

## VRYBURG RETAIL DEVELOPMENT ACCESS STUDY MEMORANDUM

## K1471/1

## FEBRUARY 2013



CIVIL AND STRUCTURAL CONSULTING ENGINEERS

Civil Concepts (Pty) Ltd
PO Box 36148, Menlo Park, 0102
Alec's Place, Lynnwood
Tel: +27 $12 \mathbf{3 6 5 1 4 1 4}$

|  | VRYBURG RETAIL DEVELOPMENT - ACCESS STUDY MEMORANDUM | PAGE1 |
| :---: | :---: | :---: |

## 1 INTRODUCTION

The site of the proposed development is located to the south of Vryburg as shown in Figure 1.1. This memorandum addresses the accesses to the proposed Vryburg Retail development.

The proposed development will consist of a $35000 \mathrm{~m}^{2}$ retail centre and will generate 1792 and 3035 trips during the Friday afternoon and Saturday peak hours, respectively.


Figure 1.1: Locality Plan

## 2 LAND USE AND TRIP GENERATI ON

### 2.1 Land Use

The Vryburg Retail development will consist of the following land-use:

- $35000 m^{2}$ retail centre.


### 2.2 Trip Generation

The trip generation rates in the Department of Transport's "South African Trip Generation Rates, 2 ${ }^{\text {nd }}$ Edition, June 1995" are used as a basis for the calculation of the proposed development trips.

The retail trips will comprise of primary and pass-by trips only. Due to the location and the lack of retail centres of similar size in the vicinity of the proposed development it is assumed that there will not be any diverted trips generated. The diverted trips were considered as primary trips for this study.

Due to the low volume of traffic along the N14 the passer-by trips were reduced by $25 \%$ and the primary trips increased by $25 \%$.

A trip reduction of $20 \%$ has been applied to account for:

- walking and public transport trips, and
- low vehicle ownership.

The Friday afternoon and Saturday peak hour trip generation for the proposed development are shown in Tables 2.1 and 2.2 , respectively.

TABLE 2.1: FRIDAY AFTERNOON PEAK HOUR TRIP GENERATION (PROPOSED RETAIL DEVELOPMENT)


TABLE 2.2: SATURDAY PEAK HOUR TRIP GENERATION (PROPOSED RETAIL DEVELOPMENT)

| Land Use | Area $\left(\mathrm{m}^{2}\right) /$ Units | $\begin{aligned} & \text { Unit } \\ & \left(\mathrm{m}^{2}\right) \end{aligned}$ | Rate | Reduction | Directional split <br> Weekday |  | Peak Hour Trips |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  | IN | OUT | TOTAL |
|  |  |  |  |  | IN | OUT |  |  |  |
| Retail | 35000 | 100 | 10.84 | 20\% | 50\% | 50\% | 1518 | 1518 | 3035 |
| PRIMARY |  |  |  |  | 90\% |  | 1366 | 1366 | 2732 |
| DIVERTED 0\% |  |  |  |  |  |  | 0 | 0 | 0 |
| PASSER-BY 10\% |  |  |  |  |  |  | 152 | 152 | 304 |
| TOTAL |  |  |  |  |  |  | 1518 | 1518 | 3035 |

Retail trips are not considered in the morning peak hour due to the fact that most shops within a retail centre normally open after 8:00, which is outside peak of the surrounding road network. Trips to a retail centre during the morning peak hour are therefore considered to be negligible.

There are two residential developments within the vicinity of the proposed retail development, namely:

- Fairview Estates (High Income Residential)
- Rosendal Estates (Low Income Residential)

Traffic studies for the above mentioned developments were not available and therefore the trips generated by these residential developments were estimated in order to determine the latent trips to be considered as part of this study.

The Saturday peak hour residential trips are assumed to be $50 \%$ of the normal weekday peak hour residential trips.

The Friday afternoon and Saturday peak hour trip generation for the Fairview Estates residential development are shown in Tables 2.3 and 2.4, respectively.

TABLE 2.3: FRIDAYAFTERNOON PEAK HOUR TRIP GENERATION (FAIRVIEW ESTATES)

| Land Use | Units | Unit$\left(\mathrm{m}^{2}\right)$ | Rate | Reduction | Directional split Weekday |  | Peak Hour Trips |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  | IN | OUT | TOTAL |
|  |  |  |  |  | IN | OUT |  |  |  |
| High Income Residential | 42 | units | 1.50 | 0\% | 75\% | 25\% | 47 | 16 | 63 |
| TOTAL |  |  |  |  |  |  | 47 | 16 | 63 |

TABLE 2.4: SATURDAY PEAK HOUR TRIP GENERATION (FAIRVIEW ESTATES)


The Friday afternoon and Saturday peak hour trip generation for the Rosendal Estates residential development are shown in Tables 2.5 and 2.6, respectively.

TABLE 2.5: FRIDAYAFTERNOON PEAK HOUR TRIP GENERATION (ROSENDAL ESTATES)

| Land Use | Units | $\begin{aligned} & \text { Unit } \\ & \left(\mathrm{m}^{2}\right) \end{aligned}$ | Rate | Reduction | Directional split <br> Weekday |  | Peak Hour Trips |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  | IN | OUT | TOTAL |
|  |  |  |  |  | IN | OUT |  |  |  |
| Low Income Residential | 274 | units | 0.50 | 0\% | 65\% | 35\% | 89 | 48 | 137 |
| TOTAL |  |  |  |  |  |  | 89 | 48 | 137 |

TABLE 2.6: SATURDAY PEAK HOUR TRIP GENERATION (ROSENDAL ESTATES)

| Land Use | Units | Unit$\left(\mathrm{m}^{2}\right)$ | Rate | Reduction | Directional split <br> Weekday |  | Peak Hour Trips |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  | IN | OUT | TOTAL |
|  |  |  |  |  | IN | OUT |  |  |  |
| Low Income Residential | 274 | units | 0.25 | 0\% | 50\% | 50\% | 34 | 34 | 69 |
| TOTAL |  |  |  |  |  |  | 34 | 34 | 69 |

## 3 THE EXISTING ROAD NETWORK

N14-is a class 2 road with one lane per direction and resorts under SANRAL. The SANRAL jurisdiction is 30 m south west of the developments boundary. The development is therefore situated along the Naledi Local Municipality jurisdiction and the N14 becomes a class 4a road along the site boundary, with closely spaced intersections.

## 4 TRIP DISTRI BUTI ON AND ASSI GNMENT

The development trips were distributed and assigned to the adjacent road network based on the expected origins and destinations to and from the development.

## 5 TRAFFIC COUNTS

### 5.1 Introduction

24 hour counts were conducted between 19 August 2009 and 15 September 2009 on the N14 between Kuruman and Vryburg. The Friday and Saturday peak hour traffic volumes were calculated from this data based on the assumption that 10\%-15\% of the AADT occurs in the peak hour..

The 2009 weekday morning and afternoon and Saturday peak hour traffic counts are shown in Table 6.1.

TABLE 5.1: 2009 TRAFFIC COUNTS

|  | Friday Peak <br> Hour | Saturday <br> Peak Hour |
| :---: | :---: | :---: |
| To Vryburg | 120 | 65 |
| To Kuruman | 110 | 70 |

### 5.2 Background Traffic Volumes

## Base Year -2014

The 2009 weekday morning and afternoon peak hour traffic counts were escalated at a $3 \%$ annual growth rate over 5 years and added to the Fairview Estates and Rosendal Estates development trips to obtain the 2014 peak hour background traffic volumes.

## Horizon Year- 2019 (5 year) and 2024 (10 year)

The proposed retail development will generate more than 2000 trips in the peak hour therefore a 5 year and 10 year horizon period was considered.

The 2009 weekday morning and afternoon peak hour traffic counts were escalated at $3 \%$ annual growth rate over 10 and 15 years and added to the Fairview Estates' and Rosendal Estates' development trips obtain the 2019 and 2024 peak hour background traffic volumes respectively.

### 5.3 Background And Development Traffic Volumes

The Friday and Saturday peak hour total development trips were added to the 2014, 2019 and 2024 background traffic volumes to obtain the 2014, 2019 and 2024 background and development trips.

## 6 TRAFFIC OPERATIONS AND CAPACITY ANALYSIS

### 6.1 Introduction

Capacity analysis was performed at the proposed retail centre access points with the N14 using the SIDRA Intersection 5.0 software package.

The accesses to the development are addressed in this section. The two accesses to the site will be referred to as the eastern and western access respectively.

### 6.2 Proposed Accesses

The traffic control at the intersections are:

- Eastern Access - roundabout.
- Western Access - roundabout.

Tables 6.1, 6.2 and 6.3 below show the results of the capacity analysis for the base year and the horizon years. Detailed capacity calculations are included in ANNEXURE A.

TABLE 6.1: 2014 BACKGROUND AND DEVELOPMENT TRAFFIC CAPACITY CALCULATION RESULTS (BASE YEAR)

| I NTERSECTI ON |  |  | FRIDAY PEAK HOUR | SATURDAY PEAK HOUR |
| :---: | :---: | :---: | :---: | :---: |
|  | N14/ <br> PROPOSED <br> WESTERN ACCESS | V/C ratio | 0.643 | 0.744 |
|  |  | LOS | A | B |
|  |  | Delay (sec/veh) | 9.4 | 11.5 |
|  | $\begin{gathered} \text { N14/ } \\ \text { PROPOSED } \\ \text { EASTERN } \\ \text { ACCESS } \end{gathered}$ | V/C ratio | 0.650 | 0.525 |
|  |  | LOS | B | B |
|  |  | Delay (sec/veh) | 10.3 | 10.2 |

TABLE 6.2: 2019 BACKGROUND AND DEVELOPMENT TRAFFIC CAPACITY CALCULATION RESULTS (5 YEAR HORIZON)

| I NTERSECTI ON |  |  | FRIDAY PEAK HOUR | SATURDAY PEAK HOUR |
| :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & 5 \\ & 0 \\ & 0 \\ & 0 \\ & 0 \\ & 0 \\ & 0 \\ & 0 \end{aligned}$ | N14/ PROPOSED WESTERN ACCESS | V/C ratio | 0.659 | 0.755 |
|  |  | LOS | A | B |
|  |  | Delay (sec/veh) | 9.4 | 11.6 |
|  |  | V/C ratio | 0.665 | 0.535 |
|  | PROPOSED EASTERN | LOS | B | B |
|  |  | Delay (sec/veh) | 10.5 | 10.2 |

TABLE 6.3: 2024 BACKGROUND AND DEVELOPMENT TRAFFIC CAPACITY CALCULATION RESULTS (10 YEAR HORIZON)

| I NTERSECTI ON |  |  | FRIDAY PEAK HOUR | SATURDAY PEAK HOUR |
| :---: | :---: | :---: | :---: | :---: |
| 5 <br> 0 <br> 0 <br> 0 <br> 0 <br> 0 <br> 0 <br>  | N14/ <br> PROPOSED WESTERN ACCESS | V/C ratio | 0.679 | 0.769 |
|  |  | LOS | A | B |
|  |  | Delay (sec/veh) | 9.5 | 11.7 |
|  |  | V/C ratio | 0.681 | 0.546 |
|  | PROPOSED EASTERN | LOS | B | B |
|  |  | Delay (sec/veh) | 10.8 | 10.3 |

VRYBURG RETAIL DEVELOPMENT - ACCESS STUDY

## 7 ACCESS

### 7.1 Introduction

The proposed roundabout accesses are to be located along a section of the N14 under the jurisdiction of the Naledi Local Municipality.

### 7.2 Proposed Accesses

The configurations of the proposed eastern and western accesses are shown in the Figures 7.1 and 7.2 respectively.


Figure 7.1:Proposed Eastern Access to Development


Figure 7.2: Proposed Western Access to Development
A detailed layout plan showing the access positions is included in ANNEXURE B.

### 7.3 Access Spacing

## Proposed Development

The N14 leading into Vryburg can be classified as an urban class 4a commercial collector road in accordance with TRH26 'South African Road Classification and Access Management Manual' Version 1.0, December 2011.

For a class 4a commercial collector, a minimum spacing of 200 m is required between intersection centre lines.

The spacing between the proposed accesses is 227 m between centre lines and therefore meets the minimum requirements in accordance with the TRH26.

## Existing Developments

Access to the existing Fairview Estate residential development is gained via a priority controlled access off the N14. Fairview Estate's access lies on a section of the N14 under the jurisdiction of SANRAL, but does not comply with the minimum access spacing requirements of SANRAL or TRH26. The access is approximately 375 m away from the nearest access to the south.

During discussions with SANRAL they indicated that they will deal with Fairview access and substandard spacing.

## 8 CONCLUSION

The proposed development will consist of a 35000 m 2 retail centre and will generate $\mathbf{1} \mathbf{7 9 2}$ and $\mathbf{3 0 3 5}$ trips during Friday and Saturday peak hours, respectively.

The proposed development will require two accesses off the N14. The accesses will be able to accommodate the base year traffic as well as the 2019 and 2024 horizon year traffic.

The two accesses to the development will be controlled by roundabouts.

The proposed retail development will not have a negative impact on the surrounding road network.

[^0]February 2013

|  | VRYBURG RETAIL DEVELOPMENT - ACCESS STUDY MEMORANDUM | PAGE11 |
| :---: | :---: | :---: |

## ANNEXURE A <br> DETAI LED CAPACITY CALCULATI ONS

N14 / Proposed East Access Roundabout 2014 Friday Peak Hour Background and Development Traffic
Option 2 Proposed Configuration
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | $\begin{aligned} & \text { Deg. } \\ & \text { Satn } \end{aligned}$ $\mathrm{v} / \mathrm{c}$ | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Eastern Access (S) 0.0 |  |  |  |  |  |  |  |  |  |  |
| 1 L | 114 | 0.0 | 0.536 | 14.2 | LOS B | 5.9 | 41.0 | 0.89 | 0.96 | 42.6 |
| 3 R | 264 | 0.0 | 0.536 | 18.3 | LOS B | 5.9 | 41.0 | 0.89 | 0.98 | 40.5 |
| Approach | 378 | 0.0 | 0.536 | 17.1 | LOS B | 5.9 | 41.0 | 0.89 | 0.97 | 41.1 |
| East: N14 (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 283 | 0.0 | 0.651 | 8.0 | LOS A | 9.7 | 67.9 | 0.58 | 0.56 | 47.6 |
| 5 T | 579 | 0.0 | 0.651 | 7.1 | LOS A | 9.7 | 67.9 | 0.58 | 0.52 | 47.7 |
| Approach | 862 | 0.0 | 0.650 | 7.4 | LOS A | 9.7 | 67.9 | 0.58 | 0.53 | 47.7 |
| West: N14 (W) |  |  |  |  |  |  |  |  |  |  |
| 11 T | 534 | 0.0 | 0.626 | 9.5 | LOS A | 8.1 | 56.5 | 0.81 | 0.74 | 46.4 |
| 12 R | 95 | 0.0 | 0.627 | 14.4 | LOS B | 8.1 | 56.5 | 0.81 | 0.81 | 44.3 |
| Approach | 628 | 0.0 | 0.627 | 10.2 | LOS B | 8.1 | 56.5 | 0.81 | 0.75 | 46.0 |
| All Vehicles | 1868 | 0.0 | 0.650 | 10.3 | LOS B | 9.7 | 67.9 | 0.72 | 0.69 | 45.6 |

Level of Service (Aver. Int. Delay): LOS B. Based on average delay for all vehicle movements. LOS Method: Delay (HCM).
Level of Service (Worst Movement): LOS B. LOS Method for individual vehicle movements: Delay (HCM).
Approach LOS values are based on the worst delay for any vehicle movement.
Roundabout LOS Method: Same as Signalised Intersections.
Roundabout Capacity Model: SIDRA Standard.

N14 / Proposed East Access Roundabout 2014 Saturday Peak Hour Background and Development Traffic
Option 2 Proposed Configuration
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | $\begin{aligned} & \text { Deg. } \\ & \text { Satn } \\ & \text { v/c } \end{aligned}$ | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Access (S) |  |  |  |  |  |  |  |  |  |  |
| 1 L | 192 | 0.0 | 0.522 | 8.2 | LOS A | 6.0 | 42.0 | 0.54 | 0.57 | 47.2 |
| 3 R | 447 | 0.0 | 0.521 | 12.3 | LOS B | 6.0 | 42.0 | 0.54 | 0.67 | 44.8 |
| Approach | 639 | 0.0 | 0.522 | 11.0 | LOS B | 6.0 | 42.0 | 0.54 | 0.64 | 45.4 |
| East: N 14 (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 479 | 0.0 | 0.525 | 8.4 | LOS A | 6.0 | 42.2 | 0.60 | 0.62 | 47.3 |
| 5 T | 134 | 0.0 | 0.524 | 7.6 | LOS A | 6.0 | 42.2 | 0.60 | 0.58 | 47.4 |
| Approach | 613 | 0.0 | 0.525 | 8.2 | LOS A | 6.0 | 42.2 | 0.60 | 0.61 | 47.3 |
| West: N14 (W) |  |  |  |  |  |  |  |  |  |  |
| 11 T | 128 | 0.0 | 0.354 | 9.8 | LOS A | 3.1 | 21.4 | 0.73 | 0.75 | 46.3 |
| 12 R | 160 | 0.0 | 0.354 | 14.7 | LOS B | 3.1 | 21.4 | 0.73 | 0.84 | 43.6 |
| Approach | 288 | 0.0 | 0.354 | 12.5 | LOS B | 3.1 | 21.4 | 0.73 | 0.80 | 44.7 |
| All Vehicles | 1540 | 0.0 | 0.525 | 10.2 | LOS B | 6.0 | 42.2 | 0.60 | 0.66 | 46.0 |

Level of Service (Aver. Int. Delay): LOS B. Based on average delay for all vehicle movements. LOS Method: Delay (HCM). Level of Service (Worst Movement): LOS B. LOS Method for individual vehicle movements: Delay (HCM).
Approach LOS values are based on the worst delay for any vehicle movement.
Roundabout LOS Method: Same as Signalised Intersections.
Roundabout Capacity Model: SIDRA Standard.

N14 / Proposed East Access Roundabout 2019 Friday Peak Hour Background and Development Traffic
Option 2 Proposed Configuration
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Access (S) |  |  |  |  |  |  |  |  |  |  |
| 1 L | 114 | 0.0 | 0.549 | 14.8 | LOS B | 6.1 | 43.0 | 0.90 | 0.98 | 42.1 |
| 3 R | 264 | 0.0 | 0.549 | 18.9 | LOS B | 6.1 | 43.0 | 0.90 | 1.00 | 40.1 |
| Approach | 378 | 0.0 | 0.549 | 17.7 | LOS B | 6.1 | 43.0 | 0.90 | 1.00 | 40.6 |
| East: N 14 (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 283 | 0.0 | 0.665 | 8.0 | LOSA | 10.2 | 71.3 | 0.60 | 0.56 | 47.6 |
| 5 T | 599 | 0.0 | 0.665 | 7.2 | LOSA | 10.2 | 71.3 | 0.60 | 0.52 | 47.6 |
| Approach | 882 | 0.0 | 0.665 | 7.5 | LOS A | 10.2 | 71.3 | 0.60 | 0.53 | 47.6 |
| West: N14 (W) |  |  |  |  |  |  |  |  |  |  |
| 11 T | 555 | 0.0 | 0.647 | 9.8 | LOS A | 8.7 | 61.2 | 0.83 | 0.75 | 46.3 |
| 12 R | 95 | 0.0 | 0.649 | 14.7 | LOS B | 8.7 | 61.2 | 0.83 | 0.82 | 44.1 |
| Approach | 649 | 0.0 | 0.647 | 10.5 | LOS B | 8.7 | 61.2 | 0.83 | 0.76 | 45.9 |
| All Vehicles | 1909 | 0.0 | 0.665 | 10.5 | LOS B | 10.2 | 71.3 | 0.74 | 0.70 | 45.5 |

Level of Service (Aver. Int. Delay): LOS B. Based on average delay for all vehicle movements. LOS Method: Delay (HCM).
Level of Service (Worst Movement): LOS B. LOS Method for individual vehicle movements: Delay (HCM).
Approach LOS values are based on the worst delay for any vehicle movement.
Roundabout LOS Method: Same as Signalised Intersections.
Roundabout Capacity Model: SIDRA Standard.

N14 / Proposed East Access Roundabout
2019 Saturday Peak Hour
Background and Development Traffic
Option 2 Proposed Configuration
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | $\begin{aligned} & \text { Deg. } \\ & \text { Satn } \end{aligned}$ $\mathrm{v} / \mathrm{c}$ | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue <br> Distance <br> m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Access (S) |  |  |  |  |  |  |  |  |  |  |
| L | 192 | 0.0 | 0.532 | 8.3 | LOS A | 6.1 | 42.9 | 0.57 | 0.59 | 47.0 |
| 3 R | 447 | 0.0 | 0.532 | 12.4 | LOS B | 6.1 | 42.9 | 0.57 | 0.68 | 44.7 |
| Approach | 639 | 0.0 | 0.532 | 11.2 | LOS B | 6.1 | 42.9 | 0.57 | 0.65 | 45.3 |
| East: N14 (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 479 | 0.0 | 0.535 | 8.5 | LOS A | 6.2 | 43.6 | 0.61 | 0.62 | 47.3 |
| 5 T | 146 | 0.0 | 0.534 | 7.6 | LOS A | 6.2 | 43.6 | 0.61 | 0.58 | 47.3 |
| Approach | 625 | 0.0 | 0.535 | 8.3 | LOS A | 6.2 | 43.6 | 0.61 | 0.61 | 47.3 |
| West: N14 (W) |  |  |  |  |  |  |  |  |  |  |
| 11 T | 140 | 0.0 | 0.369 | 9.8 | LOS A | 3.2 | 22.7 | 0.74 | 0.75 | 46.3 |
| 12 R | 160 | 0.0 | 0.370 | 14.7 | LOS B | 3.2 | 22.7 | 0.74 | 0.84 | 43.6 |
| Approach | 300 | 0.0 | 0.370 | 12.4 | LOS B | 3.2 | 22.7 | 0.74 | 0.80 | 44.8 |
| All Vehicles | 1564 | 0.0 | 0.535 | 10.2 | LOS B | 6.2 | 43.6 | 0.62 | 0.66 | 46.0 |

Level of Service (Aver. Int. Delay): LOS B. Based on average delay for all vehicle movements. LOS Method: Delay (HCM).
Level of Service (Worst Movement): LOS B. LOS Method for individual vehicle movements: Delay (HCM).
Approach LOS values are based on the worst delay for any vehicle movement.
Roundabout LOS Method: Same as Signalised Intersections.
Roundabout Capacity Model: SIDRA Standard.

N14 / Proposed East Access Roundabout 2024 Friday Peak Hour Background and Development Traffic
Option 2 Proposed Configuration
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{array}{r} \text { HV } \\ \% \end{array}$ | $\begin{aligned} & \text { Deg. } \\ & \text { Satn } \\ & \text { v/c } \end{aligned}$ | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Access (S) |  |  |  |  |  |  |  |  |  |  |
| L | 114 | 0.0 | 0.566 | 15.6 | LOS B | 6.5 | 45.6 | 0.92 | 1.01 | 41.5 |
| 3 R | 264 | 0.0 | 0.567 | 19.7 | LOS B | 6.5 | 45.6 | 0.92 | 1.03 | 39.5 |
| Approach | 378 | 0.0 | 0.567 | 18.5 | LOS B | 6.5 | 45.6 | 0.92 | 1.02 | 40.1 |
| East: N14 (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 283 | 0.0 | 0.681 | 8.1 | LOSA | 10.8 | 75.5 | 0.62 | 0.56 | 47.5 |
| 5 T | 622 | 0.0 | 0.681 | 7.2 | LOSA | 10.8 | 75.5 | 0.62 | 0.52 | 47.5 |
| Approach | 905 | 0.0 | 0.681 | 7.5 | LOS A | 10.8 | 75.5 | 0.62 | 0.53 | 47.5 |
| West: N14 (W) |  |  |  |  |  |  |  |  |  |  |
| 11 T | 579 | 0.0 | 0.670 | 10.2 | LOS B | 9.6 | 67.0 | 0.85 | 0.77 | 46.1 |
| 12 R | 95 | 0.0 | 0.672 | 15.1 | LOS B | 9.6 | 67.0 | 0.85 | 0.83 | 43.8 |
| Approach | 674 | 0.0 | 0.670 | 10.9 | LOS B | 9.6 | 67.0 | 0.85 | 0.78 | 45.8 |
| All Vehicles | 1957 | 0.0 | 0.681 | 10.8 | LOS B | 10.8 | 75.5 | 0.76 | 0.71 | 45.3 |

Level of Service (Aver. Int. Delay): LOS B. Based on average delay for all vehicle movements. LOS Method: Delay (HCM). Level of Service (Worst Movement): LOS B. LOS Method for individual vehicle movements: Delay (HCM).
Approach LOS values are based on the worst delay for any vehicle movement.
Roundabout LOS Method: Same as Signalised Intersections.
Roundabout Capacity Model: SIDRA Standard.

N14 / Proposed East Access Roundabout 2024 Saturday Peak Hour Background and Development Traffic
Option 2 Proposed Configuration
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | $\begin{aligned} & \text { Deg. } \\ & \text { Satn } \\ & \text { v/c } \end{aligned}$ | Average Delay sec | Level of Service | 95\% Back o Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Access (S) |  |  |  |  |  |  |  |  |  |  |
| L | 192 | 0.0 | 0.543 | 8.5 | LOS A | 6.3 | 43.9 | 0.60 | 0.60 | 46.8 |
| 3 R | 447 | 0.0 | 0.543 | 12.6 | LOS B | 6.3 | 43.9 | 0.60 | 0.68 | 44.6 |
| Approach | 639 | 0.0 | 0.543 | 11.3 | LOS B | 6.3 | 43.9 | 0.60 | 0.66 | 45.2 |
| East: N14 (E) |  |  |  |  |  |  |  |  |  |  |
| L | 479 | 0.0 | 0.546 | 8.5 | LOS A | 6.5 | 45.2 | 0.62 | 0.62 | 47.3 |
| 5 T | 160 | 0.0 | 0.546 | 7.6 | LOS A | 6.5 | 45.2 | 0.62 | 0.58 | 47.3 |
| Approach | 639 | 0.0 | 0.546 | 8.3 | LOS A | 6.5 | 45.2 | 0.62 | 0.61 | 47.3 |
| West: N14 (W) |  |  |  |  |  |  |  |  |  |  |
| 11 T | 153 | 0.0 | 0.386 | 9.8 | LOS A | 3.4 | 24.0 | 0.75 | 0.76 | 46.2 |
| 12 R | 160 | 0.0 | 0.386 | 14.8 | LOS B | 3.4 | 24.0 | 0.75 | 0.85 | 43.6 |
| Approach | 313 | 0.0 | 0.387 | 12.4 | LOS B | 3.4 | 24.0 | 0.75 | 0.80 | 44.8 |
| All Vehicles | 1591 | 0.0 | 0.546 | 10.3 | LOS B | 6.5 | 45.2 | 0.64 | 0.67 | 45.9 |

Level of Service (Aver. Int. Delay): LOS B. Based on average delay for all vehicle movements. LOS Method: Delay (HCM). Level of Service (Worst Movement): LOS B. LOS Method for individual vehicle movements: Delay (HCM).
Approach LOS values are based on the worst delay for any vehicle movement.
Roundabout LOS Method: Same as Signalised Intersections.
Roundabout Capacity Model: SIDRA Standard.

N14 / Proposed West Access Roundabout 2014 Friday Peak Hour
Background and Development Traffic
Option 2 Proposed Configuration
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Western Access (S) |  |  |  |  |  |  |  |  |  |  |
| 1 L | 207 | 0.0 | 0.239 | 8.7 | LOS A | 1.7 | 11.9 | 0.54 | 0.67 | 47.7 |
| 2 T | 1 | 0.0 | 0.351 | 7.2 | LOS A | 2.8 | 19.7 | 0.57 | 0.60 | 47.2 |
| 3 R | 359 | 0.0 | 0.350 | 12.9 | LOS B | 2.8 | 19.7 | 0.57 | 0.74 | 44.3 |
| Approach | 567 | 0.0 | 0.350 | 11.4 | LOS B | 2.8 | 19.7 | 0.56 | 0.72 | 45.4 |
| East: N14 (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 377 | 0.0 | 0.643 | 8.1 | LOS A | 5.8 | 40.6 | 0.58 | 0.64 | 47.8 |
| 5 T | 306 | 0.0 | 0.642 | 6.8 | LOS A | 5.8 | 40.6 | 0.58 | 0.57 | 47.9 |
| 6 R | 1 | 0.0 | 0.002 | 12.8 | LOS B | 0.0 | 0.1 | 0.41 | 0.61 | 44.9 |
| Approach | 684 | 0.0 | 0.643 | 7.5 | LOS B | 5.8 | 40.6 | 0.57 | 0.61 | 47.9 |
| North: Farm Access |  |  |  |  |  |  |  |  |  |  |
| 7 L | 1 | 0.0 | 0.003 | 9.7 | LOS A | 0.0 | 0.1 | 0.63 | 0.60 | 47.6 |
| 8 T | 1 | 0.0 | 0.003 | 8.5 | LOS A | 0.0 | 0.1 | 0.63 | 0.54 | 47.6 |
| 9 R | 1 | 0.0 | 0.002 | 15.3 | LOS B | 0.0 | 0.1 | 0.64 | 0.65 | 42.8 |
| Approach | 3 | 0.0 | 0.003 | 11.1 | LOS B | 0.0 | 0.1 | 0.63 | 0.60 | 45.9 |
| West: N14 (W) |  |  |  |  |  |  |  |  |  |  |
| 10 L | 1 | 0.0 | 0.263 | 8.7 | LOS A | 2.0 | 14.1 | 0.57 | 0.71 | 48.2 |
| 11 T | 260 | 0.0 | 0.265 | 7.4 | LOS A | 2.0 | 14.1 | 0.57 | 0.64 | 48.3 |
| 12 R | 188 | 0.0 | 0.213 | 13.3 | LOS B | 1.5 | 10.6 | 0.56 | 0.75 | 44.3 |
| Approach | 449 | 0.0 | 0.265 | 9.9 | LOS B | 2.0 | 14.1 | 0.57 | 0.69 | 46.5 |
| All Vehicles | 1704 | 0.0 | 0.643 | 9.4 | LOS A | 5.8 | 40.6 | 0.57 | 0.66 | 46.7 |

Level of Service (Aver. Int. Delay): LOS A. Based on average delay for all vehicle movements. LOS Method: Delay (HCM).
Level of Service (Worst Movement): LOS B. LOS Method for individual vehicle movements: Delay (HCM).
Approach LOS values are based on the worst delay for any vehicle movement.
Roundabout LOS Method: Same as Signalised Intersections.
Roundabout Capacity Model: SIDRA Standard.

Project: W:IRoadsITRAFFICIPROJECTSIK PROJECTSIK1471 - Vryburgl05 CalculationsIO2 SIDRAIBackground

N14 / Proposed West Access Roundabout 2014 Saturday Peak Hour
Background and Development Traffic
Option 2 Proposed Configuration
Roundabout


Level of Service (Aver. Int. Delay): LOS B. Based on average delay for all vehicle movements. LOS Method: Delay (HCM).
Level of Service (Worst Movement): LOS B. LOS Method for individual vehicle movements: Delay (HCM).
Approach LOS values are based on the worst delay for any vehicle movement.
Roundabout LOS Method: Same as Signalised Intersections.
Roundabout Capacity Model: SIDRA Standard.

N14 / Proposed West Access Roundabout 2019 Friday Peak Hour Background and Development Traffic
Option 2 Proposed Configuration
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Western Access (S) |  |  |  |  |  |  |  |  |  |  |
| 1 L | 207 | 0.0 | 0.243 | 8.9 | LOS A | 1.7 | 12.1 | 0.56 | 0.68 | 47.6 |
| 2 T | 1 | 0.0 | 0.351 | 7.3 | LOS A | 2.9 | 20.1 | 0.59 | 0.62 | 47.0 |
| 3 R | 359 | 0.0 | 0.356 | 13.0 | LOS B | 2.9 | 20.1 | 0.59 | 0.75 | 44.2 |
| Approach | 567 | 0.0 | 0.356 | 11.5 | LOS B | 2.9 | 20.1 | 0.58 | 0.73 | 45.4 |
| East: N14 (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 377 | 0.0 | 0.659 | 8.1 | LOS A | 6.1 | 42.5 | 0.59 | 0.64 | 47.8 |
| 5 T | 325 | 0.0 | 0.660 | 6.8 | LOS A | 6.1 | 42.5 | 0.59 | 0.58 | 47.9 |
| 6 R | 1 | 0.0 | 0.002 | 12.8 | LOS B | 0.0 | 0.1 | 0.41 | 0.61 | 44.9 |
| Approach | 703 | 0.0 | 0.659 | 7.5 | LOS B | 6.1 | 42.5 | 0.59 | 0.61 | 47.8 |
| North: Farm Access |  |  |  |  |  |  |  |  |  |  |
| 7 L | 1 | 0.0 | 0.003 | 9.8 | LOS A | 0.0 | 0.1 | 0.64 | 0.60 | 47.6 |
| 8 T | 1 | 0.0 | 0.003 | 8.6 | LOS A | 0.0 | 0.1 | 0.64 | 0.55 | 47.6 |
| 9 R | 1 | 0.0 | 0.002 | 15.4 | LOS B | 0.0 | 0.1 | 0.65 | 0.65 | 42.7 |
| Approach | 3 | 0.0 | 0.003 | 11.3 | LOS B | 0.0 | 0.1 | 0.64 | 0.60 | 45.8 |
| West: N14 (W) |  |  |  |  |  |  |  |  |  |  |
| 10 L | 1 | 0.0 | 0.263 | 8.7 | LOS A | 2.2 | 15.5 | 0.59 | 0.71 | 48.2 |
| 11 T | 281 | 0.0 | 0.287 | 7.4 | LOS A | 2.2 | 15.5 | 0.59 | 0.65 | 48.3 |
| 12 R | 188 | 0.0 | 0.218 | 13.4 | LOS B | 1.6 | 10.9 | 0.57 | 0.75 | 44.3 |
| Approach | 471 | 0.0 | 0.287 | 9.8 | LOS B | 2.2 | 15.5 | 0.58 | 0.69 | 46.6 |
| All Vehicles | 1744 | 0.0 | 0.659 | 9.4 | LOS A | 6.1 | 42.5 | 0.58 | 0.67 | 46.6 |

Level of Service (Aver. Int. Delay): LOS A. Based on average delay for all vehicle movements. LOS Method: Delay (HCM).
Level of Service (Worst Movement): LOS B. LOS Method for individual vehicle movements: Delay (HCM).
Approach LOS values are based on the worst delay for any vehicle movement.
Roundabout LOS Method: Same as Signalised Intersections.
Roundabout Capacity Model: SIDRA Standard.

Project: W:IRoadsITRAFFICIPROJECTSIK PROJECTSIK1471 - Vryburgl05 CalculationsIO2 SIDRAIBackground

N14 / Proposed West Access Roundabout 2019 Saturday Peak Hour
Background and Development Traffic
Option 2 Proposed Configuration
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | $\begin{aligned} & \text { Deg. } \\ & \text { Satn } \\ & \text { v/c } \end{aligned}$ | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Western Access (S) 0.0 |  |  |  |  |  |  |  |  |  |  |
| L | 352 | 0.0 | 0.323 | 7.5 | LOS A | 2.8 | 19.9 | 0.41 | 0.57 | 48.4 |
| 2 T | 1 | 0.0 | 0.526 | 6.1 | LOSA | 5.0 | 34.9 | 0.46 | 0.47 | 48.1 |
| 3 R | 607 | 0.0 | 0.466 | 11.8 | LOS B | 5.0 | 34.9 | 0.46 | 0.64 | 44.7 |
| Approach | 960 | 0.0 | 0.466 | 10.2 | LOS B | 5.0 | 34.9 | 0.44 | 0.61 | 46.0 |
| East: N14 (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 639 | 0.0 | 0.755 | 12.1 | LOS B | 11.2 | 78.2 | 0.86 | 0.87 | 45.1 |
| 5 T | 114 | 0.0 | 0.753 | 10.9 | LOS B | 11.2 | 78.2 | 0.86 | 0.85 | 45.4 |
| 6 R | 1 | 0.0 | 0.002 | 13.9 | LOS B | 0.0 | 0.1 | 0.53 | 0.62 | 44.0 |
| Approach | 754 | 0.0 | 0.755 | 11.9 | LOS B | 11.2 | 78.2 | 0.85 | 0.86 | 45.1 |
| North: Farm Access |  |  |  |  |  |  |  |  |  |  |
| 7 L | 1 | 0.0 | 0.003 | 12.0 | LOS B | 0.0 | 0.2 | 0.75 | 0.62 | 45.5 |
| 8 T | 1 | 0.0 | 0.003 | 10.8 | LOS B | 0.0 | 0.2 | 0.75 | 0.59 | 45.9 |
| 9 R | 1 | 0.0 | 0.002 | 18.1 | LOS B | 0.0 | 0.1 | 0.75 | 0.66 | 40.7 |
| Approach | 3 | 0.0 | 0.003 | 13.6 | LOS B | 0.0 | 0.2 | 0.75 | 0.62 | 43.8 |
| West: N14 (W) |  |  |  |  |  |  |  |  |  |  |
| 10 L | 1 | 0.0 | 0.211 | 12.2 | LOS B | 1.4 | 9.5 | 0.71 | 0.85 | 45.6 |
| 11 T | 107 | 0.0 | 0.205 | 10.9 | LOS B | 1.4 | 9.5 | 0.71 | 0.80 | 46.1 |
| 12 R | 320 | 0.0 | 0.399 | 15.2 | LOS B | 3.3 | 23.1 | 0.77 | 0.86 | 42.9 |
| Approach | 428 | 0.0 | 0.398 | 14.1 | LOS B | 3.3 | 23.1 | 0.75 | 0.85 | 43.6 |
| All Vehicles | 2145 | 0.0 | 0.755 | 11.6 | LOS B | 11.2 | 78.2 | 0.65 | 0.75 | 45.2 |

Level of Service (Aver. Int. Delay): LOS B. Based on average delay for all vehicle movements. LOS Method: Delay (HCM).
Level of Service (Worst Movement): LOS B. LOS Method for individual vehicle movements: Delay (HCM).
Approach LOS values are based on the worst delay for any vehicle movement.
Roundabout LOS Method: Same as Signalised Intersections.
Roundabout Capacity Model: SIDRA Standard.

N14 / Proposed West Access Roundabout 2024 Friday Peak Hour
Background and Development Traffic
Option 2 Proposed Configuration
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Western Access (S) |  |  |  |  |  |  |  |  |  |  |
| 1 L | 207 | 0.0 | 0.249 | 9.0 | LOS A | 1.8 | 12.4 | 0.58 | 0.70 | 47.5 |
| 2 T | 1 | 0.0 | 0.351 | 7.5 | LOS A | 2.9 | 20.6 | 0.61 | 0.64 | 46.9 |
| 3 R | 359 | 0.0 | 0.363 | 13.2 | LOS B | 2.9 | 20.6 | 0.61 | 0.76 | 44.1 |
| Approach | 567 | 0.0 | 0.363 | 11.7 | LOS B | 2.9 | 20.6 | 0.60 | 0.74 | 45.3 |
| East: N14 (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 377 | 0.0 | 0.679 | 8.2 | LOS A | 6.4 | 44.9 | 0.60 | 0.64 | 47.7 |
| 5 T | 348 | 0.0 | 0.679 | 6.9 | LOS A | 6.4 | 44.9 | 0.60 | 0.58 | 47.8 |
| 6 R | 1 | 0.0 | 0.002 | 12.8 | LOS B | 0.0 | 0.1 | 0.41 | 0.61 | 44.9 |
| Approach | 726 | 0.0 | 0.679 | 7.6 | LOS B | 6.4 | 44.9 | 0.60 | 0.61 | 47.7 |
| North: Farm Access |  |  |  |  |  |  |  |  |  |  |
| 7 L | 1 | 0.0 | 0.003 | 10.0 | LOS A | 0.0 | 0.1 | 0.65 | 0.60 | 47.4 |
| 8 T | 1 | 0.0 | 0.003 | 8.7 | LOS A | 0.0 | 0.1 | 0.65 | 0.55 | 47.5 |
| 9 R | 1 | 0.0 | 0.002 | 15.6 | LOS B | 0.0 | 0.1 | 0.66 | 0.65 | 42.6 |
| Approach | 3 | 0.0 | 0.003 | 11.4 | LOS B | 0.0 | 0.1 | 0.65 | 0.60 | 45.7 |
| West: N14 (W) |  |  |  |  |  |  |  |  |  |  |
| 10 L | 1 | 0.0 | 0.351 | 8.8 | LOS A | 2.5 | 17.3 | 0.60 | 0.72 | 48.1 |
| 11 T | 306 | 0.0 | 0.313 | 7.5 | LOS A | 2.5 | 17.3 | 0.60 | 0.66 | 48.2 |
| 12 R | 188 | 0.0 | 0.224 | 13.5 | LOS B | 1.6 | 11.2 | 0.58 | 0.76 | 44.3 |
| Approach | 496 | 0.0 | 0.313 | 9.8 | LOS B | 2.5 | 17.3 | 0.59 | 0.69 | 46.6 |
| All Vehicles | 1793 | 0.0 | 0.679 | 9.5 | LOS A | 6.4 | 44.9 | 0.60 | 0.67 | 46.6 |

Level of Service (Aver. Int. Delay): LOS A. Based on average delay for all vehicle movements. LOS Method: Delay (HCM).
Level of Service (Worst Movement): LOS B. LOS Method for individual vehicle movements: Delay (HCM).
Approach LOS values are based on the worst delay for any vehicle movement.
Roundabout LOS Method: Same as Signalised Intersections.
Roundabout Capacity Model: SIDRA Standard.

Project: W:IRoadsITRAFFICIPROJECTSIK PROJECTSIK1471 - Vryburgl05 CalculationsIO2 SIDRAIBackground

N14 / Proposed West Access Roundabout 2024 Saturday Peak Hour
Background and Development Traffic
Option 2 Proposed Configuration
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | $\begin{aligned} & \text { Deg. } \\ & \text { Satn } \\ & \text { v/c } \end{aligned}$ | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
|  |  |  |  |  |  |  |  |  |  |  |
| L | 352 | 0.0 | 0.331 | 7.6 | LOS A | 2.9 | 20.4 | 0.44 | 0.58 | 48.2 |
| 2 T | 1 | 0.0 | 0.526 | 6.2 | LOS A | 5.1 | 35.7 | 0.49 | 0.49 | 47.9 |
| 3 R | 607 | 0.0 | 0.477 | 11.9 | LOS B | 5.1 | 35.7 | 0.49 | 0.65 | 44.6 |
| Approach | 960 | 0.0 | 0.477 | 10.3 | LOS B | 5.1 | 35.7 | 0.47 | 0.62 | 45.8 |
| East: N14 (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 639 | 0.0 | 0.769 | 12.5 | LOS B | 11.8 | 82.6 | 0.87 | 0.88 | 44.8 |
| 5 T | 128 | 0.0 | 0.769 | 11.2 | LOS B | 11.8 | 82.6 | 0.87 | 0.86 | 45.1 |
| 6 R | 1 | 0.0 | 0.002 | 13.9 | LOS B | 0.0 | 0.1 | 0.53 | 0.62 | 44.0 |
| Approach | 768 | 0.0 | 0.769 | 12.2 | LOS B | 11.8 | 82.6 | 0.87 | 0.88 | 44.9 |
| North: Farm Access |  |  |  |  |  |  |  |  |  |  |
| 7 L | 1 | 0.0 | 0.003 | 12.1 | LOS B | 0.0 | 0.2 | 0.75 | 0.62 | 45.4 |
| 8 T | 1 | 0.0 | 0.003 | 10.8 | LOS B | 0.0 | 0.2 | 0.75 | 0.59 | 45.9 |
| 9 R | 1 | 0.0 | 0.002 | 18.1 | LOS B | 0.0 | 0.1 | 0.75 | 0.67 | 40.7 |
| Approach | 3 | 0.0 | 0.003 | 13.6 | LOS B | 0.0 | 0.2 | 0.75 | 0.63 | 43.8 |
| West: N14 (W) |  |  |  |  |  |  |  |  |  |  |
| 10 L | 1 | 0.0 | 0.211 | 11.9 | LOS B | 1.5 | 10.5 | 0.71 | 0.84 | 45.9 |
| 11 T | 121 | 0.0 | 0.219 | 10.7 | LOS B | 1.5 | 10.5 | 0.71 | 0.80 | 46.4 |
| 12 R | 320 | 0.0 | 0.401 | 15.2 | LOS B | 3.3 | 23.4 | 0.77 | 0.86 | 42.9 |
| Approach | 442 | 0.0 | 0.401 | 13.9 | LOS B | 3.3 | 23.4 | 0.76 | 0.85 | 43.8 |
| All Vehicles | 2174 | 0.0 | 0.769 | 11.7 | LOS B | 11.8 | 82.6 | 0.67 | 0.76 | 45.0 |

Level of Service (Aver. Int. Delay): LOS B. Based on average delay for all vehicle movements. LOS Method: Delay (HCM).
Level of Service (Worst Movement): LOS B. LOS Method for individual vehicle movements: Delay (HCM).
Approach LOS values are based on the worst delay for any vehicle movement.
Roundabout LOS Method: Same as Signalised Intersections.
Roundabout Capacity Model: SIDRA Standard.

## ANNEXURE B <br> DETAI LED LAYOUT PLAN




[^0]:    MM Gounden
    For CIVIL CONCEPTS (PTY) LTD

