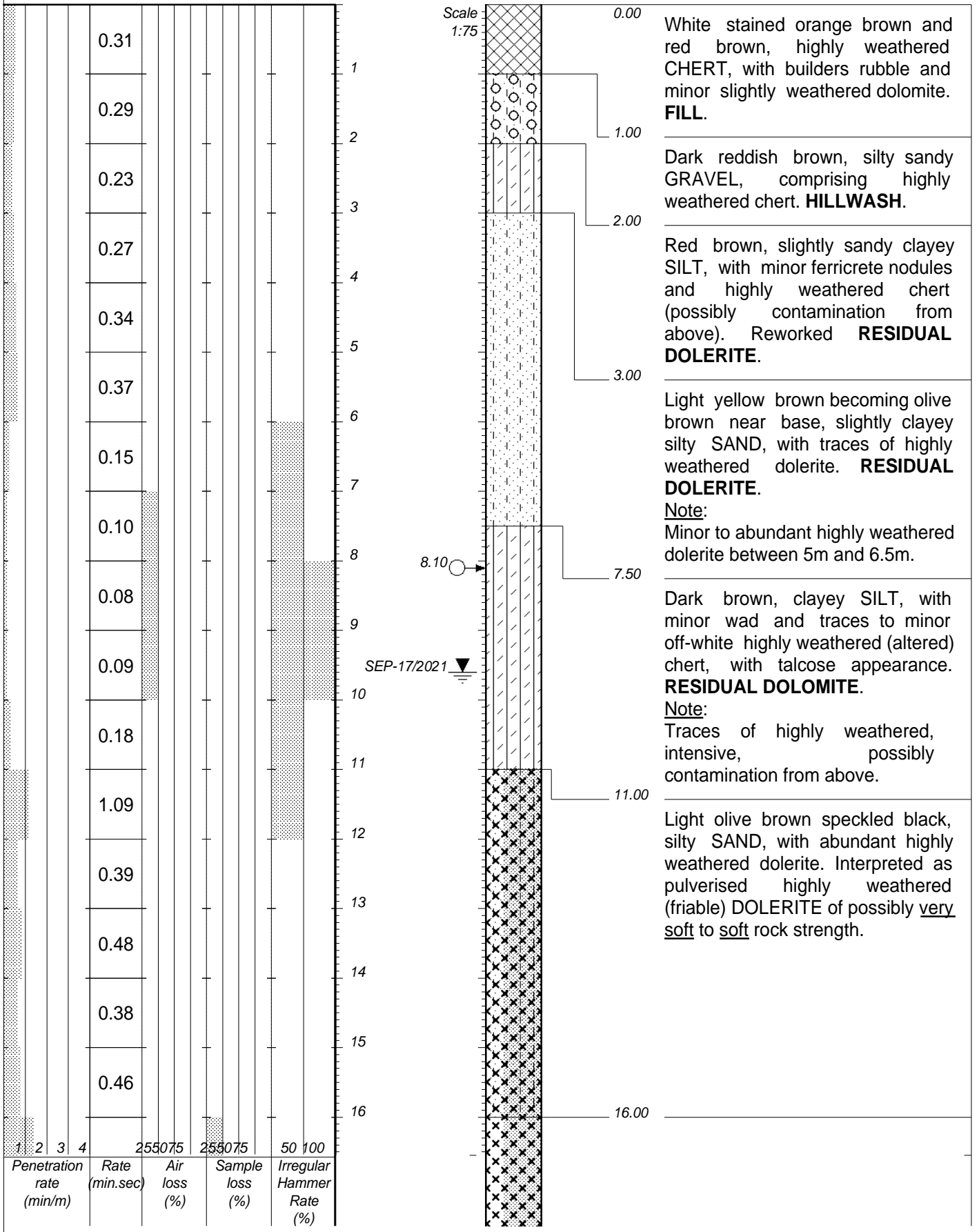
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X-COORD : 2916750.41
Y-COORD : 76198.13

APPENDIX C

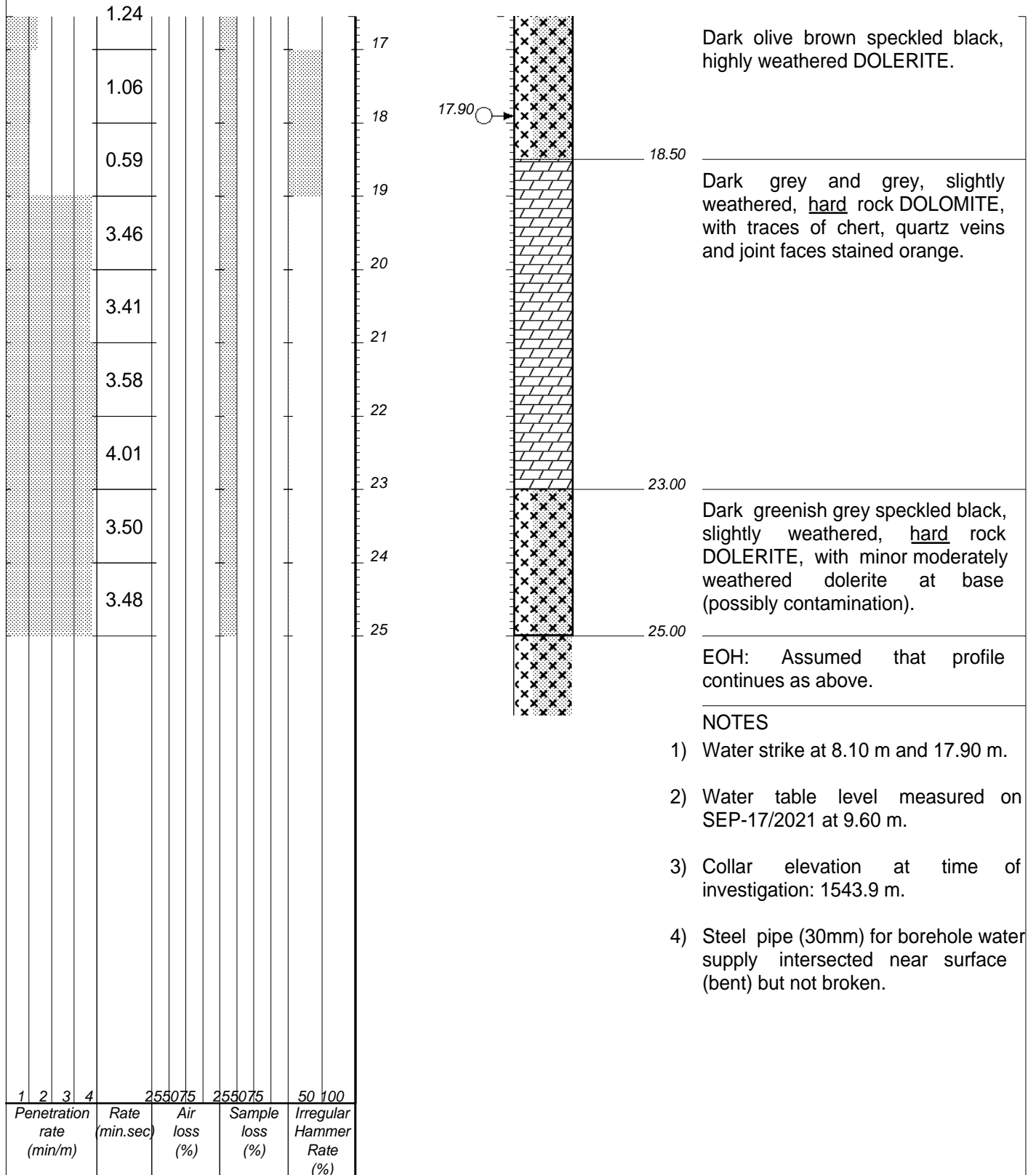
Percussion Borehole Profiles

**FEASIBILITY, GEOTECHNICAL AND
 DOLOMITE STABILITY INVESTIGATION**

**CONTRACTOR : Hennie Erwee Drilling
 MACHINE : Super Rock 1000
 DRILLED BY : Japhta
 PROFILED BY : Deon Bester (114260)**
**INCLINATION : Vertical
 DIAM : 165mm
 DATE : 17 Sep 2021
 DATE : 17 Sep 2021**
**COORDINATE SYSTEM : WGS84 (Lo29)
 X-COORD : 2916707.48
 Y-COORD : 76095.86**
**TYPE SET BY : EM
 SETUP FILE : KPTP8.SET**
**DATE : 22/11/2021 09:34
 TEXT : ..1\PROFILES\PEHPERCBH.TXT**

FEASIBILITY, GEOTECHNICAL AND
DOLOMITE STABILITY INVESTIGATION



FEASIBILITY, GEOTECHNICAL AND
DOLOMITE STABILITY INVESTIGATION



- NOTES**
- 1) Water strike at 8.10 m and 17.90 m.
 - 2) Water table level measured on SEP-17/2021 at 9.60 m.
 - 3) Collar elevation at time of investigation: 1543.9 m.
 - 4) Steel pipe (30mm) for borehole water supply intersected near surface (bent) but not broken.

CONTRACTOR : Hennie Erwee Drilling
MACHINE : Super Rock 1000
DRILLED BY : Japhta
PROFILED BY : Deon Bester (114260)

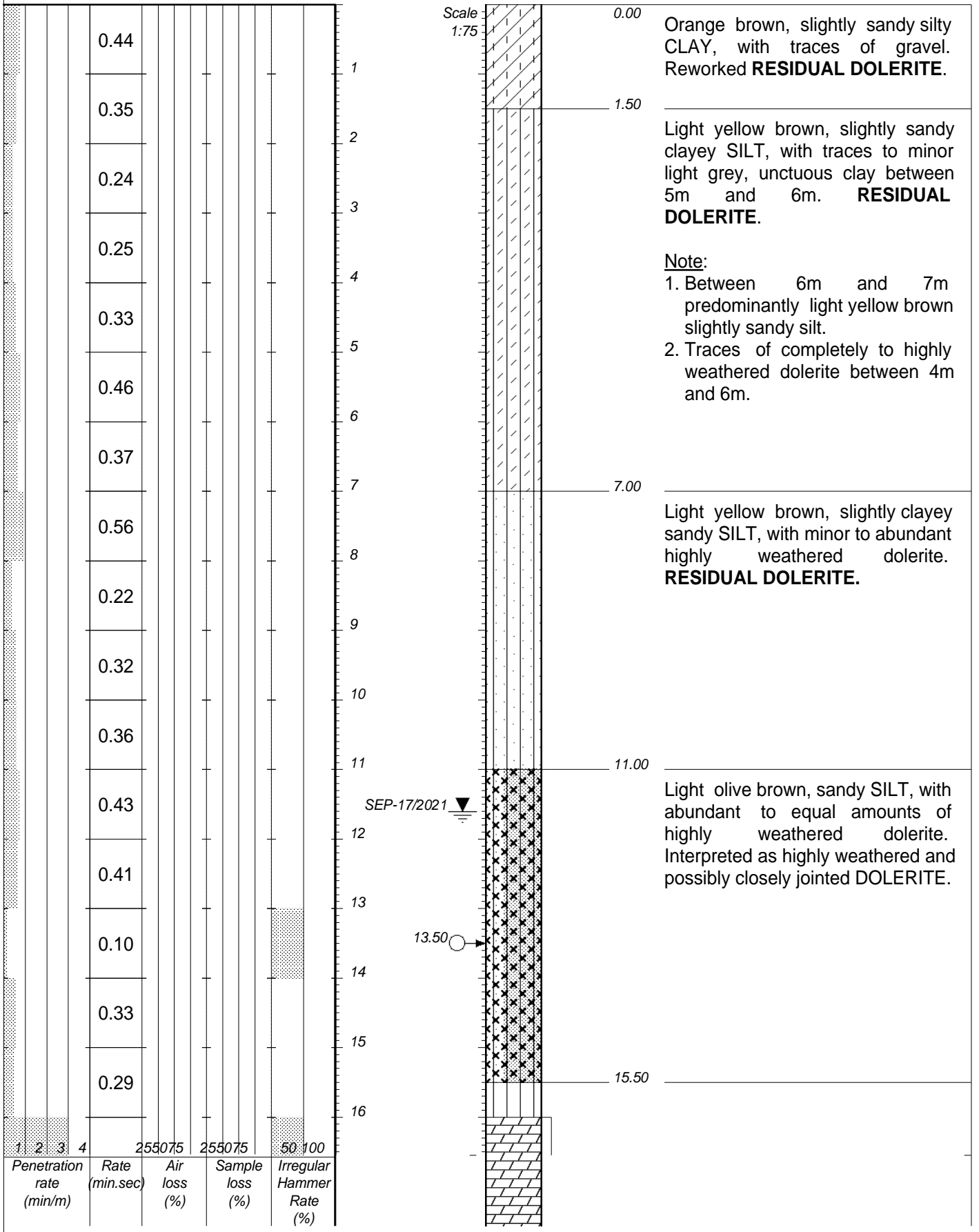
INCLINATION : Vertical
DIAM : 165mm
DATE : 20 Sep 2021
DATE : 20 Sep 2021

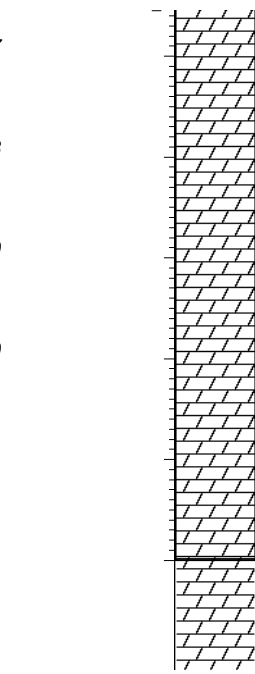
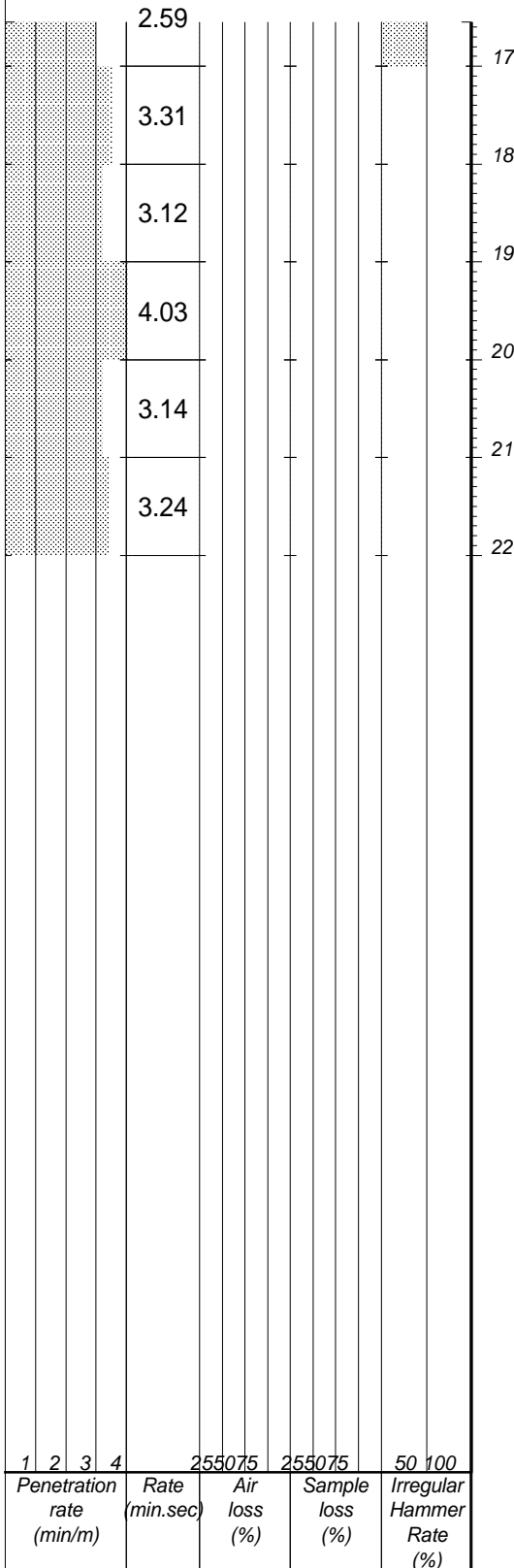
COORDINATE SYSTEM : WGS84 (Lo29)
X-COORD : 2916772.05
Y-COORD : 76279.25

TYPE SET BY : EM
SETUP FILE : KTPP8.SET

DATE : 22/11/2021 09:34
TEXT : ..1\PROFILES\PEHPERCBH.TXT

**FEASIBILITY, GEOTECHNICAL AND
DOLOMITE STABILITY INVESTIGATION**



**FEASIBILITY, GEOTECHNICAL AND
 DOLOMITE STABILITY INVESTIGATION**


Dark brown, slightly sandy SILT, with traces of highly weathered dolerite (possibly contamination). **RESIDUAL DOLOMITE**, with traces of wad.

16.00

Dark grey stained brown, moderately to slightly weathered, hard rock DOLOMITE.

Note:

- Sample heavily contaminated with residual dolomite between 16m and 18m.
- Abundant pulverised material between 19.5m and 21m.

22.00

EOH: Assumed that profile continues as above.

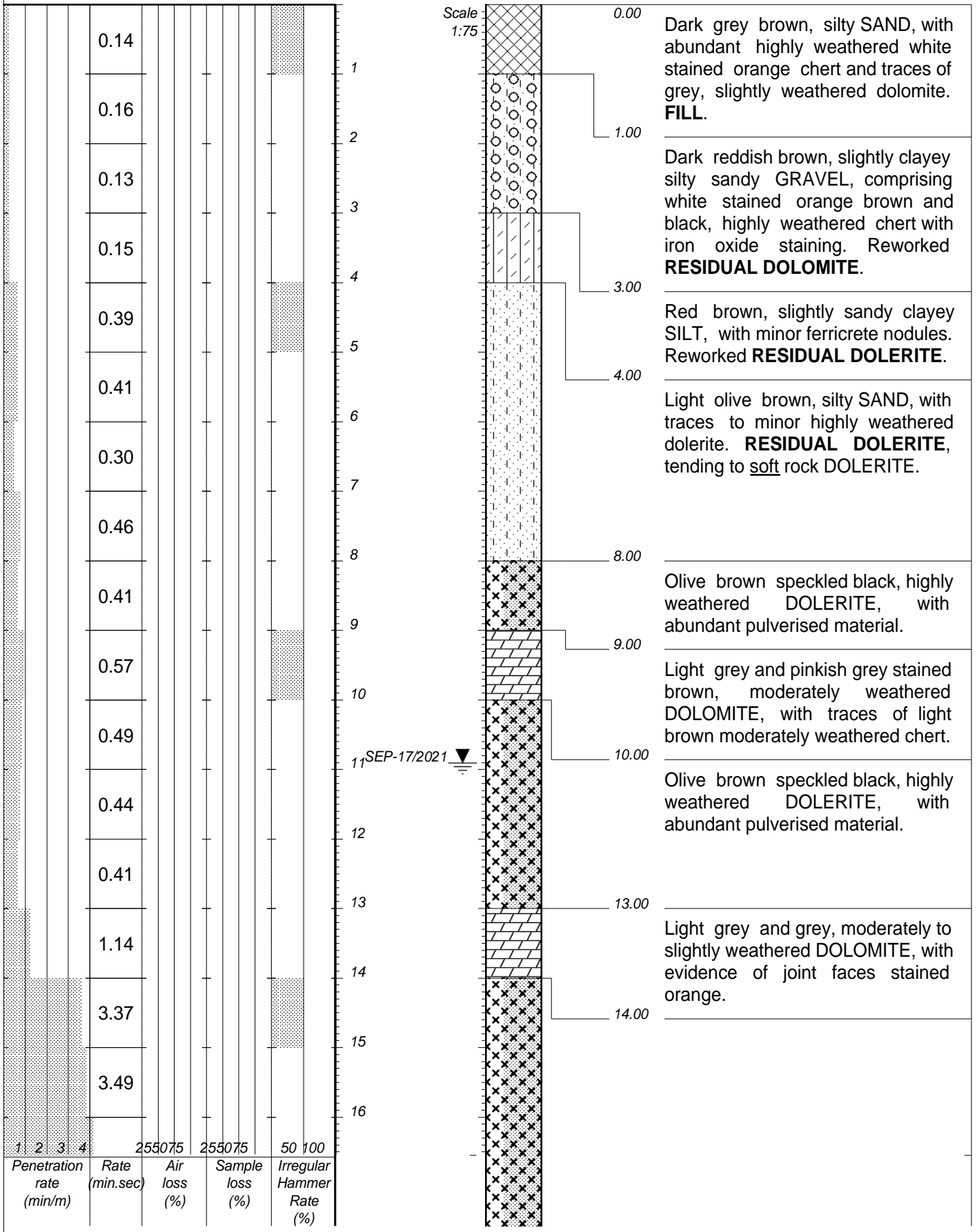
- NOTES**
- 1) Water strike at 13.50 m.
 - 2) Water table level measured on SEP-17/2021 at 11.60 m.
 - 3) Collar elevation at time of investigation: 1546.2 m.

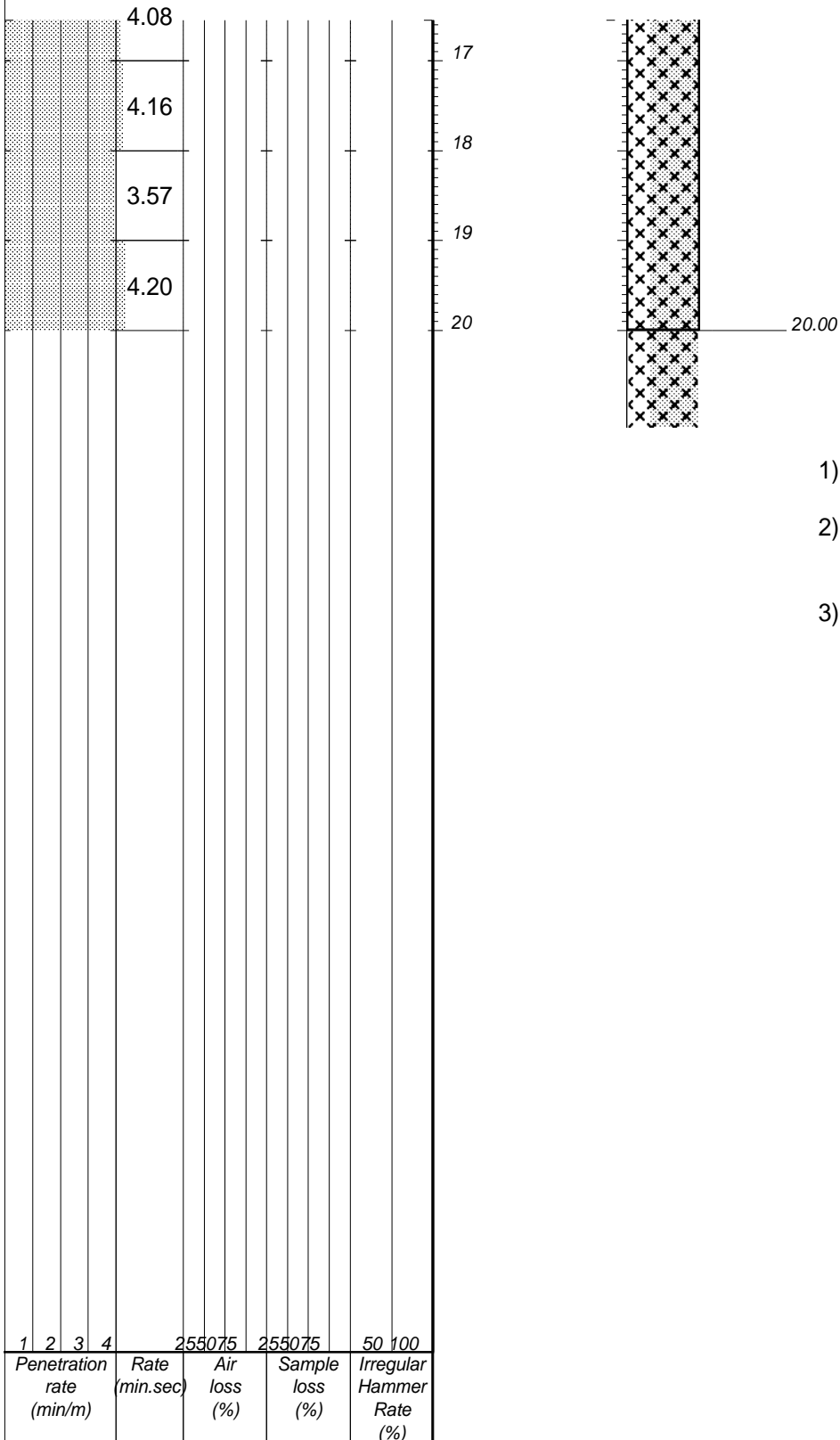
CONTRACTOR : Hennie Erwee Drilling
MACHINE : Super Rock 1000
DRILLED BY : Japhta
PROFILED BY : Deon Bester (114260)
TYPE SET BY : EM
SETUP FILE : KTP8.SET

INCLINATION : Vertical
DIAM : 165mm
DATE : 17 Sep 2021
DATE : 17 Sep 2021
DATE : 22/11/2021 09:34
TEXT : ..1\PROFILES\PEHPERCBH.TXT

COORDINATE SYSTEM : WGS84 (Lo29)
X-COORD : 2916811.75
Y-COORD : 76190.88

**FEASIBILITY, GEOTECHNICAL AND
DOLOMITE STABILITY INVESTIGATION**



**FEASIBILITY, GEOTECHNICAL AND
 DOLOMITE STABILITY INVESTIGATION**

 Dark grey speckled white, slightly weathered, hard rock DOLERITE.

Note:
 Between 16m and 17m minor slightly weathered dolomite recovered, interpreted as thin dolomite band within dolerite bedrock.

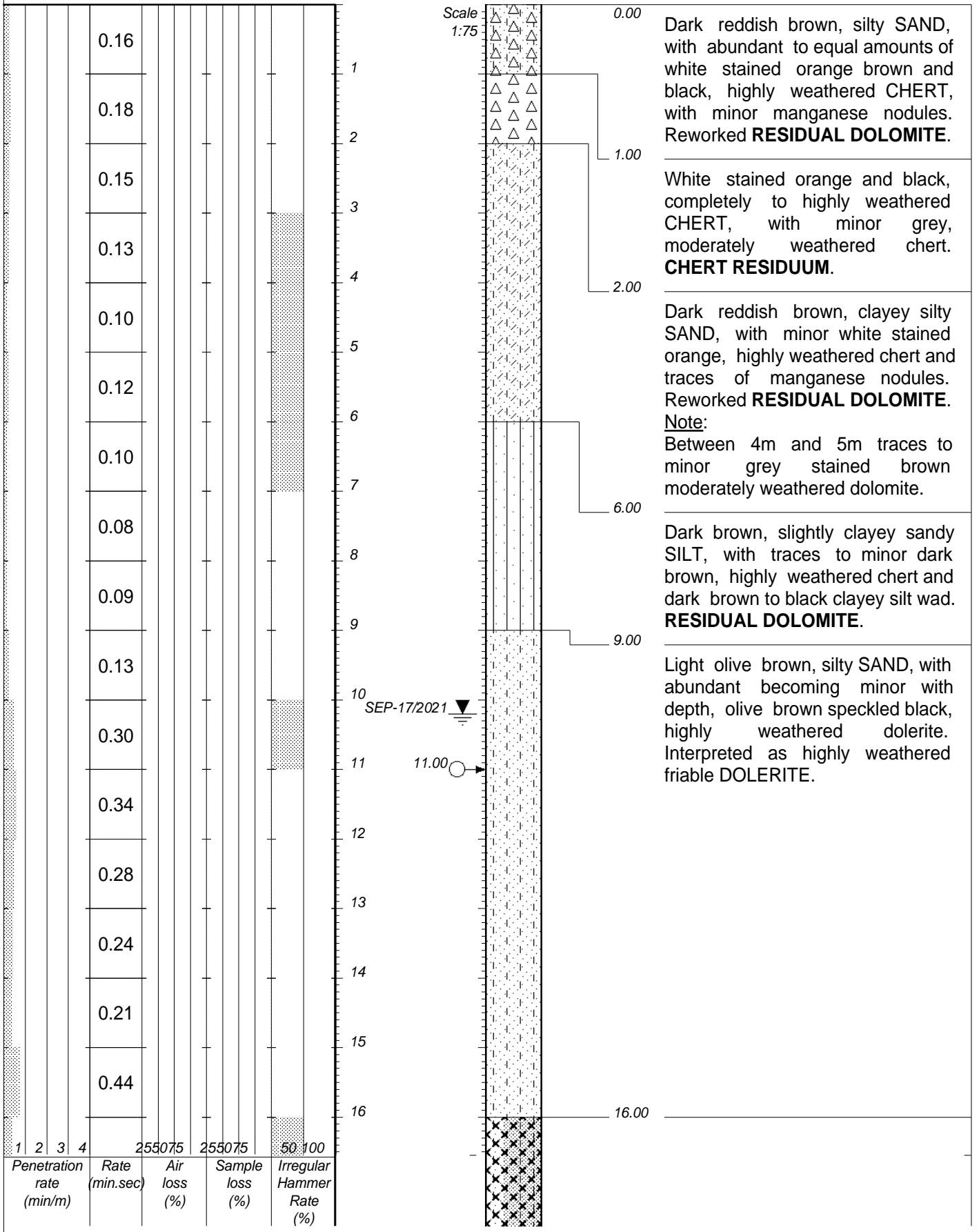
EOH: Assumed that profile continues as above.

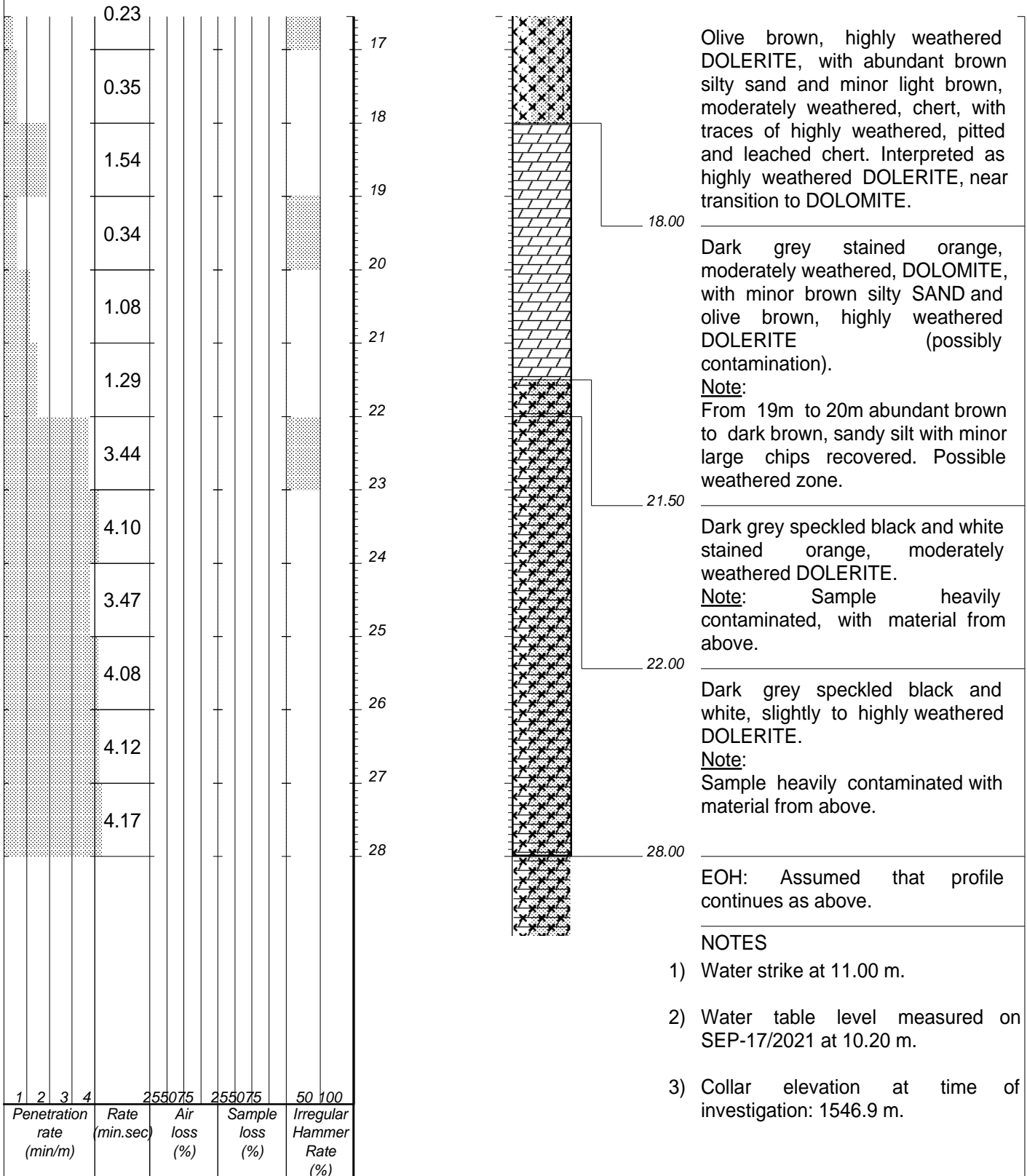
NOTES

- 1) No water strikes noticed.
- 2) Water table level measured on SEP-17/2021 at 10.90 m.
- 3) Collar elevation at time of investigation: 1543.3 m.

CONTRACTOR : Hennie Erwee Drilling
MACHINE : Super Rock 1000
DRILLED BY : Jack
PROFILED BY : Deon Bester (114260)
INCLINATION : Vertical
DIAM : 165mm
DATE : 20 Sep 2021
DATE : 20 Sep 2021
COORDINATE SYSTEM : WGS84 (Lo29)
X-COORD : 2916880.06
Y-COORD : 76273.02
TYPE SET BY : EM
SETUP FILE : KPTP8.SET
DATE : 22/11/2021 09:34
TEXT : ..1\PROFILES\PEHPERCBH.TXT

**FEASIBILITY, GEOTECHNICAL AND
DOLOMITE STABILITY INVESTIGATION**



**FEASIBILITY, GEOTECHNICAL AND
 DOLOMITE STABILITY INVESTIGATION**

CONTRACTOR : Hennie Erwee Drilling
MACHINE : Super Rock 1000
DRILLED BY : Jack
PROFILED BY : Deon Bester (114260)

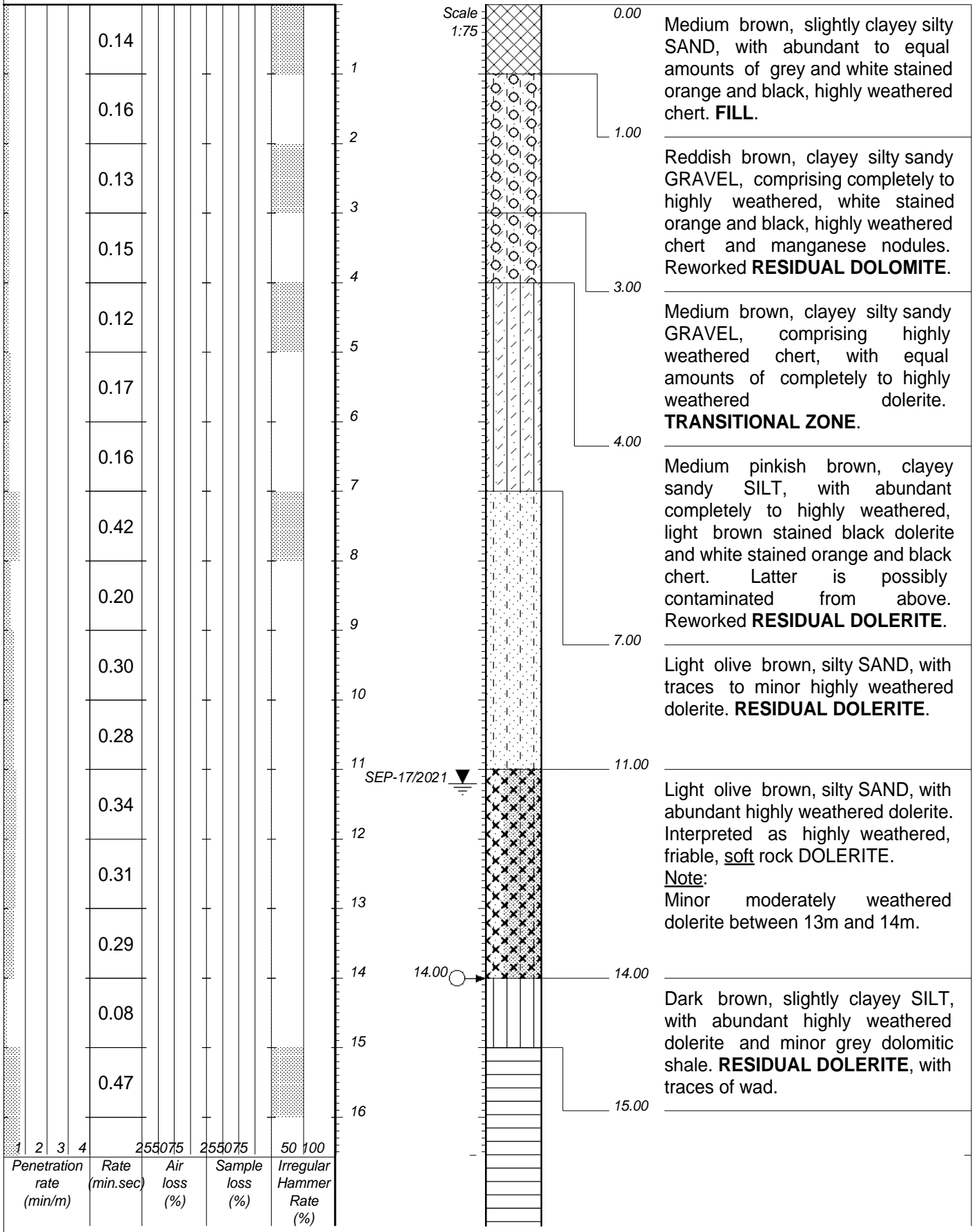
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DIAM : 165mm
DATE : 20 Sep 2021
DATE : 20 Sep 2021

COORDINATE SYSTEM : WGS84 (Lo29)
X-COORD : 2916797.02
Y-COORD : 76116

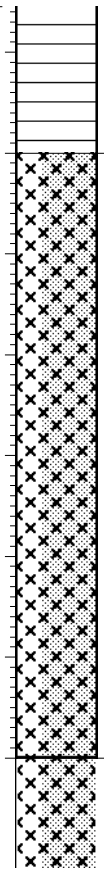
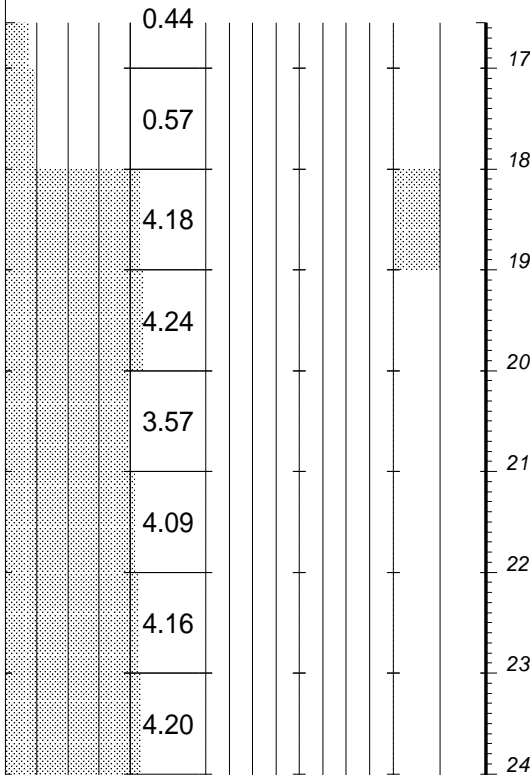
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SETUP FILE : KTP8.SET

DATE : 22/11/2021 09:34
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**FEASIBILITY, GEOTECHNICAL AND
DOLOMITE STABILITY INVESTIGATION**



**FEASIBILITY, GEOTECHNICAL AND
DOLOMITE STABILITY INVESTIGATION**



Light yellow brown stained black, highly weathered DOLERITE, with equal amounts of light grey dolomitic SHALE and minor dark brown, clayey silt and traces of wad (possibly contamination).
RESIDUAL DOLOMITIC SHALE, contaminated with highly weathered dolerite from above.

Notes:
1. Samples heavily contaminated, with material from above.

Dark grey to black speckled white, slightly weathered hard rock DOLERITE.

EOH: Assumed that profile continues as above.

- NOTES**
- 1) Water strike at 14.00 m.
 - 2) Water table level measured on SEP-17/2021 at 11.20 m.
 - 3) Collar elevation at time of investigation: 1545.7 m.

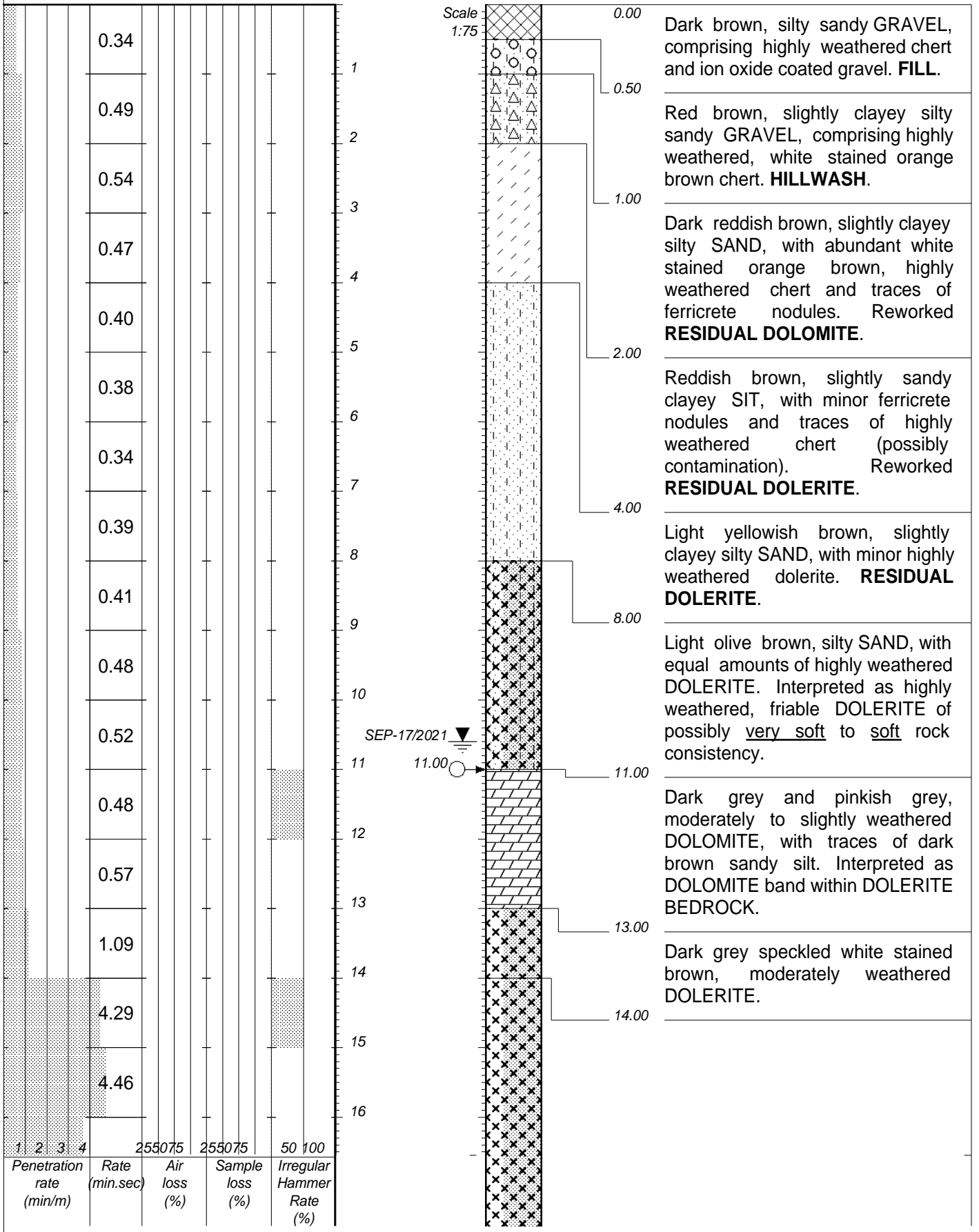
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Penetration rate (min/m)	Rate (min.sec)	Air loss (%)	Sample loss (%)	Irregular Hammer Rate (%)			

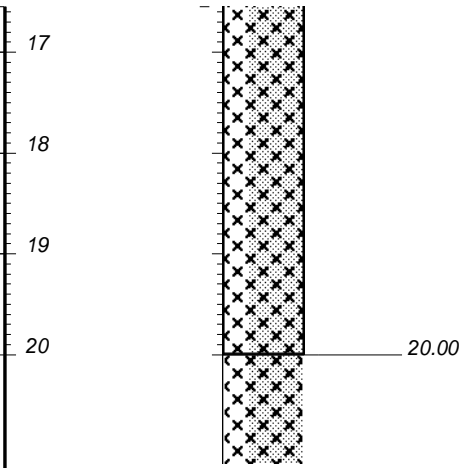
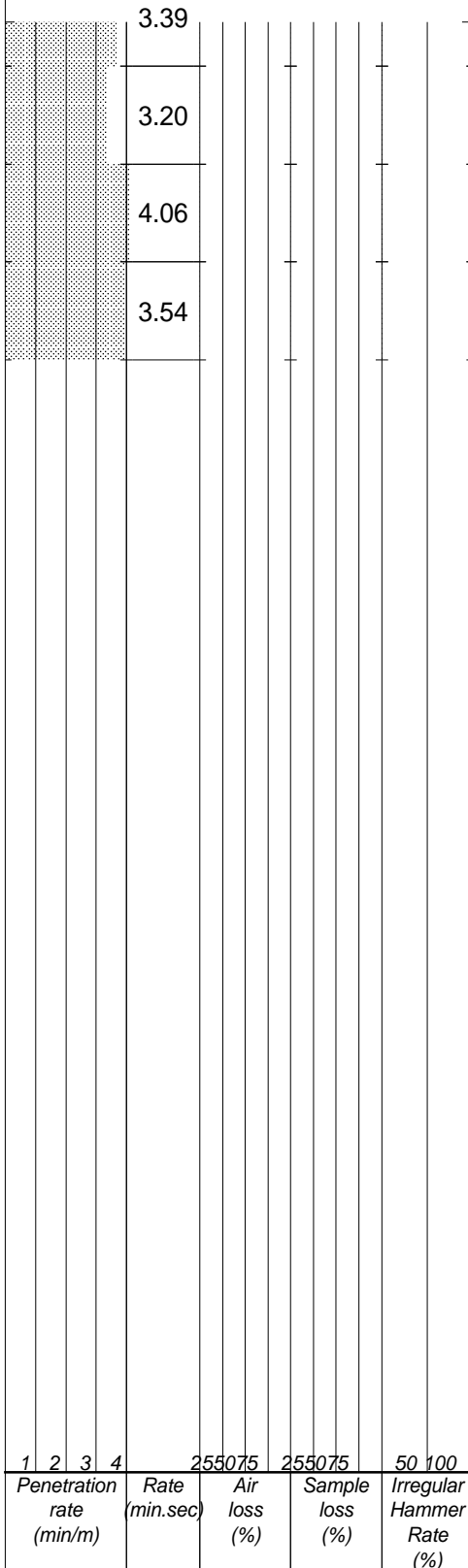
CONTRACTOR: Hennie Erwee Drilling
MACHINE: Super Rock 1000
DRILLED BY: Jack
PROFILED BY: Deon Bester (114260)
TYPE SET BY: EM
SETUP FILE: KPTP8.SET

INCLINATION: Vertical
DIAM: 165mm
DATE: 17 Sep 2021
DATE: 17 Sep 2021
DATE: 22/11/2021 09:34
TEXT: ..1\PROFILES\PEHPERCBH.TXT

COORDINATE SYSTEM: WGS84 (Lo29)
X-COORD: 2916709.25
Y-COORD: 76206.66

FEASIBILITY, GEOTECHNICAL AND
DOLOMITE STABILITY INVESTIGATION



**FEASIBILITY, GEOTECHNICAL AND
 DOLOMITE STABILITY INVESTIGATION**


Dark grey speckled white, slightly weathered, hard rock DOLERITE.
Note:
 Occasional moderately weathered chips, possibly moderately weathered/fractured in places.

EOH: Assumed that profile continues as above.

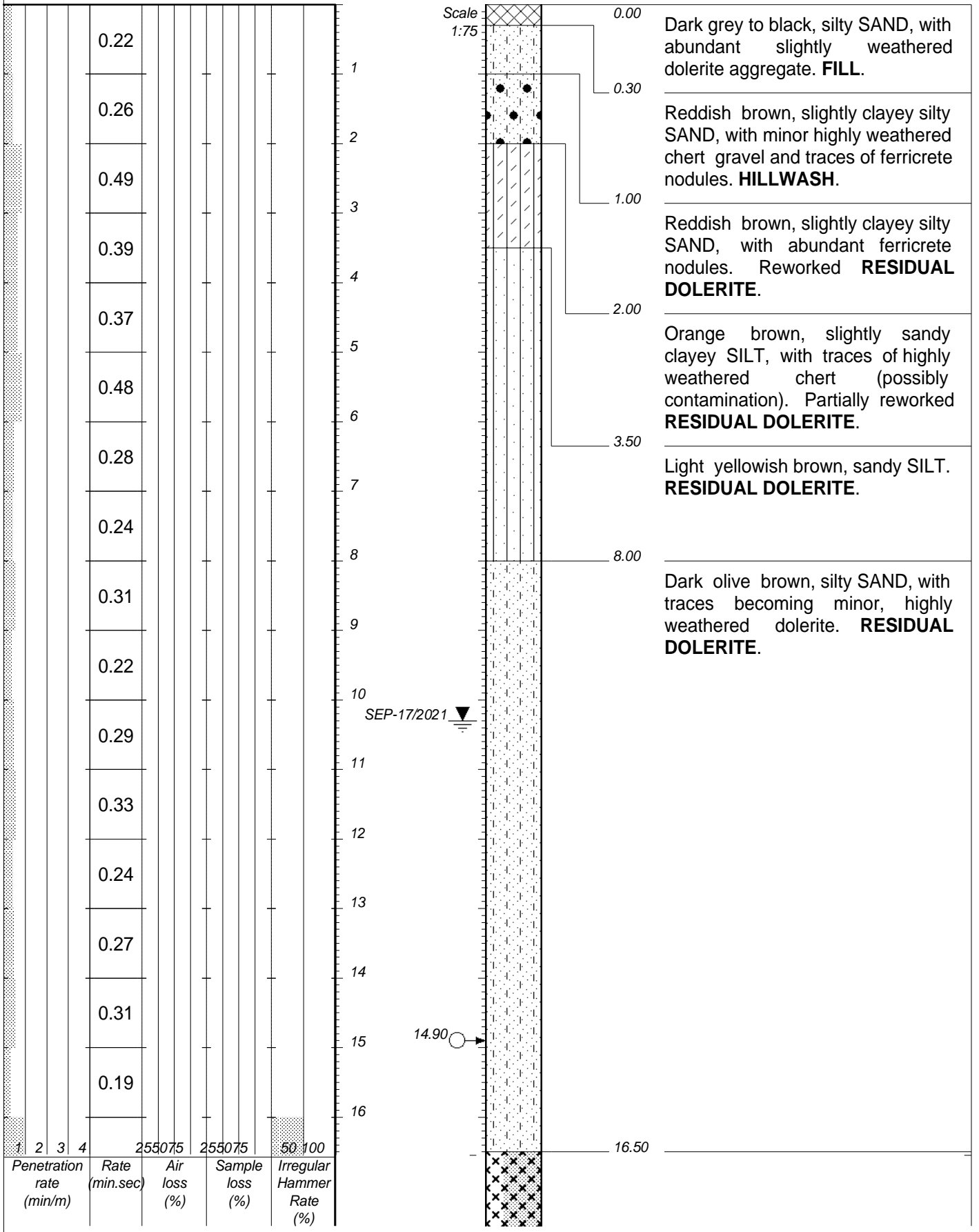
- NOTES**
- 1) Water strike at 11.00 m.
 - 2) Water table level measured on SEP-17/2021 at 10.60 m.
 - 3) Collar elevation at time of investigation: 1545 m.

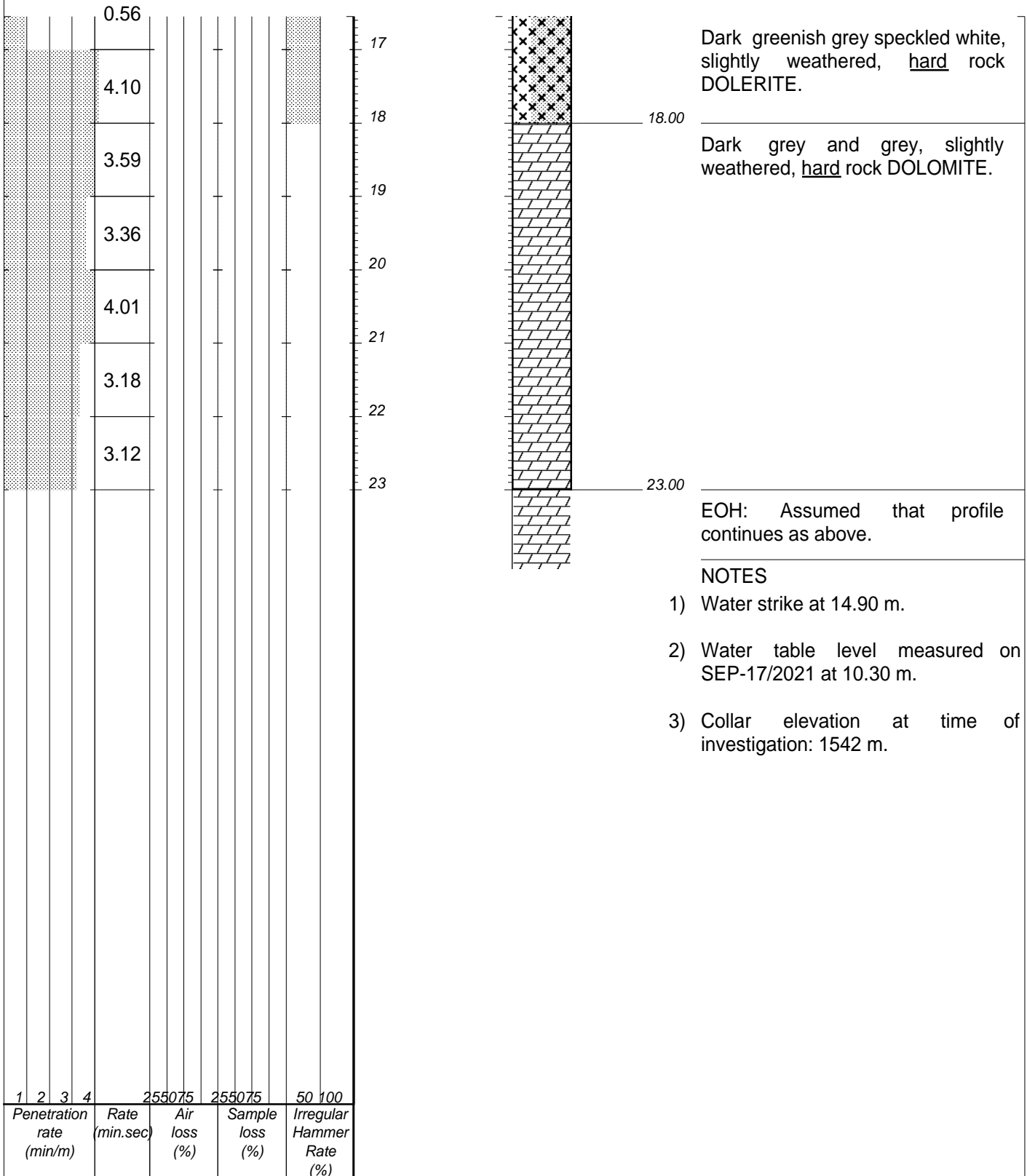
CONTRACTOR : Hennie Erwee Drilling
MACHINE : Super Rock 1000
DRILLED BY : Jack
PROFILED BY : Deon Bester (114260)
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM : 165mm
DATE : 20 Sep 2021
DATE : 20 Sep 2021
DATE : 22/11/2021 09:34
TEXT : ..1\PROFILES\PEHPERCBH.TXT

COORDINATE SYSTEM : WGS84 (Lo29)
X-COORD : 2916845.85
Y-COORD : 76222.32

FEASIBILITY, GEOTECHNICAL AND
DOLOMITE STABILITY INVESTIGATION



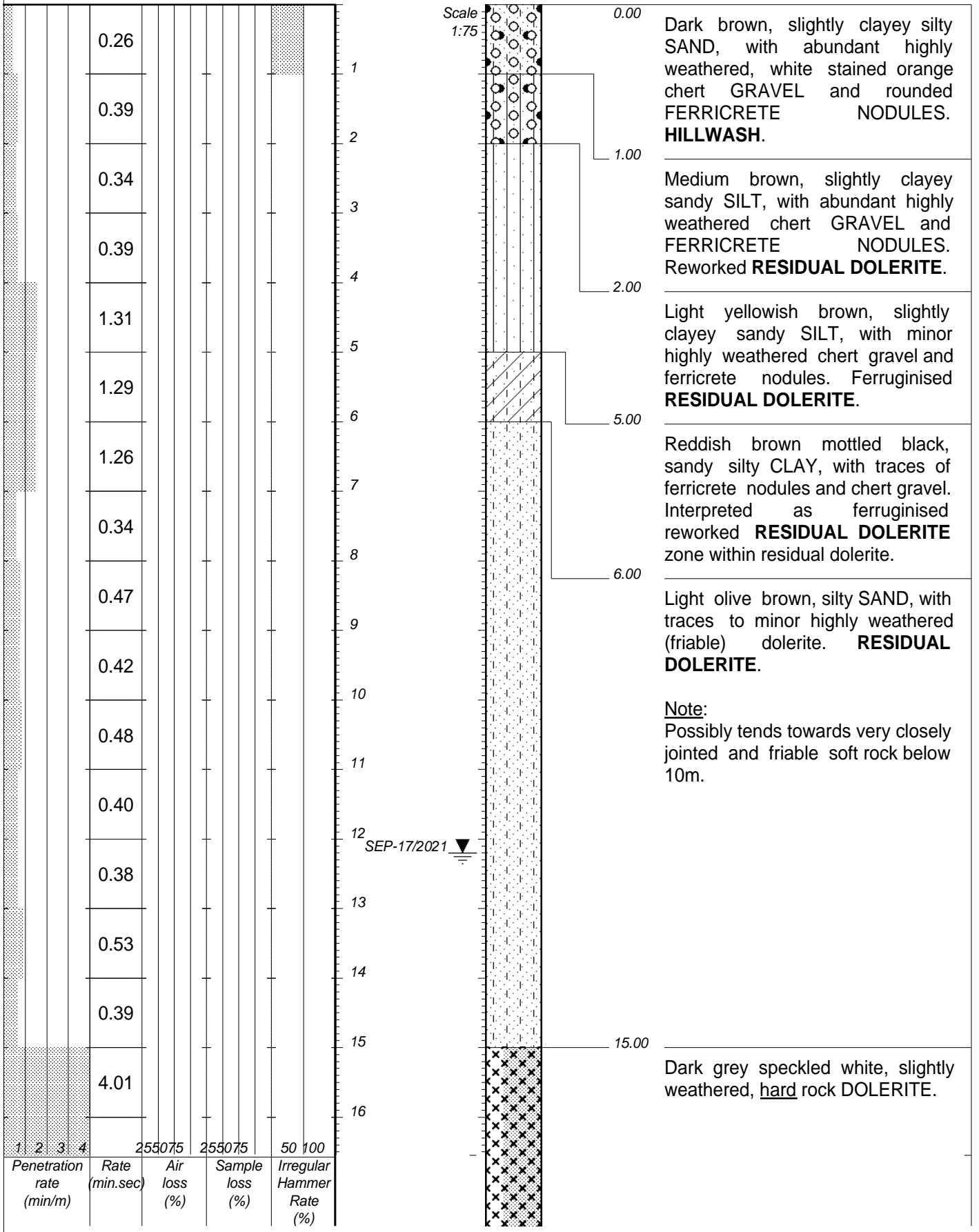
**FEASIBILITY, GEOTECHNICAL AND
 DOLOMITE STABILITY INVESTIGATION**


CONTRACTOR : Hennie Erwee Drilling
MACHINE : Super Rock 1000
DRILLED BY : Japhta
PROFILED BY : Deon Bester (114260)
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

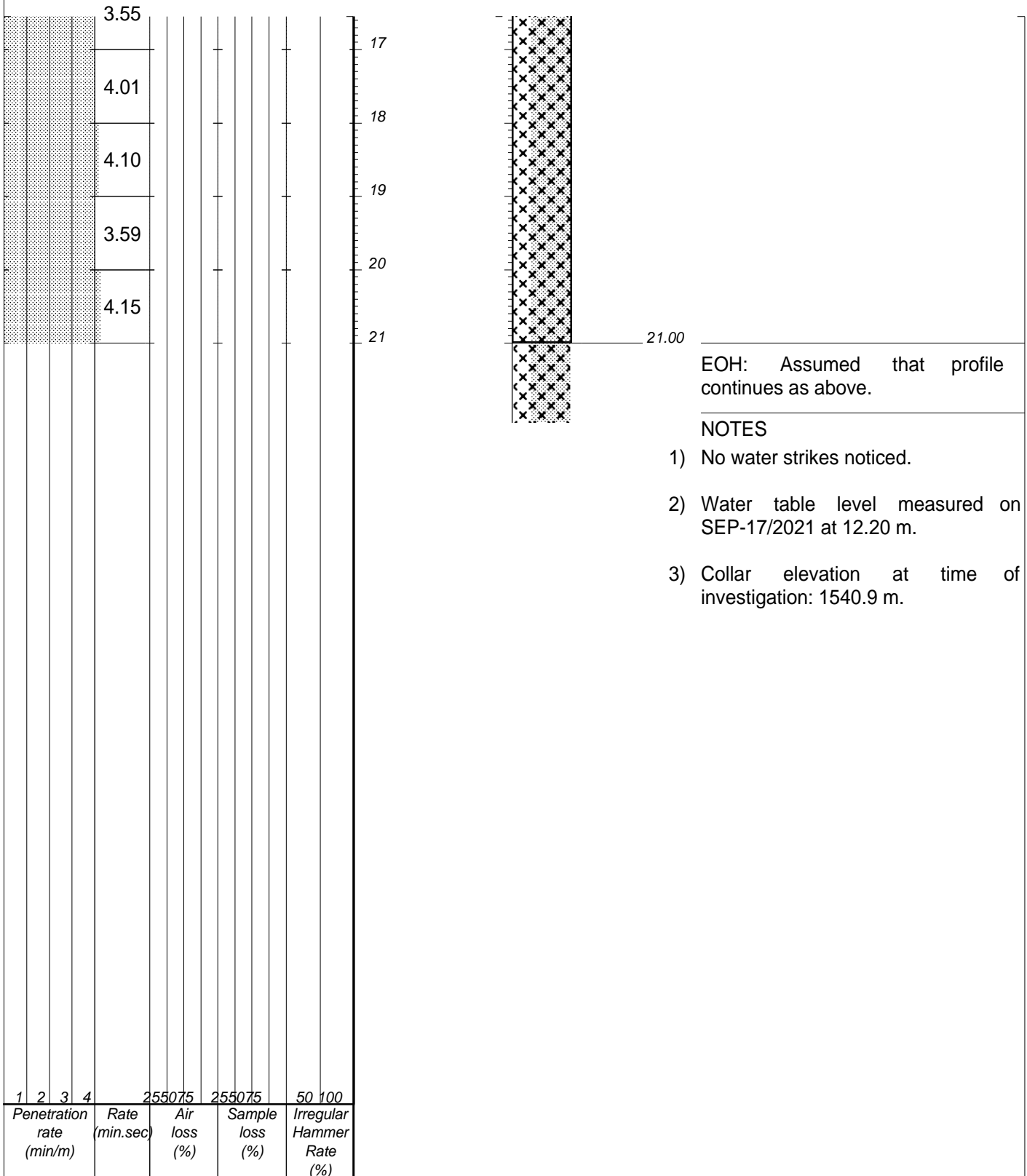
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DIAM : 165mm
DATE : 20 Sep 2021
DATE : 20 Sep 2021

COORDINATE SYSTEM : WGS84 (Lo29)
X-COORD : 2916706.89
Y-COORD : 76370.48

**FEASIBILITY, GEOTECHNICAL AND
DOLOMITE STABILITY INVESTIGATION**



1	2	3	4	255075	255075	50	100
Penetration rate (min/m)	Rate (min.sec)	Air loss (%)	Sample loss (%)	Irregular Hammer Rate (%)			

**FEASIBILITY, GEOTECHNICAL AND
 DOLOMITE STABILITY INVESTIGATION**


EOH: Assumed that profile continues as above.

NOTES

- 1) No water strikes noticed.
- 2) Water table level measured on SEP-17/2021 at 12.20 m.
- 3) Collar elevation at time of investigation: 1540.9 m.

CONTRACTOR : Hennie Erwee Drilling
MACHINE : Super Rock 1000
DRILLED BY : Japhta
PROFILED BY : Deon Bester (114260)

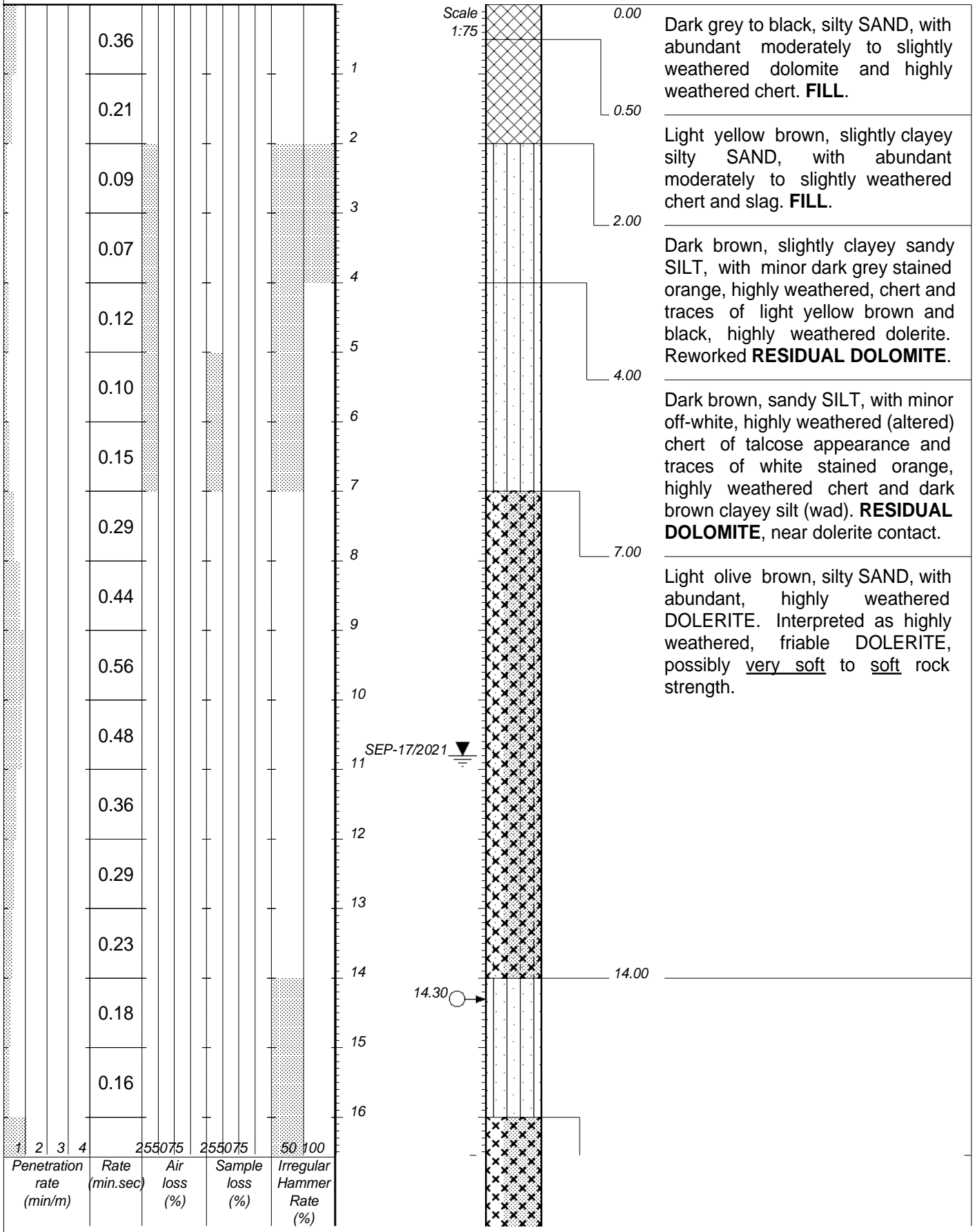
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DIAM : 165mm
DATE : 17 Sep 2021
DATE : 17 Sep 2021

COORDINATE SYSTEM : WGS84 (Lo29)
X-COORD : 2916861.72
Y-COORD : 76377.64

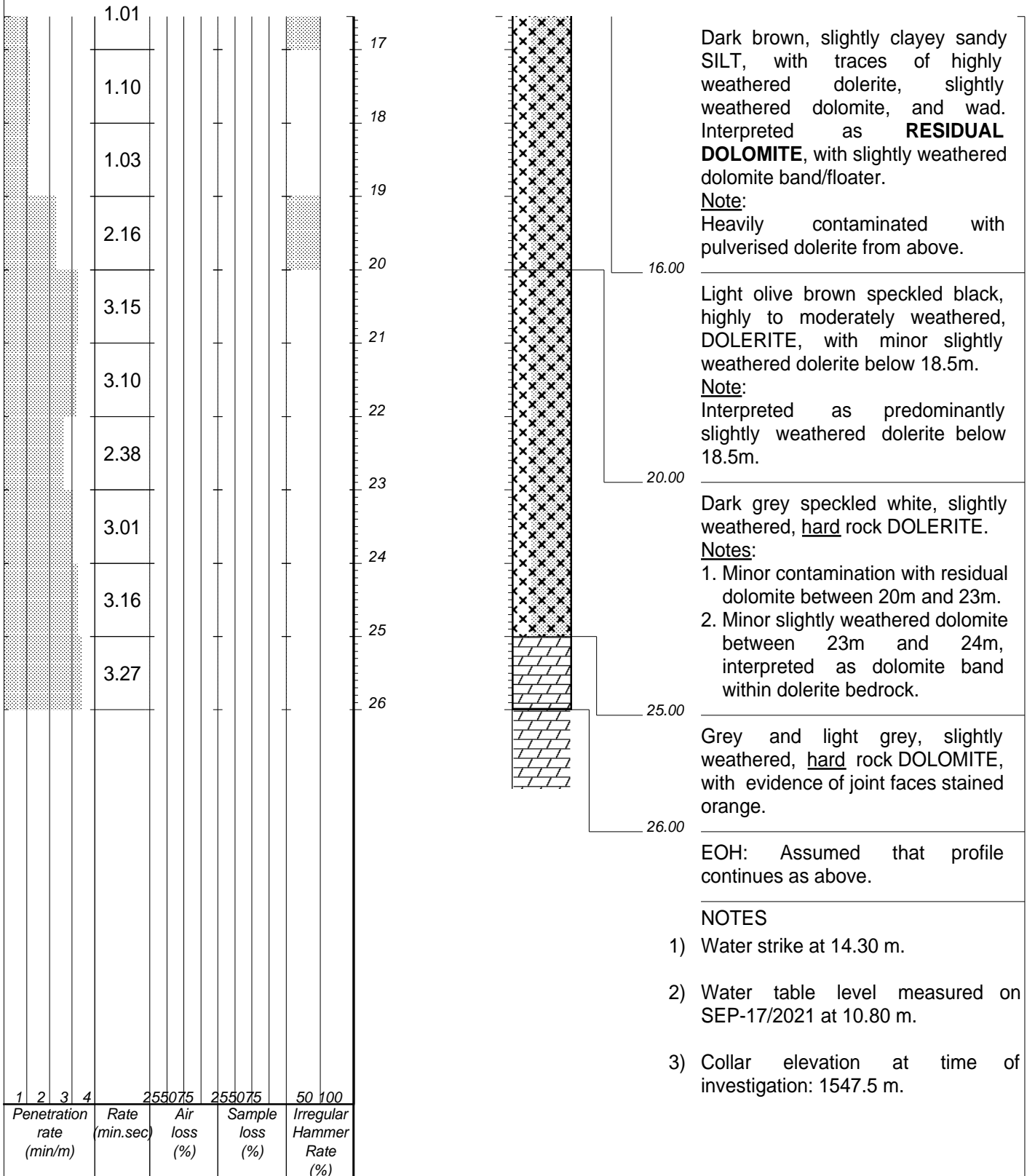
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SETUP FILE : KPTP8.SET

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TEXT : ..1\PROFILES\PEHPERCBH.TXT

**FEASIBILITY, GEOTECHNICAL AND
DOLOMITE STABILITY INVESTIGATION**



**FEASIBILITY, GEOTECHNICAL AND
DOLOMITE STABILITY INVESTIGATION**

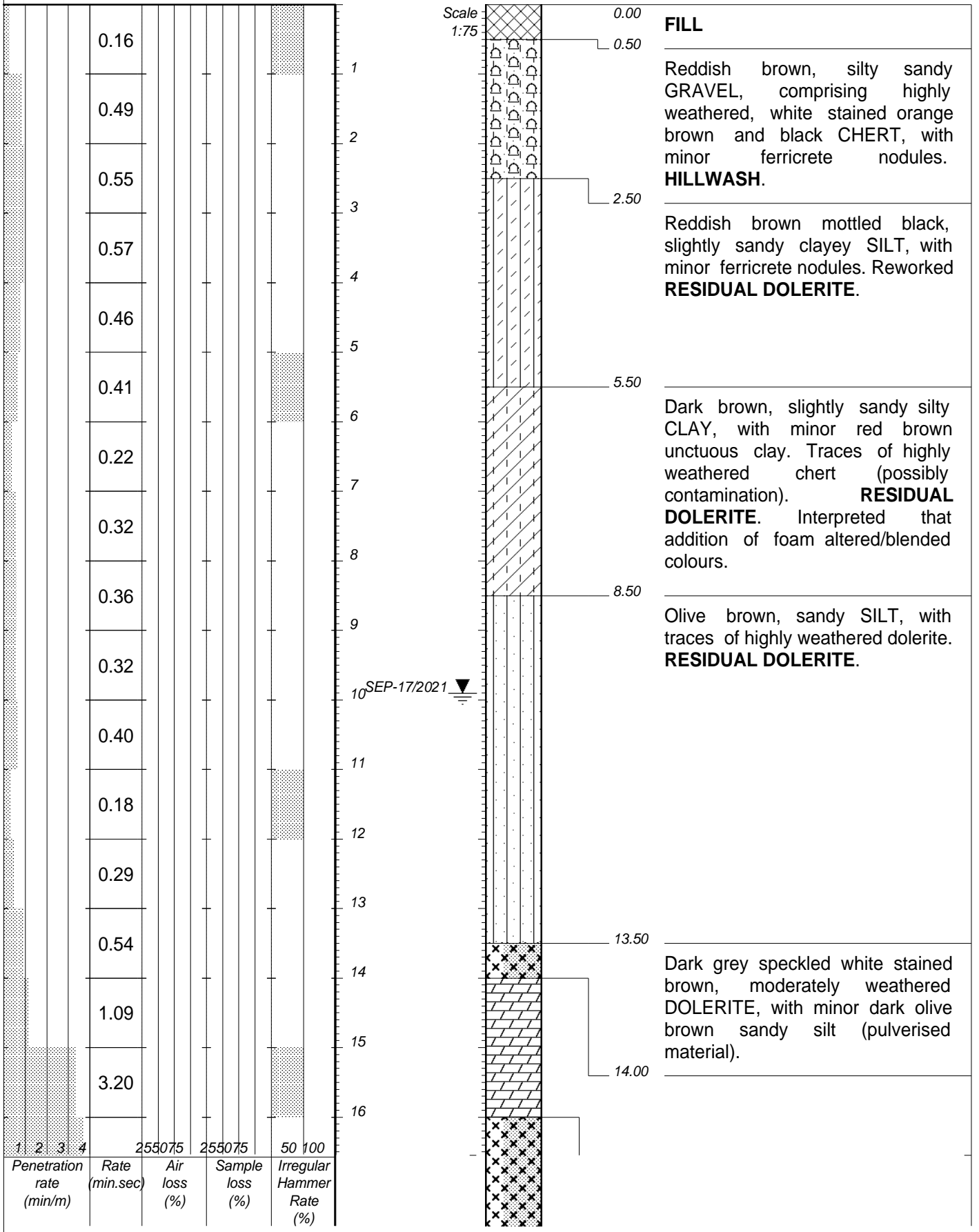


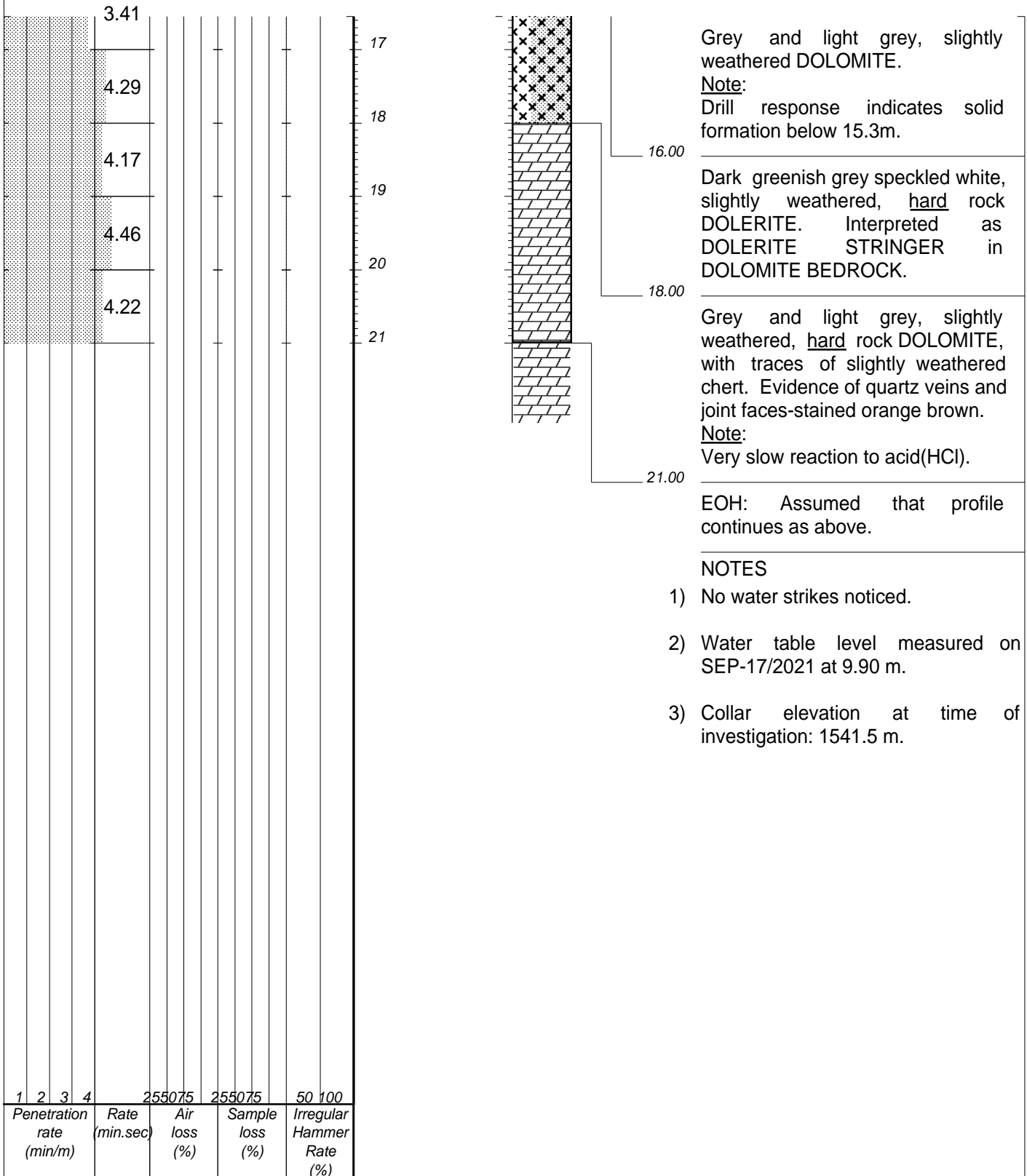
CONTRACTOR : Hennie Erwee Drilling
MACHINE : Super Rock 1000
DRILLED BY : Jack
PROFILED BY : Deon Bester (114260)
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM : 165mm
DATE : 20 Sep 2021
DATE : 20 Sep 2021

COORDINATE SYSTEM : WGS84 (Lo29)
X-COORD : 2916881.86
Y-COORD : 76109.91

FEASIBILITY, GEOTECHNICAL AND
DOLOMITE STABILITY INVESTIGATION



**FEASIBILITY, GEOTECHNICAL AND
 DOLOMITE STABILITY INVESTIGATION**


Grey and light grey, slightly weathered DOLOMITE.
Note:
 Drill response indicates solid formation below 15.3m.

Dark greenish grey speckled white, slightly weathered, hard rock DOLERITE. Interpreted as DOLERITE STRINGER in DOLOMITE BEDROCK.

Grey and light grey, slightly weathered, hard rock DOLOMITE, with traces of slightly weathered chert. Evidence of quartz veins and joint faces-stained orange brown.
Note:
 Very slow reaction to acid(HCl).

EOH: Assumed that profile continues as above.

- NOTES**
- 1) No water strikes noticed.
 - 2) Water table level measured on SEP-17/2021 at 9.90 m.
 - 3) Collar elevation at time of investigation: 1541.5 m.

CONTRACTOR : Hennie Erwee Drilling
MACHINE : Super Rock 1000
DRILLED BY : Jack
PROFILED BY : Deon Bester (114260)
TYPE SET BY : EM
SETUP FILE : KTP8.SET

INCLINATION : Vertical
DIAM : 165mm
DATE : 20 Sep 2021
DATE : 20 Sep 2021
DATE : 22/11/2021 09:34
TEXT : ..1\PROFILES\PEHPERCBH.TXT

COORDINATE SYSTEM : WGS84 (Lo29)
X-COORD : 2916780.35
Y-COORD : 76352.87

APPENDIX D

Laboratory Test Results

Client Name: Knight Piesold
Project Name: PR311-00288/72: Proposed New Chemical Plant, Vosloorus
Job Number: KNP-34
Date: 04-Nov-21
Method: SANS 3001 GR1, GR3, GR10, GR12 GR20, GR30, GR31, GR40, GR50, GR53, GR54 & BS 1377 (where applicable)

SUMMARY OF TEST DATA

Grading & Hydrometer Analysis (% Passing)

Sample	TP1/1	TP1/2	TP2/1	TP2/2	TP3/1	TP4/1	TP5/1	TP6/1
Depth (m)	2.1 - 3.0	1.0 - 1.4	1.0 - 1.4	1.4 - 2.7	1.9 - 3.1	1.3 - 3.2	1.6 - 2.7	0.85 - 1.5
Lab No	KNP-34-534	KNP-34-535	KNP-34-536	KNP-34-537	KNP-34-538	KNP-34-539	KNP-34-540	KNP-34-541
53.0	100	100	100	96	100	90	100	100
37.5	100	100	100	96	100	90	100	100
26.5	100	100	100	93	100	90	100	100
19.0	100	100	100	85	100	89	100	100
13.2	100	100	99	68	100	88	100	100
9.5	100	100	98	60	100	86	100	100
6.7	97	100	96	53	100	83	100	100
4.75	93	100	95	49	99	79	100	99
2.00	85	98	92	42	95	68	98	94
1.00	81	94	90	39	88	63	97	89
0.425	78	87	85	35	74	58	95	84
0.250	74	79	81	31	62	55	90	79
0.150	70	73	77	28	49	51	84	75
0.075	62	64	72	25	33	45	74	69
0.060	57	60	68	24	28	42	69	65
0.050	53	58	65	23	25	40	66	62
0.035	45	53	60	21	19	37	59	56
0.020	36	46	54	20	14	33	51	49
0.006	25	38	42	17	8	27	35	38
0.002	19	32	37	14	5	22	25	30
GM	0.75	0.51	0.51	1.98	0.98	1.29	0.33	0.53

Atterberg Limits

LL (%)	42	35	41	38	31	40	54	44
PI (%)	22	18	19	16	8	20	24	20
LS (%)	11.0	9.5	9.5	8.5	4.0	10.0	11.0	10.5

pH & Conductivity

pH	5.7				5.5			
EC (S/m)	0.022				0.022			

MDD / OMC

MDD (kg/m ³)				1805				
OMC (%)				14.9				

CBR

100%				24				
98%				22				
97%				21				
95%				19				
93%				16				
90%				13				
Swell (%)				0.1				

UCS (MPa)

100%								
97%								
90%								

COLTO Classification

				*				
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Remarks: * = Not Classifiable



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 Gerrie | 082 309 4448 | gerrie@stlab.co.za
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Client Name: Knight Piesold
Project Name: PR311-00288/72: Proposed New Chemical Plant, Vosloorus
Job Number: KNP-34
Date: 04-Nov-21
Method: SANS 3001 GR1, GR3, GR10, GR12 GR20, GR30, GR31, GR40, GR50, GR53, GR54 & BS 1377 (where applicable)

SUMMARY OF TEST DATA

Grading & Hydrometer Analysis (% Passing)

Sample	TP8/1	TP8/2	TP9/1	TP11/1				
Depth (m)	2.2 - 3.2	0.8 - 2.2	2.8 - 3.2	0.7 - 2.6				
Lab No	KNP-34-542	KNP-34-543	KNP-34-544	KNP-34-545				
53.0	100	79	100	70				
37.5	100	71	100	65				
26.5	100	67	98	61				
19.0	100	60	95	55				
13.2	100	54	94	47				
9.5	99	48	92	44				
6.7	97	42	90	41				
4.75	95	38	89	38				
2.00	83	28	79	27				
1.00	77	25	75	25				
0.425	72	23	70	23				
0.250	66	20	65	21				
0.150	59	18	59	19				
0.075	50	14	45	16				
0.060	47	13	39	14				
0.050	45	12	37	13				
0.035	40	11	33	11				
0.020	35	10	30	10				
0.006	30	7	25	8				
0.002	24	5	22	5				
GM	0.95	2.35	1.06	2.34				

Atterberg Limits

LL (%)	29	27	31	31				
PI (%)	14	12	13	15				
LS (%)	7.0	6.0	6.5	7.5				

pH & Conductivity

pH				5.8				
EC (S/m)				0.011				

MDD / OMC

MDD (kg/m ³)		2079		2012				
OMC (%)		8.6		9.6				

CBR

100%		35		42				
98%		31		36				
97%		30		33				
95%		26		27				
93%		24		21				
90%		20		14				
Swell (%)		0.0		0.1				

UCS (MPa)

100%								
97%								
90%								

COLTO Classification

		G6		G6				
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Remarks: * = Not Classifiable

Although everything possible is done to ensure testing is performed accurately, neither Specialised Testing Laboratory (Pty) Ltd nor any of its directors, managers, employees or contractors can be held liable for any damages whatsoever arising from any error made in performing any tests, nor from any conclusions drawn therefrom. Test results are to be published in full. Samples will be kept for 1 month after the submission of test results due to limited storage space, unless other arrangements are in place.



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Client Name: Knight Piesold
Project Name: PR311-00288/72: Proposed New Chemical Plant, Vosloorus
Job Number: KNP-34
Date: 2021-11-04
Method: SANS 3001 GR1, GR3, GR10 GR12 & BS 1377 (where applicable)

FOUNDATION INDICATOR				Sheet Reference: R-STL-011 Rev02			
Grading & Hydrometer Analysis (Particle Size (mm) & % Passing)				Atterberg Limits & Classification			
Sample	TP1/1	TP1/2	TP2/1	Sample	TP1/1	TP1/2	TP2/1
Depth (m)	2.1 - 3.0	1.0 - 1.4	1.0 - 1.4	Depth (m)	2.1 - 3.0	1.0 - 1.4	1.0 - 1.4
Lab No	KNP-34-534	KNP-34-535	KNP-34-536	Lab No	KNP-34-534	KNP-34-535	KNP-34-536
53.0	100	100	100	Liquid Limit (%)	42	35	41
37.5	100	100	100	Plastic Limit (%)	20	17	22
26.5	100	100	100	Plasticity Index (%)	22	18	19
19.0	100	100	100	Linear Shrinkage (%)	11.0	9.5	9.5
13.2	100	100	99	PI of whole sample	17	16	16
9.5	100	100	98				
6.7	97	100	96	% Gravel	15	2	8
4.75	93	100	95	% Sand	28	38	24
2.00	85	98	92	% Silt	38	28	31
1.00	81	94	90	% Clay	19	32	37
0.425	78	87	85	Activity	1.2	0.6	0.5
0.250	74	79	81				
0.150	70	73	77	% Soil Mortar	85	98	92
0.075	62	64	72				
0.060	57	60	68	Grading Modulus	0.75	0.51	0.51
0.050	53	58	65	Moisture Content (%)	N / T	N / T	N / T
0.035	45	53	60	Relative Density (SG)*	2.65	2.65	2.65
0.020	36	46	54				
0.006	25	38	42	Unified (ASTM D2487)	CL	CL	CL
0.002	19	32	37	AASHTO (M145-91)	A - 7 - 6	A - 6	A - 7 - 6
Remarks: *: Assumed							
N / T: Not Tested							
<small>Although everything possible is done to ensure testing is performed accurately, neither Specialised Testing Laboratory (Pty) Ltd nor any of its directors, managers, employees or contractors can be held liable for any damages whatsoever arising from any error made in performing any tests, nor from any conclusions drawn therefrom. Test results are to be published in full. Samples will be kept for 1 month after the submission of test results due to limited storage space, unless other arrangements are in place. Confidentiality statement: Unless the release of information is required by law or covered by confidentiality agreements all information obtained or created during the performance of laboratory activities will be kept confidential.</small>							



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Gerrie | 082 309 4448 | gerrie@stlab.co.za

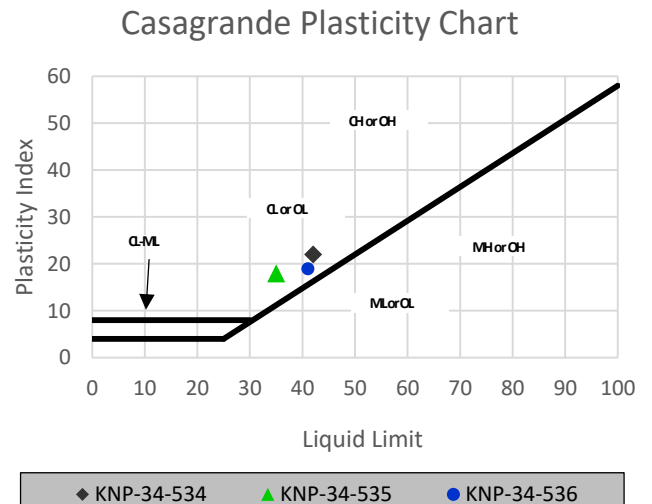
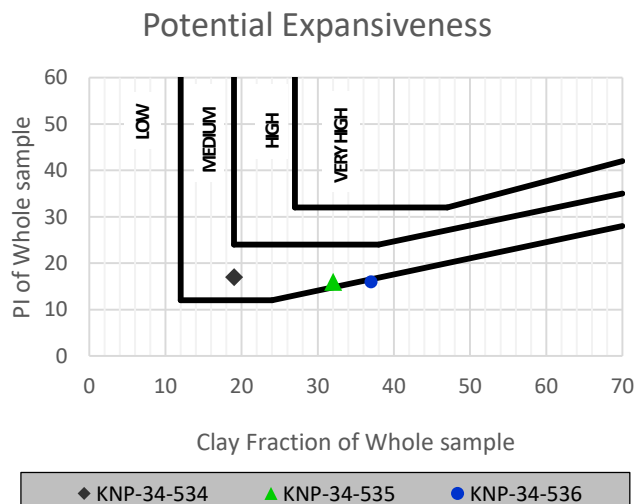
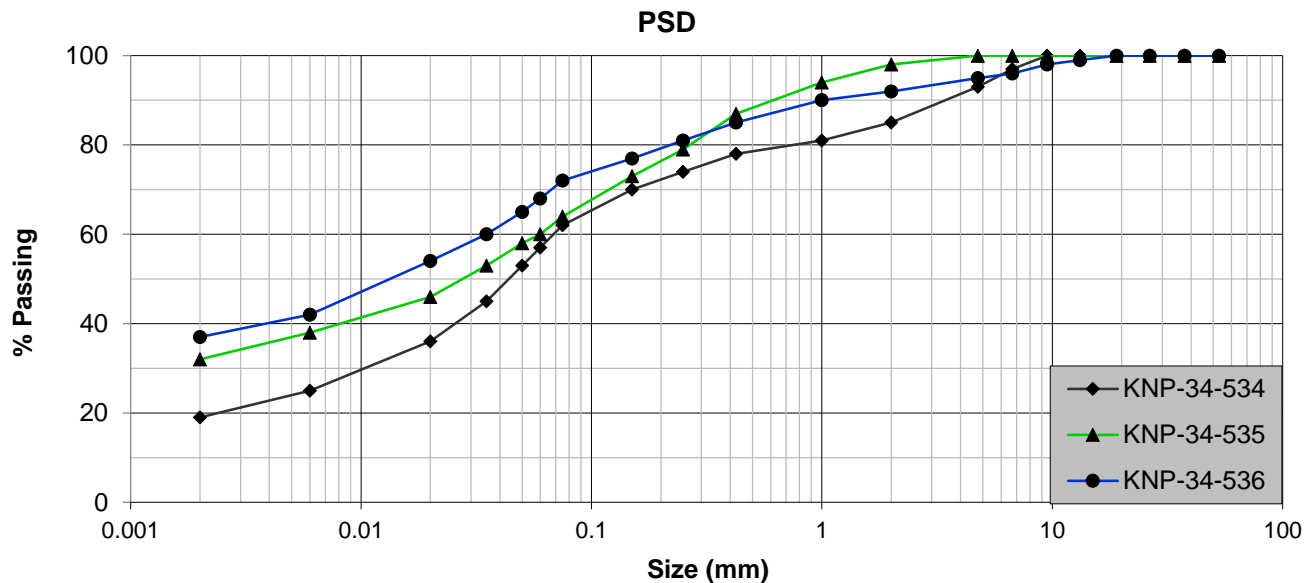
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Client Name: Knight Piesold
Project Name: PR311-00288/72: Proposed New Chemical Plant, Vosloorus
Job Number: KNP-34
Date: 2021-11-04
Method: SANS 3001 GR1, GR3, GR10 GR12 & BS 1377 (where applicable)

FOUNDATION INDICATOR

Sheet Reference:
R-STL-011 Rev02



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Client Name: Knight Piesold
Project Name: PR311-00288/72: Proposed New Chemical Plant, Vosloorus
Job Number: KNP-34
Date: 2021-11-04
Method: SANS 3001 GR1, GR3, GR10 GR12 & BS 1377 (where applicable)

FOUNDATION INDICATOR				Sheet Reference: R-STL-011 Rev02			
Grading & Hydrometer Analysis (Particle Size (mm) & % Passing)				Atterberg Limits & Classification			
Sample	TP2/2	TP3/1	TP4/1	Sample	TP2/2	TP3/1	TP4/1
Depth (m)	1.4 - 2.7	1.9 - 3.1	1.3 - 3.2	Depth (m)	1.4 - 2.7	1.9 - 3.1	1.3 - 3.2
Lab No	KNP-34-537	KNP-34-538	KNP-34-539	Lab No	KNP-34-537	KNP-34-538	KNP-34-539
53.0	96	100	90	Liquid Limit (%)	38	31	40
37.5	96	100	90	Plastic Limit (%)	22	23	20
26.5	93	100	90	Plasticity Index (%)	16	8	20
19.0	85	100	89	Linear Shrinkage (%)	8.5	4.0	10.0
13.2	68	100	88	PI of whole sample	6	6	12
9.5	60	100	86				
6.7	53	100	83	% Gravel	58	5	32
4.75	49	99	79	% Sand	18	67	26
2.00	42	95	68	% Silt	10	23	20
1.00	39	88	63	% Clay	14	5	22
0.425	35	74	58	Activity	1.1	1.6	0.9
0.250	31	62	55				
0.150	28	49	51	% Soil Mortar	42	95	68
0.075	25	33	45				
0.060	24	28	42	Grading Modulus	1.98	0.98	1.29
0.050	23	25	40	Moisture Content (%)	N / T	N / T	N / T
0.035	21	19	37	Relative Density (SG)*	2.65	2.65	2.65
0.020	20	14	33				
0.006	17	8	27	Unified (ASTM D2487)	GC	SC	SC
0.002	14	5	22	AASHTO (M145-91)	A - 2 - 6	A - 2 - 4	A - 6
Remarks: *: Assumed							
N / T: Not Tested							
<small>Although everything possible is done to ensure testing is performed accurately, neither Specialised Testing Laboratory (Pty) Ltd nor any of its directors, managers, employees or contractors can be held liable for any damages whatsoever arising from any error made in performing any tests, nor from any conclusions drawn therefrom. Test results are to be published in full. Samples will be kept for 1 month after the submission of test results due to limited storage space, unless other arrangements are in place. Confidentiality statement: Unless the release of information is required by law or covered by confidentiality agreements all information obtained or created during the performance of laboratory activities will be kept confidential.</small>							



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Gerrie | 082 309 4448 | gerrie@stlab.co.za

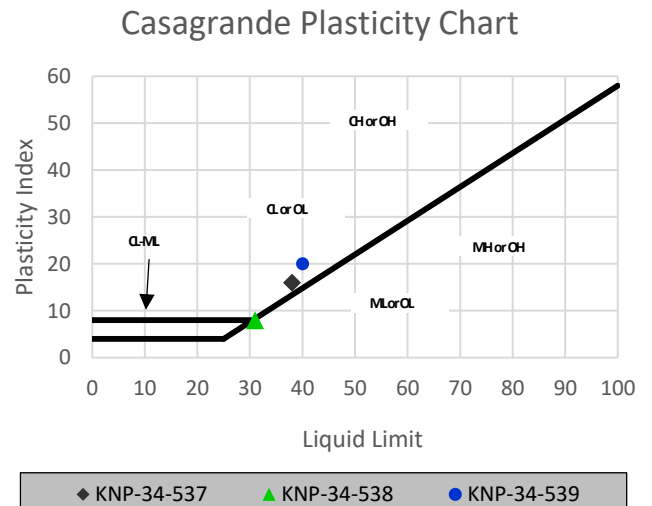
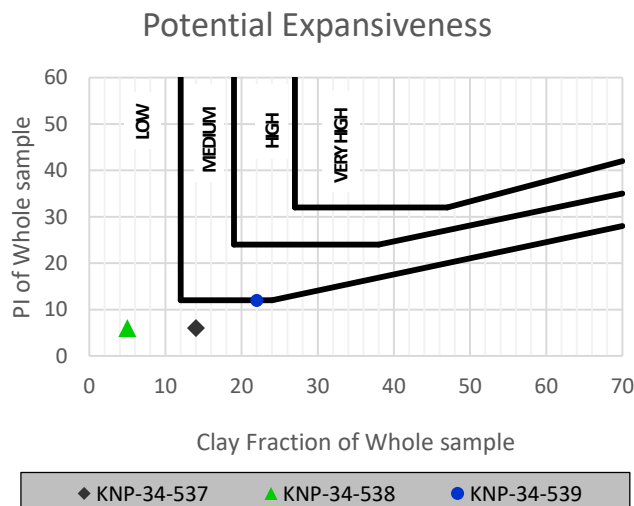
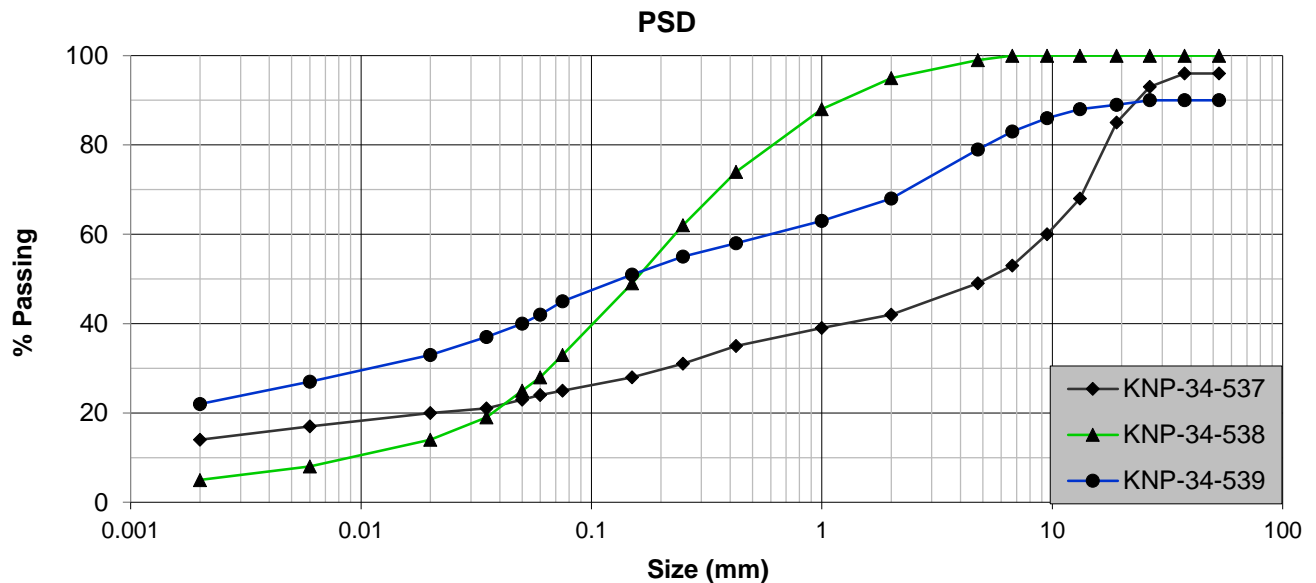
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Client Name: Knight Piesold
Project Name: PR311-00288/72: Proposed New Chemical Plant, Vosloorus
Job Number: KNP-34
Date: 2021-11-04
Method: SANS 3001 GR1, GR3, GR10 GR12 & BS 1377 (where applicable)

FOUNDATION INDICATOR

Sheet Reference:
R-STL-011 Rev02



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Quality | Excellence | On Time

Client Name: Knight Piesold
Project Name: PR311-00288/72: Proposed New Chemical Plant, Vosloorus
Job Number: KNP-34
Date: 2021-11-04
Method: SANS 3001 GR1, GR3, GR10 GR12 & BS 1377 (where applicable)

FOUNDATION INDICATOR				Sheet Reference: R-STL-011 Rev02			
Grading & Hydrometer Analysis (Particle Size (mm) & % Passing)				Atterberg Limits & Classification			
Sample	TP5/1	TP6/1	TP8/1	Sample	TP5/1	TP6/1	TP8/1
Depth (m)	1.6 - 2.7	0.85 - 1.5	2.2 - 3.2	Depth (m)	1.6 - 2.7	0.85 - 1.5	2.2 - 3.2
Lab No	KNP-34-540	KNP-34-541	KNP-34-542	Lab No	KNP-34-540	KNP-34-541	KNP-34-542
53.0	100	100	100	Liquid Limit (%)	54	44	29
37.5	100	100	100	Plastic Limit (%)	30	24	15
26.5	100	100	100	Plasticity Index (%)	24	20	14
19.0	100	100	100	Linear Shrinkage (%)	11.0	10.5	7.0
13.2	100	100	100	PI of whole sample	23	17	10
9.5	100	100	99				
6.7	100	100	97	% Gravel	2	6	17
4.75	100	99	95	% Sand	29	29	36
2.00	98	94	83	% Silt	44	35	23
1.00	97	89	77	% Clay	25	30	24
0.425	95	84	72	Activity	1.0	0.7	0.6
0.250	90	79	66				
0.150	84	75	59	% Soil Mortar	98	94	83
0.075	74	69	50				
0.060	69	65	47	Grading Modulus	0.33	0.53	0.95
0.050	66	62	45	Moisture Content (%)	N / T	N / T	N / T
0.035	59	56	40	Relative Density (SG)*	2.65	2.65	2.65
0.020	51	49	35				
0.006	35	38	30	Unified (ASTM D2487)	CH	CL	CL
0.002	25	30	24	AASHTO (M145-91)	A - 7 - 5	A - 7 - 6	A - 6
Remarks: *: Assumed							
N / T: Not Tested							
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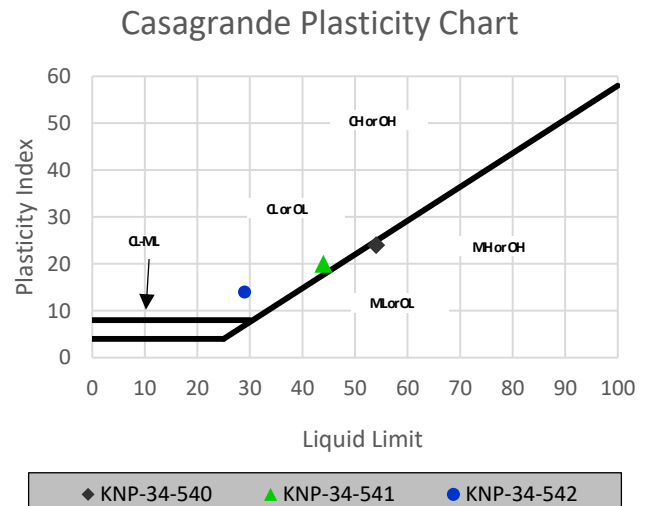
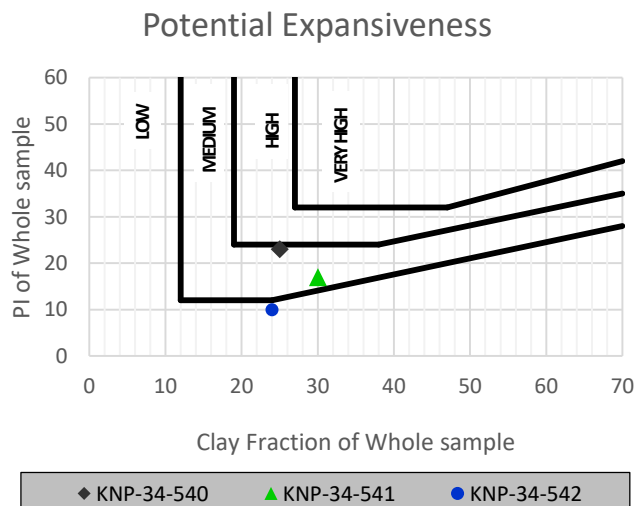
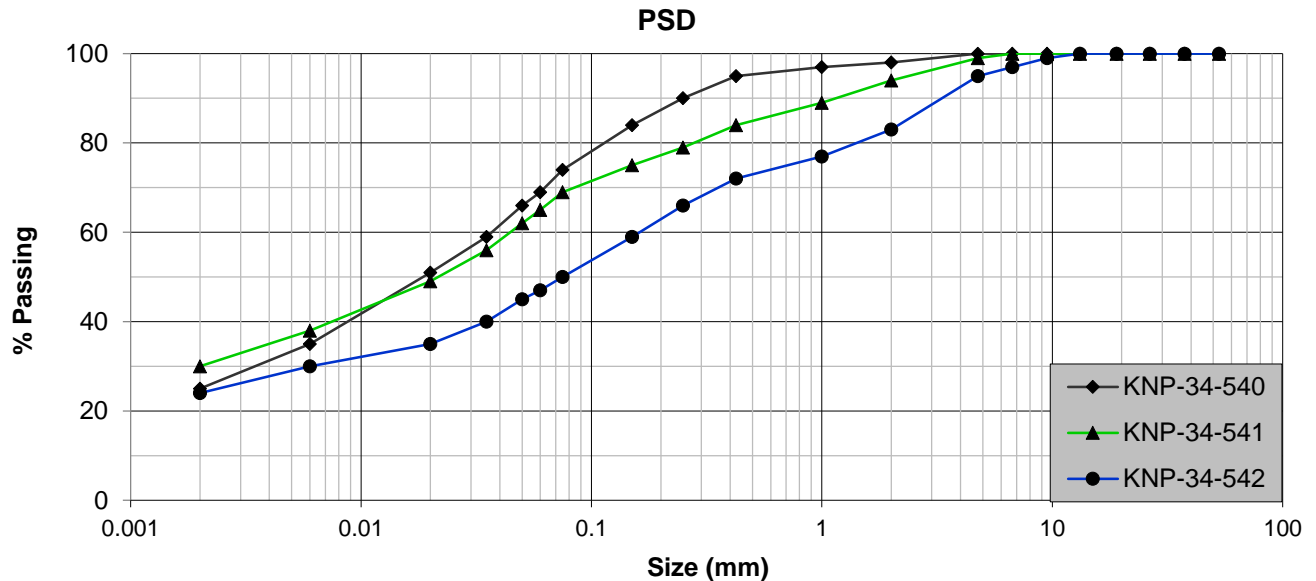


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Client Name: Knight Piesold
Project Name: PR311-00288/72: Proposed New Chemical Plant, Vosloorus
Job Number: KNP-34
Date: 2021-11-04
Method: SANS 3001 GR1, GR3, GR10 GR12 & BS 1377 (where applicable)

FOUNDATION INDICATOR

Sheet Reference:
R-STL-011 Rev02





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Client Name: Knight Piesold
Project Name: PR311-00288/72: Proposed New Chemical Plant, Vosloorus
Job Number: KNP-34
Date: 2021-11-04
Method: SANS 3001 GR1, GR3, GR10 GR12 & BS 1377 (where applicable)

FOUNDATION INDICATOR				Sheet Reference: R-STL-011 Rev02			
Grading & Hydrometer Analysis (Particle Size (mm) & % Passing)				Atterberg Limits & Classification			
Sample	TP8/2	TP9/1	TP11/1	Sample	TP8/2	TP9/1	TP11/1
Depth (m)	0.8 - 2.2	2.8 - 3.2	0.7 - 2.6	Depth (m)	0.8 - 2.2	2.8 - 3.2	0.7 - 2.6
Lab No	KNP-34-543	KNP-34-544	KNP-34-545	Lab No	KNP-34-543	KNP-34-544	KNP-34-545
53.0	79	100	70	Liquid Limit (%)	27	31	31
37.5	71	100	65	Plastic Limit (%)	15	18	16
26.5	67	98	61	Plasticity Index (%)	12	13	15
19.0	60	95	55	Linear Shrinkage (%)	6.0	6.5	7.5
13.2	54	94	47	PI of whole sample	3	9	3
9.5	48	92	44				
6.7	42	90	41	% Gravel	72	21	73
4.75	38	89	38	% Sand	15	40	13
2.00	28	79	27	% Silt	8	17	9
1.00	25	75	25	% Clay	5	22	5
0.425	23	70	23	Activity	2.4	0.6	3.0
0.250	20	65	21				
0.150	18	59	19	% Soil Mortar	28	79	27
0.075	14	45	16				
0.060	13	39	14	Grading Modulus	2.35	1.06	2.34
0.050	12	37	13	Moisture Content (%)	N / T	N / T	N / T
0.035	11	33	11	Relative Density (SG)*	2.65	2.65	2.65
0.020	10	30	10				
0.006	7	25	8	Unified (ASTM D2487)	GC	SC	GC
0.002	5	22	5	AASHTO (M145-91)	A - 2 - 6	A - 6	A - 2 - 6
Remarks: *: Assumed							
N / T: Not Tested							
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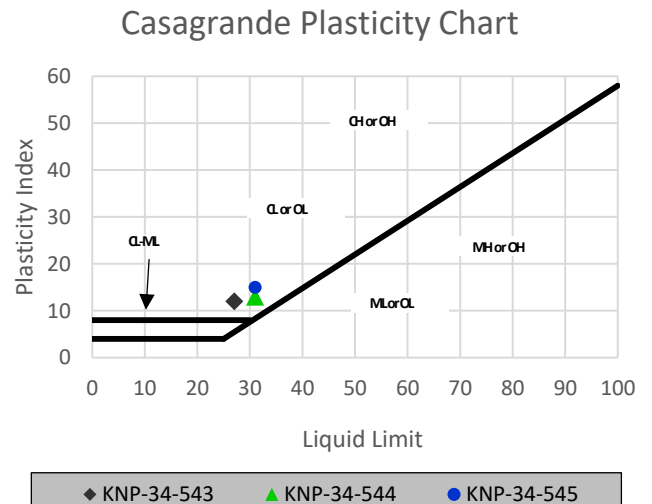
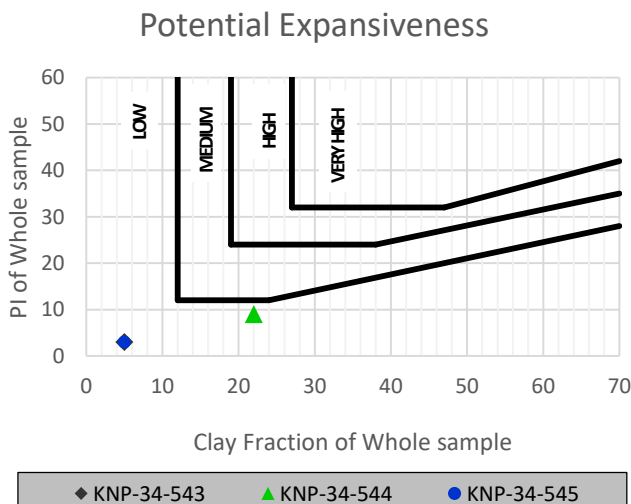
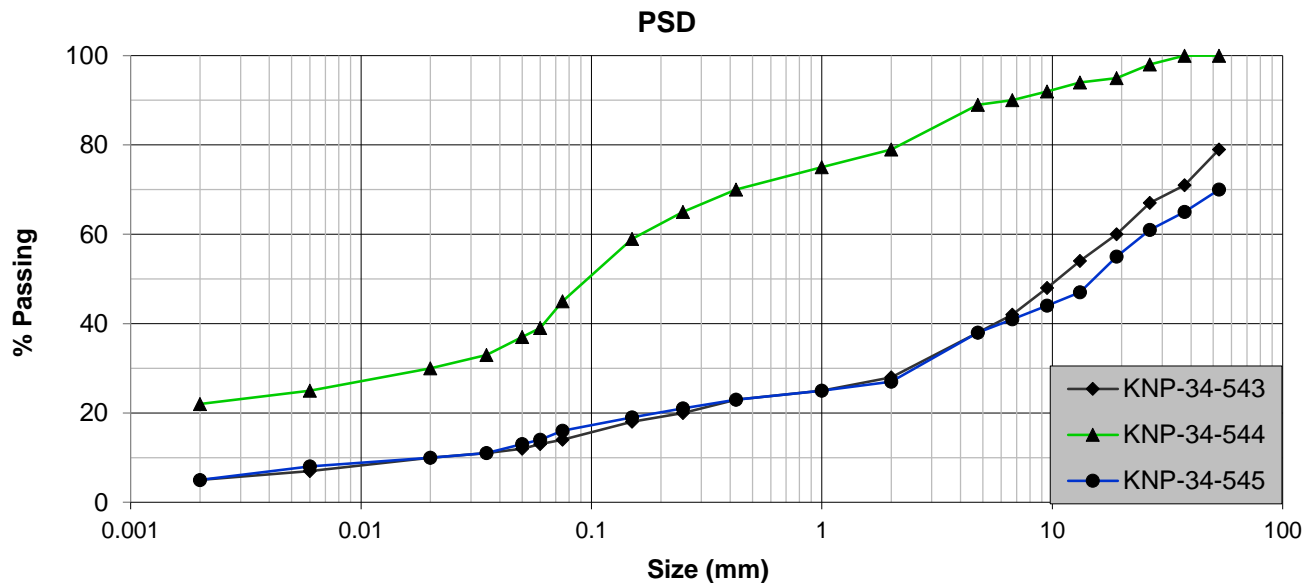
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Client Name: Knight Piesold
Project Name: PR311-00288/72: Proposed New Chemical Plant, Vosloorus
Job Number: KNP-34
Date: 2021-11-04
Method: SANS 3001 GR1, GR3, GR10 GR12 & BS 1377 (where applicable)

FOUNDATION INDICATOR

Sheet Reference:
R-STL-011 Rev02



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Client Name: Knight Piesold

Project Name: PR311-00288/72: Proposed New Chemical Plant, Voslo

Sample: TP1/2

Depth: (m) 1.0 - 1.4

Job Number: KNP-34

Lab Number: KNP-34-535

Method: BS 1377 Part 5

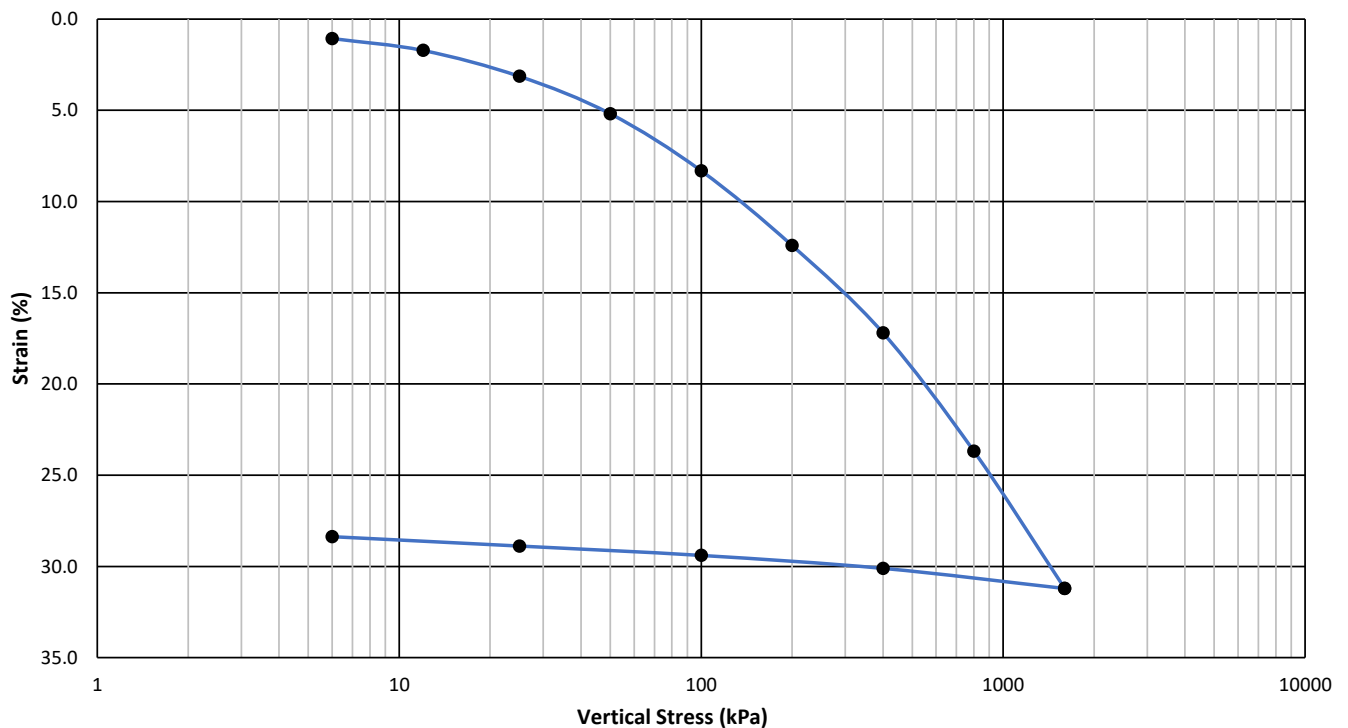
Date: 04/11/2021

ONE DIMENSIONAL CONSOLIDATION TEST

Sample Info		Unit	Initial	Test Remarks:
Test Specimen Height		mm	25.4	Undisturbed
Moisture Content	Initial	%	15.0	
	Final	%	15.4	
Dry Density		kg/m ³	1375	
Void Ratio		-	0.948	
Degree of Saturation		%	42.4	
Relative Density (SG)		-	2.679	Determined

Vertical Stress Applied:	kPa	6	12	25	50	100	200	400	800	1600	400	100	25	6
Load applied for:	Hrs	12	12	12	12	12	12	12	12	12	3	3	3	3
Height after increment	mm	25.13	24.96	24.60	24.08	23.29	22.25	21.03	19.38	17.47	17.75	17.93	18.06	18.19
Total Strain	%	1.07	1.72	3.14	5.19	8.32	12.41	17.20	23.68	31.21	30.11	29.40	28.89	28.37
Void Ratio	-	0.928	0.915	0.887	0.847	0.786	0.707	0.613	0.487	0.340	0.362	0.376	0.386	0.396
Mv (1/Mpa)	-	-	1.102	1.110	0.847	0.660	0.447	0.273	0.196	0.123	0.013	0.034	0.097	0.380

Strain vs Log Stress





Quality | Excellence | On Time

Client Name: Knight Piesold

Project Name: PR311-00288/72: Proposed New Chemical Plant, Vosloc

Sample: TP1/2

Depth: (m) 1.0 - 1.4

Job Number: KNP-34

Lab Number: KNP-34-535

Method: BS 1377 Part 5

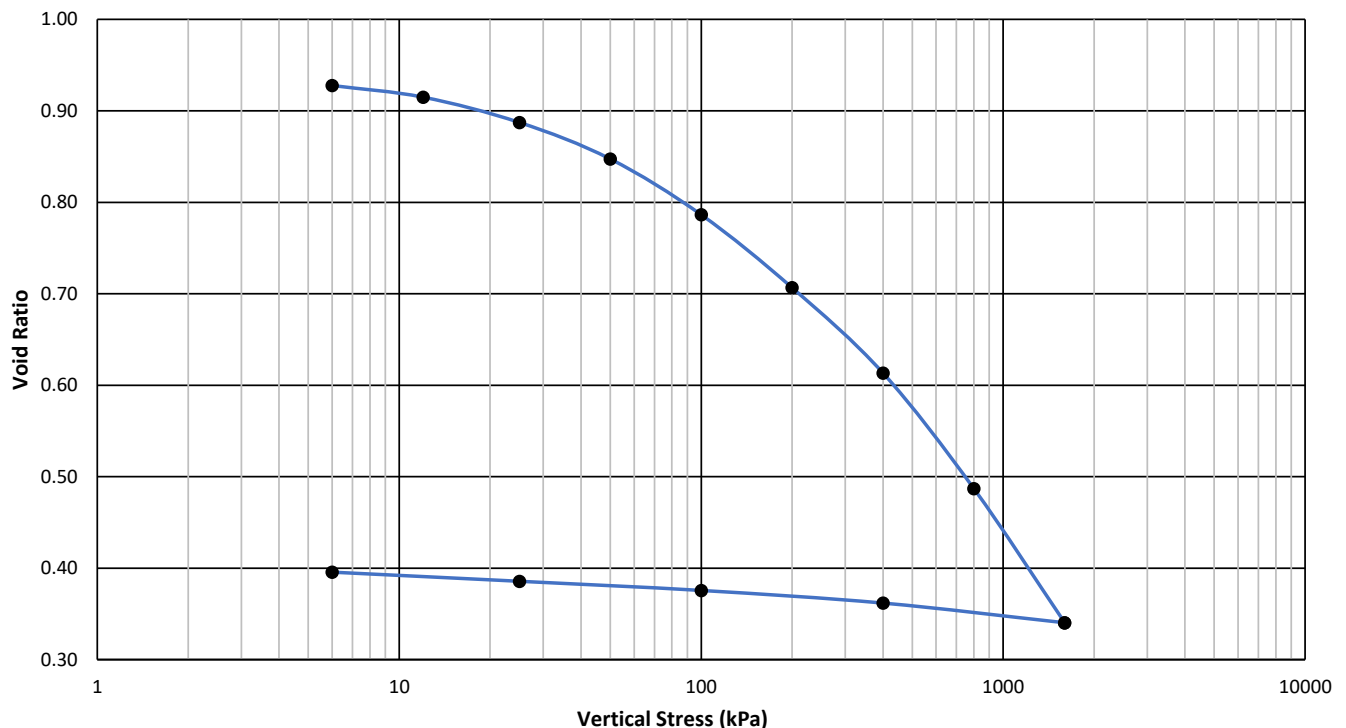
Date: 04/11/2021

ONE DIMENSIONAL CONSOLIDATION TEST

Sample Info		Unit	Initial	Test Remarks:
Test Specimen Height		mm	25.4	Undisturbed
Moisture Content	Initial	%	15.0	
	Final	%	15.4	
Dry Density		kg/m ³	1375	
Void Ratio		-	0.948	
Degree of Saturation		%	42.4	
Relative Density (SG)		-	2.679	Determined

Vertical Stress Applied:	kPa	6	12	25	50	100	200	400	800	1600	400	100	25	6
Load applied for:	Hrs	12	12	12	12	12	12	12	12	12	3	3	3	3
Height after increment	mm	25.13	24.96	24.60	24.08	23.29	22.25	21.03	19.38	17.47	17.75	17.93	18.06	18.19
Total Strain	%	1.07	1.72	3.14	5.19	8.32	12.41	17.20	23.68	31.21	30.11	29.40	28.89	28.37
Void Ratio	-	0.928	0.915	0.887	0.847	0.786	0.707	0.613	0.487	0.340	0.362	0.376	0.386	0.396
Mv (1/Mpa)	-	-	1.102	1.110	0.847	0.660	0.447	0.273	0.196	0.123	0.013	0.034	0.097	0.380

Void Ratio vs Log Stress





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Client Name: Knight Piesold

Job Number: KNP-34

Project Name: PR311-00288/72: Proposed New Chemical Plant, Vos

Lab Number: KNP-34-536

Sample: TP2/1

Method: BS 1377 Part 5

Depth: (mm) 1.0 - 1.4

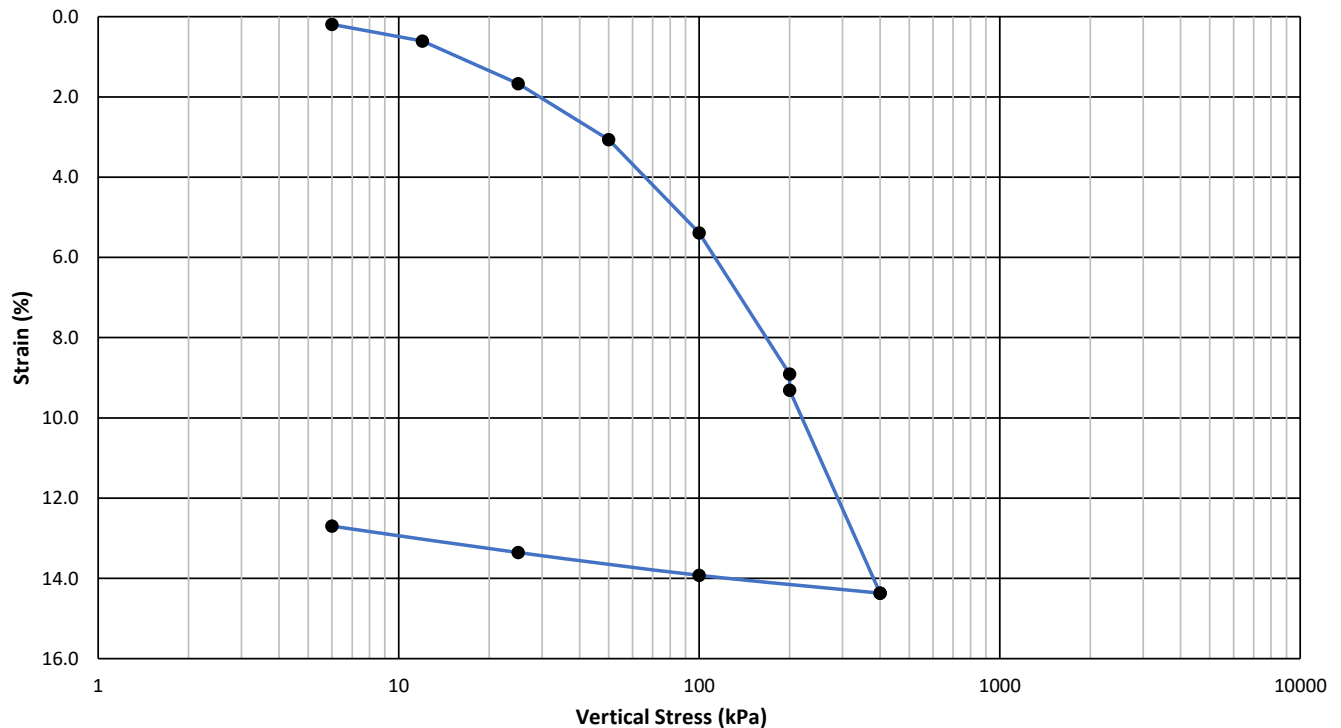
Date: 04-Nov-21

ONE DIMENSIONAL COLLAPSE POTENTIAL TEST

Sample Info		Unit	Initial	Test Remarks:
Test Specimen Height		mm	25.4	Collapse Potential: 0.40 %
Moisture Content	Initial	%	24.9	
	Final	%	28.3	
Dry Density		kg/m ³	1345	
Void Ratio		-	0.970	
Degree of Saturation		%	67.9	
Relative Density (SG)		-	2.650	Assumed

Vertical Stress Applied:	kPa	6	12	25	50	100	200	200	400	100	25	6		
Load applied for:	Hrs	1	1	1	1	1	1	21	1	1	1	1		
Height after increment	mm	25.35	25.25	24.98	24.62	24.03	23.14	23.04	21.75	21.86	22.01	22.17		
Total Strain	%	0.19	0.61	1.67	3.06	5.39	8.91	9.31	14.37	13.92	13.36	12.70		
Void Ratio	-	0.966	0.958	0.937	0.910	0.864	0.795	0.787	0.687	0.696	0.707	0.720		
Mv (1/Mpa)	-	-	0.691	0.820	0.568	0.480	0.372	-	0.279	0.017	0.088	0.400		

Strain vs Log Stress



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Client Name: Knight Piesold

Job Number: KNP-34

Project Name: PR311-00288/72: Proposed New Chemical Plant, Vos

Lab Number: KNP-34-536

Sample: TP2/1

Method: BS 1377 Part 5

Depth: (m) 1.0 - 1.4

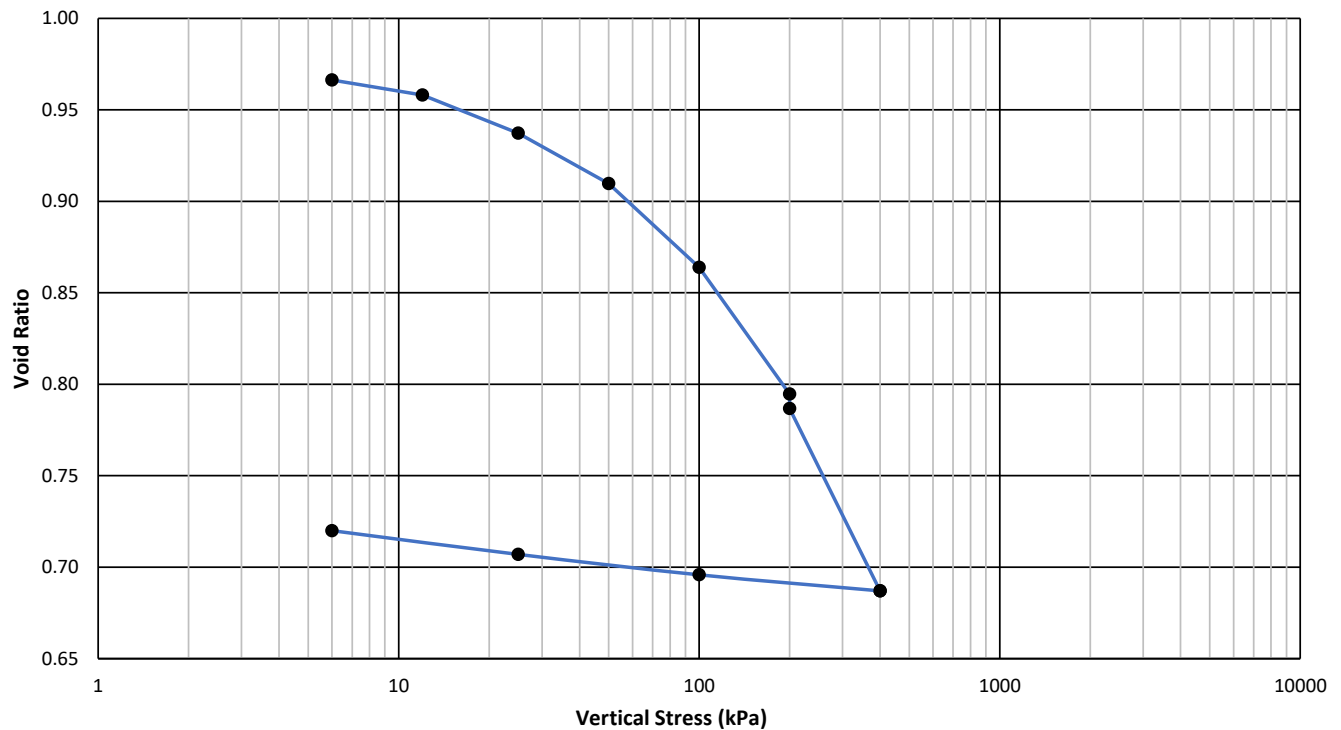
Date: 04-Nov-21

ONE DIMENSIONAL COLLAPSE POTENTIAL TEST

Sample Info		Unit	Initial	Test Remarks:
Test Specimen Height		mm	25.4	Collapse Potential: 0.40 %
Moisture Content	Initial	%	24.9	
	Final	%	28.3	
Dry Density		kg/m ³	1345	
Void Ratio		-	0.970	
Degree of Saturation		%	67.9	
Relative Density (SG)		-	2.650	Assumed

Vertical Stress Applied:	kPa	6	12	25	50	100	200	200	400	100	25	6		
Load applied for:	Hrs	1	1	1	1	1	1	21	1	1	1	1		
Height after increment	mm	25.35	25.25	24.98	24.62	24.03	23.14	23.04	21.75	21.86	22.01	22.17		
Total Strain	%	0.19	0.61	1.67	3.06	5.39	8.91	9.31	14.37	13.92	13.36	12.70		
Void Ratio	-	0.966	0.958	0.937	0.910	0.864	0.795	0.787	0.687	0.696	0.707	0.720		
Mv (1/Mpa)	-	-	0.691	0.820	0.568	0.480	0.372	-	0.279	0.017	0.088	0.400		

Void Ratio vs Log Stress



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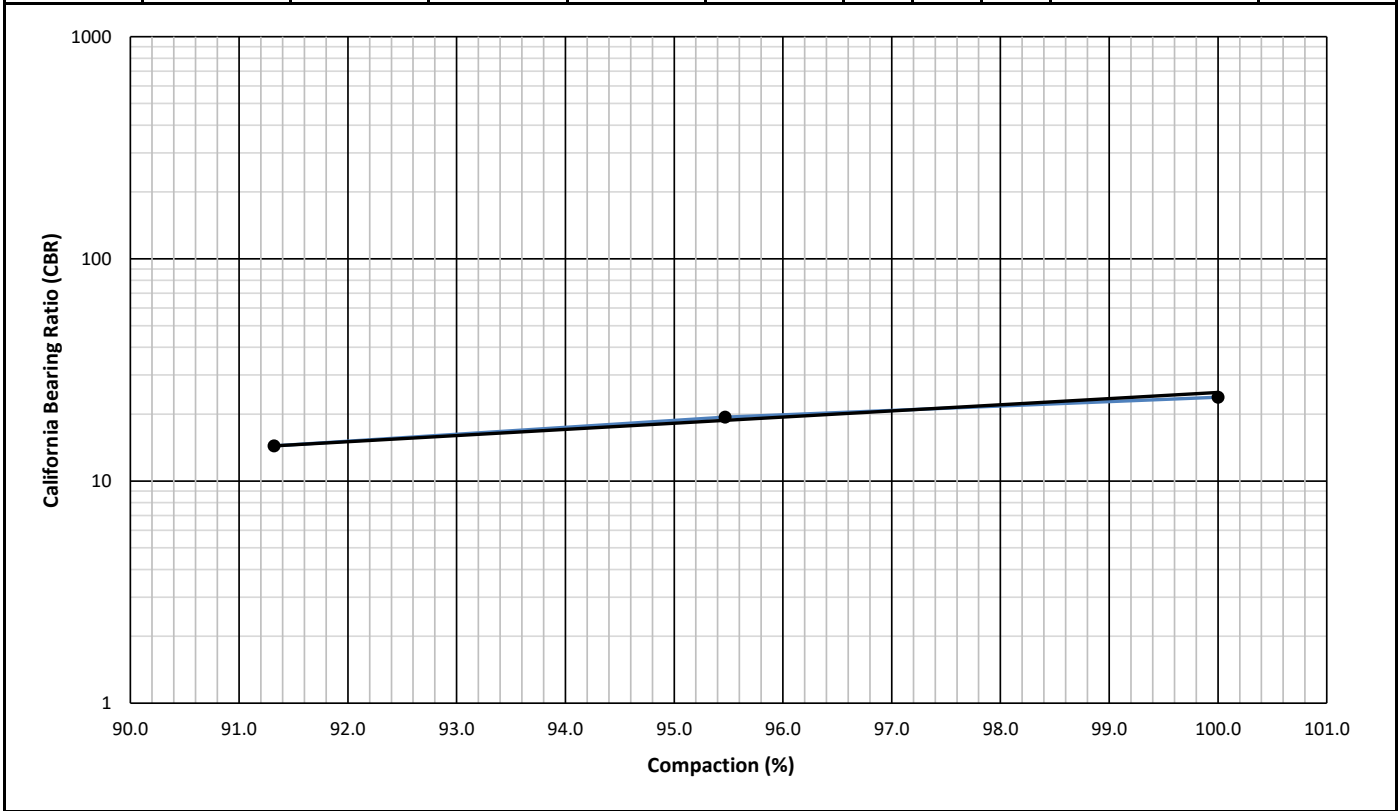
Client Name: Knight Piesold
Project Name: Proposed New Chemical Plant, Vosloorus
Client Project No.: PR311-00288/72
Sample: TP2/2
Depth: (m) 1.4 - 2.7

Job Number: KNP-34
Lab Number: KNP-34-537
Method: SANS 3001 GR40
Date: 04-Nov-21

CALIFORNIA BEARING RATIO

Sheet Reference:
R-STL-014 Rev01

Mod. AASHTO Values		Compaction Data: CBR			Swell	CBR at (mm)			CBR Values	
MDD (kg/m ³)	OMC (%)	Dry Dens. (kg/m ³)	MC (%)	Comp. (%)		2.5	5.0	7.5	Compaction (%)	CBR
1805	14.9	1809	15.6	100.0	0.1	24	26	24	100	24
1805	14.9	1727	15.6	95.5	0.1	19	20	20	98	22
1805	14.9	1652	15.6	91.3	0.2	14	13	12	97	21
									95	19
									93	16
									90	13



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Client Name: Knight Piesold
Project Name: Proposed New Chemical Plant, Vosloorus
Client Project No.: PR311-00288/72
Sample: TP2/2
Depth: (m) 1.4 - 2.7

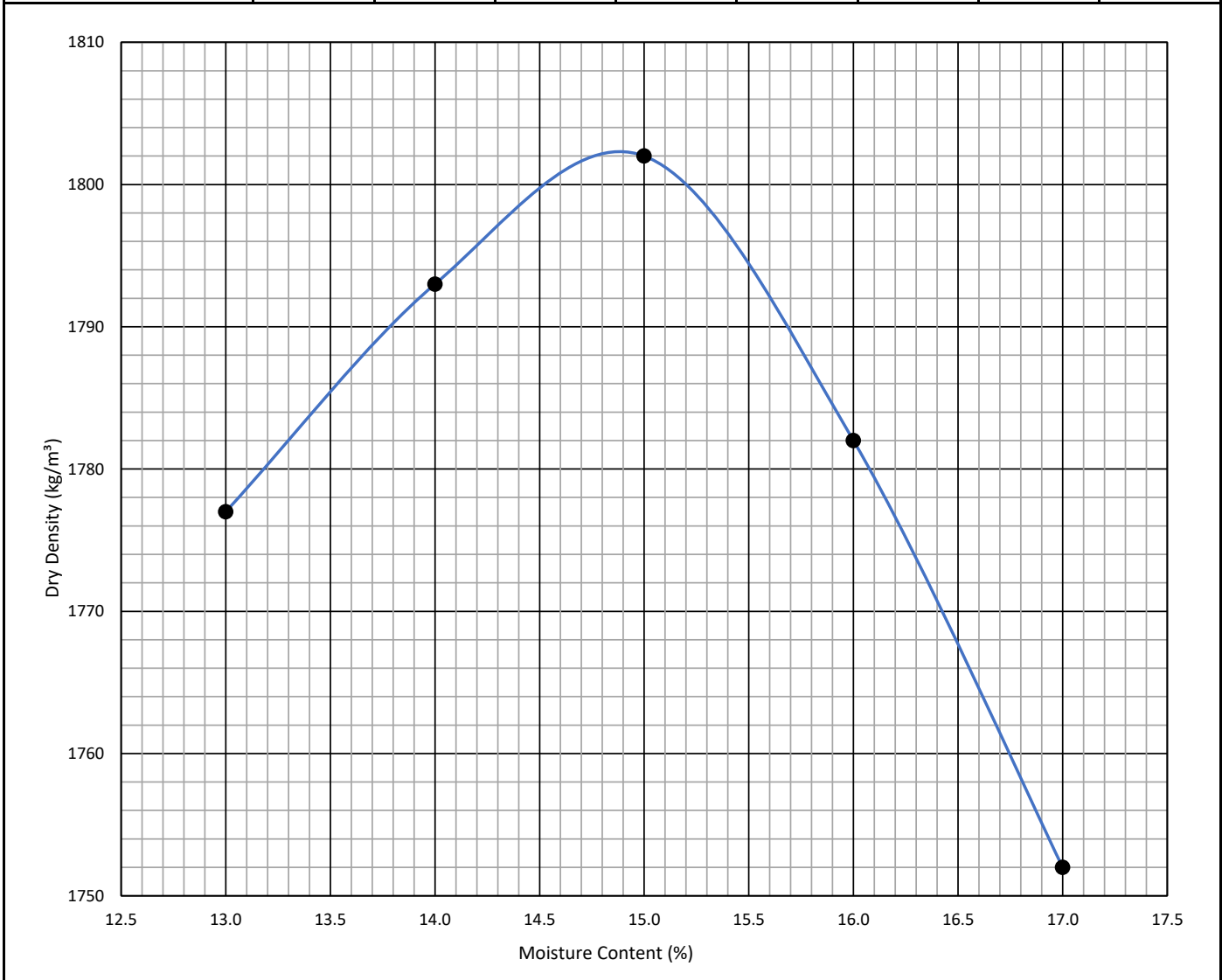
Job Number: KNP-34
Lab Number: KNP-34-537
Method: SANS 3001 GR30
Date: 19-Oct-21

MDD & OMC DETERMINATION (Mod. AASHTO) **Sheet Reference:**
R-STL-013 Rev01

Maximum Dry Density: 1805 kg/m³

Optimum Moisture Content: 14.9 %

Moisture Content (%)	13.0	14.0	15.0	16.0	17.0			
Dry Density (kg/m ³)	1777	1793	1802	1782	1752			



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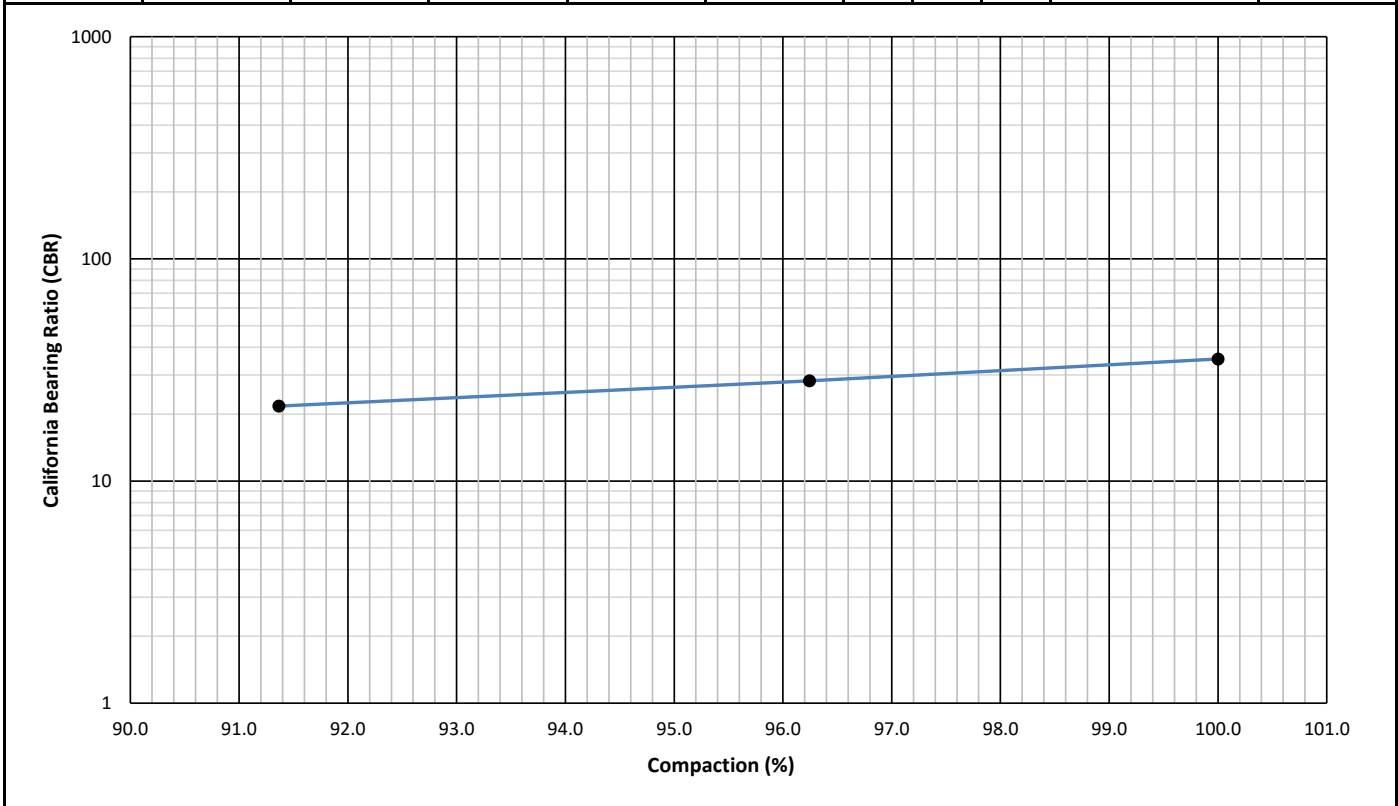
Client Name: Knight Piesold
Project Name: Proposed New Chemical Plant, Vosloorus
Client Project No.: PR311-00288/72
Sample: TP8/2
Depth: (m) 0.8 - 2.2

Job Number: KNP-34
Lab Number: KNP-34-543
Method: SANS 3001 GR40
Date: 04-Nov-21

CALIFORNIA BEARING RATIO

Sheet Reference:
R-STL-014 Rev01

Mod. AASHTO Values		Compaction Data: CBR			Swell	CBR at (mm)			CBR Values	
MDD	OMC	Dry Dens.	MC	Comp.		2.5	5.0	7.5	Compaction (%)	CBR
(kg/m ³)	(%)	(kg/m ³)	(%)	(%)	(%)					
2079	8.6	2050	9.3	100.0	0.0	35	41	42	100	35
2079	8.6	1973	9.3	96.2	0.0	28	34	35	98	31
2079	8.6	1873	9.3	91.4	0.0	22	22	20	97	30
									95	26
									93	24
									90	20



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Client Name: Knight Piesold
Project Name: Proposed New Chemical Plant, Vosloorus
Client Project No.: PR311-00288/72
Sample: TP8/2
Depth: (m) 0.8 - 2.2

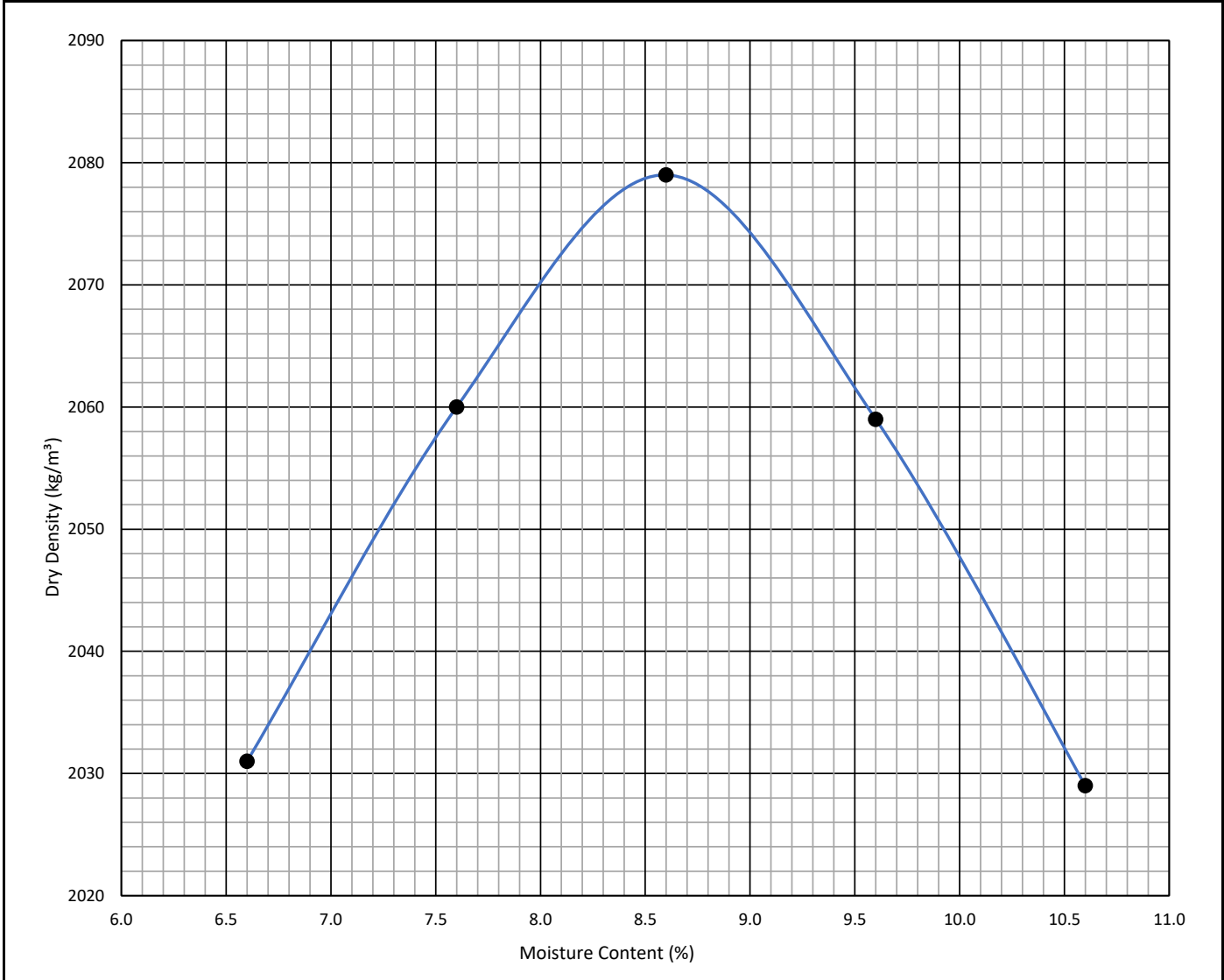
Job Number: KNP-34
Lab Number: KNP-34-543
Method: SANS 3001 GR30
Date: 19-Oct-21

MDD & OMC DETERMINATION (Mod. AASHTO) **Sheet Reference:**
R-STL-013 Rev01

Maximum Dry Density: 2079 kg/m³

Optimum Moisture Content: 8.6 %

Moisture Content (%)	6.6	7.6	8.6	9.6	10.6			
Dry Density (kg/m ³)	2031	2060	2079	2059	2029			



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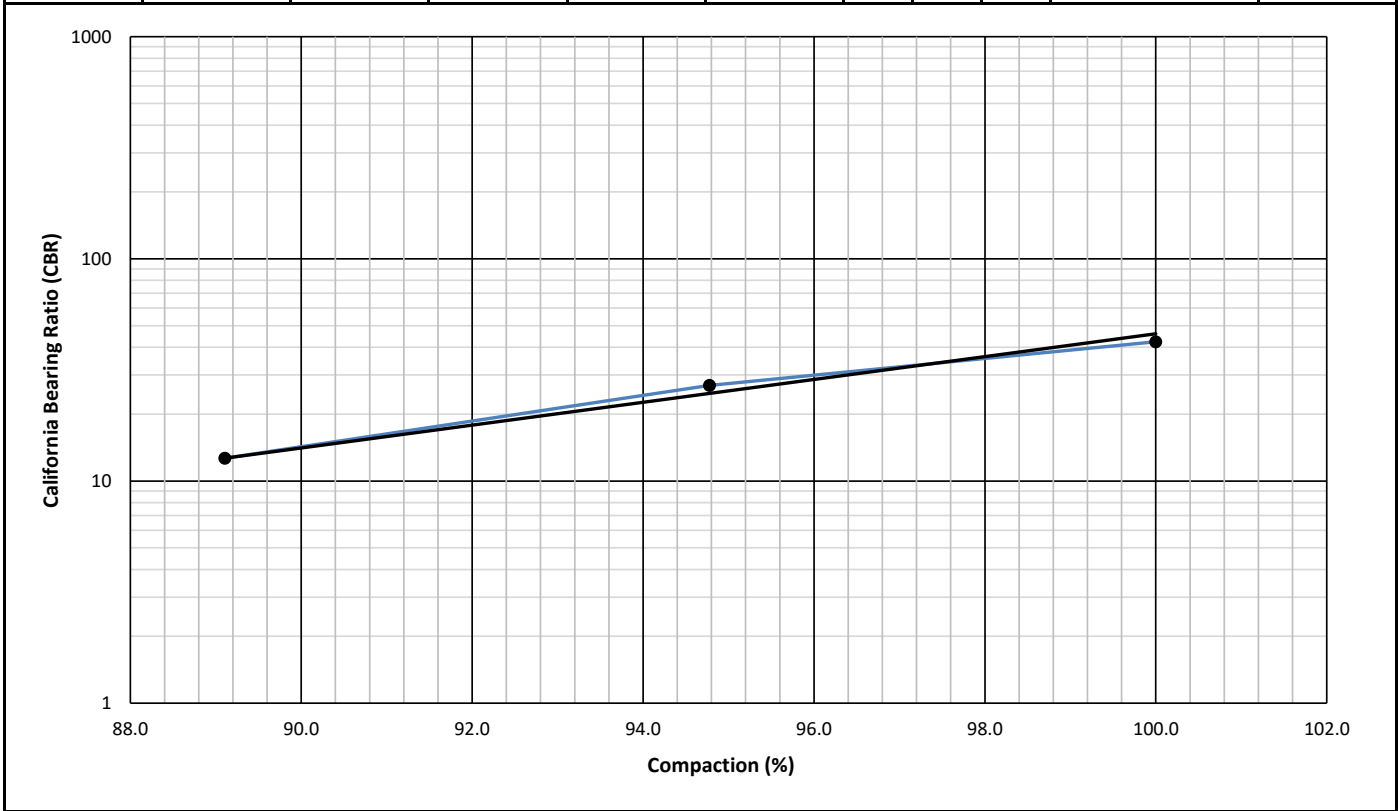
Client Name: Knight Piesold
Project Name: Proposed New Chemical Plant, Vosloorus
Client Project No.: PR311-00288/72
Sample: TP11/1
Depth: (m) 0.7 - 2.6

Job Number: KNP-34
Lab Number: KNP-34-545
Method: SANS 3001 GR40
Date: 04-Nov-21

CALIFORNIA BEARING RATIO

Sheet Reference:
R-STL-014 Rev01

Mod. AASHTO Values		Compaction Data: CBR			Swell	CBR at (mm)			CBR Values	
MDD (kg/m ³)	OMC (%)	Dry Dens. (kg/m ³)	MC (%)	Comp. (%)		2.5	5.0	7.5	Compaction (%)	CBR
2012	9.6	2010	9.3	100.0	0.1	42	48	47	100	42
2012	9.6	1905	9.3	94.8	0.1	27	24	22	98	36
2012	9.6	1791	9.3	89.1	0.2	13	10	8	97	33
									95	27
									93	21
									90	14



Although everything possible is done to ensure testing is performed accurately, neither Specialised Testing Laboratory (Pty) Ltd nor any of its directors, managers, employees or contractors can be held liable for any damages whatsoever arising from any error made in performing any tests, nor from any conclusions drawn therefrom. Test results are to be published in full. Samples will be kept for 1 month after the submission of test results due to limited storage space, unless other arrangements are in place. Confidentiality statement: Unless the release of information is required by law or covered by confidentiality agreements all information obtained or created during the performance of laboratory activities will be kept confidential.



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Client Name: Knight Piesold
Project Name: Proposed New Chemical Plant, Vosloorus
Client Project No.: PR311-00288/72
Sample: TP11/1
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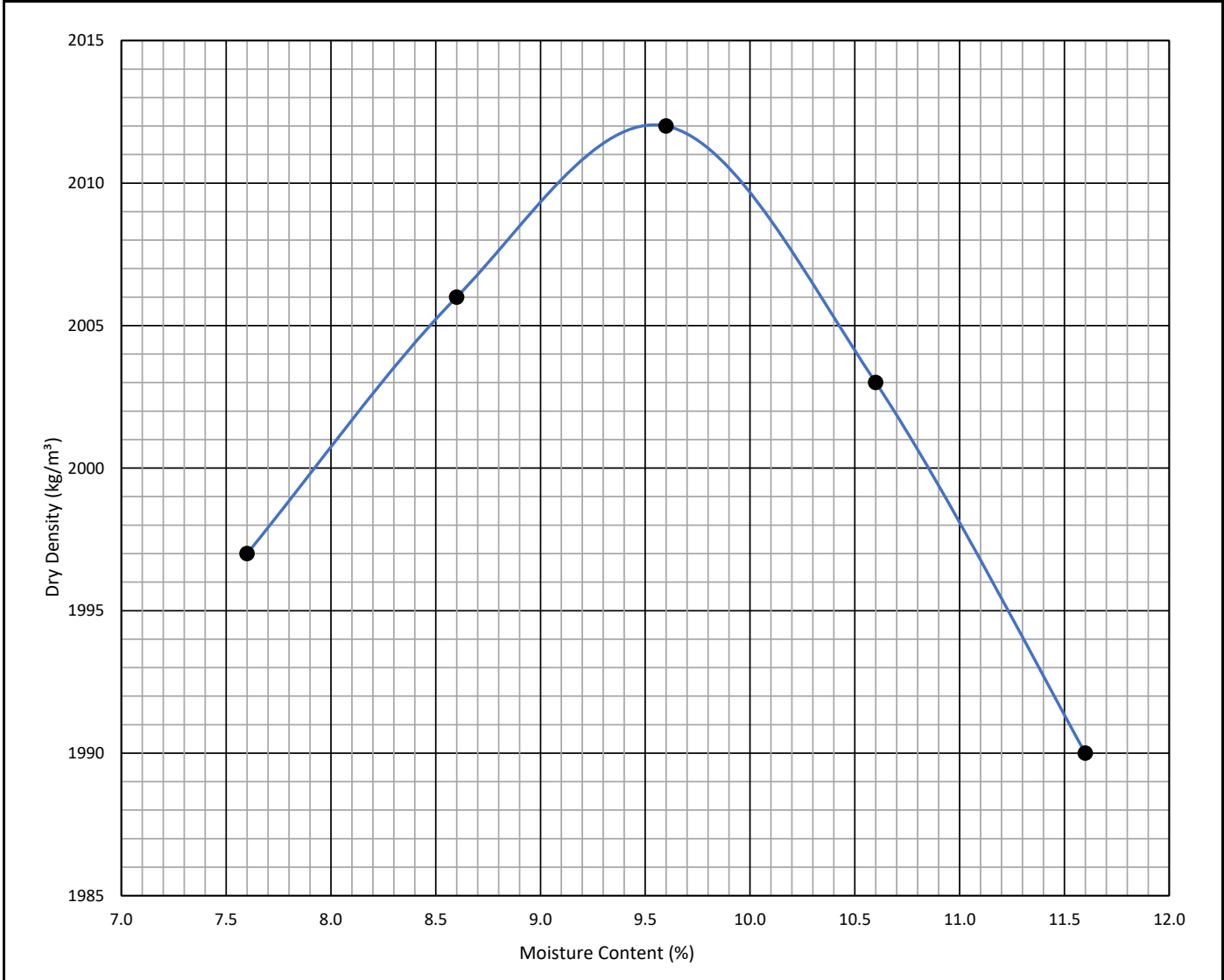
Job Number: KNP-34
Lab Number: KNP-34-545
Method: SANS 3001 GR30
Date: 19-Oct-21

MDD & OMC DETERMINATION (Mod. AASHTO)	Sheet Reference: R-STL-013 Rev01
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Maximum Dry Density: 2012 kg/m³

Optimum Moisture Content: 9.6 %

Moisture Content (%)	7.6	8.6	9.6	10.6	11.6			
Dry Density (kg/m ³)	1997	2006	2012	2003	1990			



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APPENDIX E

Report on Gravity Survey Results

12 October 2021

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Vosloorus – Waterkloof AFB Sinkhole 1 W

General

This report presents the results of a gravity survey, conducted in support of a dolomitic stability investigation, for Richards Bay Chemicals Vosloorus chemical plant located between the N3 and R103. The work was done on behalf of Knight Piésold.

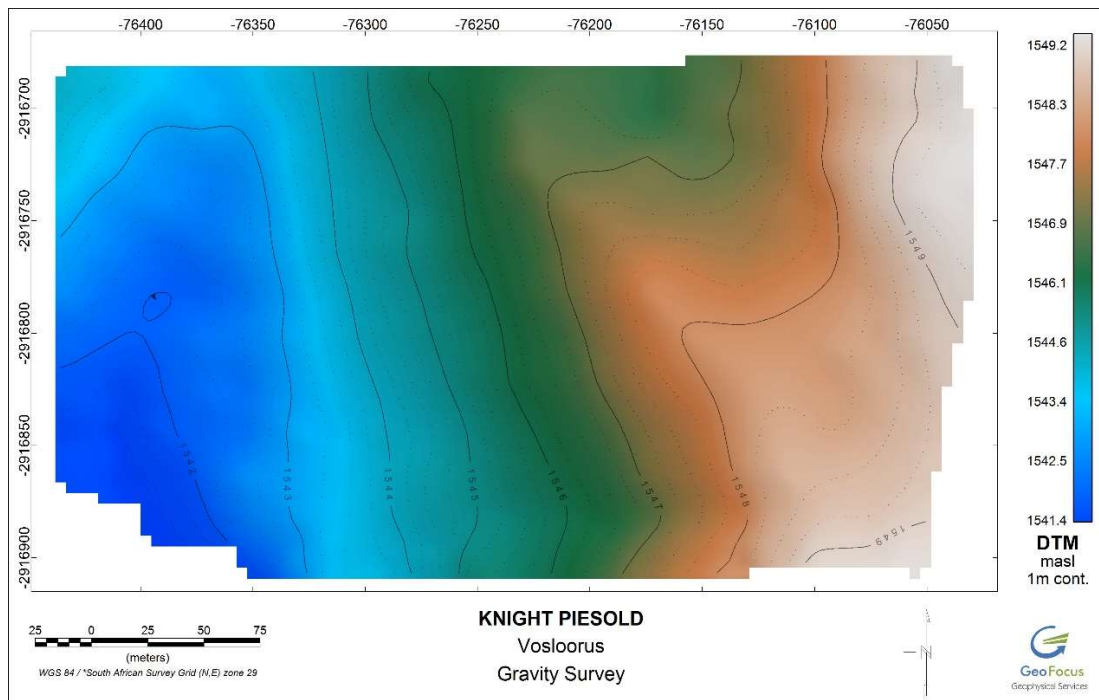
A total of 242 gravity stations were acquired on a 20m grid covering roughly 8ha, in mid-September 2021.

Methodology

Gravity was observed with a Scintrex CG5 automated gravimeter in conjunction with a high resolution GNSS (RTK) GPS, the latter is used to record accurate station locations. The topography of the site sloped very gently from E to W (Figure 1).

Standard gravity processing procedures were applied, firstly reducing the data to relative Bouguer values by applying Earth Tide (ETC), elevation and Bouguer corrections. As a final step, the regional gravity field, derived through linear regression or fitting a 1st

order plane to the data, was removed from the Bouguer gravity to produce a residual gravity map.



Results

The residual gravity results fall within a peak-to-peak envelope of circa 0.3 mGal. The results are typified by irregular highs in the extreme E and central W, the latter displaying a NW-SE orientation. The high straddle a broad low integrating multiple smaller lows. A gentle gradient is seen to the far W.

Figure 1: Elevation map & sinkhole outline (white dash)

Eleven [11] boreholes were drilled across the site (Figure 2).

Borehole PH01 intersected dolomite from near surface to 12m with the formation becoming hard at 6m deep; it is to this depth that the residual gravity was corrected. Dolerite was intersected in horizons of varying thickness (up to several meters thick) and/or as the end-of-hole lithology.

Notwithstanding the highly variable geology and dolerite present, the hard-rock intersections correlate broadly with the residual gravity; bedrock is shallowest over the gravity high in the NW, dipping over the broad central low to between 16m (PH3) and 22m (PH5), thereafter it rises very slightly to 14m (PH4) in the central S. Hard-rock intersections of between 15m and 17m are observed to the NW.

Although influenced to a great degree by the presence of dolerite, the gravity results still give a fair reflection of the changes in bedrock depth.

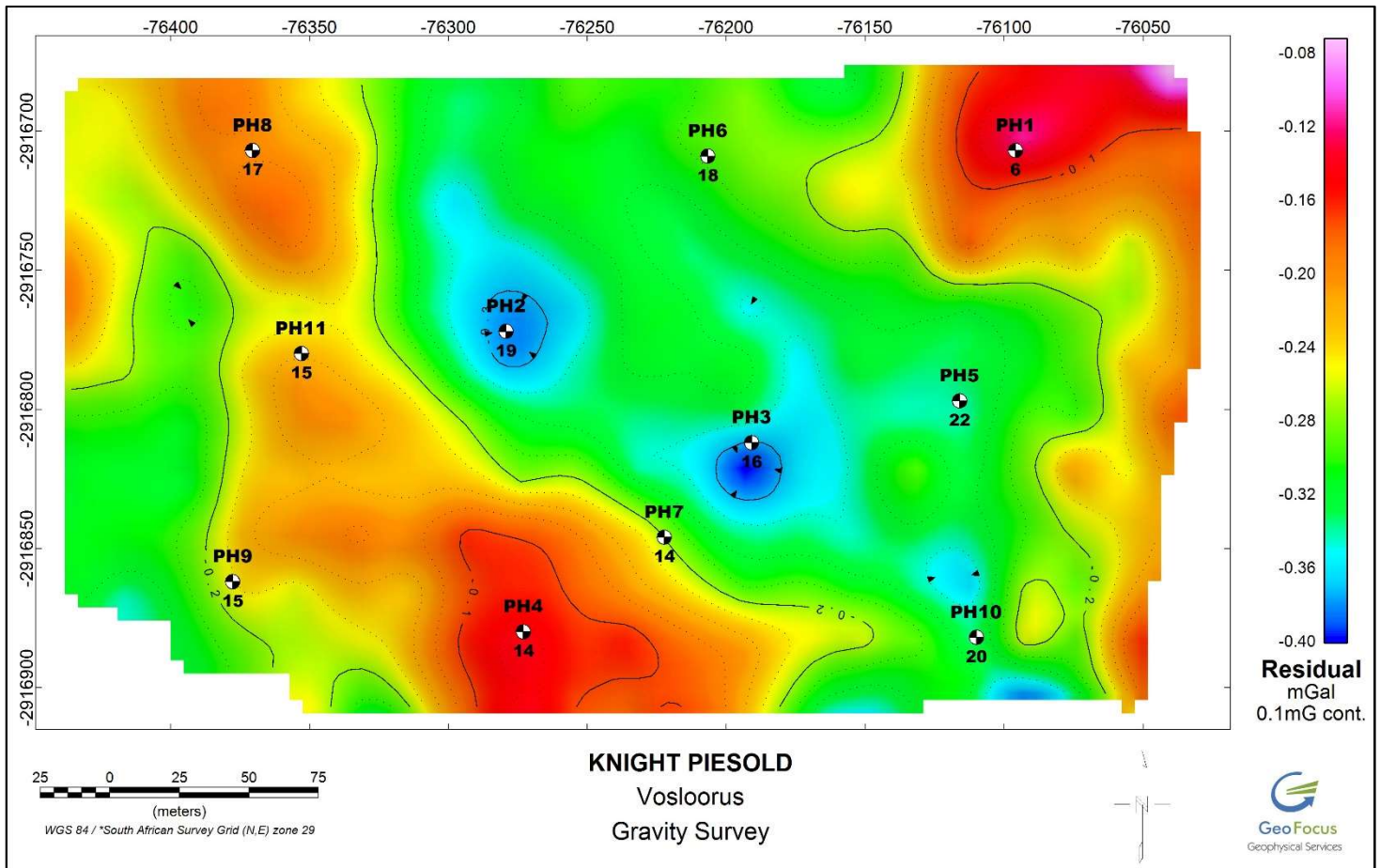


Figure 2: Residual gravity & drilling results. The depths indicated are to competent formation.