

VRYBURG RETAIL DEVELOPMENT ACCESS STUDY MEMORANDUM

K1471/1

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CIVIL AND STRUCTURAL CONSULTING ENGINEERS

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The site of the proposed development is located to the south of Vryburg as shown in Figure 1.1. This memorandum addresses the accesses to the proposed Vryburg Retail development.

The proposed development will consist of a 35000 m² retail centre and will generate 1 792 and 3 035 trips during the Friday afternoon and Saturday peak hours, respectively.



Figure 1.1: Locality Plan



2 LAND USE AND TRIP GENERATION

2.1 Land Use

The Vryburg Retail development will consist of the following land-use:

• 35000m² retail centre.

2.2 Trip Generation

The trip generation rates in the Department of Transport's "South African Trip Generation Rates, 2nd Edition, June 1995" are used as a basis for the calculation of the proposed development trips.

The retail trips will comprise of primary and pass-by trips only. Due to the location and the lack of retail centres of similar size in the vicinity of the proposed development it is assumed that there will not be any diverted trips generated. The diverted trips were considered as primary trips for this study.

Due to the low volume of traffic along the N14 the passer-by trips were reduced by 25% and the primary trips increased by 25%.

A trip reduction of 20% has been applied to account for:

- walking and public transport trips, and
- low vehicle ownership.

The Friday afternoon and Saturday peak hour trip generation for the proposed development are shown in Tables 2.1 and 2.2, respectively.

					Directional split Weekday		Pea	ak Hour T	rips
	Area	Unit							
Land Use	(m ²)/Units	(m ²)	Rate	Reduction	IN	OUT	IN	OUT	TOTAL
Retail	35000	100	6.40	20%	50%	50%	896	896	1792
				PRIMARY	90%		806	806	1613
				DIVERTED	0%		0	0	0
				PASSER-BY	10%		90	90	179
					Г	OTAL	896	896	1792

TABLE 2.1: FRIDAY AFTERNOON PEAK HOUR TRIP GENERATION(PROPOSED RETAIL DEVELOPMENT)



TABLE 2.2: SATURDAY PEAK HOUR TRIP GENERATION(PROPOSED RETAIL DEVELOPMENT)

				Directiona split		Directional split		Directional split Weekday		ak Hour T	rips
	Area	Unit			Wee						
Land Use	(m ²)/Units	(m ²)	Rate	Reduction	IN	OUT	IN	OUT	TOTAL		
Retail	35000	100	10.84	20%	50%	50%	1518	1518	3035		
				PRIMARY	90%		1366	1366	2732		
				DIVERTED	0%		0	0	0		
				PASSER-BY	10%		152	152	304		
					Т	OTAL	1518	1518	3035		

Retail trips are not considered in the morning peak hour due to the fact that most shops within a retail centre normally open after 8:00, which is outside peak of the surrounding road network. Trips to a retail centre during the morning peak hour are therefore considered to be negligible.

There are two residential developments within the vicinity of the proposed retail development, namely:

- Fairview Estates (High Income Residential)
- Rosendal Estates (Low Income Residential)

Traffic studies for the above mentioned developments were not available and therefore the trips generated by these residential developments were estimated in order to determine the latent trips to be considered as part of this study.

The Saturday peak hour residential trips are assumed to be 50% of the normal weekday peak hour residential trips.

The Friday afternoon and Saturday peak hour trip generation for the Fairview Estates residential development are shown in Tables 2.3 and 2.4, respectively.

					Directional split Weekday		Pe	ak Hour T	Frips
		Unit							
Land Use	Units	(m ²)	Rate	Reduction	IN	OUT	IN	OUT	TOTAL
High Income Residential	42	units	1.50	0%	75%	25%	47	16	63
						TOTAL	47	16	63

TABLE 2.3: FRIDAYAFTERNOON PEAK HOUR TRIP GENERATION (FAIRVIEW ESTATES)



VRYBURG RETAIL DEVELOPMENT – ACCESS STUDY MEMORANDUM

TABLE 2.4: SATURDAY PEAK HOUR TRIP GENERATION (FAIRVIEW ESTATES)

					Directional split		Pe	ak Hour T	Trips
		Unit			Wee	kday			
Land Use	Units	(m ²)	Rate	Reduction	IN	OUT	IN	Ουτ	TOTAL
High Income Residential	42	units	0.75	0%	50%	50%	16	16	32
						TOTAL	16	16	32

The Friday afternoon and Saturday peak hour trip generation for the Rosendal Estates residential development are shown in Tables 2.5 and 2.6, respectively.

TABLE 2.5: FRIDAYAFTERNOON PEAK HOUR TRIP GENERATION (ROSENDAL ESTATES)

					Directional split		Pe	ak Hour 1	Trips
		l lmit			Wee	kday			
Land Use	Units	(m ²)	Rate	Reduction	IN	OUT	IN	OUT	TOTAL
Low Income Residential	274	units	0.50	0%	65%	35%	89	48	137
						TOTAL	89	48	137

TABLE 2.6: SATURDAY PEAK HOUR TRIP GENERATION (ROSENDAL ESTATES)

					Directional split		Pe	ak Hour 1	Frips
		l lm:t			Wee	kday			
Land Use	Units	(m ²)	Rate	Reduction	IN	Ουτ	IN	Ουτ	TOTAL
Low Income Residential	274	units	0.25	0%	50%	50%	34	34	69
						TOTAL	34	34	69

3 THE EXISTING ROAD NETWORK

N14-is a class 2 road with one lane per direction and resorts under SANRAL. The SANRAL jurisdiction is 30 m south west of the developments boundary. The development is therefore situated along the Naledi Local Municipality jurisdiction and the N14 becomes a class 4a road along the site boundary, with closely spaced intersections.

4 TRIP DISTRIBUTION AND ASSIGNMENT

The development trips were distributed and assigned to the adjacent road network based on the expected origins and destinations to and from the development.



5 TRAFFIC COUNTS

5.1 Introduction

24 hour counts were conducted between 19 August 2009 and 15 September 2009 on the N14 between Kuruman and Vryburg. The Friday and Saturday peak hour traffic volumes were calculated from this data based on the assumption that 10%- 15% of the AADT occurs in the peak hour..

The 2009 weekday morning and afternoon and Saturday peak hour traffic counts are shown in Table 6.1.

TABLE	5.1:	2009	TRA	FFIC	COL	JNTS
					_	-

	Friday Peak Hour	Saturday Peak Hour
To Vryburg	120	65
To Kuruman	110	70

5.2 Background Traffic Volumes

Base Year -2014

The 2009 weekday morning and afternoon peak hour traffic counts were escalated at a 3% annual growth rate over 5 years and added to the Fairview Estates and Rosendal Estates development trips to obtain the 2014 peak hour background traffic volumes.

Horizon Year- 2019 (5 year) and 2024 (10 year)

The proposed retail development will generate more than 2 000 trips in the peak hour therefore a 5 year and 10 year horizon period was considered.

The 2009 weekday morning and afternoon peak hour traffic counts were escalated at 3% annual growth rate over 10 and 15 years and added to the Fairview Estates' and Rosendal Estates' development trips obtain the 2019 and 2024 peak hour background traffic volumes respectively.

5.3 Background And Development Traffic Volumes

The Friday and Saturday peak hour total development trips were added to the 2014, 2019 and 2024 background traffic volumes to obtain the 2014, 2019 and 2024 background and development trips.



6 TRAFFIC OPERATIONS AND CAPACITY ANALYSIS

6.1 Introduction

Capacity analysis was performed at the proposed retail centre access points with the N14 using the SIDRA Intersection 5.0 software package.

The accesses to the development are addressed in this section. The two accesses to the site will be referred to as the eastern and western access respectively.

6.2 Proposed Accesses

The traffic control at the intersections are:

- Eastern Access roundabout.
- Western Access roundabout.

Tables 6.1, 6.2 and 6.3 below show the results of the capacity analysis for the base year and the horizon years. Detailed capacity calculations are included in **ANNEXURE A**.

	INTERSECTIO	DN	FRIDAY PEAK HOUR	SATURDAY PEAK HOUR
	N14/	V/C ratio	0.643	0.744
	PROPOSED WESTERN	LOS	А	В
ABOUT	ACCESS	Delay (sec/veh)	9.4	11.5
ROUND	N14/	V/C ratio	0.650	0.525
	PROPOSED EASTERN	LOS	В	В
	AUCESS	Delay (sec/veh)	10.3	10.2

TABLE 6.1: 2014 BACKGROUND AND DEVELOPMENT TRAFFIC CAPACITY CALCULATION RESULTS (BASE YEAR)



TABLE 6.2: 2019 BACKGROUND AND DEVELOPMENT TRAFFIC CAPACITY CALCULATION RESULTS (5 YEAR HORIZON)

	INTERSECTIO	DN	FRIDAY PEAK HOUR	SATURDAY PEAK HOUR
ABOUT	N114/	V/C ratio	0.659	0.755
	PROPOSED WESTERN	LOS	А	В
	ACCESS	Delay (sec/veh)	9.4	11.6
ROUND	NI1 4 /	V/C ratio	0.665	0.535
	PROPOSED EASTERN	LOS	В	В
	ACCESS	Delay (sec/veh)	10.5	10.2

TABLE 6.3: 2024 BACKGROUND AND DEVELOPMENT TRAFFIC CAPACITY CALCULATION RESULTS (10 YEAR HORIZON)

	INTERSECTIO	ON	FRIDAY PEAK HOUR	SATURDAY PEAK HOUR
	N14/	V/C ratio	0.679	0.769
	PROPOSED WESTERN	LOS	А	В
ABOUT	ACCESS	Delay (sec/veh)	9.5	11.7
ROUND	NI1 4 /	V/C ratio	0.681	0.546
	PROPOSED EASTERN	LOS	В	В
	ACCESS	Delay (sec/veh)	10.8	10.3



7 ACCESS

7.1 Introduction

The proposed roundabout accesses are to be located along a section of the N14 under the jurisdiction of the Naledi Local Municipality.

7.2 Proposed Accesses

The configurations of the proposed eastern and western accesses are shown in the Figures 7.1 and 7.2 respectively.



Figure 7.1: Proposed Eastern Access to Development



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Figure 7.2: Proposed Western Access to Development

A detailed layout plan showing the access positions is included in **ANNEXURE B**.



7.3 Access Spacing

Proposed Development

The N14 leading into Vryburg can be classified as an urban class 4a commercial collector road in accordance with TRH26 'South African Road Classification and Access Management Manual' Version 1.0, December 2011.

For a class 4a commercial collector, a minimum spacing of 200m is required between intersection centre lines.

The spacing between the proposed accesses is 227m between centre lines and therefore meets the minimum requirements in accordance with the TRH26.

Existing Developments

Access to the existing Fairview Estate residential development is gained via a priority controlled access off the N14. Fairview Estate's access lies on a section of the N14 under the jurisdiction of SANRAL, but does not comply with the minimum access spacing requirements of SANRAL or TRH26. The access is approximately 375m away from the nearest access to the south.

During discussions with SANRAL they indicated that they will deal with Fairview access and substandard spacing.

8 CONCLUSION

The proposed development will consist of a 35 000 m2 retail centre and will generate **1 792** and **3 035** trips during Friday and Saturday peak hours, respectively.

The proposed development will require two accesses off the N14. The accesses will be able to accommodate the base year traffic as well as the 2019 and 2024 horizon year traffic.

The two accesses to the development will be controlled by roundabouts.

The proposed retail development will not have a negative impact on the surrounding road network.

MM Gounden For CIVIL CONCEPTS (PTY) LTD

February 2013



ANNEXURE A DETAILED CAPACITY CALCULATIONS

N14 / Proposed East Access Roundabout 2014 Friday Peak Hour Background and Development Traffic **Option 2 Proposed Configuration** Roundabout

Movem	ent Pe	rformance -	Vehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	f Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: E	astern	Access (S)									
1	L	114	0.0	0.536	14.2	LOS B	5.9	41.0	0.89	0.96	42.6
3	R	264	0.0	0.536	18.3	LOS B	5.9	41.0	0.89	0.98	40.5
Approacl	h	378	0.0	0.536	17.1	LOS B	5.9	41.0	0.89	0.97	41.1
East: N1	4 (E)										
4	L	283	0.0	0.651	8.0	LOS A	9.7	67.9	0.58	0.56	47.6
5	Т	579	0.0	0.651	7.1	LOS A	9.7	67.9	0.58	0.52	47.7
Approacl	h	862	0.0	0.650	7.4	LOS A	9.7	67.9	0.58	0.53	47.7
West: N1	14 (W)										
11	Т	534	0.0	0.626	9.5	LOS A	8.1	56.5	0.81	0.74	46.4
12	R	95	0.0	0.627	14.4	LOS B	8.1	56.5	0.81	0.81	44.3
Approacl	h	628	0.0	0.627	10.2	LOS B	8.1	56.5	0.81	0.75	46.0
All Vehic	les	1868	0.0	0.650	10.3	LOS B	9.7	67.9	0.72	0.69	45.6

Level of Service (Aver. Int. Delay): LOS B. Based on average delay for all vehicle movements. LOS Method: Delay (HCM).

Level of Service (Worst Movement): LOS B. LOS Method for individual vehicle movements: Delay (HCM).

Approach LOS values are based on the worst delay for any vehicle movement.

Roundabout LOS Method: Same as Signalised Intersections.

Roundabout Capacity Model: SIDRA Standard.

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N14 / Proposed East Access Roundabout 2014 Saturday Peak Hour Background and Development Traffic **Option 2 Proposed Configuration** Roundabout

Movem	Novement Performance - Vehicles													
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	f Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h			
South: A	ccess (S)												
1	L	192	0.0	0.522	8.2	LOS A	6.0	42.0	0.54	0.57	47.2			
3	R	447	0.0	0.521	12.3	LOS B	6.0	42.0	0.54	0.67	44.8			
Approac	h	639	0.0	0.522	11.0	LOS B	6.0	42.0	0.54	0.64	45.4			
East: N1	4 (E)													
4	L	479	0.0	0.525	8.4	LOS A	6.0	42.2	0.60	0.62	47.3			
5	Т	134	0.0	0.524	7.6	LOS A	6.0	42.2	0.60	0.58	47.4			
Approac	h	613	0.0	0.525	8.2	LOS A	6.0	42.2	0.60	0.61	47.3			
West: N1	14 (W)													
11	Т	128	0.0	0.354	9.8	LOS A	3.1	21.4	0.73	0.75	46.3			
12	R	160	0.0	0.354	14.7	LOS B	3.1	21.4	0.73	0.84	43.6			
Approac	h	288	0.0	0.354	12.5	LOS B	3.1	21.4	0.73	0.80	44.7			
All Vehic	les	1540	0.0	0.525	10.2	LOS B	6.0	42.2	0.60	0.66	46.0			

Level of Service (Aver. Int. Delay): LOS B. Based on average delay for all vehicle movements. LOS Method: Delay (HCM).

Level of Service (Worst Movement): LOS B. LOS Method for individual vehicle movements: Delay (HCM).

Approach LOS values are based on the worst delay for any vehicle movement.

Roundabout LOS Method: Same as Signalised Intersections.

Roundabout Capacity Model: SIDRA Standard.

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N14 / Proposed East Access Roundabout 2019 Friday Peak Hour Background and Development Traffic **Option 2 Proposed Configuration** Roundabout

Movem	lovement Performance - Vehicles													
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	f Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h			
South: A	ccess ((S)												
1	L	114	0.0	0.549	14.8	LOS B	6.1	43.0	0.90	0.98	42.1			
3	R	264	0.0	0.549	18.9	LOS B	6.1	43.0	0.90	1.00	40.1			
Approac	h	378	0.0	0.549	17.7	LOS B	6.1	43.0	0.90	1.00	40.6			
East: N1	4 (E)													
4	L	283	0.0	0.665	8.0	LOS A	10.2	71.3	0.60	0.56	47.6			
5	Т	599	0.0	0.665	7.2	LOS A	10.2	71.3	0.60	0.52	47.6			
Approac	h	882	0.0	0.665	7.5	LOS A	10.2	71.3	0.60	0.53	47.6			
West: N1	14 (W)													
11	Т	555	0.0	0.647	9.8	LOS A	8.7	61.2	0.83	0.75	46.3			
12	R	95	0.0	0.649	14.7	LOS B	8.7	61.2	0.83	0.82	44.1			
Approac	h	649	0.0	0.647	10.5	LOS B	8.7	61.2	0.83	0.76	45.9			
All Vehic	les	1909	0.0	0.665	10.5	LOS B	10.2	71.3	0.74	0.70	45.5			

Level of Service (Aver. Int. Delay): LOS B. Based on average delay for all vehicle movements. LOS Method: Delay (HCM).

Level of Service (Worst Movement): LOS B. LOS Method for individual vehicle movements: Delay (HCM).

Approach LOS values are based on the worst delay for any vehicle movement.

Roundabout LOS Method: Same as Signalised Intersections.

Roundabout Capacity Model: SIDRA Standard.

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N14 / Proposed East Access Roundabout 2019 Saturday Peak Hour Background and Development Traffic **Option 2 Proposed Configuration** Roundabout

Movem	ent Pe	erformance - `	Vehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	f Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: A	ccess ((S)									
1	L	192	0.0	0.532	8.3	LOS A	6.1	42.9	0.57	0.59	47.0
3	R	447	0.0	0.532	12.4	LOS B	6.1	42.9	0.57	0.68	44.7
Approacl	h	639	0.0	0.532	11.2	LOS B	6.1	42.9	0.57	0.65	45.3
East: N1	4 (E)										
4	L	479	0.0	0.535	8.5	LOS A	6.2	43.6	0.61	0.62	47.3
5	Т	146	0.0	0.534	7.6	LOS A	6.2	43.6	0.61	0.58	47.3
Approacl	h	625	0.0	0.535	8.3	LOS A	6.2	43.6	0.61	0.61	47.3
West: N1	14 (W)										
11	Т	140	0.0	0.369	9.8	LOS A	3.2	22.7	0.74	0.75	46.3
12	R	160	0.0	0.370	14.7	LOS B	3.2	22.7	0.74	0.84	43.6
Approacl	h	300	0.0	0.370	12.4	LOS B	3.2	22.7	0.74	0.80	44.8
All Vehic	les	1564	0.0	0.535	10.2	LOS B	6.2	43.6	0.62	0.66	46.0

Level of Service (Aver. Int. Delay): LOS B. Based on average delay for all vehicle movements. LOS Method: Delay (HCM).

Level of Service (Worst Movement): LOS B. LOS Method for individual vehicle movements: Delay (HCM).

Approach LOS values are based on the worst delay for any vehicle movement.

Roundabout LOS Method: Same as Signalised Intersections.

Roundabout Capacity Model: SIDRA Standard.

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N14 / Proposed East Access Roundabout 2024 Friday Peak Hour Background and Development Traffic **Option 2 Proposed Configuration** Roundabout

Movem	Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	f Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h		
South: A	ccess ((S)											
1	L	114	0.0	0.566	15.6	LOS B	6.5	45.6	0.92	1.01	41.5		
3	R	264	0.0	0.567	19.7	LOS B	6.5	45.6	0.92	1.03	39.5		
Approacl	h	378	0.0	0.567	18.5	LOS B	6.5	45.6	0.92	1.02	40.1		
East: N1	4 (E)												
4	L	283	0.0	0.681	8.1	LOS A	10.8	75.5	0.62	0.56	47.5		
5	Т	622	0.0	0.681	7.2	LOS A	10.8	75.5	0.62	0.52	47.5		
Approacl	h	905	0.0	0.681	7.5	LOS A	10.8	75.5	0.62	0.53	47.5		
West: N1	14 (W)												
11	Т	579	0.0	0.670	10.2	LOS B	9.6	67.0	0.85	0.77	46.1		
12	R	95	0.0	0.672	15.1	LOS B	9.6	67.0	0.85	0.83	43.8		
Approacl	h	674	0.0	0.670	10.9	LOS B	9.6	67.0	0.85	0.78	45.8		
All Vehic	les	1957	0.0	0.681	10.8	LOS B	10.8	75.5	0.76	0.71	45.3		

Level of Service (Aver. Int. Delay): LOS B. Based on average delay for all vehicle movements. LOS Method: Delay (HCM).

Level of Service (Worst Movement): LOS B. LOS Method for individual vehicle movements: Delay (HCM).

Approach LOS values are based on the worst delay for any vehicle movement.

Roundabout LOS Method: Same as Signalised Intersections.

Roundabout Capacity Model: SIDRA Standard.

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N14 / Proposed East Access Roundabout 2024 Saturday Peak Hour Background and Development Traffic **Option 2 Proposed Configuration** Roundabout

Movem	Movement Performance - Vehicles													
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	f Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h			
South: A	ccess ((S)												
1	L	192	0.0	0.543	8.5	LOS A	6.3	43.9	0.60	0.60	46.8			
3	R	447	0.0	0.543	12.6	LOS B	6.3	43.9	0.60	0.68	44.6			
Approac	h	639	0.0	0.543	11.3	LOS B	6.3	43.9	0.60	0.66	45.2			
East: N1	4 (E)													
4	L	479	0.0	0.546	8.5	LOS A	6.5	45.2	0.62	0.62	47.3			
5	Т	160	0.0	0.546	7.6	LOS A	6.5	45.2	0.62	0.58	47.3			
Approac	h	639	0.0	0.546	8.3	LOS A	6.5	45.2	0.62	0.61	47.3			
West: N2	14 (W)													
11	Т	153	0.0	0.386	9.8	LOS A	3.4	24.0	0.75	0.76	46.2			
12	R	160	0.0	0.386	14.8	LOS B	3.4	24.0	0.75	0.85	43.6			
Approac	h	313	0.0	0.387	12.4	LOS B	3.4	24.0	0.75	0.80	44.8			
All Vehic	les	1591	0.0	0.546	10.3	LOS B	6.5	45.2	0.64	0.67	45.9			

Level of Service (Aver. Int. Delay): LOS B. Based on average delay for all vehicle movements. LOS Method: Delay (HCM).

Level of Service (Worst Movement): LOS B. LOS Method for individual vehicle movements: Delay (HCM).

Approach LOS values are based on the worst delay for any vehicle movement.

Roundabout LOS Method: Same as Signalised Intersections.

Roundabout Capacity Model: SIDRA Standard.

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N14 / Proposed West Access Roundabout 2014 Friday Peak Hour Background and Development Traffic Option 2 Proposed Configuration Roundabout

Movem	ient Pe	erformance -	Vehicles								
Maria		Demand	1.11.7	Deg.	Average	Level of	95% Back o	of Queue	Prop.	Effective	Average
MOV ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
South: W	Nootorn	veh/h	%	V/C	sec		veh	m		per veh	km/h
South. V	vestern	Access (5)		0.000	0.7	100.4	4 7	44.0	0.54	0.07	47.7
1	L _	207	0.0	0.239	8.7	LOSA	1.7	11.9	0.54	0.67	47.7
2	I	1	0.0	0.351	7.2	LOS A	2.8	19.7	0.57	0.60	47.2
3	R	359	0.0	0.350	12.9	LOS B	2.8	19.7	0.57	0.74	44.3
Approac	ch	567	0.0	0.350	11.4	LOS B	2.8	19.7	0.56	0.72	45.4
East: N1	14 (E)										
4	L	377	0.0	0.643	8.1	LOS A	5.8	40.6	0.58	0.64	47.8
5	Т	306	0.0	0.642	6.8	LOS A	5.8	40.6	0.58	0.57	47.9
6	R	1	0.0	0.002	12.8	LOS B	0.0	0.1	0.41	0.61	44.9
Approac	ch	684	0.0	0.643	7.5	LOS B	5.8	40.6	0.57	0.61	47.9
North: F	arm Ac	cess									
7	L	1	0.0	0.003	9.7	LOS A	0.0	0.1	0.63	0.60	47.6
8	Т	1	0.0	0.003	8.5	LOS A	0.0	0.1	0.63	0.54	47.6
9	R	1	0.0	0.002	15.3	LOS B	0.0	0.1	0.64	0.65	42.8
Approac	ch	3	0.0	0.003	11.1	LOS B	0.0	0.1	0.63	0.60	45.9
West: N	14 (W)										
10	Ĺ	1	0.0	0.263	8.7	LOS A	2.0	14.1	0.57	0.71	48.2
11	Т	260	0.0	0.265	7.4	LOS A	2.0	14.1	0.57	0.64	48.3
12	R	188	0.0	0.213	13.3	LOS B	1.5	10.6	0.56	0.75	44.3
Approac	ch	449	0.0	0.265	9.9	LOS B	2.0	14.1	0.57	0.69	46.5
All Vehic	cles	1704	0.0	0.643	9.4	LOS A	5.8	40.6	0.57	0.66	46.7

Level of Service (Aver. Int. Delay): LOS A. Based on average delay for all vehicle movements. LOS Method: Delay (HCM). Level of Service (Worst Movement): LOS B. LOS Method for individual vehicle movements: Delay (HCM).

Approach LOS values are based on the worst delay for any vehicle movement.

Roundabout LOS Method: Same as Signalised Intersections.

Roundabout Capacity Model: SIDRA Standard.

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N14 / Proposed West Access Roundabout 2014 Saturday Peak Hour Background and Development Traffic Option 2 Proposed Configuration Roundabout

Movem	Movement Performance - Vehicles													
	Turn	Demand	н\/	Deg.	Average	Level of	95% Back o	of Queue	Prop.	Effective	Average			
	Turri	FIOW	0/2	Sath	Delay	Service	venicies	Distance	Queuea	Stop Rate	Speed km/h			
South: V	Vestern	Access (S)	/0	V/C	300		VCII				N11//11			
1	L	352	0.0	0.318	7.4	LOS A	2.8	19.6	0.39	0.56	48.5			
2	т	1	0.0	0.526	6.0	LOS A	4.9	34.3	0.43	0.46	48.4			
3	R	607	0.0	0.458	11.7	LOS B	4.9	34.3	0.43	0.64	44.8			
Approac	h	960	0.0	0.458	10.1	LOS B	4.9	34.3	0.41	0.61	46.1			
East: N1	4 (E)													
4	L	639	0.0	0.745	11.9	LOS B	10.7	74.9	0.84	0.86	45.3			
5	Т	102	0.0	0.745	10.6	LOS B	10.7	74.9	0.84	0.84	45.6			
6	R	1	0.0	0.002	13.9	LOS B	0.0	0.1	0.53	0.62	44.0			
Approac	h	742	0.0	0.744	11.7	LOS B	10.7	74.9	0.84	0.85	45.4			
North: F	arm Ac	cess												
7	L	1	0.0	0.003	12.0	LOS B	0.0	0.2	0.75	0.62	45.5			
8	Т	1	0.0	0.003	10.7	LOS B	0.0	0.2	0.75	0.58	45.9			
9	R	1	0.0	0.002	18.0	LOS B	0.0	0.1	0.74	0.66	40.7			
Approac	h	3	0.0	0.003	13.6	LOS B	0.0	0.2	0.75	0.62	43.9			
West: N	14 (W)													
10	L	1	0.0	0.211	12.5	LOS B	1.2	8.7	0.70	0.85	45.3			
11	Т	96	0.0	0.193	11.3	LOS B	1.2	8.7	0.70	0.80	45.8			
12	R	320	0.0	0.397	15.2	LOS B	3.3	22.9	0.76	0.86	42.9			
Approac	h	417	0.0	0.396	14.3	LOS B	3.3	22.9	0.75	0.85	43.5			
All Vehic	les	2122	0.0	0.744	11.5	LOS B	10.7	74.9	0.63	0.74	45.3			

Level of Service (Aver. Int. Delay): LOS B. Based on average delay for all vehicle movements. LOS Method: Delay (HCM). Level of Service (Worst Movement): LOS B. LOS Method for individual vehicle movements: Delay (HCM).

Approach LOS values are based on the worst delay for any vehicle movement.

Roundabout LOS Method: Same as Signalised Intersections.

Roundabout Capacity Model: SIDRA Standard.

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N14 / Proposed West Access Roundabout 2019 Friday Peak Hour Background and Development Traffic Option 2 Proposed Configuration Roundabout

Movem	Movement Performance - Vehicles													
Mov ID	Turn	Demand Flow	HV	Deg. Satn	Average Delay	Level of Service	95% Back o Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed			
0 (1)		veh/h	%	V/C	Sec		veh	m		per veh	km/h			
South: V	Vestern	Access (S)												
1	L	207	0.0	0.243	8.9	LOS A	1.7	12.1	0.56	0.68	47.6			
2	Т	1	0.0	0.351	7.3	LOS A	2.9	20.1	0.59	0.62	47.0			
3	R	359	0.0	0.356	13.0	LOS B	2.9	20.1	0.59	0.75	44.2			
Approac	h	567	0.0	0.356	11.5	LOS B	2.9	20.1	0.58	0.73	45.4			
East: N1	I4 (E)													
4	L	377	0.0	0.659	8.1	LOS A	6.1	42.5	0.59	0.64	47.8			
5	Т	325	0.0	0.660	6.8	LOS A	6.1	42.5	0.59	0.58	47.9			
6	R	1	0.0	0.002	12.8	LOS B	0.0	0.1	0.41	0.61	44.9			
Approac	h	703	0.0	0.659	7.5	LOS B	6.1	42.5	0.59	0.61	47.8			
North: F	arm Ac	cess												
7	L	1	0.0	0.003	9.8	LOS A	0.0	0.1	0.64	0.60	47.6			
8	Т	1	0.0	0.003	8.6	LOS A	0.0	0.1	0.64	0.55	47.6			
9	R	1	0.0	0.002	15.4	LOS B	0.0	0.1	0.65	0.65	42.7			
Approac	h	3	0.0	0.003	11.3	LOS B	0.0	0.1	0.64	0.60	45.8			
West: N	14 (W)													
10	L	1	0.0	0.263	8.7	LOS A	2.2	15.5	0.59	0.71	48.2			
11	Т	281	0.0	0.287	7.4	LOS A	2.2	15.5	0.59	0.65	48.3			
12	R	188	0.0	0.218	13.4	LOS B	1.6	10.9	0.57	0.75	44.3			
Approac	h	471	0.0	0.287	9.8	LOS B	2.2	15.5	0.58	0.69	46.6			
All Vehic	cles	1744	0.0	0.659	9.4	LOS A	6.1	42.5	0.58	0.67	46.6			

Level of Service (Aver. Int. Delay): LOS A. Based on average delay for all vehicle movements. LOS Method: Delay (HCM). Level of Service (Worst Movement): LOS B. LOS Method for individual vehicle movements: Delay (HCM).

Approach LOS values are based on the worst delay for any vehicle movement.

Roundabout LOS Method: Same as Signalised Intersections.

Roundabout Capacity Model: SIDRA Standard.

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N14 / Proposed West Access Roundabout 2019 Saturday Peak Hour Background and Development Traffic Option 2 Proposed Configuration Roundabout

Movem	Movement Performance - Vehicles												
Mov ID	Turn	Demand Flow	HV	Deg. Sato	Average Delay	Level of Service	95% Back o	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed		
		veh/h	%	V/C	sec		veh	m	Queucu	per veh	km/h		
South: V	Vestern	Access (S)											
1	L	352	0.0	0.323	7.5	LOS A	2.8	19.9	0.41	0.57	48.4		
2	Т	1	0.0	0.526	6.1	LOS A	5.0	34.9	0.46	0.47	48.1		
3	R	607	0.0	0.466	11.8	LOS B	5.0	34.9	0.46	0.64	44.7		
Approac	h	960	0.0	0.466	10.2	LOS B	5.0	34.9	0.44	0.61	46.0		
East: N1	4 (E)												
4	L	639	0.0	0.755	12.1	LOS B	11.2	78.2	0.86	0.87	45.1		
5	Т	114	0.0	0.753	10.9	LOS B	11.2	78.2	0.86	0.85	45.4		
6	R	1	0.0	0.002	13.9	LOS B	0.0	0.1	0.53	0.62	44.0		
Approac	h	754	0.0	0.755	11.9	LOS B	11.2	78.2	0.85	0.86	45.1		
North: F	arm Ac	cess											
7	L	1	0.0	0.003	12.0	LOS B	0.0	0.2	0.75	0.62	45.5		
8	т	1	0.0	0.003	10.8	LOS B	0.0	0.2	0.75	0.59	45.9		
9	R	1	0.0	0.002	18.1	LOS B	0.0	0.1	0.75	0.66	40.7		
Approac	h	3	0.0	0.003	13.6	LOS B	0.0	0.2	0.75	0.62	43.8		
West: N	14 (W)												
10	L	1	0.0	0.211	12.2	LOS B	1.4	9.5	0.71	0.85	45.6		
11	т	107	0.0	0.205	10.9	LOS B	1.4	9.5	0.71	0.80	46.1		
12	R	320	0.0	0.399	15.2	LOS B	3.3	23.1	0.77	0.86	42.9		
Approac	h	428	0.0	0.398	14.1	LOS B	3.3	23.1	0.75	0.85	43.6		
All Vehic	les	2145	0.0	0.755	11.6	LOS B	11.2	78.2	0.65	0.75	45.2		

Level of Service (Aver. Int. Delay): LOS B. Based on average delay for all vehicle movements. LOS Method: Delay (HCM). Level of Service (Worst Movement): LOS B. LOS Method for individual vehicle movements: Delay (HCM).

Approach LOS values are based on the worst delay for any vehicle movement.

Roundabout LOS Method: Same as Signalised Intersections.

Roundabout Capacity Model: SIDRA Standard.

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N14 / Proposed West Access Roundabout 2024 Friday Peak Hour Background and Development Traffic Option 2 Proposed Configuration Roundabout

Movem	ient Pe	erformance -	Vehicles								
		Demand	111/	Deg.	Average	Level of	95% Back o	of Queue	Prop.	Effective	Average
MOV ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
South	Nootoro	veh/h	%	V/C	Sec		veh	m		per veh	km/h
South. V	vestern	ALLESS (3)	0.0	0.040	0.0		4.0	40.4	0.50	0.70	47 5
1	L	207	0.0	0.249	9.0	LOSA	1.8	12.4	0.58	0.70	47.5
2	I	1	0.0	0.351	7.5	LOSA	2.9	20.6	0.61	0.64	46.9
3	R	359	0.0	0.363	13.2	LOS B	2.9	20.6	0.61	0.76	44.1
Approac	ch	567	0.0	0.363	11.7	LOS B	2.9	20.6	0.60	0.74	45.3
East: N	14 (E)										
4	L	377	0.0	0.679	8.2	LOS A	6.4	44.9	0.60	0.64	47.7
5	Т	348	0.0	0.679	6.9	LOS A	6.4	44.9	0.60	0.58	47.8
6	R	1	0.0	0.002	12.8	LOS B	0.0	0.1	0.41	0.61	44.9
Approad	h	726	0.0	0.679	7.6	LOS B	6.4	44.9	0.60	0.61	47.7
North: F	arm Ac	cess									
7	L	1	0.0	0.003	10.0	LOS A	0.0	0.1	0.65	0.60	47.4
8	Т	1	0.0	0.003	8.7	LOS A	0.0	0.1	0.65	0.55	47.5
9	R	1	0.0	0.002	15.6	LOS B	0.0	0.1	0.66	0.65	42.6
Approad	ch	3	0.0	0.003	11.4	LOS B	0.0	0.1	0.65	0.60	45.7
West: N	14 (W)										
10	L	1	0.0	0.351	8.8	LOS A	2.5	17.3	0.60	0.72	48.1
11	т	306	0.0	0.313	7.5	LOS A	2.5	17.3	0.60	0.66	48.2
12	R	188	0.0	0.224	13.5	LOS B	1.6	11.2	0.58	0.76	44.3
Approad	:h	496	0.0	0.313	9.8	LOS B	2.5	17.3	0.59	0.69	46.6
All Vehic	cles	1793	0.0	0.679	9.5	LOS A	6.4	44.9	0.60	0.67	46.6

Level of Service (Aver. Int. Delay): LOS A. Based on average delay for all vehicle movements. LOS Method: Delay (HCM). Level of Service (Worst Movement): LOS B. LOS Method for individual vehicle movements: Delay (HCM).

Approach LOS values are based on the worst delay for any vehicle movement.

Roundabout LOS Method: Same as Signalised Intersections.

Roundabout Capacity Model: SIDRA Standard.

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N14 / Proposed West Access Roundabout 2024 Saturday Peak Hour Background and Development Traffic Option 2 Proposed Configuration Roundabout

Movem	Movement Performance - Vehicles Demand Deman													
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h			
South: A	ccess	(S)												
1	L	352	0.0	0.331	7.6	LOS A	2.9	20.4	0.44	0.58	48.2			
2	т	1	0.0	0.526	6.2	LOS A	5.1	35.7	0.49	0.49	47.9			
3	R	607	0.0	0.477	11.9	LOS B	5.1	35.7	0.49	0.65	44.6			
Approac	h	960	0.0	0.477	10.3	LOS B	5.1	35.7	0.47	0.62	45.8			
East: N1	l4 (E)													
4	L	639	0.0	0.769	12.5	LOS B	11.8	82.6	0.87	0.88	44.8			
5	Т	128	0.0	0.769	11.2	LOS B	11.8	82.6	0.87	0.86	45.1			
6	R	1	0.0	0.002	13.9	LOS B	0.0	0.1	0.53	0.62	44.0			
Approac	h	768	0.0	0.769	12.2	LOS B	11.8	82.6	0.87	0.88	44.9			
North: F	arm Ac	cess												
7	L	1	0.0	0.003	12.1	LOS B	0.0	0.2	0.75	0.62	45.4			
8	Т	1	0.0	0.003	10.8	LOS B	0.0	0.2	0.75	0.59	45.9			
9	R	1	0.0	0.002	18.1	LOS B	0.0	0.1	0.75	0.67	40.7			
Approad	h	3	0.0	0.003	13.6	LOS B	0.0	0.2	0.75	0.63	43.8			
West: N	14 (W)													
10	L	1	0.0	0.211	11.9	LOS B	1.5	10.5	0.71	0.84	45.9			
11	Т	121	0.0	0.219	10.7	LOS B	1.5	10.5	0.71	0.80	46.4			
12	R	320	0.0	0.401	15.2	LOS B	3.3	23.4	0.77	0.86	42.9			
Approac	h	442	0.0	0.401	13.9	LOS B	3.3	23.4	0.76	0.85	43.8			
All Vehic	cles	2174	0.0	0.769	11.7	LOS B	11.8	82.6	0.67	0.76	45.0			

Level of Service (Aver. Int. Delay): LOS B. Based on average delay for all vehicle movements. LOS Method: Delay (HCM). Level of Service (Worst Movement): LOS B. LOS Method for individual vehicle movements: Delay (HCM).

Approach LOS values are based on the worst delay for any vehicle movement.

Roundabout LOS Method: Same as Signalised Intersections.

Roundabout Capacity Model: SIDRA Standard.

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ANNEXURE B DETAILED LAYOUT PLAN

