Upgrade of uShukela Drive and R102 Intersection

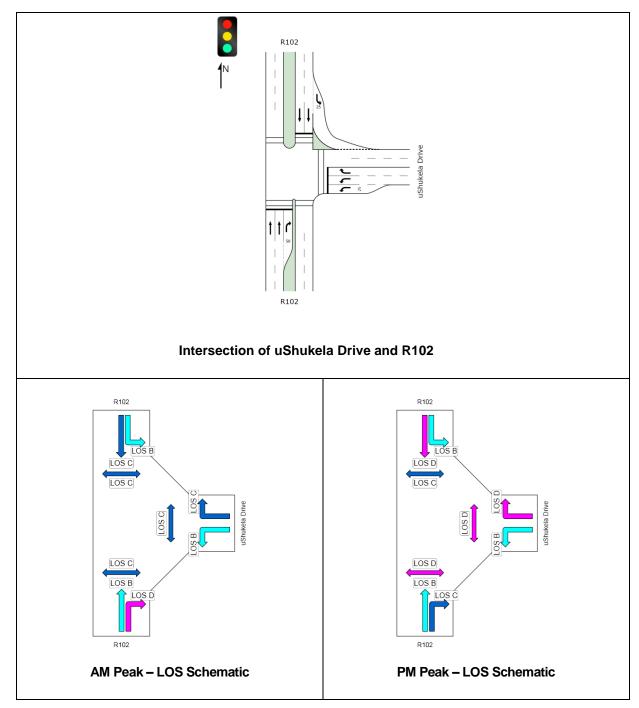


Table 19: R102 and uShukela Drive Intersection, Five Year Forecast plus Generated

An exclusive right turning lane is required on the southern approach and a left slip lane is required on the northern approach as shown in Table 19. The proposed upgrades will improve the overall efficiency of this intersection as shown in SIDRA schematics above. The average delays will be 22.5 and 29.5 seconds for the AM and PM peak hours respectively. The average queue lengths will be 137.3 and 181.2 metres for the AM and PM peak hours respectively

7.7 R102 and Brake Drive Intersection

The SIDRA analysis of this intersection shows that complete failure (LOS F) will pervade on the Brake Drive approach during both the peak hours as shown in **Table 20**. This will be as a result of the high through volumes on the R102. The average delays will be 402.6 and 221.7 seconds for the AM and PM peak hours respectively. The average queue lengths will be 2073.5 and 1380.5 metres for the AM and PM peak hours respectively, As such, this intersection will require upgrading in the 5 year horizon.

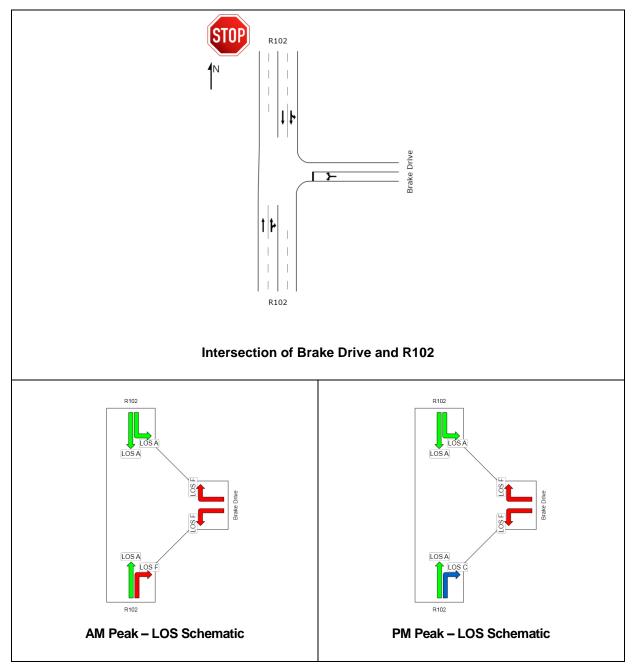


Table 20: R102 / Brake Drive Intersection, Five Year Forecast plus Generated

Upgrade of R102 and Brake Drive Intersection

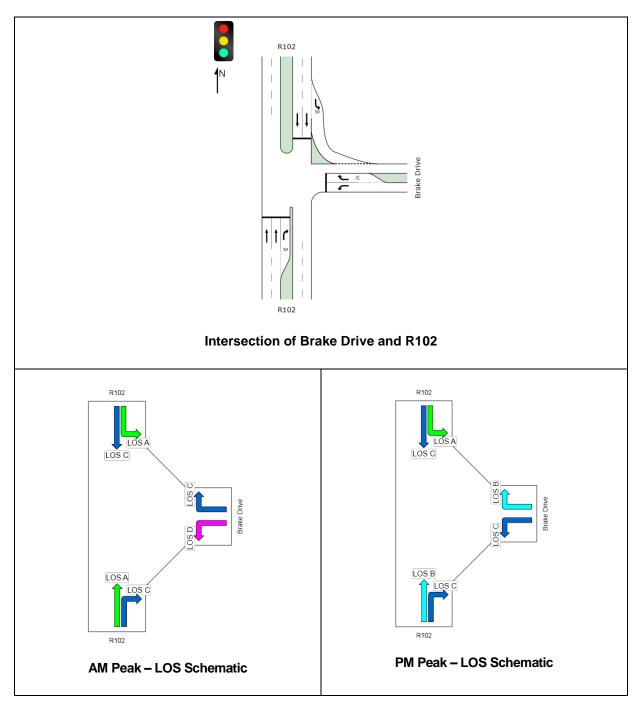


Table 21: Upgrade R102 / Brake Drive Intersection, Five Year Forecast plus Generate

This priority controlled intersection must be upgraded to a signalised intersection in the 5 year horizon. Furthermore, an exclusive right turning lane will be required on the southern approach and a left slip lane will be required on the northern approach. The eastern approach will require designated left and right turning lanes. The average delays will be 20.4 and 19.9 seconds for the AM and PM peak hours respectively. The average queue lengths will be 127.8 and 102.3 metres for the AM and PM peak hours respectively

7.8 uShukela Drive and the uShukela Development Main Spine Road

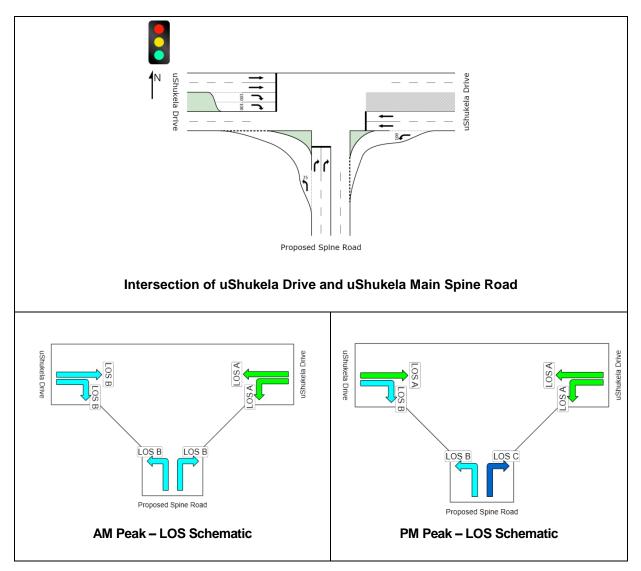


Table 22: Five Year Forecast plus Generated

The intersection configuration shown in **Table 22** was proposed by TECHSO for the main access intersection between uShukela Drive and the main spine road through the proposed development. This intersection configuration was approved by KZN DOT. Hence this configuration was analysed in this section of the report. The SIDRA analysis revealed that this intersection layout will operate efficiently in the 5 year horizon as shown in **Table 22**. The average delays will be 11 and 10.2 seconds for the AM and PM peak hours respectively. The average queue lengths will be 75 and 50.4 metres for the AM and PM peak hours respectively. No upgrades will be required to this layout to accommodate the uShukela Development generated trips in the 5-year horizon.

7.9 uShukela Main Spine Road and the New Brake Drive Link Intersection

A new road will be required to link Brake Drive and the uShukela Main Spine Road. The intersection layout shown in **Table 23** is proposed for this junction. The analysis of this proposed layout yielded good operating conditions for the 5 year horizon during both the AM and PM peak hours as shown in **Table 23**. The average delays will be 15.7 and 15.2 seconds for the AM and PM peak hours respectively. The average queue length will be 70 metres for both the AM and PM peak hours respectively.

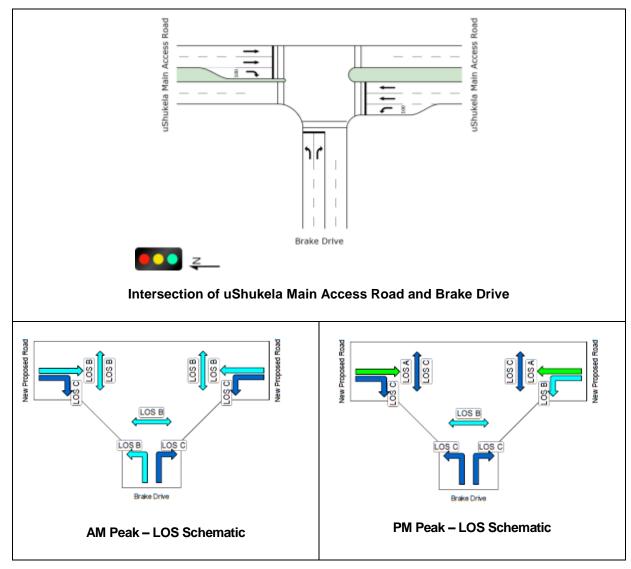


Table 23: uShukela Main Access Road and Brake Drive Intersection, Five Year Forecast plus Generated Traffic

8. Ten Year Forecast plus Development Generated Traffic Volumes

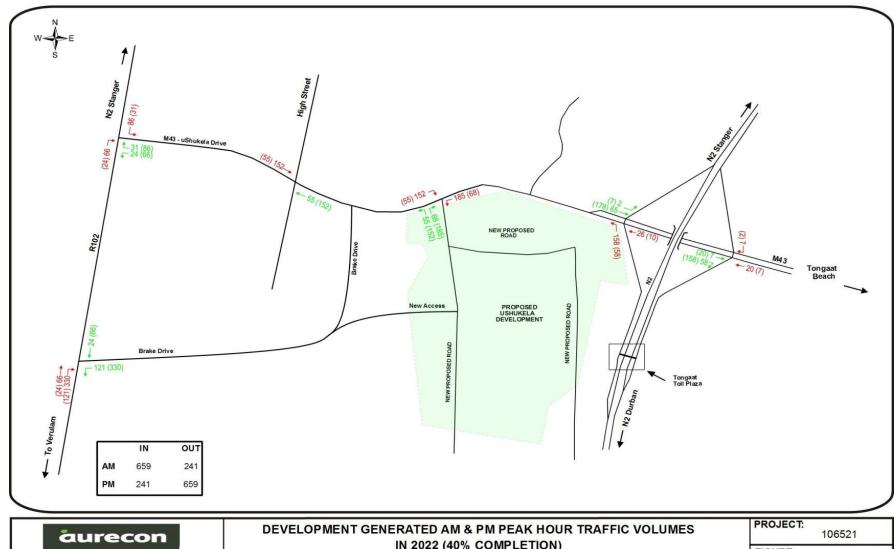
It is envisaged that the uShukela Development will attain 40% of its full developmental potential in the 10 year horizon i.e. in 2022. As such, the proposed uShukela Development will generate the peak traffic volumes shown in Table 24 below. The generated traffic volumes for the 2022 scenario at 40% completion are shown on **Figure 9**.

AM Peak Hour		PM Peak Hour			
Peak Total 2 way	ln	Out	Peak Total 2 way	ln	Out
900	659	241	900	241	900

Table 24: uShukela Development Generated Traffic Volumes in 2022

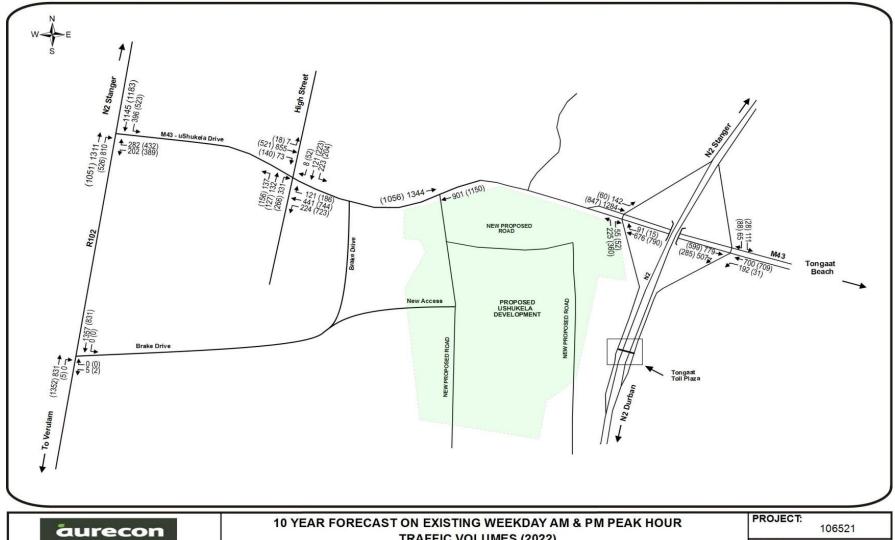
The South African Manual on Traffic Impact Studies recommends that in the case of multiphase developments that generate in excess of 2 000 trips in the peak hour, the 10 year horizon period should be analysed. Since the uShukela Development will generate in excess of 2 000 vehicle trips, an analysis of the 10 year horizon was undertaken in this chapter. A growth rate of 5% was used to forecast the 10 year traffic volumes. The 10 year forecast on the existing traffic volumes is shown on **Figure 10** (no development-generated traffic).

The combined 10 year forecasted background traffic volumes plus the uShukela Development and DTP generated traffic volumes are shown in **Figure 11**. These combined traffic volumes were analysed to determine the accumulative impact on the surrounding road network in the 10 year horizon. The analysis of these combined traffic volumes on the surrounding road network is discussed hereafter.



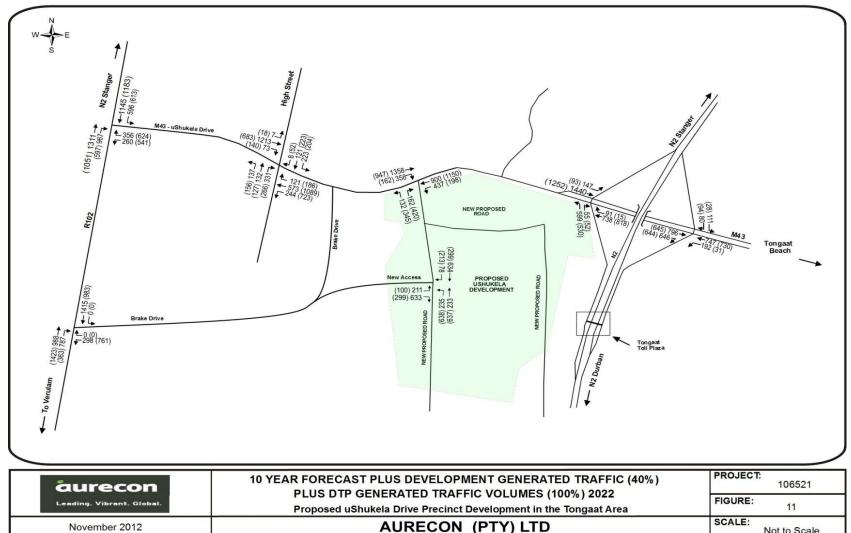
aurecon Leading, Vibrant, Global.	DEVELOPMENT GENERATED AM & PM PEAK HOUR TRAFFIC VOLUMES IN 2022 (40% COMPLETION)	PROJECT: 106521
	Proposed uShukela Drive Precinct Development in the Tongaat Area	FIGURE: 9
November 2012	AURECON (PTY) LTD	SCALE: Not to Scale

Figure 9: Development Generated Traffic Volumes at 40% completion in 2022



aurecon Leading. Vibrant. Global.	10 YEAR FORECAST ON EXISTING WEEKDAY AM & PM PEAK HOUR TRAFFIC VOLUMES (2022)	PROJECT: 106521
	Proposed uShukela Drive Precinct Development in the Tongaat Area	FIGURE: 10
November 2012	AURECON (PTY) LTD	SCALE: Not to Scale





AURECON (PTY) LTD Not to Scale

Figure 11: Ten Year Forecast plus Development Generated Traffic in 2022

8.1 N2 Interchange – Eastern Intersection

The SIDRA analysis of the combined traffic volumes revealed that all the movements at the eastern intersection (as upgraded in the 5-year horizon) will operate at a reasonable level of service during both the AM and PM peak hours. The through traffic on uShukela Drive operates fairly unconstrained by traffic on the ramps. The westbound traffic turning left onto the southbound on-ramp operates freely. The average delays will be 24.9 and 19.5 seconds for the AM and PM peak hours respectively. The average queue lengths will be 145.3 and 94.7 metres for the AM and PM peak hours respectively. The LOS schematics are shown in Table 25.

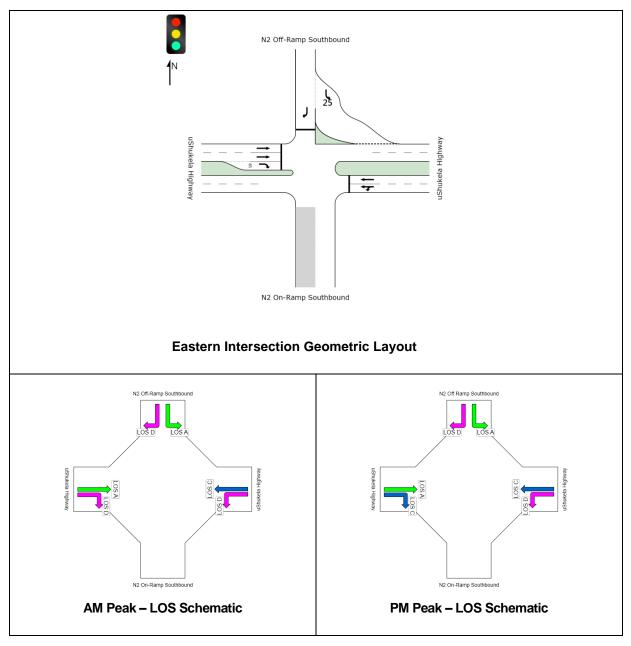


Table 25: N2 Interchange Eastern Intersection, Ten Year Horizon plus Generated Traffic

8.2 N2 Interchange - Western Intersection

The through traffic on uShukela Drive will continue to operate at a good LOS in the 10 year horizon. The traffic turning left onto the northbound on-ramp operates freely. The average delays will be 22.1 and 21.7 seconds for the AM and PM peak hours respectively. The average queue lengths will be 179.3 and 128.5 metres for the AM and PM peak hours respectively. The LOS schematics are shown in Table 26.

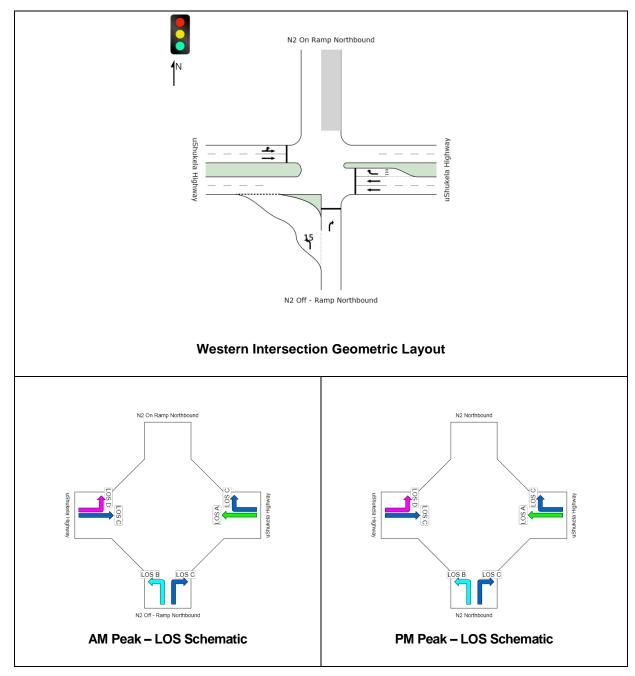


Table 26: N2 Interchange – Western Intersection, Ten Year Horizon plus Generated Traffic

8.3 uShukela Drive and High Street Intersection

Table 27 shows that this intersection will encounter severe congestion on most approaches during the AM and PM peak hours. The average delays will be 100.8 and 94.6 seconds for the AM and PM peak hours respectively. The average queue lengths will be 1278.5 and 1342.7 metres for the AM and PM peak hours respectively. Therefore, geometric improvements will be required at this intersection in the 10 year horizon.

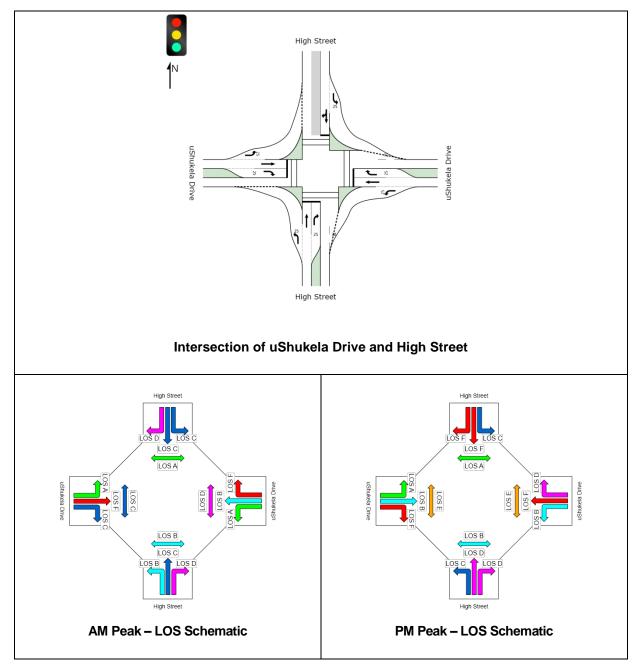


Table 27: High Street and uShukela Drive, Ten Year Horizon plus Generated Traffic

Upgrade Of uShukela Drive and High Street Intersection

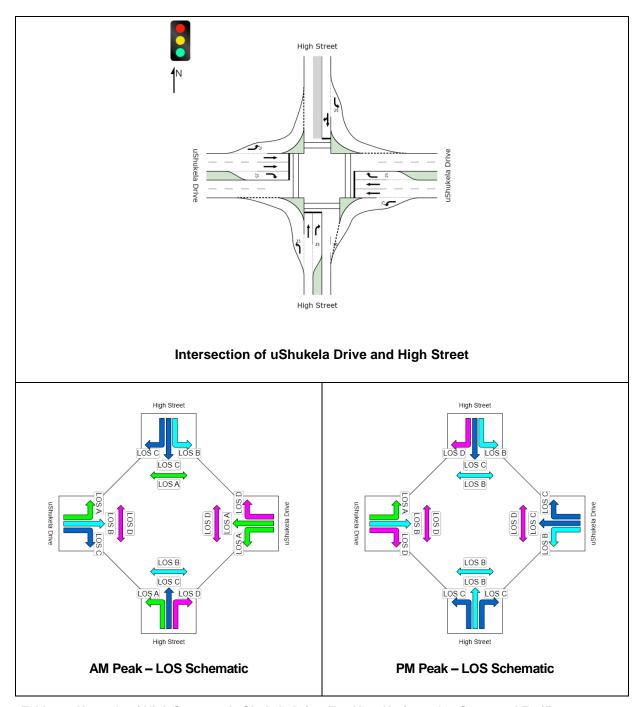


Table 28: Upgrade of High Street and uShukela Drive, Ten Year Horizon plus Generated Traffic

This intersection must be upgraded to the geometric configuration shown in **Table 28** in the 10 year horizon. The SIDRA analysis results show that this signalised intersection will operate at a good LOS during both the AM and PM peak hours despite the additional traffic volumes that will be imposed on it. The average delays will be 17.8 and 20.8 seconds for the AM and PM peak hours respectively. The average queue lengths will be 129.6 and 159.8 metres for the AM and PM peak hours respectively.

8.4 uShukela Drive and R102 Intersection

It can be seen from the LOS schematics shown in **Table 29** that all movements operate at acceptable levels of service in both peak hours. The average delays will be 23.2 and 45.3 seconds for the AM and PM peak hours respectively. The average queue lengths will be 247.8 and 356.7 metres for the AM and PM peak hours respectively. No improvements will be required at this intersection in the 10 year horizon.

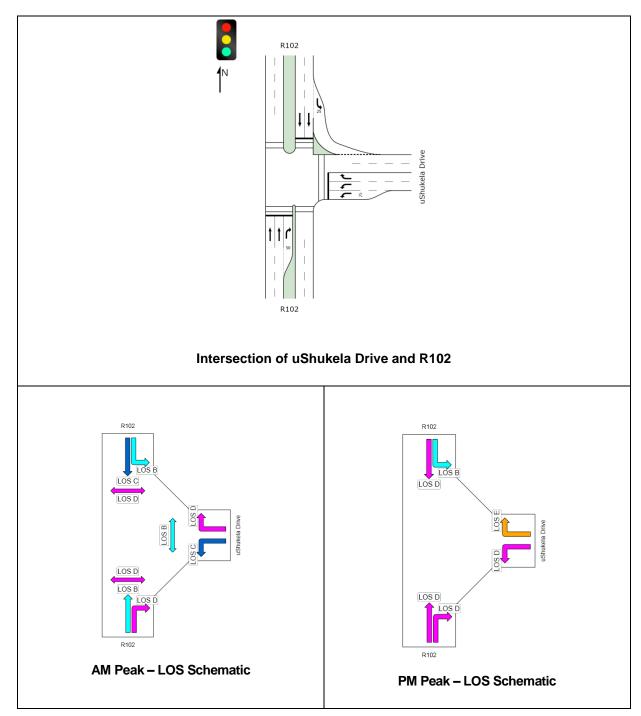


Table 29: R102 and uShukela Drive, Ten Year Horizon plus Generated Traffic

8.5 Brake Drive and R102 Intersection

The geometric improvements that will be carried out at this intersection in the 5 year horizon will provide sufficient capacity to comfortably handle the additional trips in the 10 year horizon. The levels of service at this intersection will be acceptable in both peak hours negating any need for improvements. The average delays will be 13.5 and 29.4 seconds for the AM and PM peak hours respectively. The average queue lengths will be 139.8 and 254.7 metres for the AM and PM peak hours respectively – see **Table 30**.

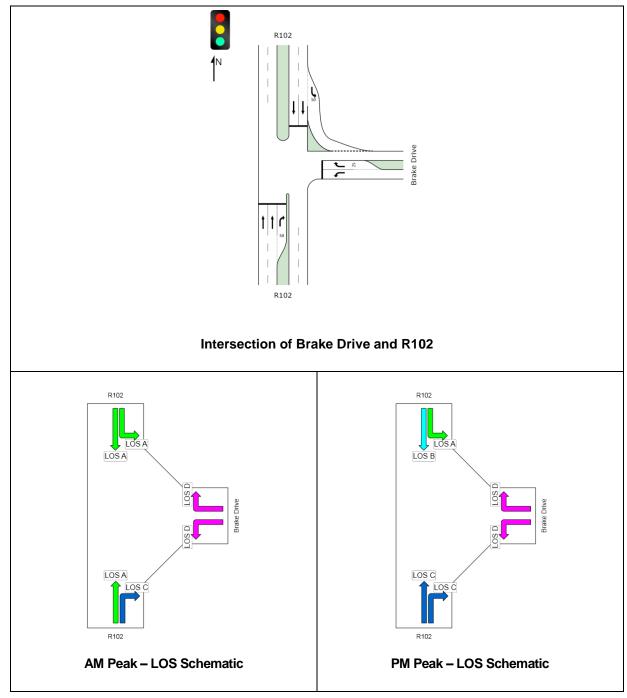


Table 30: Brake Drive and R102, Ten Year Horizon plus Generated Traffic

8.6 uShukela Drive and Main uShukela Spine Road Intersection

This access intersection will operate good levels of service in both the peak hours. The average delays will be 11 and 12.5 seconds for the AM and PM peak hours respectively. The average queue lengths will be 60.1 and 77.8 metres for the AM and PM peak hours respectively – see **Table 31**. No improvements will be required at this intersection in the 10 year horizon.

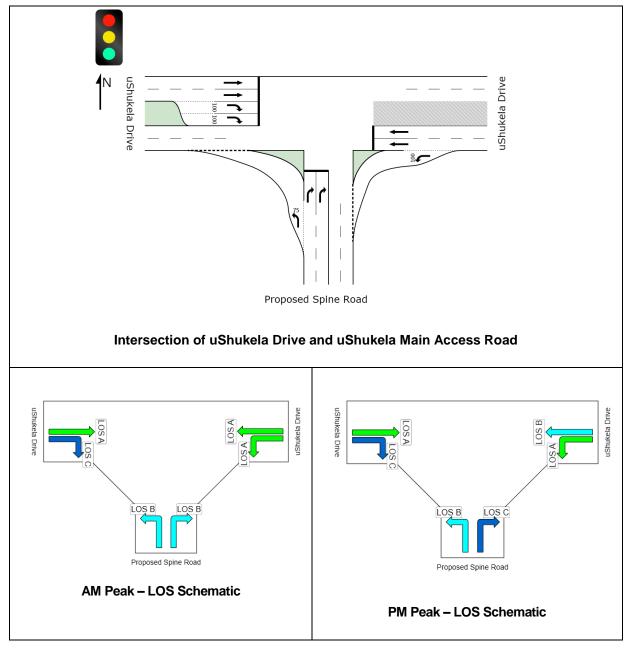


Table 31: uShukela Drive and Main uShukela Access Road Intersection, Ten Year Forecast plus Generated Traffic

8.7 Brake Drive and Main uShukela Access Intersection

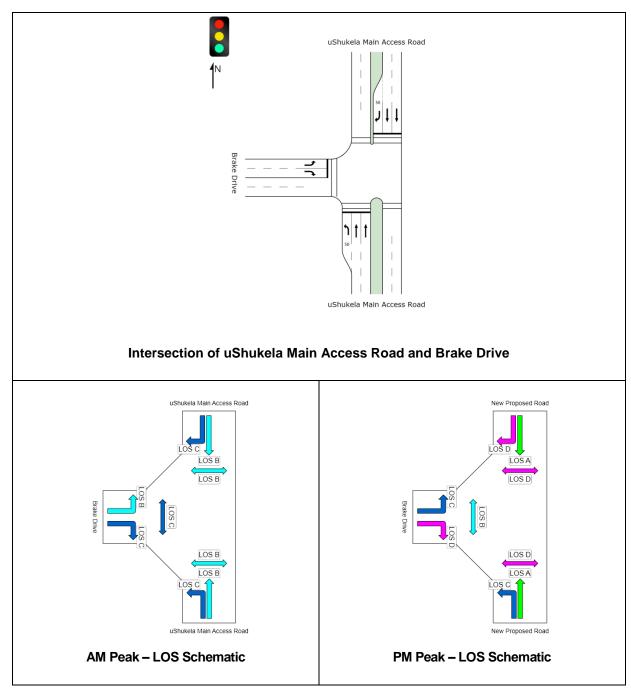


Table 32: Brake Drive and Main Access Intersection, Ten Year Forecast plus Generated Traffic

This intersection will continue to operate at a good LOS in the 10 year horizon during the both the peak hours. No improvements will be required in the 10 year horizon. The average delays will be 13.5 and 29.4 seconds for the AM and PM peak hours respectively. The average queue lengths will be 139.8 and 254.7 metres for the AM and PM peak hours respectively.

9. Fifteen Year Forecast plus Development Generated Traffic

It is envisaged that by 2027 (15 years from present) the uShukela Development will attain 100% completion. As such, the proposed uShukela Development will generate the peak hour traffic volumes shown in the Table 33 below by 2027.

AM Peak Hour		PM Peak Hour			
Peak Total 2 way	In	Out	Peak Total 2 way	In	Out
3 112	2 263	849	3 112	2 263	849

Table 33: uShukela Development Generated Traffic Volumes in 2027

Using a compound growth rate of 5%, the background traffic volumes were forecasted over a 15 year period to attain their equivalent 2027 values. These 2027 forecasted traffic volumes are shown on Figure 12.

The generated traffic volumes for the 100% completion scenario were assigned onto the road network and shown on Figure 13.

The 15 year forecasted background traffic volumes plus the uShukela Development and DTP generated traffic volumes are shown in Figure 14. These combined traffic volumes were analysed to determine the cumulative impact on the surrounding road network in the 15 year horizon. The impact of these combined traffic volumes on the surrounding road network is discussed hereafter.

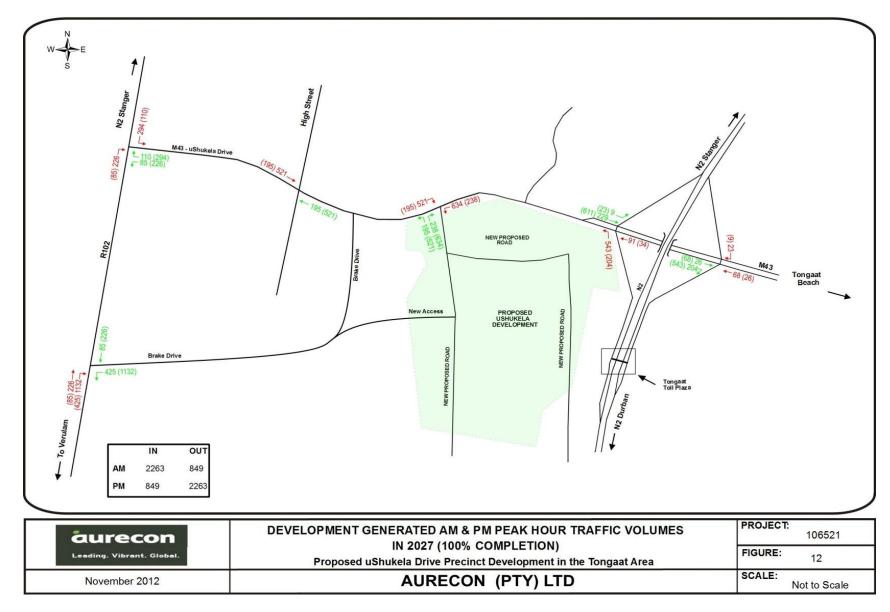


Figure 12: Development Generated Traffic Volumes in 2027

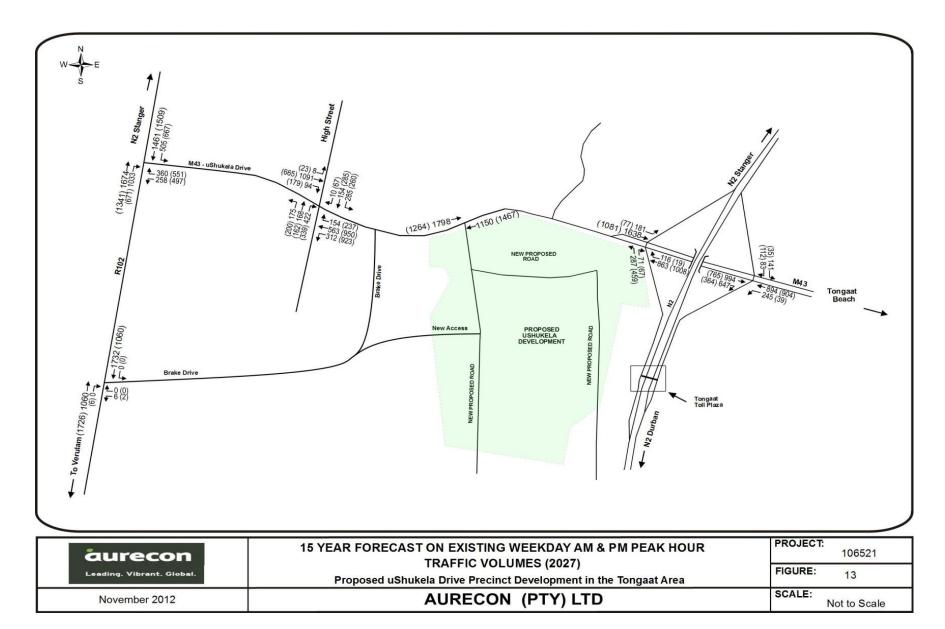
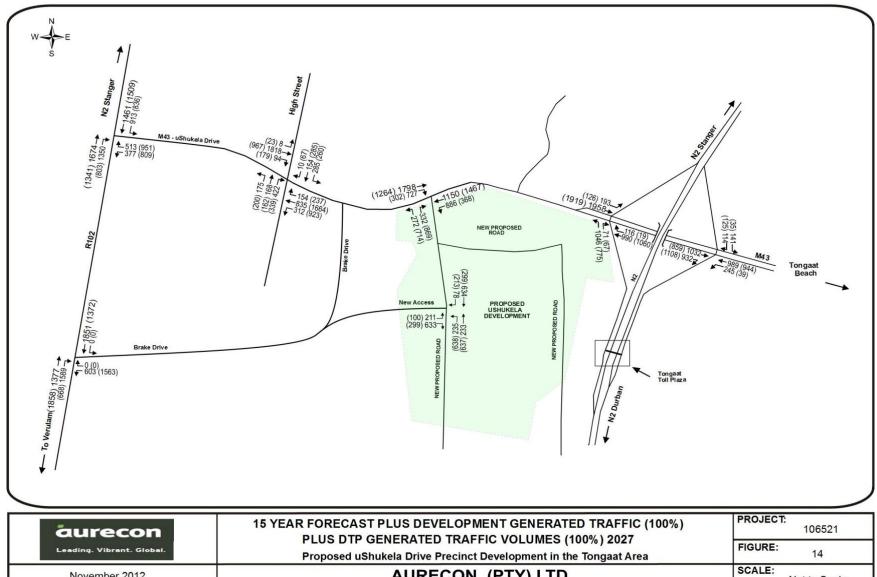


Figure 13: Fifteen Year Forecast on Existing Traffic Volumes



AURECON (PTY) LTD November 2012 Not to Scale

Figure 14: Fifteen Year Forecast plus Development Generated Traffic plus DTP Generated Traffic Volumes 2027

9.1 N2 Interchange – Eastern Intersection

The SIDRA analysis of the combined traffic volumes revealed that all the movements at the eastern intersection will operate at a reasonable level of service during both the AM and PM peak hours. The through traffic on uShukela Drive operates fairly unconstrained by traffic on the ramps. The westbound traffic turning left onto the southbound on-ramp operates freely. The average delays will be 16.2 and 17.8 seconds for the AM and PM peak hours respectively. The average queue lengths will be 178.2 and 138.6 metres for the AM and PM peak hours respectively.

No upgrades will be required in the 15 year horizon.

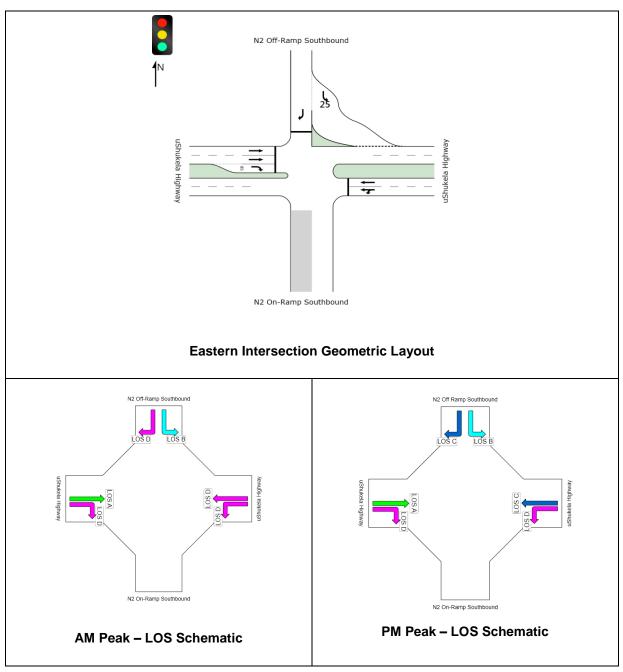


Table 34: N2 Interchange Eastern Intersection, 15 Year Horizon plus Generated Traffic

9.2 N2 Interchange – Western Intersection

The SIDRA analysis results show that this signalised off-ramp intersection will operate at a poor LOS during both the AM and PM peak hours due to the additional traffic volumes that will be imposed on it. The average delays will be 274.7 and 74.5 seconds for the AM and PM peak hours respectively. The average queue lengths will be 1727.3 and 829.6 metres for the AM and PM peak hours respectively.

Therefore, upgrades will be required in the 15 year horizon.

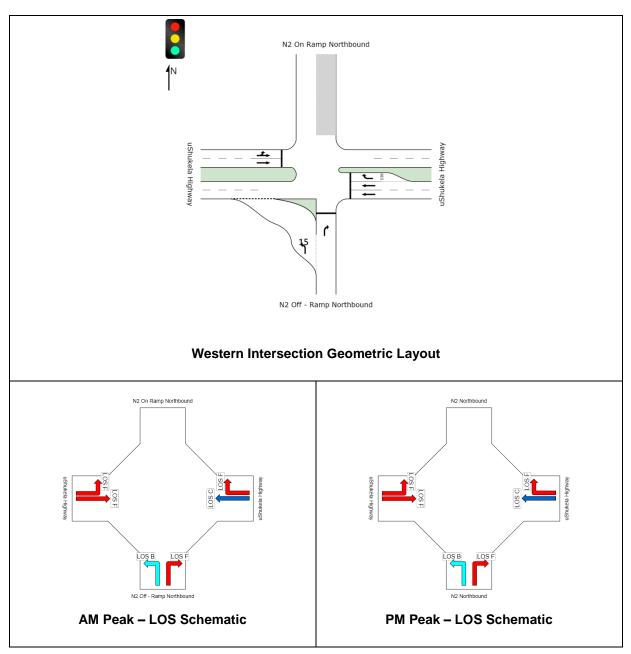


Table 35: N2 Interchange - Western Intersection, 15 Year Horizon plus Generated Traffic

Upgrade of N2 Interchange – Western Intersection

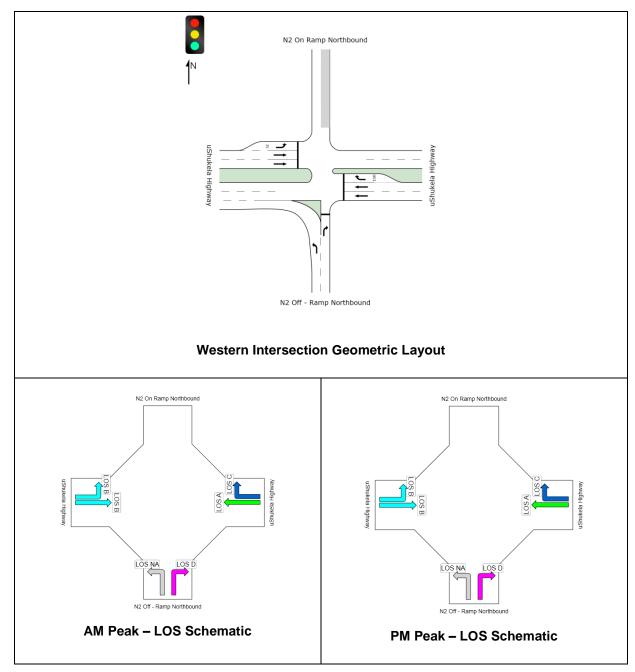


Table 36: Upgrade N2 Interchange – Western Intersection, 15 Year Horizon plus Generated Traffic

This intersection must be upgraded to the geometric configuration shown above in order to improve its overall operational efficiency. The average delays will be 13.8 and 13.2 seconds for the AM and PM peak hours respectively. The average queue lengths will be 248.3 and 265.2 metres for the AM and PM peak hours respectively.

9.3 uShukela Drive and High Street Intersection

Table 37 shows that this intersection exhibits complete failure (LOS F) at several of the movements during the AM and PM peak hours. The average delays will be 471.4 and 1175.3 seconds for the AM and PM peak hours respectively. The average queue lengths will be 3993.5 and 8483.5 metres for the AM and PM peak hours respectively.

Upgrades will certainly be required in the 15 year horizon.

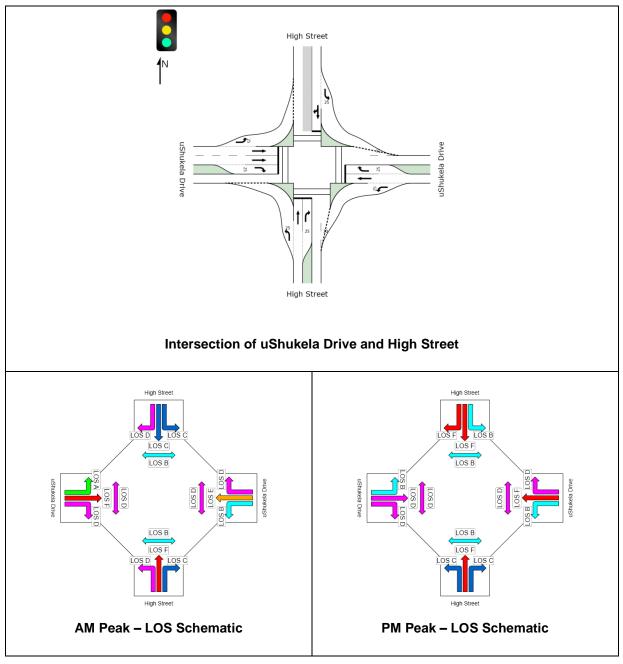


Table 37: High Street and uShukela Drive, 15 Year Horizon plus Generated Traffic

Upgrade of uShukela Drive and High Street Intersection

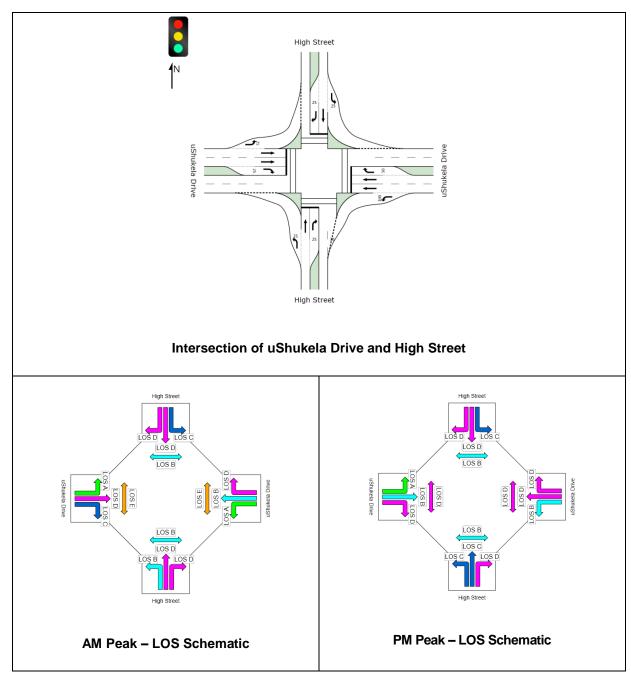


Table 38: Upgrade High Street and uShukela Drive, 15 Year Horizon plus Generated Traffic

This intersection must be upgraded to the geometric configuration shown for it to operate at a better LOS. After the upgrading of this intersection, the average delays will be 36.8 and 29.9 seconds for the AM and PM peak hours respectively. The average queue lengths will be 454 and 326.5 metres for the AM and PM peak hours respectively.

9.4 uShukela Drive and R102 Intersection

It is evident from Table 39 that this intersection will encounter immense levels of congestion in the 15 year horizon and will require desperate upgrading in 2027. However, there is no space available adjacent to this intersection to allow for any improvements due to the intense urban development alongside all three approaches to this intersection. As such, this intersection cannot be upgraded any further in 2027. The average delays will be 212.4 and 284.5 seconds for the AM and PM peak hours respectively. The average queue lengths will be 1991.6 and 1869.9 metres for the AM and PM peak hours respectively. In mitigation of the above congestion problem, it can be reasonably argued that once this intersection exhibits major signs of distress, motorists coming from the southern Tongaat area will naturally change their travel patterns and use the Brake Drive / R102 intersection to enter the uShukela Development. This redistribution of trips will alleviate the pressure placed on the uShukela Drive and R102 intersection. No further analysis will carried out at this intersection.

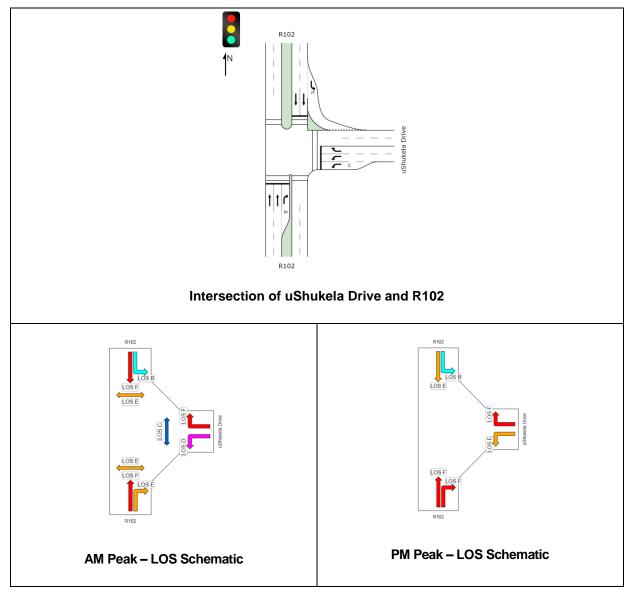


Table 39: R102 and uShukela Drive, 15 Year Horizon plus Generated Traffic

9.5 Brake Drive and R102 Intersection

This intersection operates poorly and fails at many of the movements in both the AM and the PM – see **Table 40**. The average delays will be 261.4 and 1003.2 seconds for the AM and PM peak hours respectively. The average queue lengths will be 2280.9 and 4568.3 metres for the AM and PM peak hours respectively.

Upgrades will be required in the 15 year horizon.

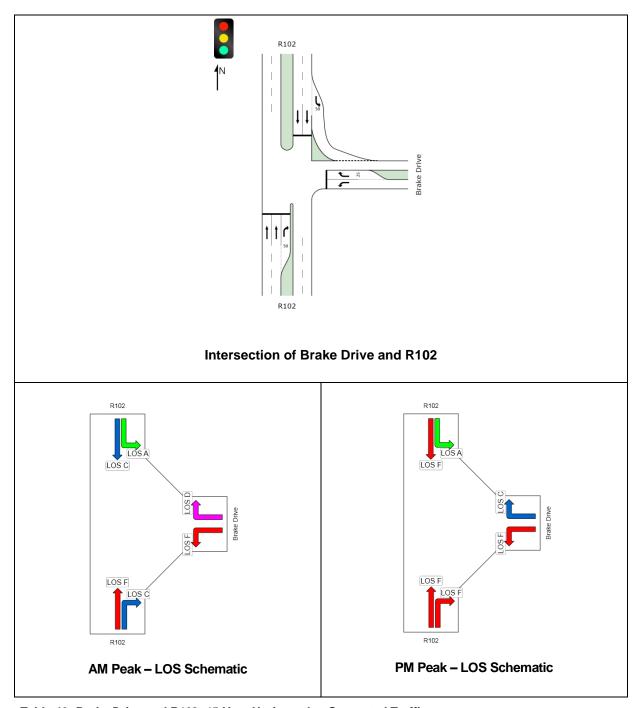


Table 40: Brake Drive and R102, 15 Year Horizon plus Generated Traffic

Upgrade of Brake Drive and R102 Intersection

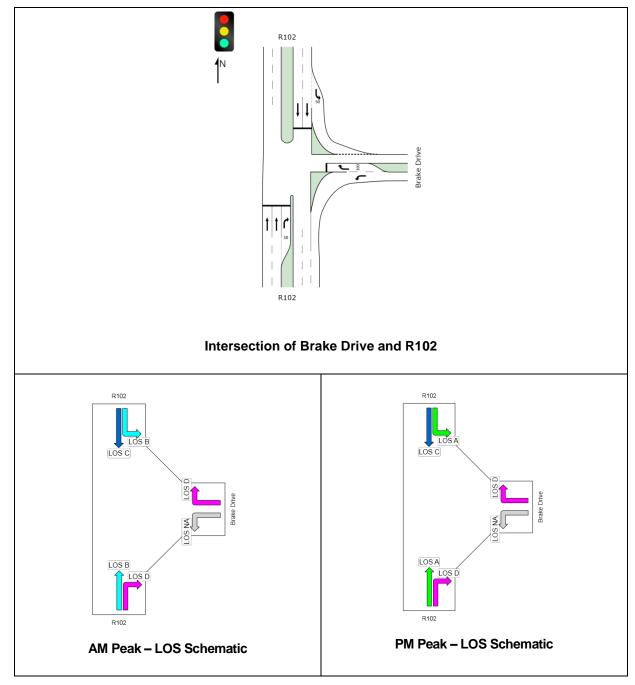


Table 41: Upgrade Brake Drive and R102, 15 Year Horizon plus Generated Traffic

Upgrades will predominantly be required on the eastern approach. A dedicated left continuous slip lane will be needed on the eastern approach. Furthermore, the exclusive right turning bay must be extended from 25m to 100m. Following the upgrades the average delays at this intersection will be 22.7 and 12 seconds for the AM and PM peak hours respectively. The average queue lengths will be 410.7 and 196.6 metres for the AM and PM peak hours respectively.

9.6 uShukela Drive and Main uShukela Access Intersection

This intersection will operate at acceptable levels of service in both the AM and PM peak hours – see Table 42. The average delays will be 17.7 and 28.5 seconds for the AM and PM peak hours respectively. The average queue lengths will be 143.3 and 244.8 metres for the AM and PM peak hours respectively.

No upgrades will be required in the 15 year horizon.

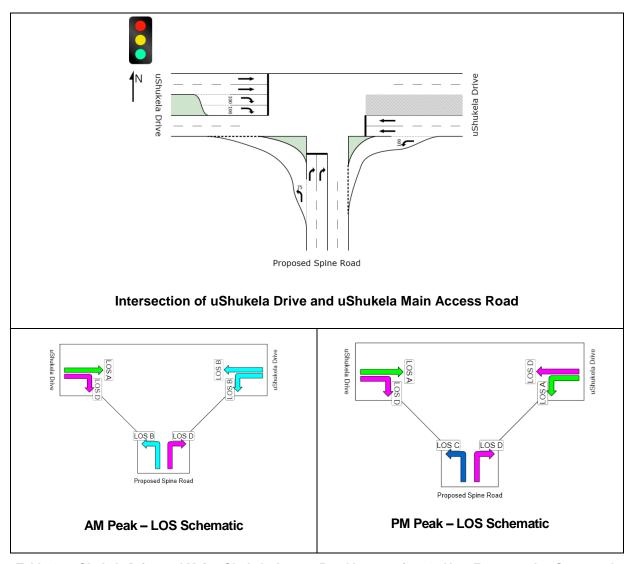


Table 42: uShukela Drive and Main uShukela Access Road Intersection, 15 Year Forecast plus Generated Traffic

9.7 Brake Drive and Main uShukela Access Intersection

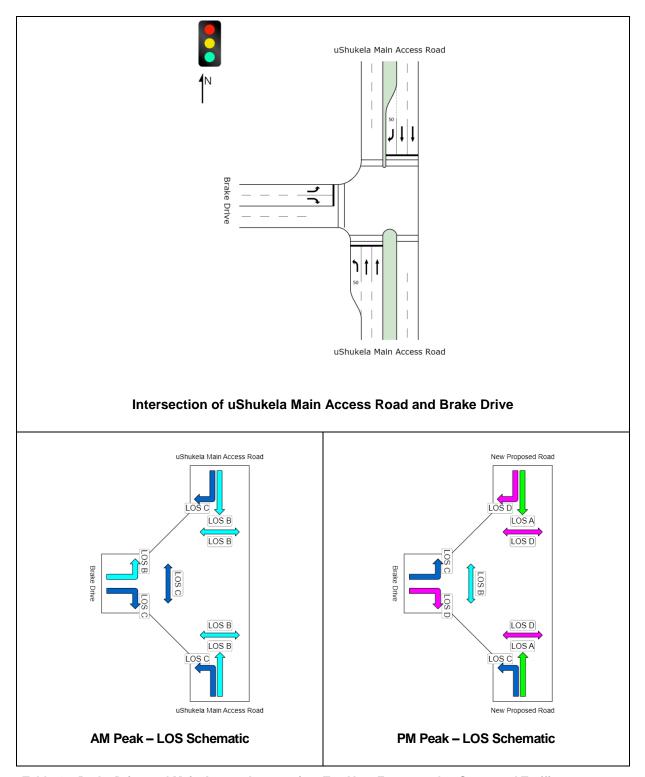


Table 43: Brake Drive and Main Access Intersection, Ten Year Forecast plus Generated Traffic

It is evident from **Table 43** that this intersection will have the capacity to handle the envisaged traffic volumes in the 15 year horizon. No upgrades will be required in the 15 year horizon. The average delays will be 17.7 and 28.5 seconds for the AM and PM peak hours respectively. The average queue lengths will be 143.3 and 244.8 metres for the AM and PM peak hours respectively.

10. Recommended Upgrades to the Surrounding Road Network

10.1 Base Year

The purpose of the base year analysis was to identify any pre-existing operational problems on the road network. This analysis revealed the following:

- The base year analysis has shown that the R102 and the uShukela Drive links encounter mounting pressure during the peak hours and are currently operating at LOS E, under the prevailing traffic conditions in the base year.
- The west to south right turn movement at the N2 interchange eastern intersection is also operating at a poor LOS of E.
- The uShukela Drive / High Street intersection is operating poorly with several movements at LOS E.

On the other hand, the remaining intersections within the study area currently operate at fairly acceptable levels of service in the base year.

10.2 Five Year Horizon

The analysis of the 5 year forecasted traffic volumes reveals that the R102 and uShukela Drive links will experience severe congestion in the 5 year horizon, before any developmentgenerated traffic is imposed onto the road network. The conclusion to this is that each of these roads will need to be upgraded to dual carriageway in the short term.

Furthermore, the inclusion of the Brake Drive access on the R102 south of Tongaat in the 5 year horizon will be highly beneficial as the uShukela Development generated traffic volumes from the south will utilise this access and not enter Tongaat as in previous scenarios.

All recommendations hereafter are based on the assumption that these upgrades are in place. The upgrades recommended for the 5 year horizon are tabulated hereafter for ease of reference.

Network Element	Proposed Upgrade	Time Horizon
uShukela Drive	Must be upgraded to a dual carriageway road with two lanes in each direction.	Prior 2017
R102 south of Tongaat	Must be upgraded to a dual carriageway road with two lanes in each direction	Prior 2017

Brake Drive	 Brake Drive must be upgraded/constructed to provide access into the uShukela Development for the generated traffic volumes approaching from the south. In addition a new link road will be required linking Brake Drive and the Main Spine Road through the uShukela Development. 	2017
N2 Interchange – Eastern Intersection	A left slip lane must be added to the northern approach. Signal warrants must be checked and if the warrants are met then the intersection must be signalised.	2017
N2 Interchange – Western Intersection	 A left slip lane must be added to the southern approach. Signal warrants must be checked and if the warrants are met then the intersection must be signalised. 	2017

uShukela Drive / High Street Intersection	 Slip lanes will be required on all approaches to the intersection. An additional through lane will be required in the eastbound direction. The above intersection configuration will adequately cater for the envisaged traffic volumes in 2017. 	2017
R102 / uShukela Drive Intersection	A left slip lane is required on the northern approach. An exclusive right turning lane is required on the southern approach.	2017

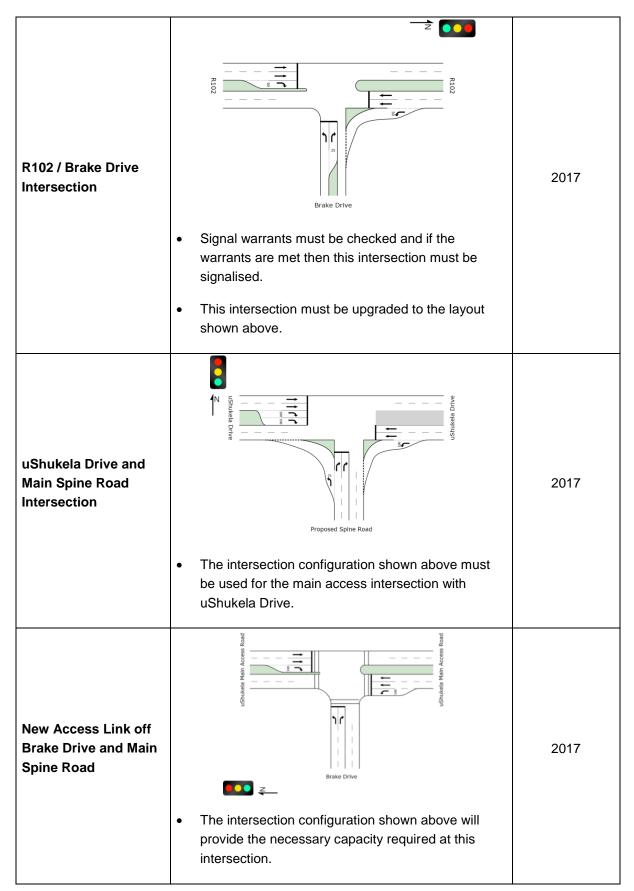


Table 44: Summary of Road Network Upgrades Required within the Five Year Horizon

10.3 Ten Year Horizon

Network Element	Proposed Upgrade	Time Horizon
uShukela Drive / High Street intersection	The above intersection configuration will adequately cater for the envisaged traffic volumes in 2022.	2022

Table 45: Summary of Road Network Upgrades Required within the 10 Year Horizon

10.4 Fifteen Year Horizon

Network Element	Proposed Upgrade	Time Horizon
N2 Interchange – Western Intersection	• This intersection must be upgraded to the geometric configuration shown above in 2027.	2027
Intersection of uShukela Drive and High Street	This intersection must be upgraded to the geometric configuration shown above in 2027.	2027

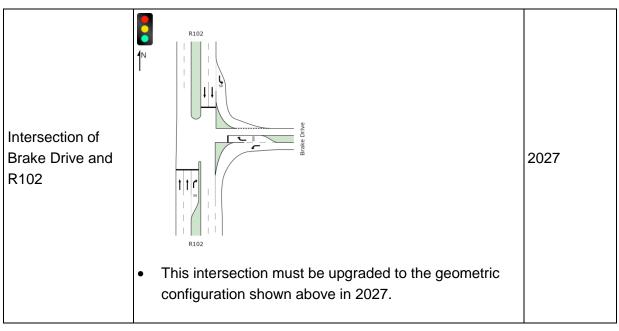


Table 46: Summary of Road Network Upgrades Required within the 10 Year Horizon

11. Literature Review of Previous Studies for the Tongaat Area

Transportation Studies and Local Area Plans (LAP) for the Tongaat area were reviewed with the primary intent of understanding the future transport and developmental plans for the Tongaat area and to determine its impact on the uShukela Development. Of particular significance, is a technical report compiled by SSI in November 2010 for the King Shaka Airport and Tongaat Area, as part of a greater study for the Northern Urban Development Corridor (NUDC). The report highlights the fact that the R102 is a strategic route in this area that needs to be enhanced as a regional mobility corridor to augment the connectivity of the surrounding areas to the envisioned Aerotropolis. However, the capacity of the R102 in the immediate vicinity of Tongaat cannot be improved due to the extensive commercial, residential and industrial activities on either side of the R102. Furthermore, the promotion of the R102 as a regional mobility corridor through the Tongaat CBD will result in several negative externalities such as unprecedented congestion, road safety problems, unfriendly pedestrian environment and high levels of gas emissions within the Tongaat CBD.

As a result, the SSI study focuses on developing a hierarchical system of roads that will promote regional mobility in this area by selecting alignments that will bypass the Tongaat CBD. According to SSI (2010), a bypass route for the Tongaat CBD has been contemplated since the late 1960's. A route alignment to the west of Tongaat, later commonly referred to as the Western Bypass, was initially proposed and proclaimed as Provincial Main Road 407. During the late 1980's, further transportation studies for this area motivated for a route on the eastern side of Tongaat, principally because the terrain was conducive to road construction. This alignment to the east of Tongaat is referred to as the Eastern Arterial. The SSI (2010) study tested various scenarios based on travel patterns and

traffic volumes in the area. The objective of this assessment was to identify a road network that provides regional mobility and access to developments while simultaneously decongesting the Tongaat CBD. The study concluded that a combination of the Western Bypass and the Eastern Arterial will improve the regional mobility and accessibility in the Tongaat area. The Western Bypass and the Eastern Arterial are shown in Figure 12, which was extracted from the SSI (2010) study.

The Western Bypass will specifically provide a high quality regional mobility link between the King Shaka Airport and the neighbouring towns to the north. Furthermore, the Western Bypass will separate the regional traffic that is currently passing through Tongaat from the CBD traffic. The proposed Western Bypass will have a minimum design speed of 100km/h with a fairly gentle geometric alignment along most of its alignment. Access to the Western Bypass will be at interchanges only, while crossing points with other roads will be achieved by means of overpasses and underpasses (SSI, 2010).

The Eastern Arterial on the other hand will function as an accessibility route providing unimpeded access to the airport and surrounding developments such as the Dube Tradeport and other developments, presumably like the proposed uShukela Development. The Eastern Arterial will unlock the land on the eastern side of Tongaat for the expansion of the town and create new development opportunities. The proposed alignment of the Eastern Arterial will most likely permit a design speed of 80km/h given the rolling terrain in the Tongaat area. Access onto the Eastern Arterial will either be at interchanges or limited to at-grade intersections. For certain sections of the Eastern Arterial there are alternative alignments proposed in the SSI study, as shown hereafter in Figure 12.

In essence, the SSI (2010) study shows that the Western Bypass and the Eastern Arterial will form critical links in ensuring the efficacy of the future road network for the NUDC, as the R102 will certainly not have the capacity to function as regional mobility corridor in the future. This traffic study for the uShukela Development directly correlates with the findings of the SSI study, as the results of analyses to date show that the R102 will not have the capacity to handle the future volumes of traffic.

In addition to the proposed bypass routes, the SSI (2010) study explicitly shows that uShukela Drive will encounter capacity problems in the future as a result of the natural growth in traffic volumes and will require an upgrade to a two lane dual carriageway highway, (SSI, 2010, p8). This finding also directly corroborates the results of the analyses conducted in this study for the uShukela Development.

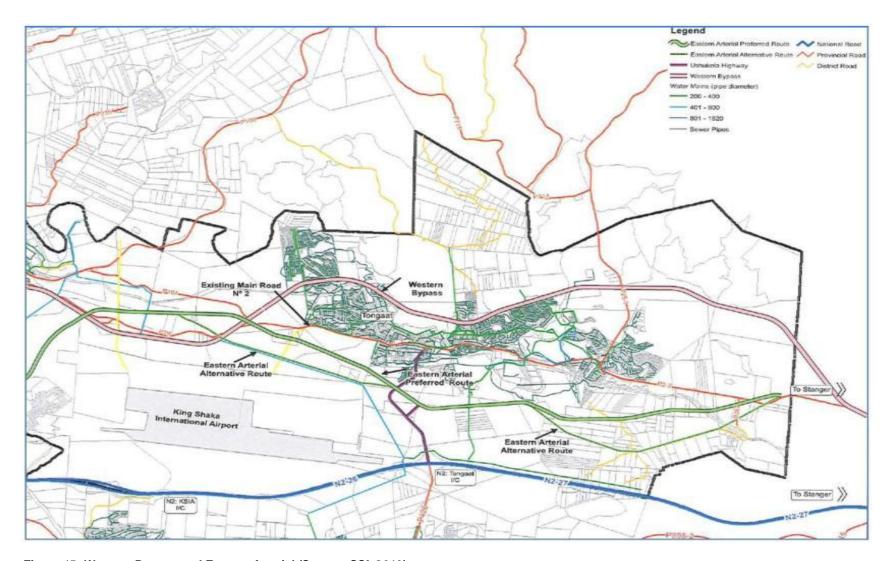


Figure 15: Western Bypass and Eastern Arterial (Source: SSI, 2010)

12. Conclusions

Tongaat-Hulett Developments and Dube Tradeport have formed a joint venture to develop a portion of land, approximately 431 000 m² of bulk area. The land is currently jointly owned by both companies. The proposed development is commonly referred to as the uShukela Drive Precinct Development. The uShukela Drive Precinct Development will be a multi-faceted development that will comprise of nine Trade Zones and a Conference Centre. Each Trade Zone will effectively comprise of Offices, Light Industry, Warehousing, Distribution and Manufacturing land use types.

Aurecon was commissioned by the joint venture to undertake a Traffic Study for the proposed uShukela Development which is located on a tract of land to the east of Tongaat and west of National Route 2 (N2). It is the primary intent of the developers of this project to symbiotically develop the land and the transportation system simultaneously such that the development can improve the economy, shape development patterns and influence the quality of life in the Tongaat Area.

It is envisaged that by 2017 the proposed uShukela Development will be 10% complete. By 2022 the development will attain 40% of its developmental potential. By 2027 the uShukela Development should be fully 100% complete.

The analysis of the existing traffic volumes in the base year showed that the R102 and the uShukela Drive links currently encounter mounting pressure during the peak hours and currently operate at a LOS E, under the prevailing traffic conditions in the base year, which is unacceptable. Most of the intersections within the study area currently operate at acceptable levels of service under the existing traffic conditions. However, there are turning movements at the N2 interchange eastern intersection and at the uShukela Drive / High Street intersection that operate at level of service E and will have to be addressed.

The analysis of the 5 year forecasted traffic volumes confirms that the uShukela Drive and R102 links will experience severe congestion, before any development-generated traffic is added onto the road network. These roads will operate at LOS E and LOS F during the peak hours purely as a result of the natural growth in the existing traffic volumes and therefore require upgrading to dual carriageway status.

The intersections within the study area display signs of distress in the 5 year horizon due to the natural growth in traffic volumes. Once the generated traffic from the uShukela Development and the Dube Tradeport are considered in the 5 year horizon, certain intersections will encounter high levels of congestion. The R102 and uShukela Drive intersection in particular will require major upgrading however this is not possible due to the space constraints adjacent to this intersection. Only minor upgrades to this intersection will be possible. Therefore, the introduction of the Brake Drive link will be highly beneficial in the 5 year horizon as this link will certainly alleviate the pressure on the road network within the



Tongaat CBD. Furthermore, the terminal intersections at the N2 interchange will also operate at poor levels of service and will require upgrading.

The analysis of the combined development-generated traffic volumes and the forecasted 10 year traffic volumes show that most of the surrounding road network (as upgraded in 2017) will have the capacity to handle the envisaged volumes of traffic. The intersection of uShukela Drive and High Street will be the only intersection within the study area that will require capacity upgrades in the 10 year horizon.

The analysis of the 15 year horizon revealed that 4 intersections within the study area will not have the capacity to cope with the demand required. As highlighted in Section 10.4, it is recommended that 3 of the 4 intersections be upgraded in the 15 year horizon. The link roads within the study area will have sufficient capacity and therefore will not require any upgrading.

Therefore, from a traffic perspective it is recommended that the uShukela Development be supported and approved.

Public transport usage in and through the uShukela Development must be given serious consideration into the future as this could drastically reduce the private vehicle flows discussed in the analyses tabled in this report. Measures such as the IRPTN (integrated rapid public transport network) will need to be applied to this northern area of eThekwini to ensure that unnecessary expenditure on private vehicle road network elements is contained.

Appendix 4: Ushukela – Watson Highway Development Site Stormwater Management Plan





CONTRACT No. 243/203

MARCH 2013







USHUKELA HIGHWAY

STORMWATER
MANAGEMENT PLAN
REVISION 2





uSHUKELA HIGHWAY

STORMWATER MANAGEMENT PLAN REVISION 2

PROJECT No. : 243/203	DATE: March 2013
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DISCIPLINE:

STORMWATER MANAGEMENT PLAN



· Consulting Engineers · Project Managers

QUALITY VERIFICATION

This report has been prepared under the control of the Bosch Stemele Quality Management System which meets the requirements of ISO 9001:2008 as independently certified by international auditors (Certificate No. 20705704/1)



Verification	Capacity	Name	Signature	Date
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Reviewed by:	Project Manager/Director	C Kroeger	May	17/4/13
Accepted by:	Client Authorised Representative	R Wilkinson	01	
Accepted by:	Client Authorised Representative			

uSHUKELA HIGHWAY

STORMWATER MANAGEMENT PLAN

REVISION 2

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uSHUKELA HIGHWAY

STORMWATER MANAGEMENT PLAN

REVISION 2

1. INTRODUCTION

1.1. Description of the Development

The development is situated near Tongaat on the KwaZulu Natal north coast and falls within the jurisdiction of the eThekwini Municipality (see *Annexure A*). The proposed development consists of an industrial trade-zones, and minor mixed use precincts.

It is bounded by the N2 freeway and Toll Plaza to the east, the uShukela Drive (Watson Highway) to the north and the Hlawe River in the west.

Tongaat Hulett Developments and Dube Tradeport currently own the land making up the development area, and propose to jointly plan the development of the land.

The site is approximately 136 ha in size.

1.2. Purpose of the Stormwater Management Plan

The purpose of this document is to provide a guideline policy for the holistic management of stormwater for the overall development of the site.

This SWMP has the following purposes:

- To ensure compliance of the overall site with relevant legislation from a stormwater runoff perspective,
- To provide a practical and achievable plan and methodology of managing stormwater runoff from site,
- To protect the health, welfare and safety of the public from damage by stormwater and floods,
- To protect against property damage from stormwater and floods,
- To prevent erosion of soil by runoff.
- To conserve the fauna and flora of the natural environment including wetland and riparian zones,
- To protect and enhance the natural water resources in the sub catchments from pollution and siltation, and
- To develop a conceptual surface water runoff management policy.



1.3. Current Use & Topographical Features of the Site

The development will take place on existing cultivated farmland (sugarcane).

The area is relatively undulating with slopes ranging from between 5% up to 20% in certain places. It is also characterised by a number of small tributaries, with the bulk of them draining to the Hlawe River. The site is mostly divided into two primary sub-catchments which drain north and west to existing wetland features. A smaller sub-catchment drains to the east which eventually links up to the northern wetland.

Sub-surface conditions are dominated by the Berea Red formation of deep loose sands overlying firm clayey sands, with a smaller portion of the area consisting of the Vryheid Formation of siltstones and sandstones where firm to stiff sandy clays and clayey silts overlay soft siltstone and mudstone, interbedded with sandstone.

1.4. Proposed Use

The proposed developed will "flatten" out the current steep grades and make provision for a number of platforms with overland slopes in the region of 2% to 5%. The proposed platforms will be linked by internal access roadways between platforms. The proposed topography of the development will retain in general the north/west sub-catchment drainage areas.

1.5. Legal Requirements

The management of stormwater on site is governed by two main acts:

- National Water Act (36 of 1998) which deals with pollution control and the protection of existing watercourses, and
- National Environmental Management Act (107 of 1998) which deals with compliance and duty of care and remediation to the existing environment.

This SWMP includes recommendations for compliance to the applicable legislation.

1.6. Local Authority Requirements

The eThekwini Municipality design guidelines and policy for the design of Stormwater Drainage and Stormwater Management Systems (May 2008) has been incorporated in this Management Plan.



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2. MANAGEMENT RESPONSIBILITIES

2.1. Stormwater Management

The development's bulk stormwater infrastructure will be developed by Tongaat Hulett Developments/Dube Tradeport, and will include all stormwater infrastructure within the open spaces and road reserves, and that connecting road reserves to open spaces via servitudes.

On completion, these facilities will be handed over to Ethekwini Municipality who will then become responsible for their operation and maintenance.

The purchasers/developers of the individual sites will be responsible for construction of the on-site stormwater facilities to the standards required by Tongaat Hulett Developments/Dube Tradeport, and approved by the Ethekwini Municipality. The Ethekwini Municipality will then assume responsibility for stormwater discharge from the sites.

A management association for the whole development will be constituted by Tongaat Hulett Developments/Dube Tradeport when the conditions of establishment of the first phase are met.

This management association may, in agreement with Ethekwini Municipality, perform operations and management functions including monitoring stormwater management on the individual properties, as well as the operation and management of stormwater facilities falling outside these individual properties.

3. EXCLUSIONS

The following are excluded from the SWMP:

- The SWMP provided a holistic overview of stormwater management on site. A more detailed layout and operation of the stormwater system would need to be defined in the detail design of the development.
- This SWMP excludes the delineation of wetland and river areas (undertaken by Wetland Consulting Services (Pty) Ltd.), and
- The determination of floodlines is excluded from this SWMP. The
 development proposal is situated significantly above any 1:100 year
 floodlines in the area and won't be affected by any floodlines. For
 reference Ethekwini determined floodlines are indicated on the
 appropriate drawings in the annexures.

4. MAJOR IDENTIFIED RISKS

The major risks applicable to this development are as follows:



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4.1. Potential Flooding

The development of the area will result in increased "hardening" of the area (i.e. increased impermeable surface areas). This will result in increased overland runoff that will have to be catered for. Potential risks include flooding of on-site facilities (proposed industrial facilities, etc) as well as off-site (existing access roads, wetland areas and existing downstream properties).

4.2. Erosion

An increase in runoff will result in the increased likelihood of erosion. Based on the preliminary geotechnical investigation report, the site is overlain by loose sands which have a high erosion potential.

4.3. Pollution

The development of the area to primarily light industrial trade-zone uses will increase the risk of pollution, especially to the existing wetlands and river. Primary anticipated increased pollution includes hydrocarbons from vehicles, total suspended solids (TSS) from the light industrial portion of the development and domestic waste from the office and ablution components. In addition pollution from construction materials for the proposed development and associated facilities is also a potential risk.

4.4. Sedimentation

Accumulated material on hardened surfaced which is transported to the existing wetlands during storm events can lead to a build up of transported material in the wetlands. This material normally contains the bulk of the pollutants.

4.5. Environmental Impacts

The wetland receives water from the existing property via overland runoff and subsurface drainage through the perched water table of the more clayey sands (between the Berea Red and Vryheid Formation). Improper management of stormwater on site could negatively affect these areas by insufficient recharging of the wetlands.

5. DESIGN PHILOSOPHY

5.1. Stormwater System

The stormwater system for the proposed development must take cognisance of the impacts of both the minor and major stormwater system in terms of runoff, potential flooding, wetlands recharging, stormwater attenuation etc.



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The minor stormwater system consists of all measures to address runoff from individual sites and road reserves, buildings and car lots to the major stormwater system. This includes kerbing, gutters, conduits, channels, infiltration systems etc. The minor system normally deals with low/medium rainfall events with high occurrence intervals (normally up to a once in 2 or 5-year interval) which are likely to cause a level of nuisance to users if not controlled. For this development it is proposed that the minor system on all individual sites be designed to handle a 1 in 5-year storm event.

The major stormwater system will consist of the natural waterways, wetlands and streams draining in a generally western and northern direction. It also includes attenuation dams and other structures to control stormwater runoff. The major system also controls runoff for high rainfall events with low occurrence intervals (usually 1 in 10 years or longer as in this case) with a high risk of flooding.

For this development it is proposed that the major system be designed to handle a 1 in 50-year storm event. Cognisance is to be taken though of risks by storms of a higher magnitude (i.e. 1 in 100-year).

As such the design philosophy for the minor and major stormwater system should allow for the following:

- Restrict stormwater flow to within a 10% increase of the pre-development flows using attenuating devices such as attenuation dams/structures or infiltration devices,
- Prevent the concentration of stormwater runoff at any point where erosion is a possibility. This will be prevalent near areas with high impermeably (roof structures, large surfaced areas) and embankments.
- · Avoid ponding on site, especially near building structures,
- · Avoid destabilisation of existing and proposed embankments,
- · Ensure compliance to local authority standards,
- · Construction of pollution reducing systems, and
- Ensure that the construction of stormwater control systems is executed in a safe and acceptable manner.

In addition the design philosophy should cater for other environmental factors that are potentially impacting on the surrounding habitat. The most important aspect is the recharging of the natural wetlands.



5.2. Conceptual Stormwater System Management for Individual Sites

The extent of proposed platforming on the site makes it more suitable to manage minor storm events on the individual site platforms instead of discharging directly to the wetlands and stream. Cognisance of recharging of the natural streams is to be taken into account as well.

As such it will be required that:

- Runoff from individual sites be limited to within an increase of less than 10% of the pre-development runoff for a 1:5-year storm event.
- Allowance be made to retain the first 25mm of precipitation for any storm event on the individual sites for infiltration and recharging of the wetlands and stream. Retaining this first 25mm can be done via various approved attenuation devices (storage tanks, permeable paving, swales, irrigation ponds etc).

The design and implementation of these measures will be the responsibility of the entity that develops each individual site, with the designs requiring approval by both the management association and the Ethekwini Municipality.

Runoff in excess of the above requirements is to be routed to defined points exiting the sites on roadways and streams, and attenuated to pre-development levels via attenuation devices (attenuation ponds, buffer dams etc).

5.3. Pollution Control

Section 4.3 deals with potential pollutant impact by the proposed development. Measures to be applied to reduce pollutants include bio-attenuation swales, artificial wetlands and infiltration measures. These measures will be applied at the point source (individual sites and road reserves) as far as possible.

5.4. Concept Stormwater Modelling Parameters

5.4.1. Rainfall Data

The following depth-duration-frequency rainfall data was used for the development:

Table 1: Local Depth Duration Frequency Rainfall

Station	Frasers (SAWB 241302)					
MAP	971mm					
Rainfall	1-Day De	1-Day Design Rainfall (mm)				
Duration (Days)	2	5	10	20	50	100
1	84	122	153	188	241	287



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5.4.2. Design Storm Frequencies

The following design storm frequency is to be used:

- i) Minor stormwater system: 1 in 5 year frequency (the proposed development is predominantly industrial & commercial in use and a smaller design return period is not advisable as it will lead to more periodic nuisance flooding).
- ii) Minor stormwater system pollution control: 25mm precipitation within a 24-hr period.
- iii) Minor stormwater system wetlands recharging: 25mm precipitation within a 24-hr period.
- iv) Major stormwater system/pollution control: 1 in 50 & 10 year frequency. Building floor levels to be above the 1:100 year flood level. Cognisance is to be taken of risks for a 1:100 year flood event.

5.4.3. Runoff Generation

Due to the size of the catchment the SCS instead of the Rational Formula was used to calculate runoff. Runoff was thus calculated using the Autodesk Storm and Sanitary Sewer Analysis Package that utilises the EPA SWMM analysis engine.

Parameters used are as follows:

Table 2: SWMM Modelling Parameters

Hydrology Method	SWMM
Distribution Curve	SA SCS Type 2
Link Routing	Kinematic Wave
Pre-development Weighted SCS Curve No.	Various
Post-development Weighted SCS Curve No.	Various
Conduit Manning's roughness factor	0.015
Stream Manning's roughness factor	0.032

The SCS curve numbers selected for use are discussed in detail in Bosch Stemele's report on catchment hydrology dated October 2012 which is included in its entirety as *Annexure E*.

For the development of the individual sites, 60% of the coverage will be hard surfaces (roofs) with 30% parking and 10% soft landscaping for the post-development runoff. Once the development proposals are finalised further adjustments can be made to fine tune expected runoff figures.



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It is expected that due to the platforming earthworks operations and compaction efforts that infiltration be significantly reduced from the current scenario. The overall layout of the soils formation in the area (see the Catchment Hydrology Report – *Annexure E*) and the formation of platforms suggests that the bulk of the proposed development will be constructed on the silty clayey Vryheid

- Roof areas 98.
- Impermeable traffic surfaces 98
- Soft landscaping (incl. open areas in road reserves) 80

formation soils. The SCS curve numbers thus adopted are as follows:

Undeveloped areas - 80

Subsequent changes to the layout must be taken into account and runoff recalculated before commencing with detail designs of the stormwater system.

5.4.4. Subcatchment Runoff

Annexure B contains a tabulated summary of expected runoff flows for the <u>pre-development</u> scenario. Also included in Annexure B are Dwg. 243/203/022 showing the pre-development subcatchments, and Dwg. 243/203/023 indicating the expected pre-development flows for the various sub-catchments.

Due to the extensive earthworks reshaping form the individual platforms, the drainage subcatchments are modified in extent. A direct comparison of subcatchments runoff for the pre- and post-development scenarios is therefore not possible.

Annexure C contains Dwg. 243/203/024 which depicts the <u>post-development</u> subcatchments, as well as a tabulated summary of expected runoff flows for the post-development scenario. The anticipated runoff for the 1:2, 1:5, 1:10, 1:50 & 1:100 year storm events for the post-development sub-catchments is shown on Dwg. 0243/203/025 (also in Annexure C).

5.5. Proposed Attenuating Measures

For the attenuating of stormwater runoff to predevelopment flow, a dual approach is to be considered.

5.5.1. Attenuation Ponds

The most common stormwater management method to intercept runoff and reduce flow to pre-development levels in hardened areas is through the use of attenuation ponds. Drawings illustrating the stormwater management are included in *Annexure D*.

They include *Dwgs.* 0243/203/026 & 027 showing stormwater drainage proposals and the positioning of attenuation ponds required to handle major events including platform links to the ponds.



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It is proposed that the attenuation pond walls be constructed using gabion type structures (see Dwg. 0243/203/028) or concrete masonry and lined with gunnite for additional protection. This will provide a more "natural looking" appearance to the landscape.

Dwg. 0243/203/029 shows the proposed method of roadways crossing stream lines.

A summary of the attenuation pond requirements is indicated on Table 3:

Table 3: Attenuation Pond Requirements

Catchment Reference	Size
RPF4B	1, 449m³
RPF16A	675m³
STR4	4, 015m³
STR2 (two ponds)	9, 037m³
STR9	2, 156 m³
Total attenuation requirement	17, 332m³

5.5.2. Infiltration Measures (permeable surfacing, soakaways)

To ensure maximisation of groundwater recharging, it is recommended that infiltration measures be constructed on the platformed sites. The eThekwini Municipality guidelines for soakaways for 1m³ of storage per 40m² of hardened land will be sufficient to allow the infiltrating of 25mm precipitation in the platformed areas. Based on a conceptual 60% hardening to roof structures, 30% paving and the remainder soft landscaping, this equates to 225m³ of storage for each hectare of hardened area.

It is recommended that interception of roof runoff be retained within the paving areas via permeable paving. Based on a 30% voids ratio in stone this would equate to a 250mm stone layer thickness. Note that the above won't reduce peak runoff volumes which would still have to be managed by the major storm system.

5.5.3. Major Stormwater Runoff Control

Where embankments are constructed, careful consideration will have to be applied to prevent erosion of the embankment face. No water may be permitted to run off from the platform down the embankment face, either in sheet flow or concentrated form. In this regard it is recommended that kerbing is constructed upstream of embankment slopes to divert flow away to an underground conduit or stabilised channel and on to the existing major stormwater system.



The outlet to the major stormwater system will only be allowed at developer specified outlet points. Energy dissipaters will be required where erosion is a possibility.

5.5.4. Roadways

Due to the large open nature of the development, the road reserves form a relatively small portion of the total area and thus do not contribute significantly to runoff.

Although roadways will be impermeable surfaces, runoff will be directed to roadside swales for infiltration. An overall layout showing the conceptual attenuation ponds, swales and internal stormwater pipework is shown on *Dwg.* 0243/203/027.

5.6. Wetlands Recharging

An assessment of the pre- and post-development scenario was undertaken to determine the impact that the development would have on groundwater and surface water recharging of the existing wetlands and streams. The assessment was based on a storm event with 25mm precipitation over a 24-hour period.

Table 4: Expected Groundwater Infiltration

Pre-development	41, 184m³		
Post-development combined	32, 770m³		

From the above nearly 80% of runoff for minor storm events will infiltrate into the soil to the wetlands within a 24-hour period. The remaining 20% will enter the wetlands area via overland flow.

Careful consideration must be given that the soakaways are not constructed on a perched water table. Should this be the case, outlets to the soakaways can be constructed to drain towards areas with higher infiltration potential.

5.7. Special Considerations

5.7.1. Subcatchment RPF4B

This subcatchment drains towards the uShukela Highway and the proposed hardening will lead to an increase in runoff leading to the roadway. The existing canal next to the roadway won't be able to cope with a major storm event. An attenuation pond with a capacity of approximately 1, 449m³ will be required next to the highway to reduce flow to pre-development runoff.



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5.7.2. General Embankment protection

The proposed platform embankments are in general at a slope of 1:2. Due to the sandy nature of soil on site, there is potential for erosion of the embankment face. Specific measures will therefore be required to minimise runoff from the platforms to the embankments, especially for large storm events.

Even with the bulk of overland runoff being redirected via kerbing or a berm, a certain level of erosion can be expected from precipitation on the embankments. It is recommended that these embankments be stabilised as soon as possible during the construction phase by placing of erosion protection measures and planting.

6. SITE ESTABLISHMENT AND PRELIMINARY ACTIVITIES

The following general conditions must be adhered to and maintained during the site establishment and preliminary activities of the project.

6.1. Existing Stormwater Systems

All existing drainage systems (streams, channels) are to be maintained by the main developer in accordance with normal agricultural soil conservation practices and local authority guidelines as far as possible (except where the town planning layout makes provision for the development of land over existing drainage systems).

6.2. Access Routes

Access routes to the construction site must follow the existing access roads as far as possible. Should new access roads be required these must be constructed in a way to minimise concentrated flow runoff and pollution to the existing wetlands.

6.3. Contractors Site Camp

The clearing of vegetation for the contractor's site camp is to be limited to the site camp area only.

The creation of hardened surfaces within the site camp area is to be kept to a minimum and is to be agreed to by the Engineer prior to construction.

Any soil or topsoil stockpiles created during site establishment are to be maintained as flat as possible, with no side slope greater than 1 in 4. The stockpiles are to be covered with cut brush found on site to provide wind screening and prevent soil loss.



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7. CONSTRUCTION STAGE ACTIVITIES

7.1. Programming

Stripping of vegetation to allow commencement of construction of the earthworks platform shall only be undertaken immediately prior to that element of construction commencing.

Construction of the embankment shall be done in segments up to full height, before moving on to the next area, clearing vegetation, and constructing embankment, etc.

The construction of internal stormwater piped systems are to be programmed for construction immediately on completion of the bulk earthworks for the road works.

7.2. Stockpiles

Any soil or topsoil stockpiles created during the construction phase are to be maintained as flat as possible, and shall not exceed 6m in height. Materials from stockpiles are to be used as soon as is practically possible or spread and spoiled in designated areas.

7.3. Haulage and Temporary Access Roads on Site

Construction vehicles must be restricted to demarcated access routes and turning areas.

7.4. Exposed Surfaces

To minimize the time that an area is exposed, the stripping of vegetation is to be carried out progressively and immediately prior to commencement of construction activities in a particular area.

Topsoiling and re-vegetation of exposed surfaces is to commence immediately after the completion of all construction activity.

All embankments or cut slopes, unless otherwise directed by the Engineer, shall be protected by a cut off drain to prevent water from cascading down the face of the slope.

7.5. Stormwater Systems

No dumping of construction rubble or spoil is to occur in completed stormwater drains, pipes, channels or natural drainage lines (existing wetland, stream, & riparian zone).

Weekly checks are to be carried out on the site's drainage system to ensure that the water flow is unobstructed. These are to be repaired or cleared of silt if required.



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7.6. Contract Completion

All undeveloped surfaces hardened due to construction activities are to be ripped, topsoiled and vegetated as soon as possible.

8. MAINTENANCE

See Annexure F relating to Operation & Maintenance.

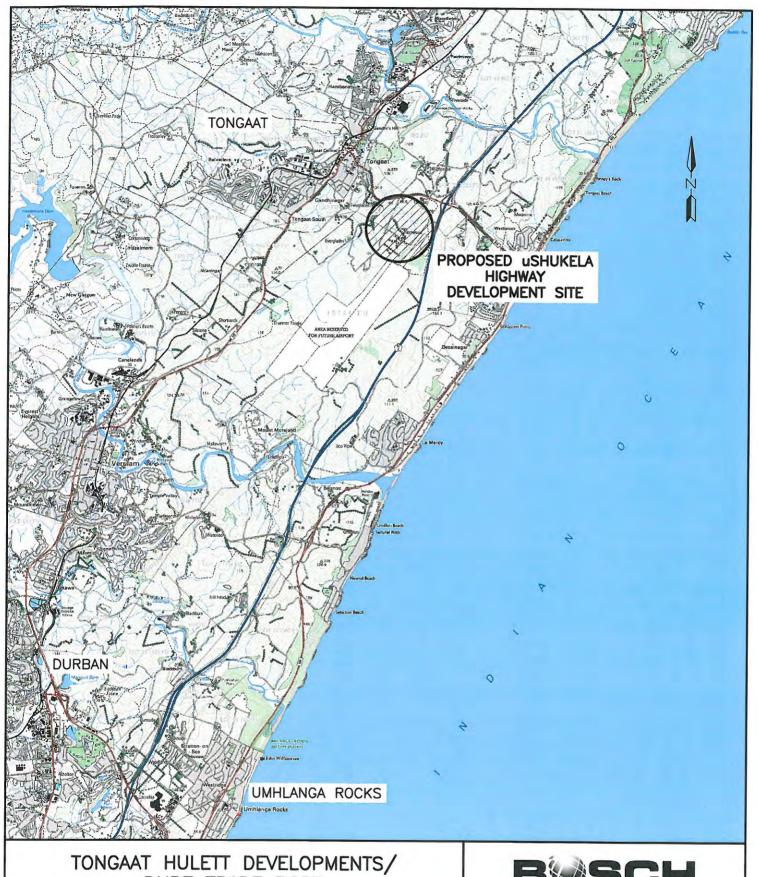
Prepared by L. Streicher Bosch Stemele (Pty) Ltd



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ANNEXURE A:

LOCALITY PLAN Dwg No. 243/203/21



TONGAAT HULETT DEVELOPMENTS/ DUBE TRADE PORT uSHUKELA HIGHWAY

LOCALITY PLAN

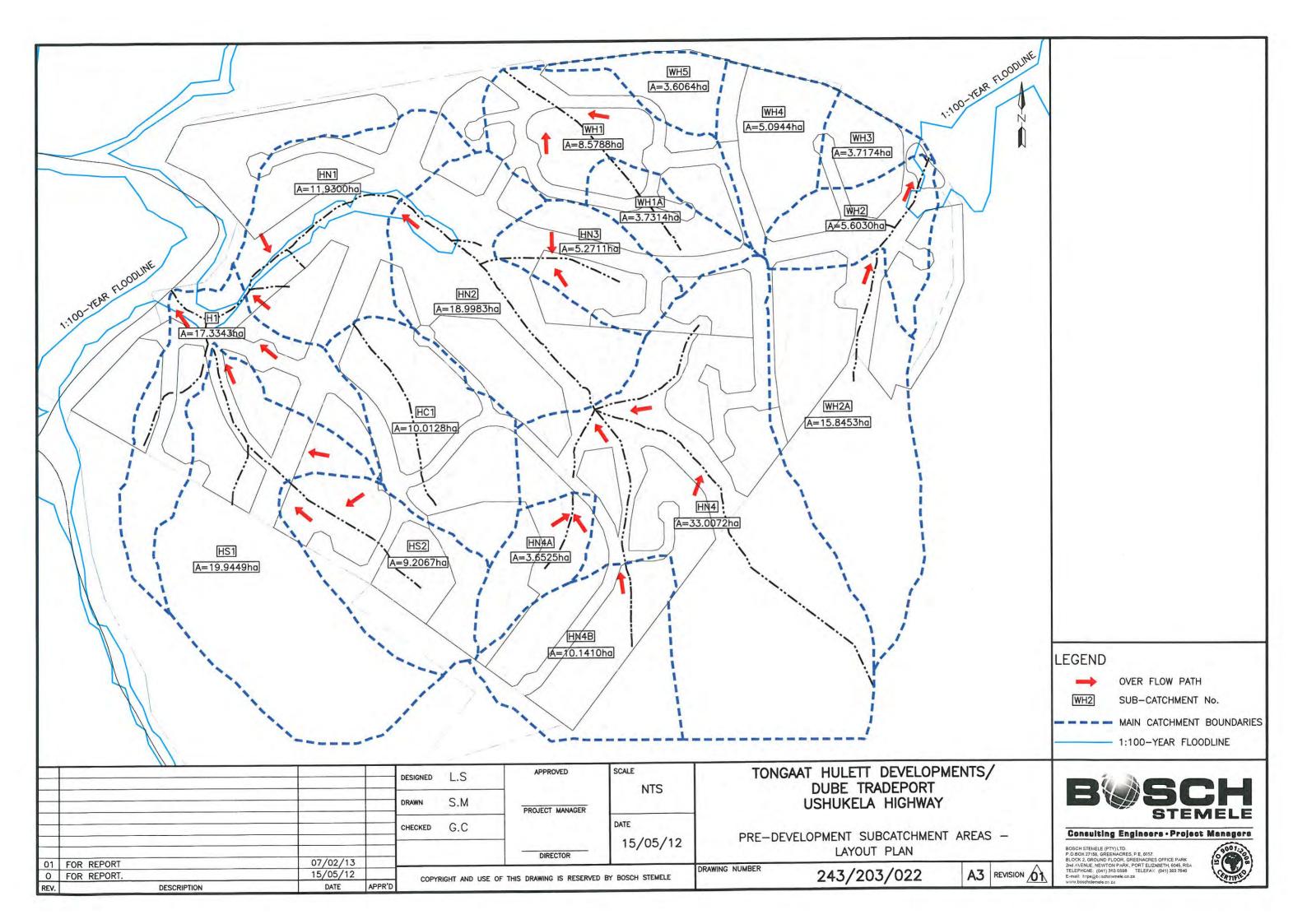


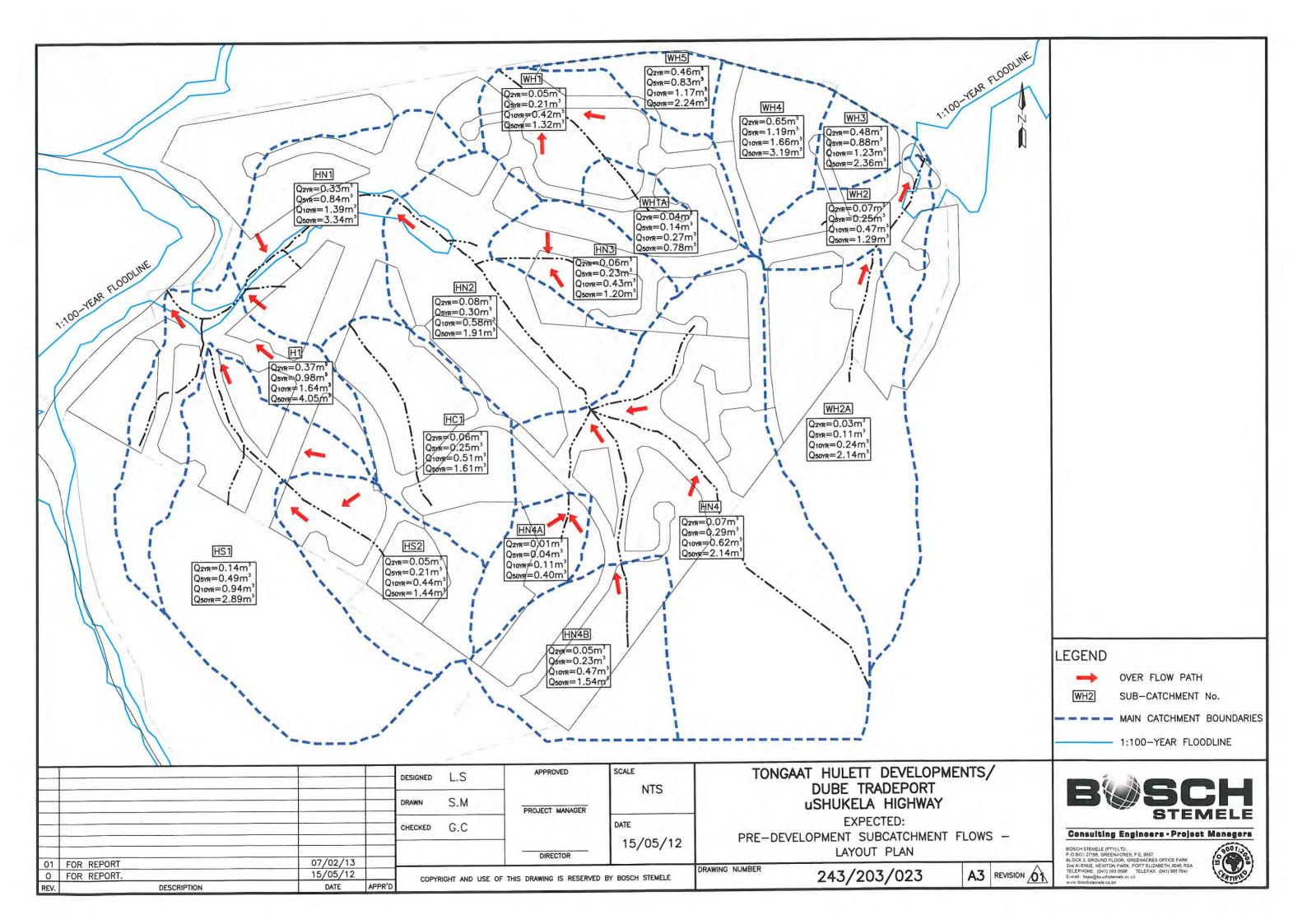
DESIGNED	L.S	APPROVED	
DRAWN	S.M		
CHECKED	G.C	DRAWING No.	REV
SCALE	NTS		100
DATE	05/12/2011	243/203/021	01

ANNEXURE B:

PRE-DEVELOPMENT RUNOFF DATA

Dwg No's: 243/203/22 243/203/23

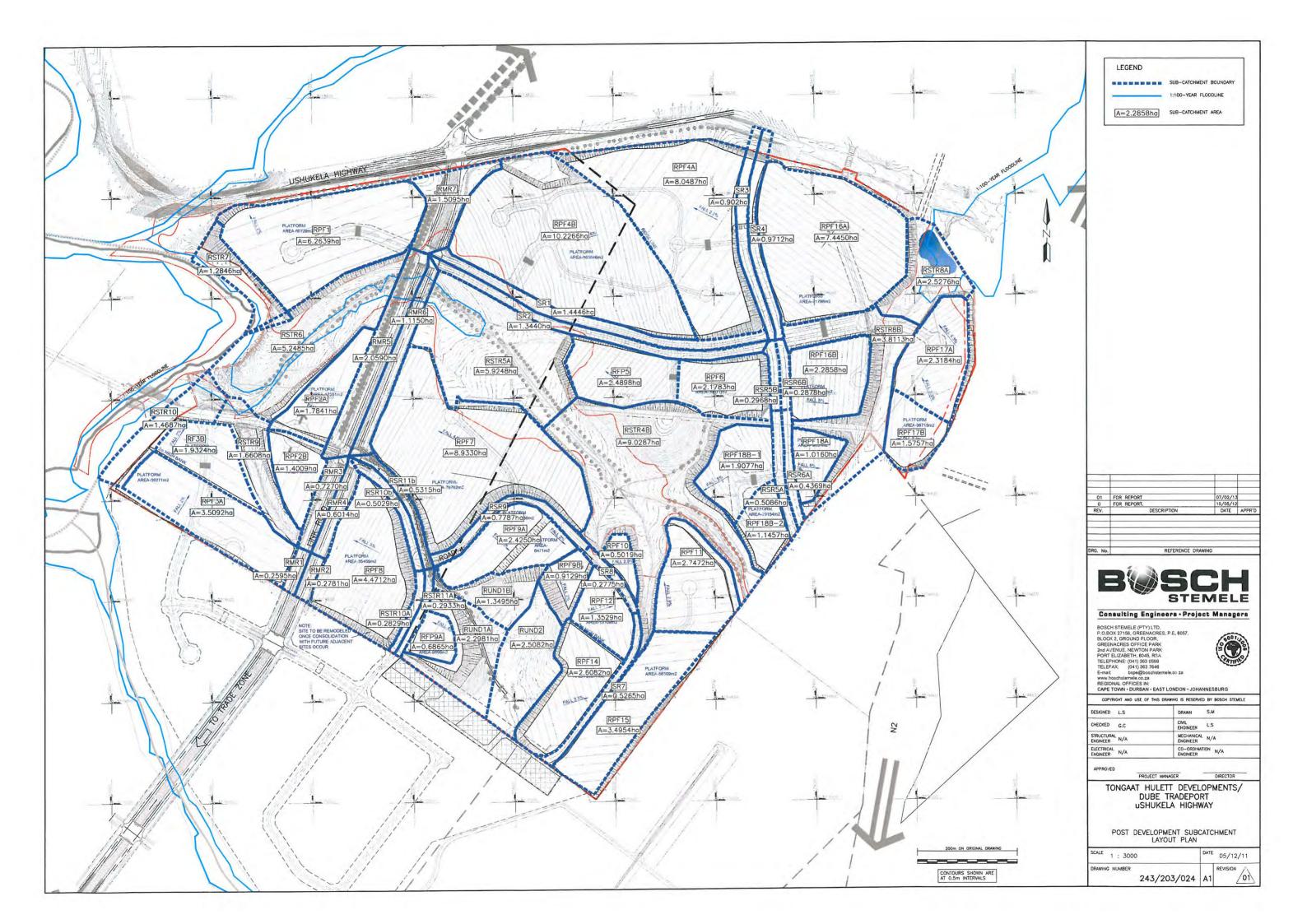


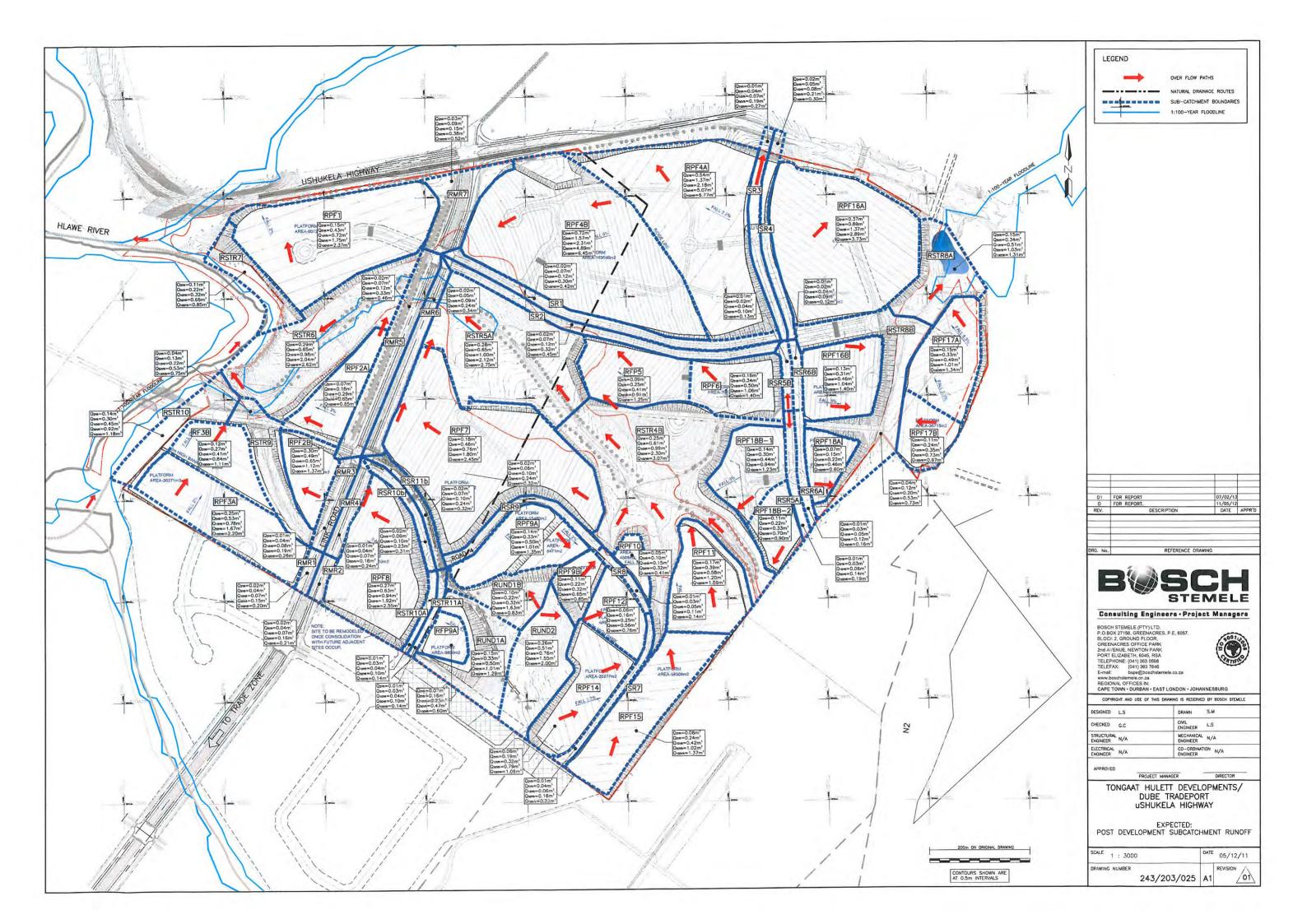


ANNEXURE C:

POST-DEVELOPMENT RUNOFF DATA

Dwg No's: 243/203/24 243/203/25





ANNEXURE D:

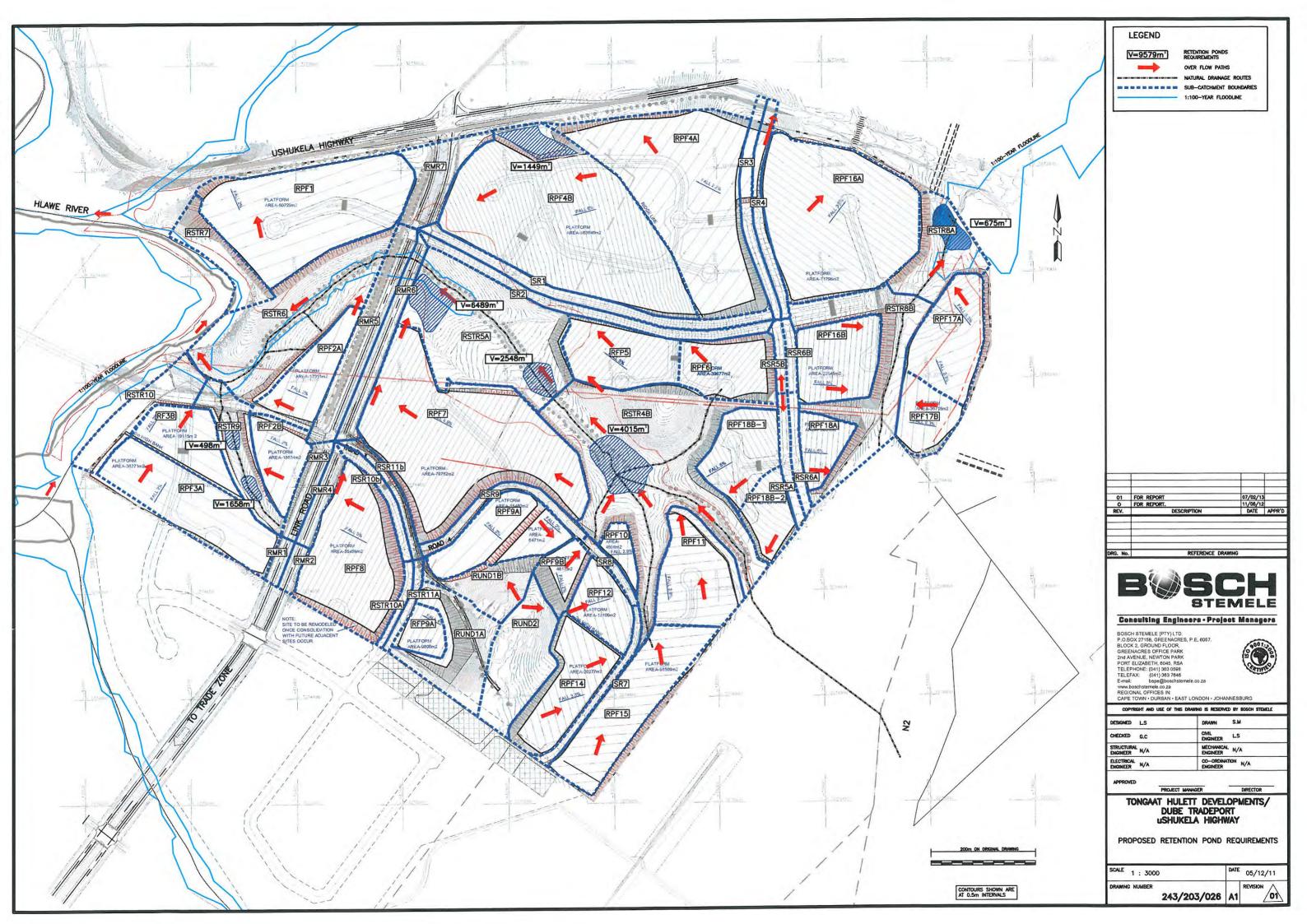
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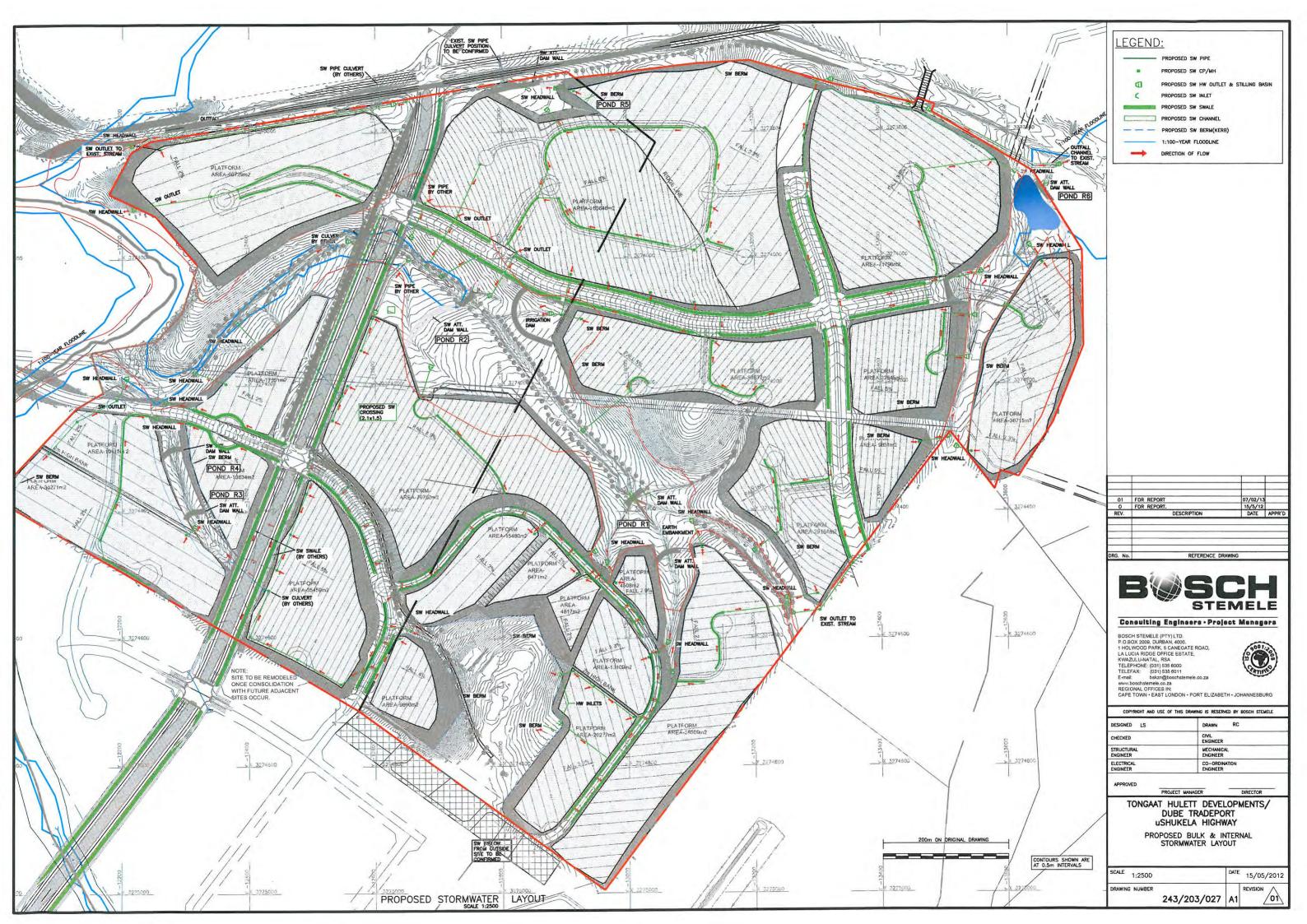
243/203/26

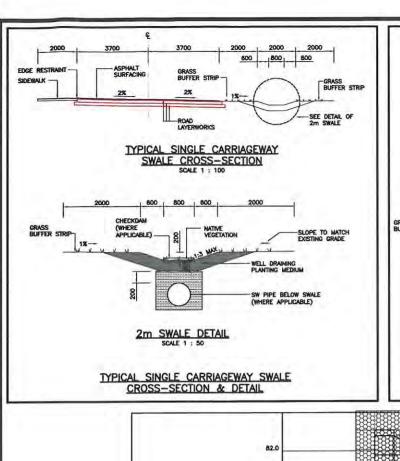
243/203/27

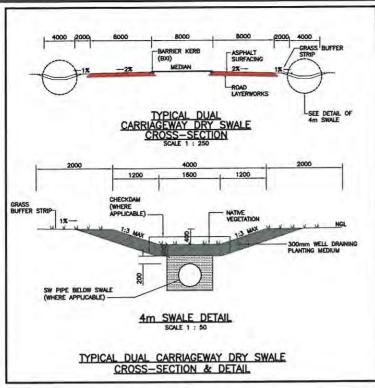
243/203/28

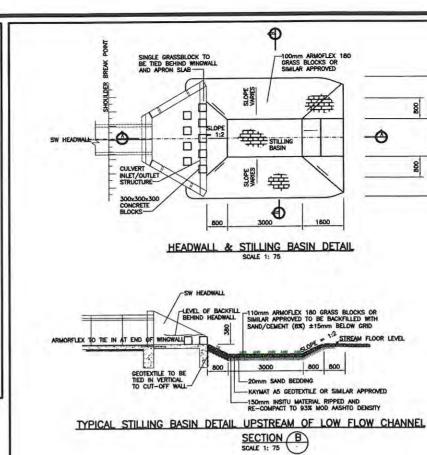
243/203/29











1600 1500 -IN-SITU MATERIAL RIPPED, SHAPED AND COMPACTED TO 90% MOD AASHTO DENSITY TYPICAL LOW-FLOW CANAL SECTION

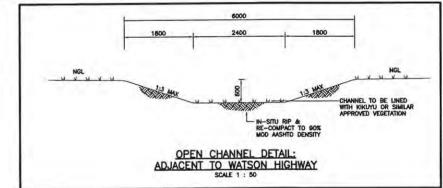
SECTION B SCALE 1: 75

NOTE: STILLING BASIN DIMENSIONS FOR PIPES UP TO 750mm#

TYPICAL HEADWALL STILLING BASIN DETAIL (PIPES UP TO 750mmø)

STREAM PROFILE 76.0 DATUM : 74.00 PEG DISTANCE (m) GROUND LEVEL (m)

TYPICAL SECTION THROUGH ATTENUATION POND CENTRE SECTION (POND R1)
SCALE: SEE ABOVE



07/02/13 15/05/12 DATE APPR'D 01 FOR REPORT.

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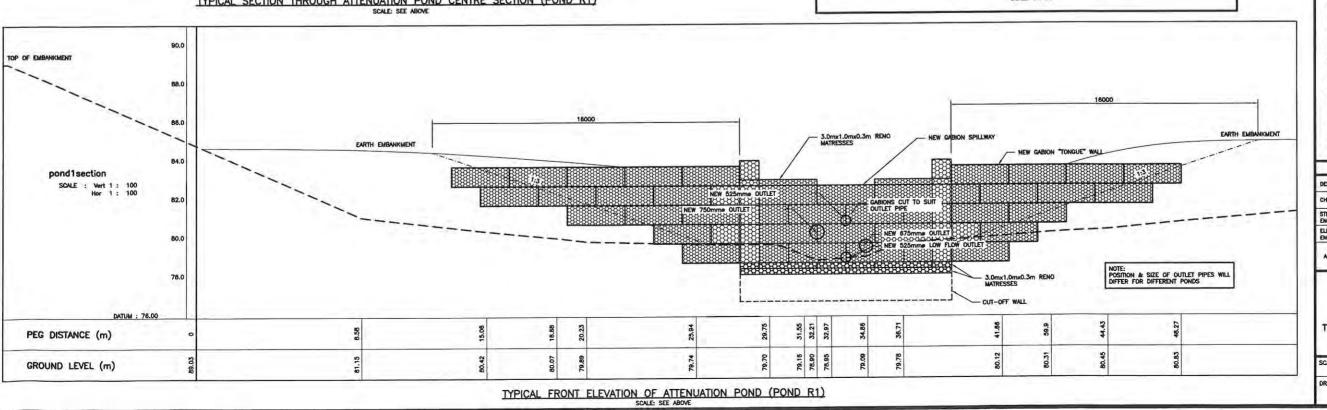
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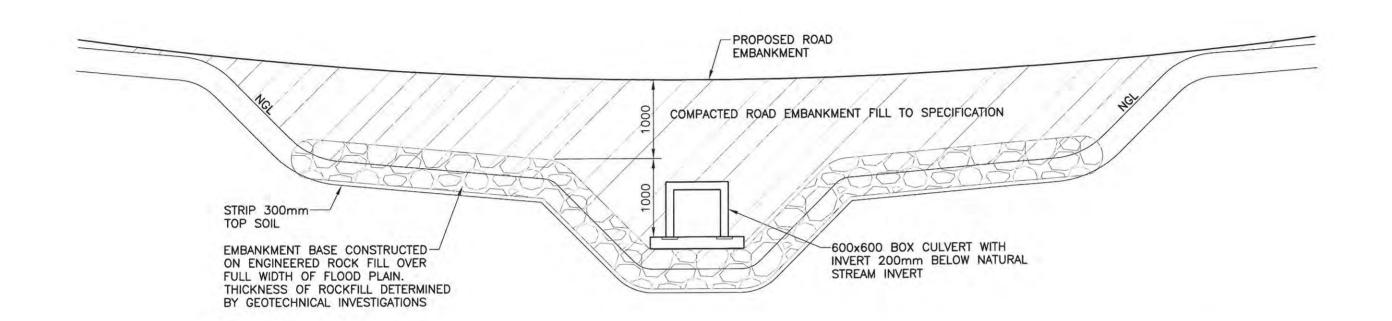
CIVIL ENGINEER MECHANICAL ENGINEER

> TONGAAT HULETT DEVELOPMENTS/ DUBE TRADEPORT USHUKELA HIGHWAY

TYPICAL STORMWATER INFRASTRUCTURE DETAILS

SCALE 1:2500 28/03/2012 RAWING NUMBER OT 243/203/028 A1





TYPICAL LONG SECTION OF ROAD EMBANKMENT OVER WETLAND CROSSING

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DRAWING NUMBER

		DESIGNED A.M.K.	APPROVED	SCALE 1 . 50
		DRAWN H.W.	PROJECT MANAGER	1 : 50
		CHECKED		DATE
			DIRECTOR	MAY 2012
01 FOR REPORT	07/02/13		DIKECTOR	

APPR'D

21/05/12

DATE

O FOR INFORMATION.

DESCRIPTION

TONGAT HULETT DEVELOPMENTS/ DUBE TRADEPORT uSHUKELA HIGHWAY

TYPICAL ROAD CROSSING OVER STREAM LINES RSTR8B AND RSTR9

RSTR8B AND RSTR9

243/203/029

A3 REVISION (A) TELEPHONE: Email: bescaring were beschate with the control of th



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ANNEXURE E:

ANALYSIS OF SITE CATCHMENT HYDROLOGY AND INFLUENCES
OF DEVELOPMENT ON CATCHMENT STORAGE AND
SURFACE RUN-OFF REV. 3





USHUKELA HIGHWAY

ANALYSIS OF SITE CATCHMENT HYDROLOGY & INFLUENCES OF DEVELOPMENT OF CATCHMENT STORAGE & SURFACE RUN-OFF REVISION 3





uSHUKELA HIGHWAY

ANALYSIS OF SITE CATCHMENT HYDROLOGY AND INFLUENCES OF DEVELOPMENT ON CATCHMENT STORAGE AND SURFACE RUN-OFF

REVISION 3

PROJECT No.: 243/203	DATE: March 2013
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DISCIPLINE:

ANALYSIS OF SITE CATCHMENT HYDROLOGY - REVISION 3



• Consulting Engineers • Project Managers

QUALITY VERIFICATION

This report has been prepared under the control of the Bosch Stemele Quality Management System which meets the requirements of ISO 9001:2008 as independently certified by international auditors (Certificate No. 20705704/1)



Verification	Capacity	Name	Signature	Date
Checked by:	Lead Engineer	A Knox	Im	17/4/13
Reviewed by:	Project Manager/Director	C P Kroeger	Xhun	17/4/13
Accepted by:	Client Authorised Representative	T Wilkinson		
Accepted by:	Client Authorised Representative			

uSHUKELA HIGHWAY

ANALYSIS OF SITE CATCHMENT HYDROLOGY AND INFLUENCES OF DEVELOPMENT ON CATCHMENT STORAGE AND SURFACE RUN-OFF – REVISION 3

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uSHUKELA HIGHWAY

ANALYSIS OF SITE CATCHMENT HYDROLOGY AND INFLUENCES OF DEVELOPMENT ON CATCHMENT STORAGE AND SURFACE RUN-OFF

REVISION 3

1. INTRODUCTION

1.1. Background

Dube Tradeport, in conjunction with Tongaat Hulett Developments, is proposing the development of a light industrial, office park and retail complex near Tongaat on the KwaZulu Natal north coast.

The development is situated at the northern end of the King Shaka International Airport and is bounded by the N2 freeway and Toll Plaza to the east, uShukela Drive (Watson Highway) to the north, and the Hlawe River in the west.

The development site measures approximately 136ha in extent and is currently under cultivated sugarcane. The overall catchment including areas outside the development and draining to the Hlawe River and the Watson Highway (Tongaat River) is 186ha.

The terrain is relatively gentle for the area, with sub-surface conditions dominated by the Berea Red formation of deep loose sands overlying firm clayey sands, with a smaller portion of the area consisting of the Vryheid Formation of siltstones and sandstones where firm to stiff sandy clays and clayey silts overlay soft siltstone and mudstone, interbedded with sandstone.

Various town planning options have been proposed for the development by town planning consultants, Urban Design, with the preferred option as set out on Dwg No. SK01 Rev 19 (See Annexure E1) dated Jan 2013.

The existing site has been entirely altered, with the primary land use being commercial sugar cane production and offers little biodiversity value. However wetlands specialists view its environmental value in its hydrological processes and the influence this has on the receiving environment. It is considered that the Berea Red formations underlying a high percentage of the site provides substantial storage value for rainfall, which will move through the soil horizons until it reaches the underlying clayey sands, at which point the water moves laterally with the natural slopes, daylighting as seasonal baseflow in the valley lines, driving the surface streams which occur on the site.

The concern of the wetland specialists with the development planned for the site is that the land use changes will intercept rainfall that was previously intercepted by the vegetation or stored in the soil profile, and convert this to surface run-off.

The perceived consequences of this surface run-off is:

- Increased volume and rate of runoff.
- Extensive erosion of the sandy soils.
- The development of erosion gullies and unsightly donga systems.
- Widespread alien plant infestation.
- Exacerbated sediment deposition downstream; and
- Potential flooding of properties downstream.

Approximately 43ha of the site includes wetland and riparian habitat, which will present a challenge to the cost effectiveness of the proposed development if these areas plus buffer areas are not encroached on.

In an effort to satisfy environmental objectives, Tongaat Hulett Developments employed the services of Wetland Consulting Services to investigate a number of alternative scenarios aimed at achieving a potentially viable development.

Their investigations and findings are set out in a report headed "Wetland/Riparian Development Layout Recommendation – uShukela – Watson Highway Development Site", dated May 2011.

Following a meeting on 13 June 2011 it was agreed that a more detailed hydrological assessment and report of how run-off will be dealt with on the site will be carried out by Bosch Stemele that would show:

- Pre-development and post-development run-off for the 1 in 2 year rainfall event for various sub-catchments within the site.
- Indicate the volume of infiltration that will occur within these subcatchments within the site.
- Provide sketch layouts of infiltration measures to be provided on the site.
- Typical details of stormwater dissipation structures.

1.2. Objective

The objective of the analysis will be to show how the mitigating measures can be used to:

- Manage the post-development hydrology to within 10% variance of the pre-development scenario.
- Retain upland catchment infiltration, such that seepage to central stream channels and wetlands are maintained.
- Prevent any further soil erosion from the site.



1.3. Proposed Stormwater Controls on the Site

1.3.1. Individual Sites

In terms of the conditions of establishment, these will be governed to a maximum coverage of 60% of the site area. The remainder of the site area will be taken up by parking and soft landscaping. These wil be split on approximately 30% coverage for parking and 10% for soft landscaping. All roof run-off will be directed to soakaways, sized in accordance with eThekwini Municipality's requirements of one cubic metre of clear volume to drain every 40 square metres of roofed area on the site.

All paved areas on the sites will be required to make use of "permeable paving" which provides a built-in infiltration function as well as an attenuation function.

All potential stormwater is therefore "attenuated" and encouraged to infiltrate the sub-soils in either permeable paving or soakaways.

Any overflows from large storm events will be piped down embankment slopes and discharged at natural water course levels or piped to the formalised stormwater reticulation within the road reserves.

A series of stormwater attenuation structures are proposed throughout the development, within the public open spaces, to deal with excesses from large storms.

Energy dissipaters will be utilised to discharge stormwater at the natural water course level, reducing the possibility of scour. Typical examples of energy dissipaters are enclosed as Annexure E2. The type and dimension of these will depend on storm flows and grade of discharged flow.

1.3.2. Road Reserves

Road reserves will consist of a trafficable impermeable surface making up approximately 50% of the road reserves, with the balance of the reserve consisting of soft landscaping (50%).

Stormwater run-off from the impermeable road surfaces will be contained in a formalised stormwater "swale" system from where it will drain to attenuation/infiltration structures located at strategic points within the "open spaces" within the development.

Energy dissipaters as detailed in Annexure E2, will be utilised on the discharge outlets from the stormwater system and attenuation/infiltration structures.



2. APPROACH AND METHODOLOGY

Catchment hydrology is not an exact science and the results are subject to the assumptions made and the analytical technique used.

For consensus of the results, agreement between the parties on the main factors that effect surface run-off and infiltration must be reached. These main factors are:

- Analytical methods (computer simulation model).
- Sub-catchment boundaries.
- Hydrological soil group for the sub-catchments.
- · Land cover classification within the sub-catchment.
- Overall (SCS) Curve Number for estimating run-off.
- Design rainfall and intensity distribution type.

This report sets out the analytical methods used and assumptions made for each of the main factors.

2.1. Analytical Method

To determine rainfall run-off hydrographs and the initial abstraction or the amount of water before run-off, such as infiltration or rainfall interception by vegetation, use is made of the Autodesk Storm and Sanitary Analysis software utilising the US Environmental Protection Agency's (EPA) SWMM modelling engine. For modelling purposes the SCS runoff curve number method was utilised.

A separate model utilising the Visual SCS computer software was developed and is used for comparative purposes in this report. This software is an adaption of the widely used SCS method by the School of Bio-Resources Engineering and Environmental Hydrology of the University of KwaZulu Natal, which makes use of local rainfall records and intensity distribution types as well as local catchment parameters.

The SCS method is an empirical relationship of estimating initial abstraction and run-off as a function of soil type and land use.

The rainfall-run-off relationship incorporates an initial abstraction, direct run-off and actual retention in the soils. These are calculated making use of a run-off curve number (CN), which is an empirical parameter developed from analysis of run-off from small catchments (generally less than 1 mile²) and hill slope plots monitored by the USDA Natural Resources and Conservation Services.

The curve number CN combines hydrologic soil group factors and land use factors.



In view of the concern related to the amount of surface run-off related to the change in land use and the amount of infiltration taken into account during both the pre-development and post-development scenarios, we believe the SCS method of analysis ideally suits this application.

2.2. Sub-Catchment Boundaries

The accompanying Figure 1, Annexure E3, shows the sub-catchment boundaries used in the analysis for the pre-development scenario.

The surface drainage of the development can be split into three distinct areas.

The majority of the development area drains to the west to the Hlawe River which flows in a northerly direction along the western boundary of the development. This catchment area comprises moderately steep topography, with narrow, well-defined drainage lines, with permanent sub-surface seepage zones and seasonal surface flow.

The catchment is further divided into a number of sub-catchments which have been designated the suffix HN (Hlawe North), HC (Hlawe Central) and HS (Hlawe South), all of which discharge to the Hlawe River through catchment H1.

The balance of the development drains northwards towards the Watson Highway and the Tongaat River.

There are two distinctive catchments in this drainage zone, designated WH1 and WH2.

These comprise gently rolling hills with flatter/plainer topography, with broad, poorly defined drainage and seepage areas.

A breakdown of each catchment and sub-catchment area is given in Table 1a -Hlawe River and 1b - Tongaat River.



Table 1a: Hlawe River Pre-development Sub-Catchment Areas

SUB-CATCHMENT	AREA Ha		
Hlawe River			
H1	17.33		
HC1	10.01		
HN1	11.93		
HN2	19.00		
HN3	5.27		
HN4A	3.65		
HN4B	10.14		
HS1	19.94		
HS2	9.21		
Sub-15 (Part of HN4)	6.52		
HN4	20.53		
Sub-18 (Part of HN4)	2.66		
Sub-19 (Part of HN4)	3.28		
Total towards Hlawe River	139.47		

Table 1b: Tongaat River Pre-development Sub-Catchment Areas

SUB-CATCHMENT	AREA Ha	
Tongaat River		
WH1	8.58	
WH1A	3.73	
WH2	5.60	
WH2A	15.85	
WH3	3.72	
WH4	5.09	
WH5	3.61	
Total towards Watson Highway	46.18	
Combined catchment area	185.65	

Hydrological Soils Groups 2.3.

Use is made of TGC Engineers, Preliminary Geotechnical Investigation Report dated September 2009 to identify hydrological soils groups falling within the sub-catchment boundaries.

The overlay of sub-catchments on the geology and associated soils is shown in Figure 2, Annexure E3.



There are two distinct formations that cover the area under investigation, these are:

- Berea Red Formation, which are characterised by:
 - Generally 2.0m 4.0m of loose sands overlying firm clayey soils.
 - The upper sands will be moderately to highly permeable.
 - The sands and clayey sands are highly erodible.
 - Perched water tables are common in the sands overlying the less permeable clayey sands.
- Vryheid Formation Siltstone/Sandstone, which are characterised by:
 - Generally 0 3.0m of soft/firm/stiff sandy clays and silty clays overlying soft through to hard dolerite bed rock.
 - Water movement through the soil is restricted to very restricted.
 - The cohesive soils of this formation are expected to have a low to moderate erosion potential.

Using the NRCS Hydrologic Soil Group Definitions, the above two sub-soil formations fall within the following Hydrologic Soil Group:

Sub-Soil Formation	Group
Berea Red Formation:	Α
Vryheid Formation:	D

A breakdown of hydrologic soil group area for each sub-catchment and cumulative catchment area is given in Table 2:



Table 2: Hydrologic Soil Group Areas by Pre-Development Sub-Catchment

SUB-CATCHMENT	SUB- CATCHMENT	AREA BY HYDROLOGICAL SOILS GROUP (Ha)			
	AREA (Ha)	Α	D		
H1	17.33	4.47	12.86		
HN1	11.93	9.02	2.91		
HN2	19.00	16.01	2.99		
HN3	5.27	3.53	1.74		
HN4	20.53	18.73	1.80		
HN4A	3.65	100.00	0.00		
HN4B	10.14	8.88	1.26		
HC1	10.01	7.80	2.21		
HS1	19.94	13.00	6.94		
HS2	9.21	7.58	1.63		
Sub15 (part of subcatchment HN4)	6.52	5.95	0.57		
Sub18 (part of subcatchment HN4)	2.66	2.43	0.23		
Sub19 (part of subcatchment HN4)	3.28	2.99	0.29		
Total towards Hlawe River	139.47	200.39	35.43		
WH1	8.58	6.36	2.22		
WH1A	3.73	2.65	1.08		
WH2	5.6	5.05	0.55		
WH2A	15.85	15.41	0,45		
WH3	3.72	3.72	0.00		
WH4	5.09	5.09 0.			
WH5	3.61	3.61	0.00		
Total towards Watson Highway	46.18	41.89	4.30		
TOTAL SUB-CATCHMENTS	185,65	242.28	39.73		

2.4. Land Cover Classification

Use is made of Urban Design's town planning layout option shown on Dwg. No. Sk 01 Rev 19.

The town planning layout has been overlaid with the sub-catchment boundaries and the Hydrological Soils Groups and is shown in Figure 3, Annexure E3.

A breakdown of land use in accordance to sub-catchment boundaries and hydrological soils groups is attached as a number of spreadsheets enclosed as Annexure E4.

2.5. SCS Curve Number

Use is made of the SCS Urban Hydrology for Small Watersheds, 2nd Ed., June 1986, SCS Curve Number Table for various land uses, see Table 3.



Table 3: SCS Curve Numbers

Land Use	Treatment / Practice / Description	Hydrological Condition	Hydrological Soil Group						
	Tradition 1 Tradition 2 Society Business	Hydrological Colldition	A A/B B B/C C C/D						D
Fallow	Straight Row		77	82	86	89	91	93	94
Row Crops	Straight Row	Poor	72	77	81	85	86	90	91
	Straight Row	Good	67	73	78	82	85	87	89
	Planted on Contour	Poor	70	75	79	82	84	86	88
	Planted on Contour	Good	65	70	75	79	82	84	86
	Conservation Structures	Poor	66	70	74	77	80	81	82
	Conservation Structures	Good	62	67	71	75	78	80	81
Garden and Truck Crops	Straight Row	Good	45 68	56 71	66 75	72 79	77 81	80 83	83 84
Small Grain	Straight Row	Poor	65	71	76	80	84	86	88
Oman Gran	Straight Row	Good	65	69	75	79	83	85	87
	Planted on Contour	Poor	63	69	74	79	82	84	85
	Planted on Contour	Good	61	67	73	78	81	83	84
	Planted on Contour – winter rainfall region	Good	63	66	70	75	78	80	81
	Conservation Structures	Poor	61	67	72	76	79	81	82
	Conservation Structures	Good	59	65	70	75	78	80	81
Close seeded	Straight Row	Poor	66	72	77	81	85	87	89
legumes or rotational meadow	Straight Row	Good	58	65	72	75	81	84	85
Totalional meadow	Planted on Contour	Poor	64	70	75	80	83	84	85
	Planted on Contour	Good	55	63	69	74	78	81	83
	Conservation Structures	Poor	63	68	73	77	80	82	83
	Conservation Structures	Good	51	60	67	72	76	78	80
Sugarcane	Straight row, trash burnt		43	55	65	72	77	80	82
	Straight row, trash mulch	4	43	56	66	72	77	80	83
	Straight row, limited cover	Q.	67	73	78	82	85	87	89
	Straight row, partial cover	1 4	49	60	69	73	79	82	84
	Straight row, complete cover	16	39	50	61	68	74	78	80
	Planted on contour, limited cover		65	70	75	79	82	84	86
	Planted on contour, partial cover	172	25	46	59	67	75	80	83
	Planted on contour, complete cover		6	14	35	59	70	75	79
Pasture or veld		Poor	68	74	79	83	86	88	89
(range)	•	Fair	49	61	69	75	79	82	84
	-	Good	39	51	61	68	74	78	80
	Planted on Contour	Poor	47	57	67	75	81	85	88
	Planted on Contour	Fair	25	46	59	67	75	80	83
	Planted on Contour	Good	6	14	35	59	70	75	79
Irrigated pasture		Good	35	41	48	57	65	68	70
Meadow	in a contract of the contract	Good	30	45	58	65	71	75	81
Woods		Poor	45	56	66	72	77	80	83
Woods		Fair	36	49	60	68	73	77	79
		Good	25	47	55	64	70	74	77
Scrub	Brush – winter rainfall region		28	34	44	53	60	64	66
Orchards	Winter region, understory of crop cover	Good	39	44	53	61	66	69	71

March 2013

Table 3: SCS Curve Numbers (contd.)

Forests / plantations	Humus depth 25mm compactness:	compact		52	62	72	77	82	85	87
S (1911) (1911) (1911)	Moderate		46	58	68	73	78	82	85	
		Loose/friable		37	49	60	66	71	74	77
	Humus depth 50mm compactness:	compact		48	58	68	73	78	82	85
		Moderate		42	54	65	70	75	78	81
		Loose/friable		32	45	57	62	67	71	74
	Humus depth 100mm compactness:	compact		41	53	64	69	74	77	80
		Moderate		34	47	59	64	69	72	75
		Loose/friable		23	37	50	56	61	64	67
	Humus depth 150mm compactness:	compact		37	49	60	66	71	74	77
		Moderate		30	43	56	61	66	69	72
		Loose/friable		18	33	47	52	57	61	65
Urban/suburban land uses	Open spaces, parks, cemeteries		Good (75% + grass cover)	39	51	61	68	74	78	80
iand uses			Fair (50 – 75% grass cover)	49	61	69	75	79	82	84
	Commercial/business areas		85% impervious	89	91	92	93	94	95	95
	Industrial districts		72% impervious	81	85	88	90	91	92	93
	Residential lot size: 500m²		65% impervious	77	81	85	88	90	91	92
	1000m ²		38% impervious	61	69	75	80	83	85	87
	1350m ²		30% impervious	57	65	72	77	81	84	86
	2000m ²		25% impervious	54	63	70	76	80	83	85
	4000m ²		20% impervious	51	61	68	75	78	82	84
	Paved parking lots, roofs, etc.			98	98	98	98	98	98	98
	Streets/roads: Tarred, with storm se	ewers, kerbs		98	98	98	98	98	98	98
	Gravel			76	81	85	88	89	90	91
	Dirt			72	77	82	85	87	88	89
	Dirt – hard sur	face		74	79	84	88	90	91	92

From Table 3, the following CN numbers have been adopted for the analysis:

Table 4: Adopted Curve Numbers

LAND USE DESCRIPTION	HYDROLOGICAL SOILS GROUP			
LAND USE DESCRIPTION	Α	D		
Buildings (roof run-off to soakaways)	71	84		
Trafficable Surfaces (Impermeable)	98	98		
Soft Landscaping	39	80		
Open Space	39	80		
Permeable Paving	25	32		
Sugarcane	25	83		

Current documentation does not provide for CN values for permeable paving. However, studies of long term stormwater quantity and quality performance of permeable pavement systems have been carried out by the Centre for Water and Watershed Studies at the University of Washington and by the Biological and Agricultural Engineering Department of North Carolina State University.



Both studies have found that with the use of permeable paving, surface run-off comprises between 0% and 2% of the total outflow volume for storms with rainfall depths less than 50mm.

Based on curves developed for Typical Run-off depth for various CN numbers, see Annexure E, we propose using a CN value of 25 for the Berea Red soils (0% run-off) and 32 for the Vryheid formation soils for permeable paving (2mm or about 2% of a 1 in 2 year storm depth).

For the soakaways for roof run-off the 1m³ storage per 40m² of roof area equates to a 25mm rainfall depth. There is then a certain amount of soil infiltration within the soakaways during a one-day design rainfall event. This is estimated to be 27mm for the Berea Red soils and 9mm for the Vryheid Formation soils. Total infiltration in the soakaways is therefore estimated to be 52mm in the Berea Red soils and 34mm in the Vryheid Formation soils.

Using the Typical Run-off depth for various CN numbers, (Annexure E5), the CN for roof run-off to soakaways is 71 for the Berea Red soils and 84 for the Vryheid Formation soils.

2.6. Design Rainfall and Intensity Distribution Type

2.6.1. Rainfall Depth

It had been agreed with Wetland Consulting Services that the analysis of run-off and rainfall interception/infiltration be based on the 1 in 2 year design storm. This has been adopted because it is felt that a 1 in 2 year rainfall event is more likely to represent the more frequent rainfall events which impact on the normal functioning of the wetlands.

Design Storm Frequencies for the design of the stormwater system within the development will however be based on the 1 in 5 year rainfall event, with critical points, such as road culverts, natural drainage lines or attenuation structures being designed for the 1 in 20 year rainfall event, and checked for impacts of the 1 in 100 year rainfall event.

Use is made of data from the SAWB Rainfall Station No. 241 302 (Frasers), which is located approximately 7.6km from the site and at a similar altitude and distance from the coast.

The 1 in 2 year, 24 hour Design Rainfall Depth adopted for the analysis is 84mm.

A breakdown of available rainfall stations in the area and the adopted station No. 241 302 24 hour Design Rainfall Depths is attached as Annexure E6.



2.6.2. Time Distribution of Design Rainfall Intensity

The design rainfall intensity distribution over a day varies regionally in Southern Africa. Four distribution "types" have been identified over the region, with Type 4 producing the highest intensities and Type 1 the lowest intensities.

A map showing the delineation of the distribution types is attached in Annexure E6.

A Type 2 distribution has been used in the analysis of the uShukela site.

ANALYSIS

Two scenarios, pre-development and post-development, have been analysed, making use of assumptions set out in Section 2.

A summary of input data in the analysis & output results using the SWMM model is given in Annexure E7.

A summary of the results utilising the Visual SCS software for the catchments draining to the Hlawe River is shown in Annexure E8.

4. DISCUSSION OF RESULTS

4.1. Areas Draining to Hlawe River

From the Visual SCS software results, for those catchments draining to the Hlawe River (H1) the total run-off volume has increased from the predevelopment scenario of 11 600m³ to 24 200m³, with the balance, 12 600m³ needing to be taken up in the attenuation structures. The SWMM model shows and increased runoff figure from 19 331 to 36498m³ for the 1:2 year storm. Considering that the total precipitation volume is 156 000m³ in the catchment area and considering that stormwater runoff calculation is not an exact science, the results are considered reasonable.

The additional volume required for attenuation can be accommodated at the outlets of each of the sub-catchments draining this area and retention facilities sized in accordance with the various sub-catchment run-off excesses.

One solution is to design the permeable paving car parks within this catchment to accept ponding of up to a 60mm depth for the larger storm events, or a combination of this measure, which accepts some ponding, say 30mm and some attenuation, say 6 300m³. This will assist in increasing catchment interception/infiltration.



4.2. Areas Draining to the Tongaat River

Using the Visual SCS software for catchments WH1 and WH2, the total run-off volume has increased from the pre-development scenario of 800m³ for both catchments to 2300m³ and 2 200m³ respectively. Based on this attenuation structures will be required at the sub-catchment outlets of both catchments to attenuate discharge peaks to within to within 10% of pre-development levels.

For catchment WH1 interception/infiltration volumes have decreased from 9 600m³ at pre-development stage to 8 000m³ at post-development stage, a 1 600m³ or 17% decrease.

Again, by designing the permeable paving in the sub-catchment to accept some ponding, allowing for further storage from road reserves in roadside swales and attenuation of roof runoff in soakaways, the target of 10% is achievable.

For catchment WH2, interception/infiltration volumes have decreased from 16 900m³ at pre-development stage to 15 600m³ at post-development stage, a 1 400m³ or 8% decrease, which is within the target of 10%.

4.3. SWMM model results

From the results of the rainfall interception/infiltration in the SWMM model (Annexure E7), it can be seen that interception/infiltration has decreased from a pre-development volume of 43 312m³ to 32 770m³ for a 25mm precipitation storm event via normal infiltration. This is a decrease of 10 542m³, or 25% of pre-development volumes. This is outside of the target of 10%. For the 1:2 post-development scenario, the infiltration ratio decreases (124 430m³ vs 68 808m³). However, taking into consideration further infiltration via the proposed swales in the road reserves and runoff storage from roofs into soakaways, infiltration can be increased to 102589m³ which is within 82% of the pre-development infiltration. Using the attenuation facilities the target of 10% will be achievable.

A summary of the required attenuation facilities from the SWMM model that will be required is as follows (see table 5 below):

Table 5: Attenuation Facility Storage Requirements

Catchment Reference	Volume
RPF4B	1, 449m³
RPF16A	675m³
STR4	4, 015m³
STR2 (two ponds)	9, 037m³
STR9	2, 156 m³
Total attenuation requirement	17, 332m³



5. CONCLUSIONS

Based on the analysis carried out, the following conclusions can be made:

- Making use of the various proposed interception/infiltration measures over the development site will not attenuate the post-development run-off peaks to within 10% of the pre-development peaks and attenuation facilities will be required on each of the catchments.
- 5.2 The position of these attenuation facilities would be ideally located in the open space wetland areas.

6. **RECOMMENDATIONS**

Based on the review of the analysis, the following is recommended:

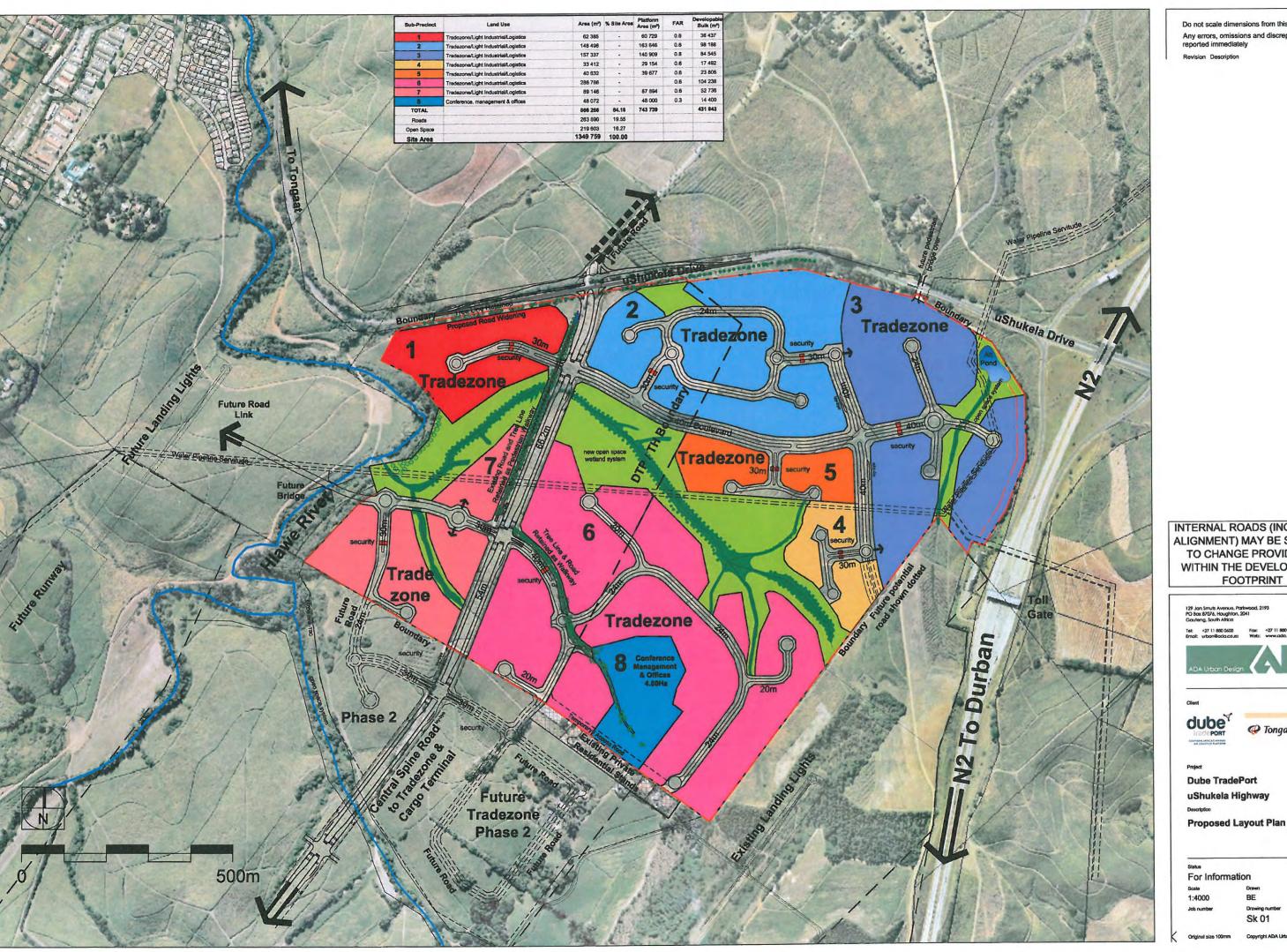
- 6.1 Attenuation structures be considered at the discharge point of each subcatchment or where ideally located and be designed to attenuate the postdevelopment discharge peak to within 10% of the pre-development discharge peak.
- 6.2 Permeable paving throughout the development be designed to allow for ponding of up to 30mm for all design rainfall depths greater than 50mm,
- 6.3 Soakaways to intercept and retain roof runoff be installed as per the eThekwini Municipality's requirements, and
- 6.4 Swales be installed in road reserves to intercept and retain runoff from impermeable road surfaces.

Prepared by L Streicher Bosch Stemele (Pty) Ltd



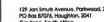
ANNEXURE E1:

Town Planning Layout Dwg No. Sk01 Rev. 19



Do not scale dimensions from this drawing Any errors, omissions and discrepancies to be reported immediately

INTERNAL ROADS (INCLUDING ALIGNMENT) MAY BE SUBJECT TO CHANGE PROVIDED IS WITHIN THE DEVELOPMENT **FOOTPRINT**











Dube TradePort uShukela Highway

ANNEXURE E2:

TYPICAL STORMWATER ENERGY DISSIPATERS PLACED AT OUTFALLS TO STORMWATER SYSTEMS

Dwg Nos.:

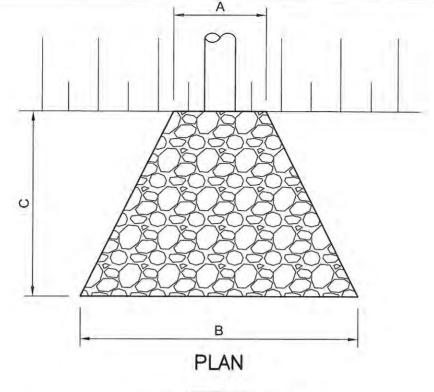
S226

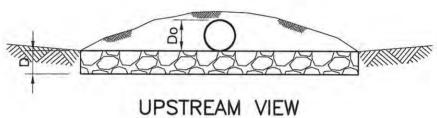
S227

S228

S229

TYPICAL DETAILS OF PERMEABLE PAVING
DWG. Nos.:
ANNEXURE E2-1 AND
243/203/10





TYPE 1 STONE

 $D_{50} = 0.0883 \ Q^{1.333}/D_0^{2.333}$ (m) $C = 3.0792 \ Q/D_0^{1.8} + 2.4384$ (m)

MINIMUM DOWNSTREAM DEPTH D = 0.5 D_0 F_0 D = 0.5 D_0 F_0 D = 0.5 D_0 F_0 C D_0 D_0 D

LIST OF SYMBOLS

D - BED THICKNESS

 D_{50} - 50% FINER PER MASS

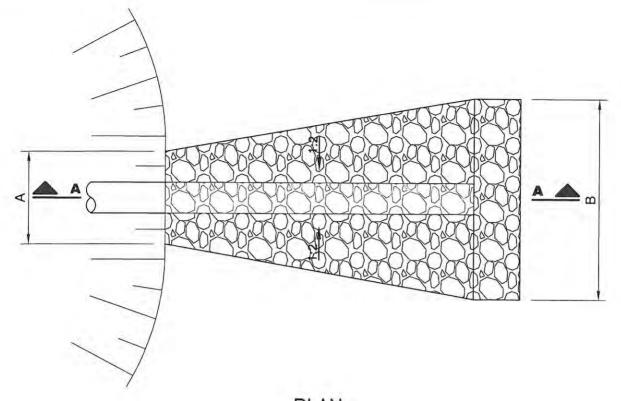
Do - VERTICAL CULVERT DIMENSION

F₀ - FROUDE NUMBER
A, B, C - SEE TYPES 1 TO 3
Y₃ - DOWNSTREAM DEPTH

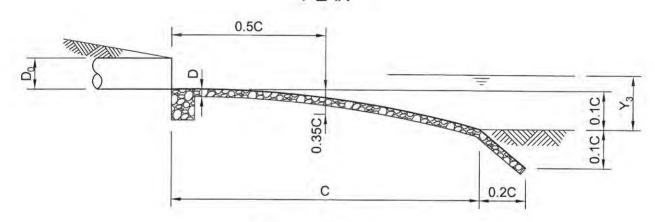
WATERWAYS STONE STILLING BASINS TYPE 1



		-
DESIGNED A.M.K.	APPROVED	
DRAWN H.W.		
CHECKED	DRAWING No.	REV
SCALE NTS	200000000000000000000000000000000000000	
DATE SEPT. 2011	S 226	A



PLAN



SECTION A-A

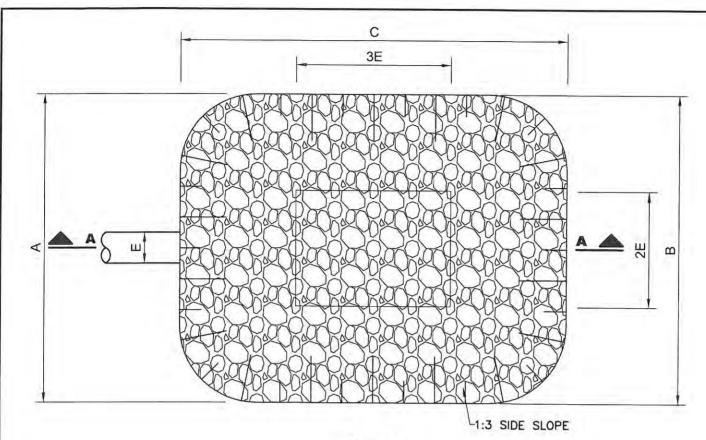
TYPE 2 STONE

D ₅₀	=	0	.0707	$Q^{1.333}/D_0^{2.33}$	(m)
C			Do		(m)
Α	=	3	Do		(m)
В	=	5	Do		(m)
D	=	2	D ₅₀		(m)

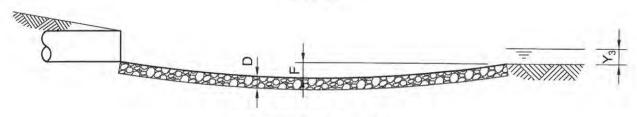
WATERWAYS STONE STILLING BASINS TYPE 2



		-
DESIGNED A.M.K.	APPROVED	
DRAWN H.W.		
CHECKED	DRAWING No.	REV
SCALE NTS		
DATE SEPT. 2011	S 227	Α



PLAN



SECTION A-A

TYPE 3(a) STONE SHALLOW STILLING BASIN

TYPE 3(b) STONE
DEEP STILLING BASIN

$$D_{50} = 0.0552 Q^{1.333}/D_0^{2.333}$$
 (m)
 $C = 3D_0 + 6F$ (m)
 $A = 2D_0 + 6F$ (m)

$$D_{50} = 0.0362 Q^{1.333}/D_0^{2.333}$$
 (m)

$$B = 2D_0 + 6F$$
 (m)
 $D = 2D_{50}$ (m)

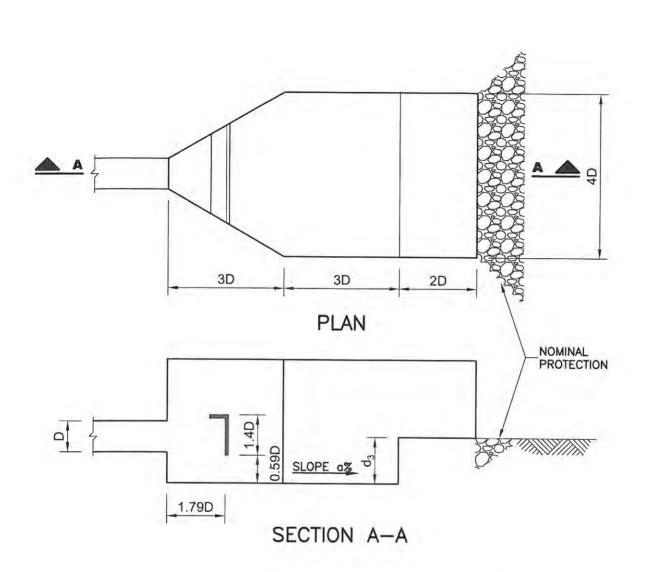
$$F = D_0 \tag{m}$$

_	_	D0
F	=	$0.5D_0$

WATERWAYS STONE STILLING BASINS TYPE 3



		-
DESIGNED A.M.K.	APPROVED	
DRAWN H.W.		
CHECKED	DRAWING No.	REV
SCALE NTS		100
DATE SEPT. 2011	S 228	A



a%	d ₃ /D
1	0.59
2	0.59
4	0.71
8	0.92
16	1.25
32	1.92
64	NOT RECOMMENDED

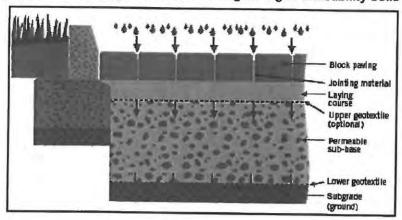
TYPE V STILLING BASIN



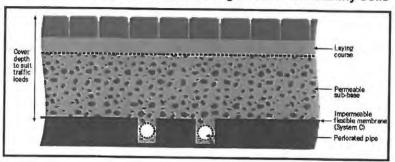
		WHELE
DESIGNED A.M.K.	APPROVED	
DRAWN H.W.		
CHECKED	DRAWING No.	REV
SCALE NTS	A STATE OF THE STA	1
DATE SEPT. 2011	S 229	A

Annexure E2-1

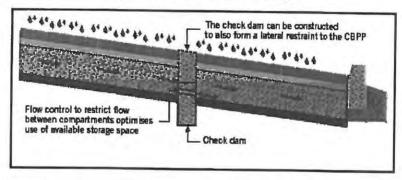
Typical Section Through Permeable Paving on High Permeability Soils

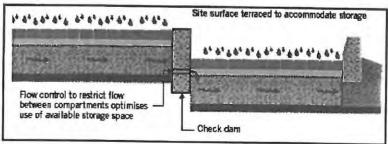


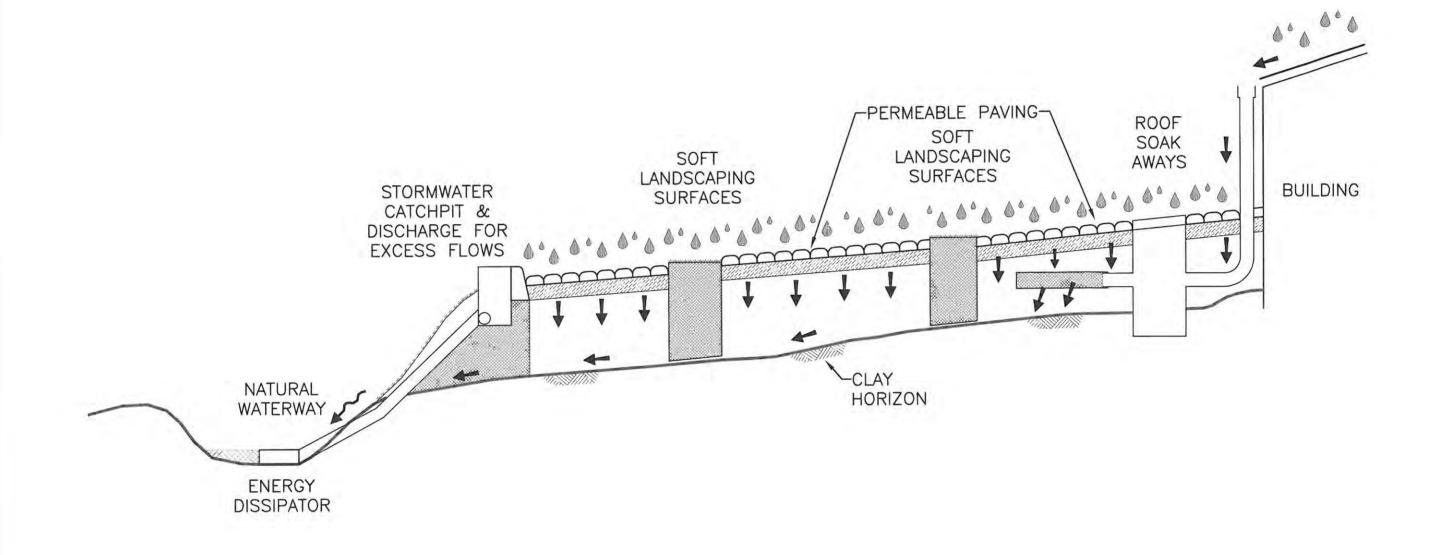
Typical Section Through Permeable Paving on Low Permeability Soils



Typical Details on Sloping Sites







NTS

SEPT. 2011

BOSCH STEMELE

DRAWING NUMBER

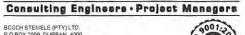
				DESIGNED A.M.K	APPROVED	SCALE
	17 =			DRAWN H.W.	PROJECT MANAGER	
				CHECKED		DATE SEP
					DIRECTOR	JEI
0	FOR INFORMATION.	20/09/11		COPYRIGHT AND USE C	OF THIS DRAWING IS RESERVED	BY BOSCH
REV.	DESCRIPTION	DATE	APPR'D	COLUMNIA THE SEE S	1110-1210-1110-1	101.0.3334

TONGAAT HULETT DEVELOPMENTS/ DUBE TRADEPORT uSHUKELA HIGHWAY

TYPICAL SECTION THRO' DEVELOPMENT PLATFORM

BCSCH STEMELE (PTY) LTD.
P.O.BOX 2009, DURBAN, 4000.
1 HOLWOOD PARK, S. CANECASTE ROAD,
LA LUCIA RIDGE OFFICE ESTATE, K.WAZULU-NATAL, RSA
TELEPHONE: (031) 535 6000 TELEFAX: (031) 535 6011
E-mail: Lekzn@boschskimele.co.za A3 REVISION O 243/203/010





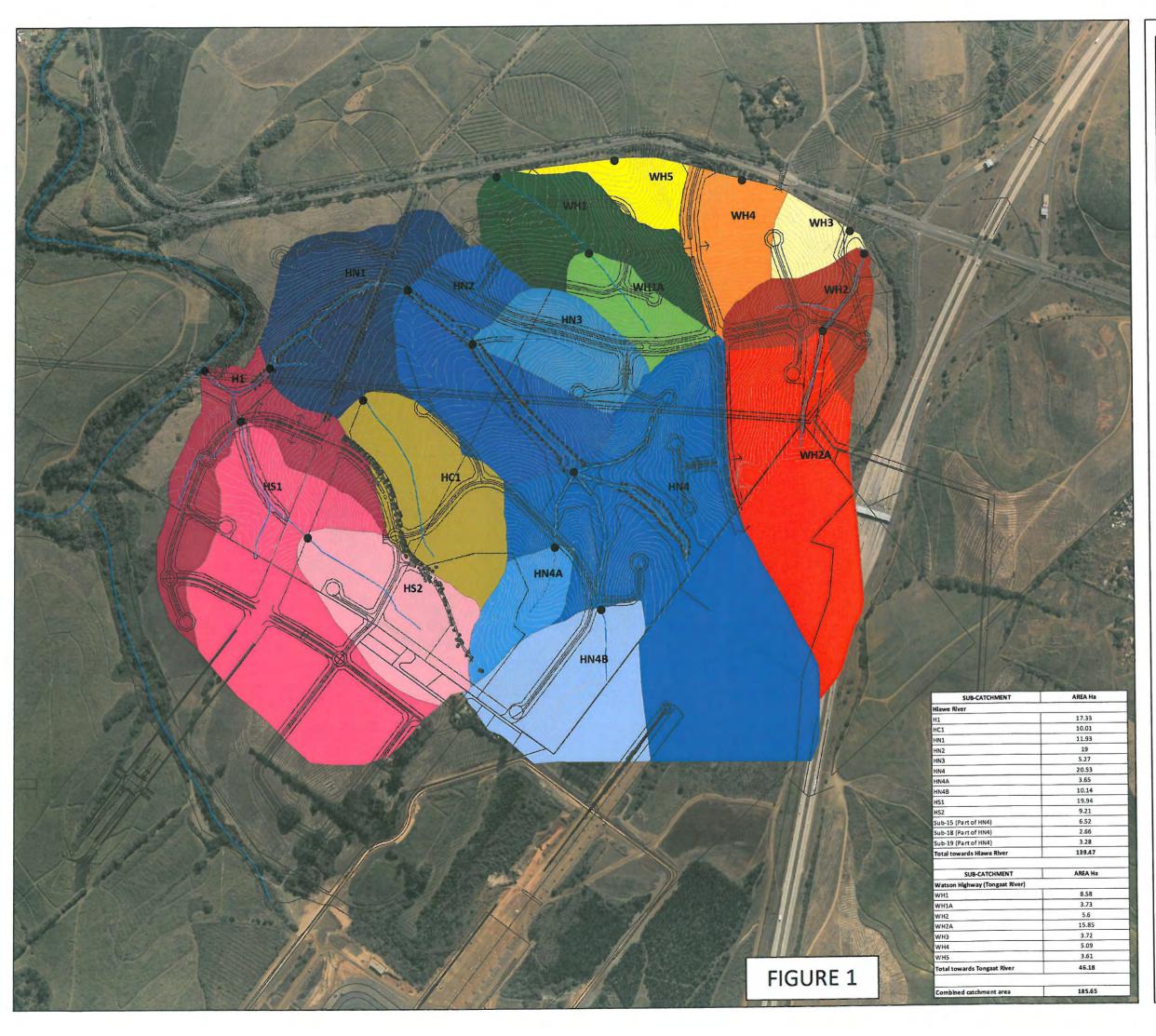


ANNEXURE E3:

FIGURE 1 - SUB CATCHMENT BOUNDARIES

FIGURE 2 – OVERLAY OF SUB-CATCHMENT BOUNDARIES ON GEOLOGY AND ASSOCIATED SOILS

FIGURE 3 – TOWN PLANNING LAYOUT, SUB-CATCHMENT BOUNDARIES AND SOILS FORMATIONS





PROJECT No. 0243-203

TONGAAT HULETT DEVELOPMENTS

DUBE TRADE PORT

uSHUKELA HIGHWAY

VISUAL SCS ANALYSIS
SUB-CATCHMENT BOUNDARIES

MAP REFERENCE No.: 0243-203-007

REVISION No.: 3

REVISION DATE: 09/10/2012

A3 SHEET

SCALE 1:7 500

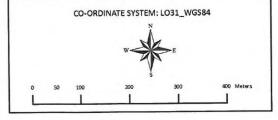
MAP KEY PLAN

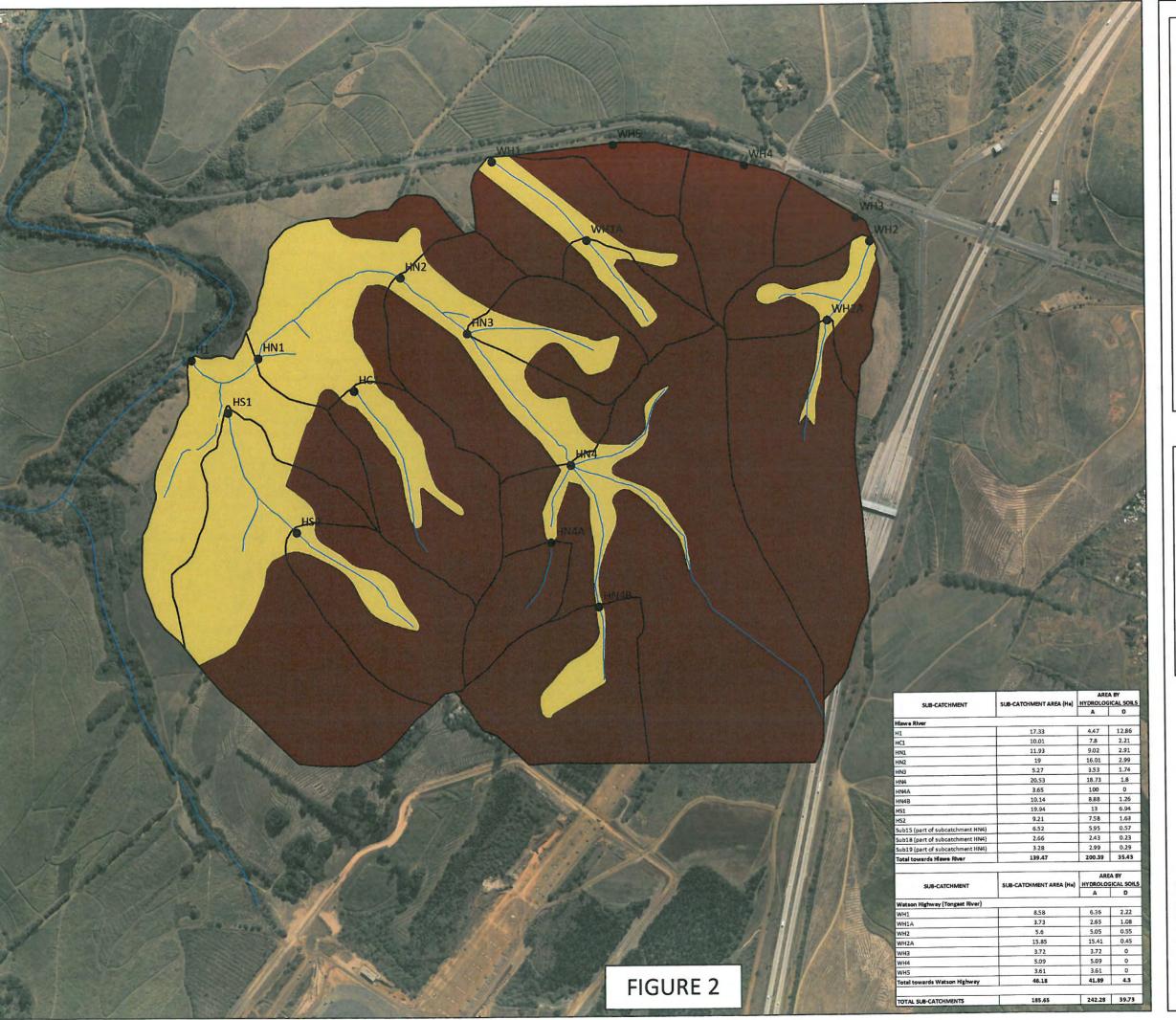


LEGEND

Sub-catchment Nodes Rev 3
 Contours

___ Streamlines







PROJECT No. 0243-203

TONGAAT HULETT DEVELOPMENTS

DUBE TRADE PORT

uSHUKELA HIGHWAY

VISUAL SCS ANALYSIS

OVERLAY OF SUB-CATCHMENTS ON
GEOLOGY AND ASSOCIATED SOILS

MAP REFERENCE No.: 0243-203-008 REVISION No.: 2

REVISION DATE: 09/10/2012
A3 SHEET

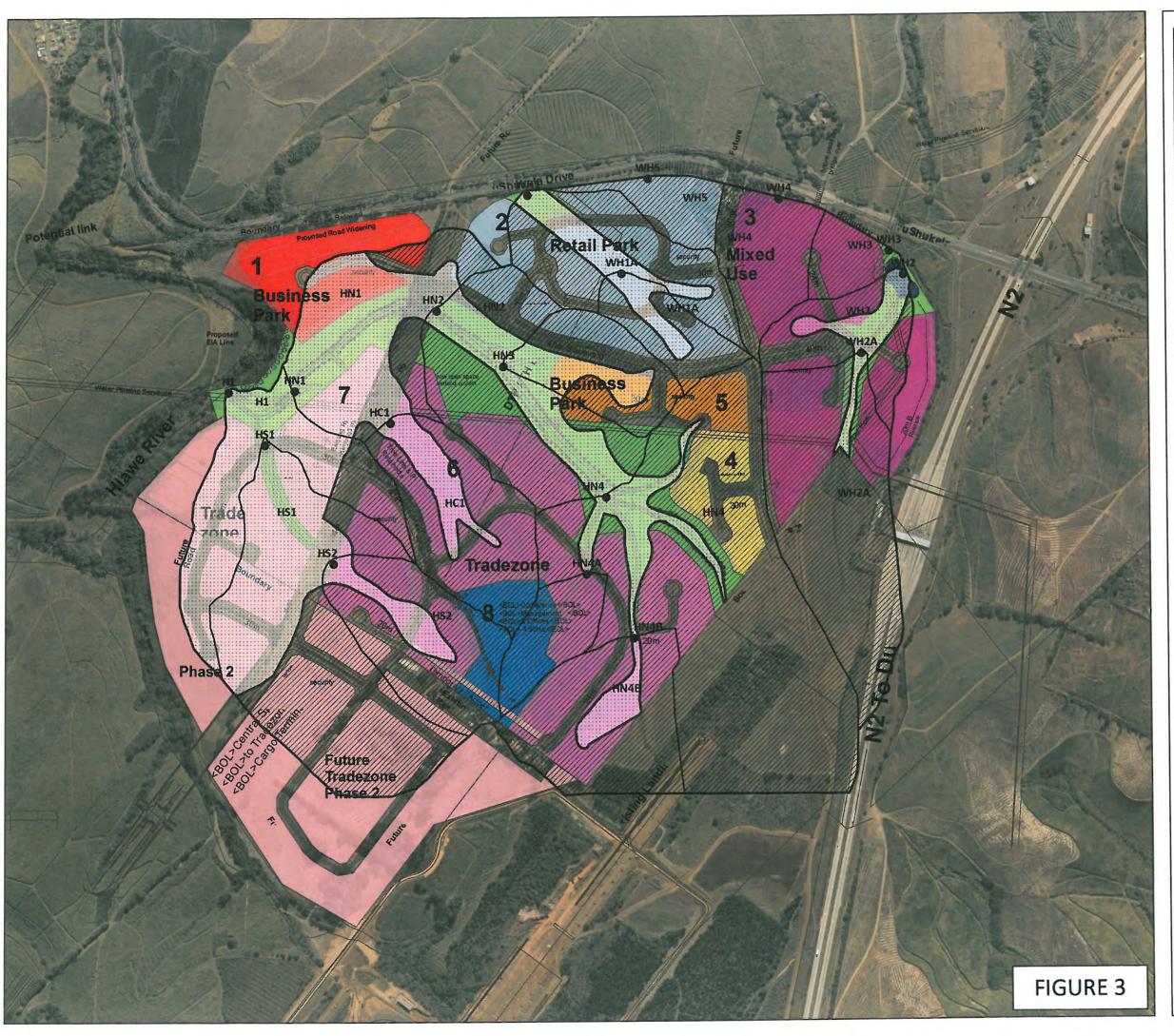
SCALE 1:7 500

MAP KEY PLAN



LEGEND

- Sub-catchment Nodes Rev 3
- ___ Streamlines
- Berea Red within Catchment
 Vryheid Soils within Catchment





PROJECT No. 0243-203

TONGAAT HULETT DEVELOPMENTS

DUBE TRADE PORT

uSHUKELA HIGHWAY

TOWN PLANNING LAYOUT
SUB-CATCHMENT BOUNDARIES
SOILS FORMATIONS

MAP REFERENCE No.: 0243-203-009

REVISION No.: 2

REVISION DATE: 09/10/2012

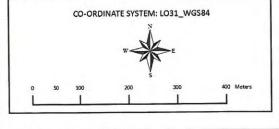
A3 SHEET

SCALE 1:7 500

MAP KEY PLAN



LEGEND Berea Red within Catchment Vryheid Soils within Catchment Contours Sub-Precinct 1_Business Park 2_Retail Park 3_Mixed Use 4_Business Park 5_Business Park 6_Tradezone 7_Tradezone 8_Conference_Mngt_Offices Attenuation Pond Open Space Phase 2



ANNEXURE E4:

BREAKDOWN OF LAND USE AND CN NUMBER IN ACCORDANCE WITH SUB-CATCHMENT BOUNDARIES AND HYDROLOGICAL SOILS GROUPS

E4-1: POST DEVELOPMENT

E4.2: PRE DEVELOPMENT

ANNEXURE E4-1 Post Development

CN for Developed Area Rev 8

		Develo	Developed Area			Road Servitudes		Open Space	Cane Areas	
Sub-Precinct	Gross Area In Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
		%09	10%	30%	1.22	20%	%05	U U	0.00	
1	00'0	0.00	0.00	0.00		0.61	0.61	Rich	000	
7	0.00	0.00	0.00	0.00		0.00			-	
3	00'0	0.00	0.00	0.00		00.0	000			
4	000	0.00	0.00	0.00		0.00	000			
2	00'0	0.00	0.00	0.00		00:00	000			
6	1.22	0.73	0.12	0.37		00.0	00.0			
7	00'0	0.00	0.00	0.00		00.0	000			
8	0.00	0.00	0.00	0.00		000	00.0			
Area Totals	1.22	0,73	0.12	0.37	1.22	0.61	0.00	000	90.0	
Total Soil Type 1	2.44							2000	0000	
% Solit to Total		30.00%	%UU 'S	15,00%		7400 10	, age - 4		***	
CN for use Class		11	000	35		23.00%	25.00%	0.00%	0.00%	100.00%
Composite CN		31.30	10.1	57 .		80	39	339	52	
combosite day		0.17		87.8		24.50	9.75	0.00	0.00	61.25
n soil bescribabili.	VIVIEN POLITIATION	nyrological soil group:	9							,
		Develo	Developed Area			Road Servitudes		Open Space	Cane Areas	
Sub-Precinct	Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
		%09	10%	30%	1.69	20%	20%	0.47	2.64	
H	0:0	0.0	0.0	0.0		0.8	80) 1		
2	0.0	0.0	0.0	0.0		0.0	0.0			
m	0.0	0.0	0.0	0.0		0.0	00			
4	0.0	0.0	0.0	0.0		0.0	0.0			
Ŋ	0,0	0.0	0.0	0.0		0.0	0.0			
9	0.0	0.0	0.0	0.0		0.0	0.0			
7	2.2	1.3	0.2	9.0		0.0	00			
80	0.0	0.0	0.0	0.0		0.0	0.0	1		
Area Totals	2.2	1.3	0.2	0.7	1.7	0.8	8.0	0.5	26	
Total Soil Type 2	7.01									
Catchinent Area		7								
% Split to Total		18.90%	3.15%	9,45%		12.06%	12.06%	6.71%	37.67%	100.00%
CIN for use Class		84	80	32		86	80	08	83	
Composite CN:		15.88	2.52	3.02		11.82	9.65	5.37	31.27	79.52
Total Catchment Area	9.45									
										74 00

HN1 - POST DEVELOPMENT - SPLIT OF AREA FOR CN CALCULATION: Sub Soil Description: Berea Red Formation. Hyrological soil group:

1	Course Acces							2000	200	
Sub-Predict	Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
		809	10%	30%	0.00	%05	%US	900	00.0	
1	KA	0.24	0.04	0.12	000	000	000	anvo	חמים	
2	1.01	0.61	0.10	0.30		900	000	-	201	
m	000	0.00	0.00	00'0		900	000			
Q	00'0	0.00	0.00	0.00		900	000			
נט	00.00	0.00	0.00	0.00		2000	0.00			
9	1.60	0.96	0.16	0.48		900	000			
7	0,00	0.00	0.00	0.00		5000	800			
80	00.00	0.00	0.00	0.00		90.0	000			
Area Totals	3.00	1.80	0.30	0.90	0.00	0.00	000	0000	000	
Total Soil Type 1 Catchment Area	3.00							000	000	
% Split to Total		60.00%	10.00%	3000		2000	, 200			S.C. Aller
CN for use Class		11	39	25		800.0	3.00%	300.0	0.00%	100.00%
Composite CN:		42.60	3.90	7.50		0.00	0.00	000	000	24 00
Sub Soil Description:	Vryheid Formation	Hyrological soil group:	٥							04:00
		Develor	Developed Area			Boad Sanitudae		Canal Canal		_
Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
		%09	10%	30%	1.00	20%	205	0.80	00.00	-200
1	77	1.4	0.2	0.7		20	90	W.004	000	
2	0.0	0.0	0.0	0.0		000				
m	0.0	0.0	0.0	0.0		000	0.0			
4	0.0	0.0	0.0	0.0		0	3 6			
יט	0.0	0.0	0.0	0.0		0.0	200			
9	0.0	0.0	0.0	0.0		00	00			
7	2.8	17.	0.3	0.8		0.0	0.0			
88	0,0	0.0	0.0	0.0		00	000			
Area Totals	5.1	3.1	0.5	1.5	1.0	0.5	5.0	2.8	00	
Total Soil Type 2	8 80							2.3	0.0	7
Catchment Area										
% Split to Total		34.36%	5.73%	17.18%		5,62%	5.62%	31 49%	2000	100 000
CN for use Class		84	80	32		86	80	8	83	20000
Composite CN:		28.86	4.58	5.50		5.51	4.50	25.19	0.00	74.14
Total Catchment Area	11.89									
										37.73

Hyrological soil group:
Berea Red Formation
Sub soil Description:

		Develo	Developed Area			Road Servitudes		Onen Snace	Cana Great	_
Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
		%09	10%	30%	0.91	20%	20%	1.05	00.0	
	0.00	0.00	0.00	0.00		0.46	0.46	200	nain	
7	1.04	0.62	0.10	0,31		000	000			
m	0.00	00.0	0.00	0,00		0.00	000			
4	0.00	00'0	0.00	0.00		00.0	000			
S	130	0.78	0.13	0.39		900	00.0			
9	3.91	2.35	0.39	1.17		0.0	000			
7	000	00'0	0.00	00'0		0.00	000			
80	0.00	0.00	0.00	0.00		0.00	000			
Area Totals	6.25	3.75	0.63	1.88	0.91	0.46	0.46	1 05	50.0	
Total Soil Type 1 Catchment Area	9.11						050	66.4	0000	_
% Split to Total		41.16%	6.86%	20.58%		%66 P	7600 P	21 61%	70000	/000 DOS
CN for use Class		71	39	25		86	39	39	35	700.007
Composite CN:		29.23	2,68	5.15		4.89	1.95	8.35	0.00	52.24
Sub Soil Description:	Vryheid Formation	Hyrological soil group:	٥							
		Develo	Developed Area			Road Servitudes		Onen Space	Cana Arase	_
Sub-Precinct	Gross Area In Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
		20%	15%	35%	•	20%	705	2.04	000	
1	0'0	0.0	0.0	0.0		00	00		ono	
2	0.0	0.0	0.0	0.0		0.0	0.0			
m	0.0	0.0	0.0	0.0		0.0	00			
4	010	0.0	0.0	0.0		0.0	0.0			
ıs	0.0	0.0	0.0	0.0		0.0	0.0			
9	0.0	0.0	0.0	0.0		0.0	0.0			
7	000	0.0	0.0	0.0		0.0	0.0			
8	0.0	0.00	0.00	0.0		0.0	0.0			
Area Totals	0.0	0.0	0.0	0.0	0.0	0.0	0.0	3.0	00	
Total Soil Type 2	3.91									
catchment Area										
% Split to Total		%00.0	0.00%	0.00%		0.00%	0.00%	100.00%	0.00%	100.00%
CN for use Class		84	80	32		86	80	80	83	
Composite CN:		0.00	0.00	0.00		0.00	0.00	80.00	0.00	80.00
Total Catchment Area	13.02									
Total constitutions in the										60.57

HNB - POST DEVELOPMENT - SPLIT OF AREA FOR CN CALCULATION:
Sub Soil Description: Berea Red Formation Hyrological soil group: A

		DEACH	Developed Area			Road Servitudes		Open Space	Cane Areas	
Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	_
		%09	10%	30%	1.19	20%	20%	0.00	000	
1		0.00	00'0	0.00		950	0.59	200	00.0	-
2		0.69	0.12	0.35		000	00.0			
m		00:00	0.00	0.00		00.0	00.0			
4		00.00	0.00	0.00		0.00	000			
S	1001	0.97	0.16	0.48		00.0	00.0			
9		00.00	0.00	0.00		0.00	0.00			
7	3	0.00	0.00	0.00		0.00	0.00			
80		0.00	0.00	00:00		0.00	0.00			
Area Totals	2.76	1.66	0.28	0.83	1.19	0.59	0.59	0.02	0.00	
Total Soil Type 1	3,97									1
Catchment Area										
% Split to Total		41.76%	896'9	20.88%		14.92%	14.92%	0.57%	%00.0	100.00%
CN for use Class		7.1	39	25		98	39	39	25	
Composite CN:		29.65	2.71	5.22		14.62	5.82	0.22	0.00	58.24
Sub Soil Description:	Vryheid Formation	Hyrological soil group:	Q							
		Develo	Developed Area			Road Servitudes		Open Space	Cane Areas	
Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
		%09	10%	30%	0.05	20%	20%	0.86	90.0	
H		0.0	0.0	0.0		0.0	00		8	
2		0.0	0.0	0.0		0.0	0.0			
m		0.0	0.0	0.0		0.0	0.0			
4		0.0	0.0	0.0		0.0	0.0			
Ŋ		9.0	0.1	0.3		0.0	0.0			
9		0.0	0.0	0.0		0.0	0.0			
7		0.0	0.0	0.0		0.0	0.0			
8		0.00	0.00	0.0		0.0	0.0			
Area Totals	1.1	9.0	0.1	0.3	0.0	0.0	0.0	0.9	0.0	
Total Soil Type 2	1.96									1
% Split to Total		32.27%	5.38%	16.14%		1 170/	10TF P	Value 6.4	,,,,,	
CN for use Class		84	80	32		86	80	97 (8.5)	0.00%	100.00%
Composite CN:		27.11	4.30	5.16		1.15	0.94	35.10	0.00	73.76
Total Catchment Area	5.93									

HN4 - POST DEVELOPMENT - SPLIT OF AREA FOR CN CALCULATION: Sub Soil Description: Berea Red Formation Hyrological soil group:

			Part I			DOOR CONTINUES		Canada Canada	Cana Beans	
	Canen Aven in		BO TO ADD TO THE			Board Sci Victoria		Open space	Cane Areas	
Sub-Precinct	Catchment(ha)	Bullding Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
		%09	10%	30%	3.24	20%	20%	1.77	50	
н		0.00	0.00	0.00		1.62	162		Leria .	
7		0.00	0.00	0.00		900	201			
8		0.00	0.00	0.00		000	00.0			
4	100	3.08	0.51	1.54		000	00:00			
'n		0,51	0.08	0.25		60.0	00.0			
9		6.60	1.10	3.30		000	90.0			
7		0.00	0.00	0.00		0.00	8.0			
8	100	1.22	0.20	0.61		0.00	000			
Area Totals	19.01	11.40	1.90	5.70	3.24	1.62	1.62	177	0.54	
Total Soil Type 1 Catchment Area	24.55								t	
% Split to Total		46.46%	7.74%	23.23%		6.60%	6 60%	7 10%	3 10%	700000
CN for use Class		7.1	39	25		86	39	33	25	T00,000,0
Composite CN:		32.98	3.02	5.81		6.47	2,57	2.80	0.55	54.20
Sub Soil Description:	Vryheid Formation	Hyrological soil group:	d :qu							
		Develo	ped Area			Road Servitudes		Open Space	Cane Areas	
Sub-Prednct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
		%09	10%	30%	0,13	20%	%US	2.58	000	
1		0.0	0.0	0.0			0.1			
2		0.0	0.0	0.0		0.0	0.0			
3		0.0	0.0	0.0		0.0	000			
**		0.0	0.0	0.0		0.0	0.0			
2		0.0	0.0	0.0		0.0	0:0			
9		0.8	0.1	0.4		0.0	0.0			
7		0,0	0.0	0.0		0.0	0.0			
8	A STATE OF THE PARTY OF THE PAR	0.0	0.0	0.0		0.0	0.0			
Area Totals	1.3	0.8	0.1	0.4	0.1	0.1	0.1	2.7	0.0	
Total Soil Type 2 Catchment Area	4.12									
% Split to Total		19.02%	3.17%	9.51%		1.59%	1.59%	65.12%	0.00%	100 00%
CN for use Class		84	80	32		86	80	80	83	100.001
Composite CN:		15.97	2.54	3.04		1.56	1.28	52.10	0.00	76.48
Total Catchment Area	28.67									F
	000	-								57.41

`
Hyrological soil group:
Berea Red Formation
Sub Soll Description:

(ha) Soft Landscaping (ha) (ha) 50% 0.05 0.00 0.00 0.00 0.00 0.00 0.00			Develo	Developed Area			Road Servitudes		Open Space	Cana Great	_
Control Cont	Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	,
1.00			%09	10%	30%	0.06	20%	20%	50.0	000	
Control Cont	1		0.00	0.00	00'0		0.03	D.Ca		8	
1.00	2		0.00	0.00	00'0		0.00	0.0			
1.04 0.00	3		0.00	0000	0.00		0.00	0.00			
1,040 0.00	4		0.00	0.00	0.00		0.00	0.00			
1,04 0,05	25	-0.11	0.00	0.00	0.00		0.00	0.00			
3.57 2.14 0.00 0.05 0.05 0.05 0.05 0.00	9		1.04	0.17	0.52		0.00	0.00			
3.56 S.2134 D.214 D.215 D.215 D.2107 D.215 D.215 D.2107 D.215 D.2107 D.215 D.2107 D.215 D.2107 D.215 D.2107	7		0.00	0.00	00:00		0.00	0.00			
3.66 2.34 2.34 0.36 1.07 0.06 0.03 0.03 0.05 0.00 A 1.34 3.9 3.70 3.35 3.3 3.9	8	The second	1.10	0.18	0.55		0.00	0.00			
3.68 S8.215% S.8.215% S.8.22 S.8.215% S.8.215% S.8.215% S.8.215% S.8.215% S.8.22 S.8.215% S.8.215	Area Totals	3.57	2.14	0.36	1.07	0.06	0.03	0.03	0.05	0.00	
Sea 212.5 Sea 21.5	otal Soil Type 1	3.68									-
Cardomenton Hyrological soil group: Developed Area 3.78 7.28 9.8 9.9 3.9 2.5 2.5	% Split to Total		58.21%	9.70%	29.10%		0.76%	%97.0	1.46%	0.00%	100 00%
Carchineritian Hyrological soil group: D Developed Area D D D D D D D D D	CN for use Class		7.1	39	22		86	39	39	25	
Cyryheid Formation Hyrological soil group: D Appen Space Areas Road Servitudes Road Servitudes Care Areas Care Areas Road Servitudes Cone Areas Care Areas Care Areas Road Servitudes Care Areas Care Areas Road Servitudes Care Areas Ca	Composite CN:		41.33	3.78	7.28		0.75	0.30	0.57	00.00	54.00
Groups Area in Carthmentinal Carthmentina Carthmentina Carthmentina Carthmentina Carthmentina Carthmentina Carthmentina Carthmentinal	Soil Description:	Vryheid Formation	Hyrological soil group:	D							
Gross Area in Carchment(ha) Building Area (ha) Soft Landscaping (ha) Permeable Paving (ha) Gross Area (ha) Impermeable Surface (ha) Soft Landscaping (ha) (ha) (ha) 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00			Develo	ped Area			Road Servitudes		Open Space	Cane Areas	
660% 10% 30% - 50% 50% 50% 0.00 0.00 0.00 0.00 0.00 0.	Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0			%09	10%	30%	•	20%	20%	0.00	0.00	
0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0	1	7	0.0	0.0	0.0		0.0	0.0			
0.00 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	2		0.0	0.0	0.0		0.0	0.0			
0.00 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	3		0.0	0.0	0.0		0.0	0.0			
0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	4		0.0	0.0	0.0		0.0	0.0			
0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	2		0.0	0.0	0.0		0.0	0.0			
0.00 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	9		0.0	0.0	0.0		0.0	0.0			
0.00 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	7		0.0	0.0	0.0		0.0	0.0			
0.00 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	8		0.0	0.0	0.0		0.0	0.0			
0.00 #DIV/0! #	Area Totals	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
#DIV/0! #DIV/0	otal Soil Type 2 atchment Area	0.00									
84 80 32 98 80 83 #DIV/0! #DIV/0! #DIV	6 Split to Total		#DIV/0!	#DIV/01	#DIV/0!		#DIV/0!	#DIV/0i	#DIV/0!	#DIV/OI	#DIV/OI
#DIV/01 #DIV/0	CN for use Class		84	80	32		86	80	80	83	
	Composite CN:		#DIV/0!	#DIV/0!	#DIA/01		#DIV/0!	#DIV/0!	#DIV/0!	#DIV/01	#DIV/01
	Catchingua Acco	3.68									

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	A
CALCOLATION:	Hyrological soil group:
THE STATE OF THE STATE OF AREA FOR CHICAGO	Berea Red Formation
THAT I COLUMN	Sub Soil Description:

Sub-Prednat	Gross Area in							and inda	CHAIL CHAIR	
	Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
		%09	10%	30%	0.87	20%	20%	0.00	D 54	
1		0.00	0.00	0.00		0.44	0.04		Local	
2		0.00	00:0	0.00		000	0.00			
ю		0.00	0.00	0.00		0.00	0.00			
Þ		0.00	0.00	0.00		0.00	000			
15	ļ	0.00	0.00	0.00		000	0.00			
9	-100	2,60	0.43	1.30		0.00	0.00			
7	- 484	0.00	0.00	0.00		0.00	0.00			
8	10.00	0,11	0.02	0.05		0.00	0.00			
Area Totals	4.52	2.71	0.45	1.35	0.87	0.44	0.44	0.00	0.54	_
Total Soil Type 1 Catchment Area	5.92									
% Split to Total		45.74%	7.62%	22.87%		7.36%	7.36%	0.00%	9.06%	100 00%
CN for use Class		7.1	39	25		86	39	39	25	20.00
Composite CN:		32.47	2.97	5.72		7.21	2.87	0.00	2,26	53.51
Sub Soil Description:	Vryheid Formation	Hyrological soil group:	0							
L		Develor	Developed Area			Board Constructor		Onen Canco	Cana Assaul	
Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
		%09	10%	30%	0.00	20%	20%	0.00	00.0	
1		0.0	0.0	0.0		0.0	0.0		0	
2		0.0	0.0	0.0		0.0	0.0			
ĸ		0.0	0.0	0.0		0.0	0.0			
4		0.0	0.0	0.0		0.0	0.0			
10		0.0	0.0	0.0		0.0	0.0			
19		0.8	0.1	0.4		0.0	0.0			
7		0.0	0.0	0.0		0.0	0.0			
80		0.0	0.0	0.0		0.0	0.0			
Area Totals	1.3	8.0	0.1	0.4	0.0	0.0	0.0	0.0	0.0	
Total Soil Type 2	1.27									
% Split to Total		29,99%	10.00%	30.00%		0.01%	0.01%	0.00%	0.00%	100 00%
CN for use Class		84	80	32		886	80	80	83	700.007
Composite CN:		50.39	8.00	9.60		0.01	0.00	0.00	0.00	68.00
Total Catchment Area	7.20									1
	70.0	Lond	00000							26.07

CN for Developed Area Rev 8

		Develor	Developed Area			Road Servitudes		Open Space	Cane Areas	
Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
		%09	10%	30%	1,67	20%	206%	000	0000	
T	100	0.00	0.00	0.00	107	0.83	0.83	2000	00'0	
7	Die	0.00	0.00	0.00		00:00	000			
8	8,00	0.00	0.00	0.00		000	85			
¥	N.ED	0.00	0.00	0.00		00:00	000			
S	1000	0.00	0.00	0.00		00:0	000			
9	- Kri	2.87	0.48	1.44		000	200			
7	0000	0.00	0.00	0.00		000	8.6			
8	110	0.36	0.48	0.36		00.0	8.6			
Area Totals	5.99	3.24	96'0	1.80	1,67	0.83	0.83	000	50.00	
Total Soil Type 1 Catchment Area	7.66								000	7
% Solit to Total		42 27%	7973 61	197 E.C.		1000 00	***************************************	, , , ,		
CN for use Class		11	96	37		10.89%	10.89%	0.00%	0.00%	100.00%
Commonly Cal		10	er.	9 !		S .	39	39	25	
composite CN:		29.98	4.89	2.87		10.67	4.25	0.00	0.00	55.65
Sub Soil Description:	Vryheid Formation	Hyrological soil group:	Q					U		
		Develop	Developed Area			Road Servitudes	+0	Open Space	Cane Areas	
Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	_
		%09	10%	30%	0.14	20%	20%	000	2	
1	0.4	0.0	0.0	0.0	110	0.1	0.1		0000	
2	90	0.0	0.0	0.0		00	100			
3	000	0.0	0.0	0.0		0.0	0.0			
*	010	0.0	0.0	0.0		0.0	00			
2	AT	0.0	0.0	0.0		0.0	0.0			
9	- 54	1.2	0.2	9.0		0.0	0.0			
7	241	0.0	0.0	0.0		0.0	0.0			
80	0.0	0.00	0.00	0.0	-	0.0	0.0			
Area Totals	2.0	1.2	0.2	9.0	0.1	0.1	0.1	0.0	0.0	
Total Soil Type 2	217									7
Catchment Area	-									
% Split to Total		56.21%	9.37%	28.11%		3,15%	3.15%	%00'0	0.00%	100 00%
CN for use Class		84	80	32		98	80	80	68	
Composite CN:		47.22	7.50	8.99		3.09	2.52	0.00	0.00	69.33
Total Catchment Area	9.83									
										28.6/

Cane Areas	(ha)	29 67	i circ								196		207 720	35	12.68 42.31		Cano Areas	(ha)	2.53									2.5			24.50% 100.00%	20.67 77.80	
Open Space	-	0.28									0.28		1.85%	36	0.56		Open Space	(ha)	1.24									1.2		177 170	80	9.74	
	Soft Landscaping (ha)	20%	2.13	0.00	00.0	0.00	000	00.0	00.0	0.00	2.13		11 16%	36	4.35			Soft Landscaping (ha)	20%	1.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.1		10 07%	80	8.77	
Road Servitudes	Impermeable Suface (ha)	20%	2.13	0.00	0.00	0.00	0.00	00.0	0.00	0.00	2.13		11.16%	86	10.94		Road Servitudes	Impermeable Suface (ha)	20%	1.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.1		70.61	86	10.75	
	Gross Area (ha)	4.25	4.55								4.25							Gross Area (ha)	2.23	1								2.2					
	Permeable Paving (ha)	30%	00'0	0.00	0.00	0.00	0.00	1.03	0.01	0.43	1,46		7.65%	25	1.91			Permeable Paving (ha)	30%	0.0	0.0	0.0	0.0	0.0	0.4	6.0	0.0	1.3		12.30%	32	3.94	
d Area	Soft Landscaping (ha)	201	00:00	0.00	0.00	0.00	0.00	0.34	0.00	0.14	0.49		2.55%	39	0.99	0	d Area	Soft Landscaping (ha)	10%	0:0	0.0	0.0	0.0	0.0	0.1	0.3	0.0	0.4		4.10%	88	3.28	
Developed Area	Building Area (ha)	%09	0.00	00:00	0.00	0.00	0.00	2.05	0.01	0.85	2.92		15.30%	71	10.87	Hyrological soil group:	Developed Area	Building Area (ha)	%09	0.0	0.0	0.0	0.0	0.0	8.0	1.7	0.0	2.5		24.60%	25	20.66	
	Gross Area in Catchment(ha)		100	No.	100	i.	46.	100	ora-	100	4.86	19.06				Vryheid Formation		Gross Area in Catchment(ha)		- 77	-	11	- 100	- 64	10	0.1	0.0	4.2	10.16				
	Sub-Precinct		1	2	3	47	S	9	7	80	Area Totals	Total Soil Type 1	% Split to Total	CN for use Class	Composite CN:	Sub Soil Description:		Sub-Precinct		1	2	3	4	2	9	7	×0	Area Totals	Catchment Area	% Split to Total	CN for use Class	Composite CN:	

CN for Developed Area Rev 8

		Develo	Developed Area			Road Servitudes		Onen Space	Cana Areas	-
Sub-Prednct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	1
		%09	701	30%	127	7605	, eu	0.00		
1		0.00	0.00	0.00	101	200	30%	97.0	2.12	
2		0.00	0.00	0.00		000	40.0			
3	14/4	0.00	0.00	0.00		000	000			
4		00.00	0.00	0.00		000	200			
S		0.00	0.00	0.00		000	900			
9	100	1.52	0.25	0.76		00:0	0.00			
7	1	0.00	0.00	000		0.00	00'0			
80	141	0.85	0.14	0.00		0.00	0.00			
Area Totals	3,95	2.37	0.40	1.19	1.27	000	0.00	000	2.42	
Total Soil Type 1	7.61					100	***************************************	0.20	777	1
% Solit to Total		31,13%	7,10%	AC 24						
CN for use Class		27	B/CT:C	15,5178		8.35%	8,35%	3.62%	27.79%	100.00%
Composite Chi.		17	50	52		200	39	39	25	
Composite CN:		07.77	2.02	3.89		8.18	3.26	1.41	6.95	47.82
Sub Soil Description:	Vryheid Formation	Hyrological soil group:	Q							
		Develo	Developed Area			Road Servitudes		Open Space	Cane Areas	_
Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	_
		%09	10%	30%	0.10	20%	20%	0.37	500	
1		0.0	0.0	0.0	A COLUMN TWO IS NOT THE OWNER, TH	0.0	00	760	Onio	
2		0.0	0.0	0.0		000	000			
3		0.0	0.0	0.0			000			
4		0.0	0.0	0.0			0.0			
טז		0.0	0.0	0.0		0.0	0.0			
90	0	0.7	0.1	0.4		0.0	00			
7		0.0	0.0	0.0		0.0	00			
8		0.0	0.0	0.0		0.0	00			
Area Totals	1.2	0.7	0.1	0.4	0.1	00	000	700	90	
Total Soil Type 2	1 64					1	25	***	0.0	7
Catchment Area	1.04									
% Split to Total		42.90%	7.15%	21.45%		2.95%	2.95%	22.59%	2000	100 000
CN for use Class		28	80	32		86	80	08	83	Toor
Composite CN:		36,04	5.72	6.86		2.89	2.36	18.08	0.00	71.95
Total Catchment Area	9.25							ŀ		
										52 10

100.001 100.00% 56.11 71.00 59.76 Cane Areas Cane Areas 25 0.00 00'0 00.0 (ha) (ha) 0.00% 83 Open Space Open Space 39 0.00 80 (ha) 000 000 0.00 (ha) 0.0 Soft Landscaping (ha) Soft Landscaping (ha) 39 2.84 7.14% 80 5.71 50% 0.0 0.0 0.0 0.0 0.0 0.0 Impermeable Suface (ha) Impermeable Suface (ha) Road Servitudes Road Servitudes 7.28% 98 7.13 7.14% 98 7.00 50% 0.00 0.00 0.00 0.00 0.00 0.00 0.00 50% 0.0 0.0 0.0 0.0 0.0 0,30 Gross Area (ha) Gross Area (ha) 0.94 0.94 0.3 Permeable Paving (ha) Permeable Paving (ha) 25.63% 25.71% 25 6.41 30% 0.5 0.5 0.0 0.0 0.0 0.0 32 0.5 Soft Landscaping (ha) Soft Landscaping (ha) 8.54% 39 3.33 8.57% 80 6.86 90.0 90.0 90.0 90.0 90.0 90.0 Developed Area Developed Area ٥ Berea Red Formation Hyrological soil group: Vryheid Formation Hyrological soil group: Building Area (ha) Building Area (ha) 51.27% 36.40 60% 0.00 0.00 0.00 0.00 0.00 0.00 51.43% 84 3.31 60% 0.0 0.0 0.0 0.0 1.1 km2 Gross Area in Catchment(ha) Gross Area in Catchment(ha) 5.52 5.52 6.46 1.8 2,10 8.56 0.09 1.8 Total Catchment Area Sub Soil Description: CN for use Class Composite CN: % Split to Total CN for use Class Total Soil Type 1 Catchment Area Composite CN: Total Soil Type 2 Catchment Area % Split to Total Sub-Precinct Area Totals Sub-Precinct Area Totals

WH1 - POST DEVELOPMENT - SPLIT OF AREA FOR CN CALCULATION:

Sub Soil Description:

Groot Area III. Building Area (ha) Soft Lundscaping (ha) Permeable Paving (ha) Groot Area (ha) Impermentable Soliding (ha) Groot Area (ha)<			Develo	Developed Area			Bond Constructor				F
Circle C		Gross Area in					was selvinges		Open space	Cane Areas	
Column C	Sub-Prednct	Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
1.60			%09	10%	30%	0.75	30%	20%	000	50	
116 0.000	-		0.00	0.00	0.00		0.37	200	orion	0.00	
1,53	2	1990	1.16	0.19	850		0.37	0.57			
1.58	8		0.00	0.00	0.00		0.00	0.00			
1,33	4		0.00	000	00.0		0.00	0.00			
1.56 0.00	N.		0.00	000	00.0		0.00	0.00			
1.33 1.16	9	-100-	0.00	000	0000		0.00	0.00			
1.35	7	-	000	900	000		0.00	0.00			
1.39 1.16 0.19 0.550 0.75	8		000	900	0.00		0.00	00.0			
13.96% 13.94% 1	Area Totals	1.93	1.16	0.19	0.58	0.75	0.00	0.00	000		
13,345 13,345 13,345 10,005 1	Total Soil Type 1	2.68					0.57	0.37	0.00	0.00	
Table Tabl	Split to Total		276 28	7 210	24 7 400			2000			
33.72 3.54 3.54 3.54 3.54 3.54 3.55 3.54 3.55 3.54 3.55	N for use Class		7.1	34	25		13.94%	13.94%	0.00%	0.00%	100,001
Concest Area In Building Area (ha) Developed Area Concest Area (ha) Edition (has been concest) Concest Area (ha) Edition (ha) Concest Area (ha)	composite CN:		30.72	2.81	5.41		12.66	39	39	25	
Catchment(ha) Building Area (ha) Soft Landscaping (ha) Permeable Paving (ha) Permeable Paving (ha) Gross Area (ha) Road Servitudes Go Pon Space Cane Areas Catchment(ha) Building Area (ha) Soft Landscaping (ha) Permeable Paving (ha) Gross Area (ha) Impermeable Suface (ha) Soft Landscaping (ha) (ha) <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>200</td> <td>1</td> <td>0.00</td> <td>0.00</td> <td>28.04</td>							200	1	0.00	0.00	28.04
Gross Area in Carthment(ha) Building Area (ha) Soft Landscaping (ha) Permeable Paving (ha) Gross Area (ha) Gross Area (ha) Gross Area (ha) Froad Servitudes Cart Landscaping (ha) Permeable Paving (ha) Gross Area (ha) Impermeable Sufface (ha) Soft Landscaping (ha) (ha) (ha) (ha) Cartchment(ha) 600 0.0	Soil Description:	Vryheid Formation	Hyrological soil group:	D							
Gross Area in Catchment(ha) Building Area (ha) Soft Landscraping (ha) Permeable Paving (ha) Gross Area (ha) Impermeable Surface (ha) Soft Landscraping (ha) (ha) (ha) (ha) 0.0<			Develop	oed Area			Road Servitudes		Open Space	Cane breac	-
66% 10% 30% 001 50% 50% 50% 000 <td>Sub-Precinct</td> <td>Gross Area in Catchment(ha)</td> <td>Building Area (ha)</td> <td>Soft Landscaping (ha)</td> <td>Permeable Paving (ha)</td> <td>Gross Area (ha)</td> <td>Impermeable Suface (ha)</td> <td>Soft Landscaping (ha)</td> <td>(ha)</td> <td>(ha)</td> <td>, -</td>	Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	, -
1.09 0.00			%09	10%	30%	0.01	20%	2005	00.0	000	
1.09 1.7 1.09 1.00 1	1		0.0	0.0	0.0	100	0.0	200	ana	onio	
0.0 0.0 <td>2</td> <td></td> <td>0.7</td> <td>0.1</td> <td>0.3</td> <td></td> <td>000</td> <td></td> <td></td> <td></td> <td></td>	2		0.7	0.1	0.3		000				
1.09	3		0.0	0.0	0.0		0.0	0.00			
1.109 0.00	4		0.0	0.0	0.0		0.0	00			
0.0 0.0 <td>2</td> <td></td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td></td> <td>0.0</td> <td>0.0</td> <td></td> <td></td> <td></td>	2		0.0	0.0	0.0		0.0	0.0			
1.09	9		0.0	0.0	0.0		0.0	0.0			
1.1 0.00	7		0.0	0.0	0.0		00	0.0			
1.1 0.7 0.1 0.3 0.0 0.0 0.0 0.0 0.0 0.0 1.09 59.72% 9.95% 29.86% 0.23% 0.23% 0.00% 0.00% 84 80 32 98 80 83 50.16 7.96 9.56 0.23 0.19 0.00 0.00	8		0.00	0.00	0.0		0.0	000			
1.09 59.72% 9.95% 29.86% 0.23% 0.23% 0.00%	Area Totals	11	7:0	0.1	0.3	0.0	00	00	000		
59.72% 9.95% 29.86% 0.23% 0.00% 0.00% 84 80 32 98 80 83 50.16 7.96 9.56 0.23 0.19 0.00 0.00	Total Soil Type 2 Catchment Area	1.09							20	05	-
84 80 32 0.00% 50.16 7.96 9.56 0.23 0.19 0.00 0.00	Split to Total		59.72%	9.95%	29.86%		%500	73%	2000	,,,,,,	
50.16 7.96 9.56 0.23 0.19 0.00 0.00 3.77	N for use Class		84	8	32		86	8 8	80.00%	0.00%	100,00%
	omposite CN:		50.16	7.96	9:26		0.23	0.19	00'0	0.00	68.10
	Catchment Area	3.77									

CN for Developed Area Rev 8

		Develo	Developed Area			Road Servitudes		Open Space	Cane Areas	
Sub-Precinct	Gross Area In Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
		%09	10%	30%	138	20%	20%	0.05	0.00	
1	- 1	0.00	0.00	0.00		69.0	69.0	2010	2000	
2		0.00	00'0	0.00		0.00	0.00			
3	1192	1.15	0.19	0.58		0.00	00.0			
4		0.00	0.00	0.00		0.00	00'0			
S		0.00	0.00	0.00		0.00	0.00			
9		00:00	0.00	00.00		0.00	0.00			
7		00:0	0.00	00.00		0.00	0.00			
60		0.00	0.00	0.00		0.00	00'0			
Area Totals	1.92	1.15	0.19	0.58	1.38	0.69	0.69	0.05	0.00	_
Total Soil Type 1	3,35									
% Split to Total		34.39%	5.73%	17.19%		20.60%	20.60%	1 49%	7,000	200,000
CN for use Class		7.1	39	25		98	39	39	25	-
Composite CN:		24.42	2.24	4.30		20.19	8.03	0.58	00'0	59.75
Sub Soil Description:	Vryheid Formation	Hyrological soil group:	٥							
		Develo	Developed Area			Road Servitudes		Open Space	Cane Areas	
Sub-Precinct	Gross Area In Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
		%09	10%	30%	0.27	20%	20%	091	0.00	
1		0.0	0:0	0'0	12	0.1	0.1			
2		0.0	0.0	0.0		0.0	0.0			
E	0.3	0.2	0.0	0.1		0.0	0.0			
4		0.0	0.0	0.0		0.0	0.0			
מג		0.0	0.0	0.0		0.0	0.0			
9		0.0	0.0	0.0		0.0	0.0			
7		0'0	0.0	0.0		0.0	0.0			
80		0.0	0.0	0.0		0.0	0.0			
Area Totals	0.3	0.2	0.0	0.1	0.3	0.1	0,1	1.6	0.0	
Total Soil Type 2 Catchment Area	2.17									
% Split to Total		8.45%	1.41%	4.23%		6.15%	6.16%	73.60%	0.00%	100 00%
CN for use Class		84	80	32		86	80	80	83	
Composite CN:		7.10	1.13	1.35		6.03	4.93	58.88	0.00	79.42
Total Catchment Area	\$52									
200000000000000000000000000000000000000										1

	A
R CN CALCULATION:	Hyrological soil group:
MENT - SPLIT OF AREA FO	Berea Red Formation
WHZA - POST DEVELOPMENT - SPLIT OF AREA FOR CN CALCULATION	Sub Soil Description:

-		Develo	Developed Area			Road Servitudes		Open Space	Cana Areas	
Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	-
		%09	10%	30%	0.41	20%	2005	920	L	
1		0.00	0.00	0.00	0.00	0.31	200	0.40	8,57	
7		0.00	0.00	00.00		130	17.0			
6	Ann	3.14	0.52	151		00.00	0.00			
4		0.00	000	500		000	0.00			
un		000		00.0		0.00	0.00			
v		000	00:0	00:0		00:0	0.00			
		000	0.00	0.00		0.00	0.00			
		0.00	0.00	0.00		0.00	0.00			
80		00.00	0.00	0.00		0.00	0.00			
Area Totals	5.24	3.14	0.52	1.57	0.41	0.21	021	0.46	623	
Total Soil Type 1	14.68							200	10:0	
Catchment Area										
% Split to Total		21.41%	3.57%	10.70%		1.41%	1.41%	217%	28 3897	1000 000
CN for use Class		71	39	25		86	39	30	30,300	100.00
Composite CN:		15.20	1.39	2.68		138	25.0	3 5	20.01	***
						2	0.53	777	74.60	37,01
Sub Soil Description:	Vryheld Formation	Hyrological soil group:	O							
		Develo	Developed Area			Road Servitudes		Onen Spare	Cana Areas	-
Sub-Predinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
		%09	10%	30%		20%	20%	000	00.0	
1		0.0	0.0	0.0	14.	00	00	000	ONYO	
2		0.0	0.0	0.0		000	2 6			
e		0.0	0.0	0.0		0				
4		0.0	0.0	0.0		000	0.00			
25		0.0	0.0	0.0		0 0	0.0			
9		0.0	0.0	0.0		0.00	0.0			
7		0.0	0.0	0.0			0.00			
8	100	0.0	0.0	0.0		0.00	0.0			
Area Totals	0.0	0.0	0.0	0.0	0.0	000	0.0	0,	0.0	
Total Soil Type 2	80.0						n'o	7''0	0.0	
Catchment Area	200									
% Split to Total		0.00%	0.00%	0.00%		%00.0	%00 U	100 000	/900 0	100 000
CN for use Class		84	80	32		866	S S	80	83.0	100.002
Composite CN:		0.00	0.00	0.00		0.00	0.00	80.00	0.00	80.00
Total Catchment Area	15.66									
										39.70

ANNEXURE E4-2 PRE DEVELOPMENT

H1 - PRE-DEVELOPMENT - SPLIT OF AREA FOR CN CALCULATION:
Sub Soil Description: Berea Red Formation Hyrological soil group: A

		Deven	Developed Alica			Road Servitudes		Onen Spare	Cano Areas	
Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	_	(ha)	
		909	10%	30%	0.17	20%	20%	00:0	7.77	
H	000	0.00	00'0	0.00	0.17	60.0	000	Me	1311	
2	20%	0.00	00:0	00.0		000	60%			
60	300	0.00	00'0	0.00		00.0	00.0			
4	202	0.00	000	00.0		800	000			
S	101	0.00	000	000	1	8000	0.00			
y	200	000	900	000		0.0	0.00			
o 1º		0000	0000	0.00		0.00	0.00			
		0,00	00.00	00'0		0.00	0.00			
60	200	0.00	00:00	00'0		0.00	0.00	A. A. A.	-	
Area Totals	00:0	0.00	0.00	0.00	0.17	60'0	60'0	0.00	1.27	
Total Soil Type 1	2.44									
Catchinent Area			100000							
% Split to lotal		0.00%	0.00%	%000		3.50%	3.50%	%00.0	93.00%	100.00%
CN for use Class		7.1	39	22		, 86	39	39	25	
Composite CN:		00'0	00'0	0.00		3.43	1.37	0.00	23.25	28.05
Sub Soil Description:	Vryheid Formation	Hyrological soil group:	۵							
		Develo	Developed Area			Road Servitudes		Open Space	Cane Areas	
Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	-	(ha)	
		%09	10%	30%	0.49	20%	20%	00'0	6.53	
1	15	0.0	0.0	0.0	40	0.25	0.25			
7	-	0.0	0.0	0.0		0.00	000			
m	- 500	0.0	0.0	0.0		0.00	0.00			
4	**	0.0	0.0	0.0		0.00	0.00			
25	7	0.0	0.0	0.0		0.00	0.00			
9	-	0.0	0.0	0.0		0.00	0.00			
7		0.0	0.0	0.0		0.00	0.00			
60	0.0	0.0	0.0	0.0		0.00	0.00			
Area Totals	0.0	0.0	0.0	0.0	0.5	0.2	0.2	0.0	6.5	
Total Soil Type Z	7.01									
% Split to Total		0.00%	7000	70000		,				
CN for use Class		84	80	32		9.30%	3,30%	0.00%	93.00%	100.00%
Commocite CN .		000				00	90	80	83	
composite civ.		0000	0.00	0.00		3.43	2.80	0.00	77.19	83.42
Total Catchment Area	9.45									3
										69 17

HN1 - PRE DEVELOPMENT - SPLIT OF AREA FOR CN CALCULATION:
Sub Soil Description: Berea Red Formation Hyrological soil procure

		Develo	Developed Area			Road Servitudes		Onen Spare	Cano Areas
Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)		(ha)
1		20%	15%	35%	0.21	20%	20%	000	2.79
1	0979	0.00	0.00	0.00	0.23	0.11	0.11	00 0	1.70
7	2010	0.00	0.00	0.00		0.00	00.00		
8	000	0.00	00.00	0.00		0.00	0.00		
4	GVD	0.00	0.00	0.00		0.00	0.00		
2	000	0.00	0.00	00'0		0.00	000		
9	202	0.00	00:00	0.00		0.00	0.00		
7	0.00	0.00	0.00	0.00		0.00	0.00		
80	000	0.00	0.00	0.00	Mary Contract	0.00	0.00		
Area Totals	0.00	0.00	0.00	0.00	0.21	0.11	0.11	0.00	779
Total Soil Type 1. Catchment Area	3.00								
% Split to Total		0.00%	0.00%	%00.0		3.50%	3,50%	0.00%	93.00%
CN for use Class		71	39	25		86	39	39	25
Composite CN:		0.00	0.00	0.00		3.43	1.37	0.00	23.25
Sub Soil Description:	Vryheid Formation	Hyrological soil group:	۵						
								The second second	

		Develo	Developed Area			Road Servitudes		Open Space	Cane Areas
Sub-Precinct	Gross Area In Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)
		20%	15%	35%	0.62	%05	20%	00'0	8.27
1	23	0.0	0.0	0.0	Die	0.31	0.31		-
2	- 17	0.0	0.0	0.0		0.00	0.00		
3	2	0.0	0.0	0.0		0.00	0.00	l	
4	0.0	0.0	0.0	0.0		0.00	0.00		
2	22	0.0	0.0	0.0		00.0	0.00		
9	90	0.0	0.0	0.0		0.00	0.00		
7	8	0.0	0.0	0.0		0.00	0.00		
8	3	0.0	0.0	0.0		0.00	00.00		
Area Totals	0.0	0.0	0.0	0.0	9.0	0.3	0.3	00	83
Total Soil Type 2 Catchment Area	8.89								
% Split to Total		0.00%	%0000	0.00%		3.50%	3.50%	0.00%	93.00%
or use class		84	08	32		86	80	80	83
Composite CN:		0.00	00.0	0.00		3,43	2.80	000	77.10

	km2
11.89	0.12
11.89	0.12

69.44

HN2 - PRE DEVELOPMENT - SPLIT OF AREA FOR CN CALCULATION:
Sub Soil Description: Berea Red Formation Hyrological soil group: A

3.50% 0.00 8.47 3.50% 0.00 8.47 0.32 0.00 0.0	6		nevel	Jueu Aire			Dosd Comittinion				
Circinmenting Building Area (ha) Soft undictaping (ha) Coop Area (ha) Impermeable Suffice (ha) Soft undictaping (ha) Coop Area (ha) Impermeable Suffice (ha) Soft undictaping (ha) Coop Area (ha) Impermeable Suffice (ha) Soft undictaping (ha) Coop Area (ha) Impermeable Suffice (ha) Soft undictaping (ha) Coop Area (ha) Impermeable Suffice (ha) Soft undictaping (ha) Coop Area (ha) Impermeable Suffice (ha) Coop Area (ha) Impermeable Suffice (ha) Soft undictaping (ha) Impermeable Suffice (ha) Soft undictaping (ha) Impermeable Suffice (ha) Impermeable Suffice (ha) Soft undictaping (ha) Impermeable Suffice (ha)	Park Barriera	Groce Area in					MORO SCI MICAGES		open space		
Continue of the continue of	Sup-Frecinct	Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
Color Colo			%09	10%	30%	0.64	20%	20%	000	8.47	
Control Cont	1	0.0	0.00	0.00	0.00	0.64	0.32	0.37			
Catchment(halles Control Contr	2	200	0.00	0.00	0.00		000	000			
Control Cont	€	1	0.00	0.00	0.00		000	00.0			
Control Cont	4	98.8	0.00	0.00	0.00		000	00.0			
Cachment(tha) Cachment(tha	'n	200	0.00	0.00	0.00		000	000			
Solution	9	007	0.00	0.00	0.00		0.00	0.00			
State Stat	7	*	0.00	0.00	0.00		00:00	0.00			
10,000 0,0	8	0.00	0.00	0.00	0.00		0.00	0.00			
Digitary	Area Totals	0.00	0.00	00.0	0.00	0.64	0.32	0.32	0.00	8.47	
Catchment(na) Ayrological soil group: Developed Area Catchment(na) C	Total Soil Type 1	9.11									
The Contraction Hypological soil group: December Area Area December Area December Area Area December Area Area December Area Area December Area Ar	% Split to Total		0.00%	00'0	0.00%		3.50%	3.50%	2000	43.00%	100.00%
National Formation Hypological soil group: D D	CN for use Class		17	39	22		86	39	39	25	2000
Gross Area In Catchment(ha) Building Area (Catchment(ha)) Soft Landscaping (ha) (ha) Permeable Paving (ha) (ha) Gross Area (ha) (ha) (ha) Road Serv/fludes Catchment(ha) (ha) (ha) (ha) (ha) Catchment(ha) (ha) (ha) (ha) (ha) (ha) (ha) (ha)	Composite CN:		0.00	0.00	0.00		3.43	1.37	0.00	23.25	28.05
Carciment(ha) Building Area (ha) Soft Landscaping (ha) Permeable Paving (ha) Impermeable Surface (ha) Soft Landscaping (ha) Carciment(ha) Soft Landscaping (ha) Soft Landscaping (ha) Carciment(ha) Carciment(ha	Soil Description:	Vryheid Formation	Hyrological soil group:	٥							
Gross Area in Catchment(ha) Building Area (ha) Soft Landscaping (ha) Permeable Paving (ha) Gross Area (ha) Impermeable Surface (ha) Soft Landscaping (ha) (ha) 6.0% 1.0% 3.0% 0.27 5.0% 5.0% 0.00 3.64 0.0 0.0 0.0 0.0 0.0 0.0 0.0 3.64 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 </td <td></td> <td></td> <td>Develo</td> <td>oped Area</td> <td></td> <td></td> <td>Road Servitudes</td> <td></td> <td>Open Space</td> <td>Cane Areas</td> <td></td>			Develo	oped Area			Road Servitudes		Open Space	Cane Areas	
60% 10% 30% 0.27 50% 50% 0.00 3.64 0.0 <td>Sub-Precinct</td> <td>Gross Area in Catchment(ha)</td> <td>Building Area (ha)</td> <td>Soft Landscaping (ha)</td> <td>Permeable Paving (ha)</td> <td>Gross Area (ha)</td> <td>Impermeable Suface (ha)</td> <td>Soft Landscaping (ha)</td> <td>(ha)</td> <td>(ha)</td> <td></td>	Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
1.00			%09	10%	30%	0.27	20%	20%	00.00	3.64	
Columbia	н	- 440	0.0	0.0	0.0	.01	0.14	0.14	20	9.0	
0.00 0.00 <th< td=""><td>2</td><td>*</td><td>0.0</td><td>0.0</td><td>0.0</td><td></td><td>0.0</td><td>0.0</td><td></td><td></td><td></td></th<>	2	*	0.0	0.0	0.0		0.0	0.0			
0.0 0.0 <td>m</td> <td>96</td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td></td> <td>0.0</td> <td>0.0</td> <td></td> <td></td> <td></td>	m	96	0.0	0.0	0.0		0.0	0.0			
0.00 0.00 <th< td=""><td>4</td><td>166</td><td>0.0</td><td>0.0</td><td>0.0</td><td></td><td>0.0</td><td>0.0</td><td></td><td></td><td></td></th<>	4	166	0.0	0.0	0.0		0.0	0.0			
0.00 0.00 <th< td=""><td>20</td><td>2</td><td>0.0</td><td>0.0</td><td>0.0</td><td></td><td>0.0</td><td>0.0</td><td></td><td></td><td></td></th<>	20	2	0.0	0.0	0.0		0.0	0.0			
0.0 0.0 <td>9</td> <td>- 99</td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td></td> <td>0.0</td> <td>0.0</td> <td></td> <td></td> <td></td>	9	- 99	0.0	0.0	0.0		0.0	0.0			
0.0 0.0 <td>1</td> <td>***</td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td></td> <td>0.0</td> <td>0.0</td> <td></td> <td></td> <td></td>	1	***	0.0	0.0	0.0		0.0	0.0			
3.91 0.00 0.0 0.0 0.00 0.3 0.1 0.1 0.1 3.6 3.6 3.6 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5	8	00	0.0	0.0	0.0		0.0	0.0			
3.91 0.00% 0.00% 3.50% 3.50% 0.00% 93.00% 84 80 80 83 0.00 0.00 0.00 3.43 2.80 0.00 77.19	Area Totals	0.0	0.0	0.0	0.0	0.3	0.1	0.1	0.0	3.6	
13.02 0.00% 0.00% 3.50% 3.50% 0.00% 93.00% 93.00% 13.50% 0.000 0.000 0.000 0.000 13.02 13.02 13.02	Total Soil Type 2	3.91									
13.02 13.02	% Split to Total		0.00%	2000	70000		2000	7000	2000	, , , , , ,	
13.02 0.00 0.00 3.43 2.80 0.00 77.19	CN for use Class		84	80	32		9,30%	9/30%	0.00%	93.00%	100.00%
13.02 0.13 tent	Composite CN:		0.00	0.00	0.00		3.43	2.80	0.00	77.19	83.42
013 bm2	al Catchment Area	13.02									13
		0.12	t and								44.67

HN3 - PRE DEVELOPMENT - SPLIT OF AREA FOR CN CALCULATION:
Sub Soil Description: Berea Red Formation Hyrological soil group: A

		Develo	Developed Area			Road Servitudes		Open Space	Cane Areas	
Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)		(ha)	
		%09	10%	30%	0.28	20%	20%	00.0	3.69	
1		0.00	0.00	0.00		0.14	0.14			
2		0.00	0.00	0.00		0.00	000			
		00'0	0.00	00'0		0.00	0.00			
4		0.00	0.00	0.00		0.00	0.00			
S		0.00	0.00	0.00		0.00	0.00			
9	1	0.00	0.00	0.00		0.00	0.00			
7	-	0.00	0.00	00'0		0.00	0.00			
8	MA	000	00'0	00'0	Service Service	0.00	0.00			
Area Totals	0.00	0.00	0.00	00'0	0.28	0,14	0.14	0.00	3.69	
Total Soil Type 1	3.97									
Catchment Area										
% Split to Total		0.00%	%00'0	%00.0		3.50%	3.50%	0.00%	93.00%	100.00%
CN for use Class		71	39	22		86	39	39	25	
Composite CN:		0.00	0.00	00.00		3.43	1.37	0.00	23.25	28.05
Sub Soil Description:	Vryheid Formation	Hyrological soil group:	Д							
		Develo	Developed Area			Road Servitudes		Open Space	Cane Areas	
Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	-	(ha)	
		%09	10%	30%	0.14	20%	20%	00.00	1.83	
1		0.0	0.0	0.0	100	0.07	20.0			
2		0.0	0.0	0.0		0.00	0.00			
63		0.0	0.0	0.0		0.00	0.00			
4		0.0	0.0	0.0		0.00	0.00			
Ŋ		0.0	0.0	0.0		0.00	0.00			
9		0.0	0.0	0.0		0.00	0.00			
7	5	0.0	0.0	0.0		0.00	0.00			
88		0.0	0.0	0.0		0.00	0.00			
Area Totals	0.0	0.0	0.0	0.0	0.1	0.1	0.1	0.0	1.8	
Total Soil Type 2	1.96									
% Split to Total		2000	70000	7900.0		-		2000		
CN for use Class		84	80	3.7		3.50%	3.50%	2,00%	93.00%	100.00%
Composite CN:		000	800	35		96	80	08	83	
· un passed una		0000	0.00	0.00		3,43	2.80	0.00	77.19	83.45
Total Catchment Area	5.93									
										1 1 1

HN4 - PRE DEVELOPMENT - SPLIT OF AREA FOR CN CALCULATION: Sub Soil Description: Berea Red Formation Hyrological soil group:

		Develo	Developed Area			Road Servitudes		Open Space	Cane Areas	
Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
		%09	10%	30%	1.72	20%	20%	000	22.83	
1		0.00	0.00	0.00		0.85	0.86	9	60.77	
2		0.00	0.00	0.00		0.00	0.00			
33		0.00	0.00	0.00		0.00	0.00			
4		0.00	0.00	0.00		0.00	0.00			
'n	- 66	0.00	0.00	0.00		0.00	0.00			
9	70.0	0.00	00'0	0.00		0.00	0.00			
7		0.00	0.00	0.00		0.00	0.00			
80	0.00	0.00	0.00	0.00		0.00	0.00			
Area Totals	0.00	0.00	0.00	00'0	1.72	0.86	0.86	0.00	22.83	
Total Soil Type 1 Catchment Area	24.55									
% Split to Total		0.00%	0.00%	0.00%		3.50%	3.50%	0.00%	93.00%	100 00%
CN for use Class		71	39	25		86	39	39	25	
Composite CN:		0.00	0.00	0.00		3.43	1.37	000	23.25	28.05
Sub Soil Description:	Vryheid Formation	Hyrological soil group:	Q							
		Develo	Developed Area			Road Servitudes		Open Space	Cane Areas	
Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
		%09	10%	30%	0.29	20%	20%	000	3.83	
1		0.0	0.0	0.0	10	0.14	0.14			
2		0.0	0.0	0.0		0.00	0.00			
3		0.0	0.0	0.0		0.00	0.00			
4		0.0	0.0	0.0		0.00	0.00			
50		0.0	0.0	0.0		00'0	0.00			
9		0.0	0.0	0.0		0.00	0.00			
7		0.0	0.0	0.0		0.00	0.00			
80		0.0	0.0	0.0		0.00	0.00			
Area Totals	0.0	0.0	0'0	0.0	0.3	0.1	0.1	0.0	300	
Total Soil Type 2 Catchment Area	4.12									
% Split to Total		0.00%	0.00%	0.00%		3.50%	3.50%	0.00%	93 00%	100.000
CN for use Class		84	80	32		86	80	80	83	700.007
Composite CN:		0.00	0.00	0.00		3.43	7.80	000	77 10	62.43
	d								CT-1/2	74.00
Total Catchment Area	28.67									16.01
	0.29	km2								20.01

HN4A - PRE DEVELOPMENT - SPLIT OF AREA FOR CN CALCULATION:
Sub Soil Description:
Berea Red Formation Hyrological soil group:

						Road Servitades		Open Space	Cane Areas	
Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
		60%	10%	30%	0.26	20%	20%	000	C7 E	
1	Onto	0.00	0.00	0.00	200	0.13	0.13		41.1	
2		0.00	0.00	0.00		000	000			
8		0.00	00'0	0.00		0.00	0.00			
4	1	0.00	0.00	0.00		0.00	000			
25	100	0.00	0.00	0.00		0.00	0.00			
9	- 100	0.00	0.00	0.00		00:00	0.00			
7	3	0.00	0.00	00'0		0.00	0.00			
88	all a	0.00	0.00	00'0	The same of	0.00	0.00			
Area Totals	00'0	0.00	0.00	00'0	0.26	0.13	0.13	00'0	3.42	
Total Soil Type 1	3.68									
% Split to Total		2000	2900 0	7800 0		7000	7004.0	7000		
CN for use Class		21	3000	35		5.30%	3.30%	0.00%	53.00%	100.00%
Composite CN .		000	66	5 60		80 5	£ .	39	52	1
combosite oils.		0.00	חיים	00.0		3.43	1.37	0.00	23.25	28.05
Sub Soil Description:	Vryheid Formation	Hyrological soil group:	٥							
		Develo	Developed Area			Road Servitudes		Onen Space	Cane Areas	-
Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
		%09	10%	30%		20%	20%	00:00	0.00	
1	0.0	0.0	0.0	0.0		0.00	0.00	0.0	100	
2	2	0.0	0.0	0.0		0.00	0.00			
m		0.0	0.0	0.0		0.00	0.00			
4	900	0.0	0.0	0.0		0.00	0.00			
S	100	0.0	0.0	0.0		0.00	0.00			
9		0.0	0.0	0.0		0.00	0.00			
7	- 08 5 -	0.0	0.0	0.0		0.00	0.00			
80		0.0	0.0	0.0		0.00	0.00			
Area Totals	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Soil Type 2 Catchment Area	0.00									20
% Split to Total		#DIV/01	#DIV/01	#DIV/OI		TOTAL CHA	TO THE STATE OF TH	100		
CN for use Class		84	80	32		98	10/AIG#	#0/A/O#	#DIV/UI	#DIA/Oi
Composite CN:		io/AIQ#	#DIV/0!	#DIV/01		#DIV/OI	10/NIQ#	10/NG#	10/AIG#	#DIV/0i
Total Catchment Area	3.68							l		

HN4B - PRE DEVELOPMENT - SPLIT OF AREA FOR CN CALCULATION: Sub Soil Description: Berea Red Formation Hyrological soil group:

Gross Area in Carchment(ha) Building Area (ha) Soft Landscaping (ha) Permeable Paving (ha) Impermeable Surface (ha) Soft Landscaping (ha) (ha) Carchment(ha) 56% 15% 35% 0.41 50% 50% 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 71 39 25 98 343 1.37 0.00 Vryheid Format			Develo	Developed Area			Road Servitudes		Open Space	Cane Areac	_
S	Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)		_
0.00 0.00 0.00 0.21 0.21 0.21 0.21 0.21			20%	15%	35%	0.41	20%	20%	000	5.53	_
0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0	-	9	00'0	0.00	0.00	-0.61	0.21	0.21	200	16.6	
0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0	2	666	00.0	0.00	00'0		000	1770			
0.000 0.000 <t< td=""><td>m</td><td>2</td><td>0.00</td><td>0.00</td><td>0.00</td><td></td><td>000</td><td>800</td><td></td><td></td><td></td></t<>	m	2	0.00	0.00	0.00		000	800			
0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0	4	1	0.00	0.00	00'0		0.00	000			
0.00 0.00 <t< td=""><td>S</td><td>000</td><td>00'0</td><td>0.00</td><td>0.00</td><td></td><td>0.00</td><td>000</td><td></td><td></td><td></td></t<>	S	000	00'0	0.00	0.00		0.00	000			
0.00 <	9	8	00.00	0.00	000		00:00	0.00			
5.92 0.00% 0.00% 0.00% 0.00% 0.00% 0.00 0.00 71 39 25 98 3.50%	1	1	0.00	0.00	0.00		0.00	0.00			
0.00 0.00 0.00 0.41 0.21 0.21 5.92 0.00% 0.00% 3.50% 3.50% 3.50% 71 39 25 98 39 0.00 0.00 0.00 3.43 1.37 Vryheid Formation Hyrological soil group: D Acceloped Area Road Saruhindes	80	California .	0.00	0.00	00'0		0.00	0.00			
5.92 0.00% 0.00% 0.00% 3.50% 3.50% 3.50% 3.50% 3.50% 3.50% 3.50% 3.50% 3.50% 3.50% 3.50% 3.50% 3.60 0.00 0.00 Vryheid Formation Hyrological soli group: D Developed Area	Area Totals	00:00	0.00	0.00	0.00	0.41	0.21	0.21	000	5 54	_
0.00% 0.00% 0.00% 3.50% 3.50% 71 39 25 98 39 0.00 0.00 0.00 3.43 1.37 Vryheid Formation Hyrological soil group: D Road Saruhinies	otal Soil Type 1 atchment Area	5:92								100	-
71 39 25 98 39 39 25 0.00 0.00 3.43 1.37 Vryheid Formation Hyrological soil group: D	% Split to Total		0.00%	0.00%	0.00%		3.50%	3.50%	0.00%	93.00%	
0.00 0.00 3.43 1.37 Vryheid Formation Hyrological soil group: D Developed Area Road Serultrides	N for use Class		7.1	39	25		98	39	39	25	
Vryheid Formation Hyrological soil group: D Boad Serultrides Developed Area	Composite CN:		0.00	0.00	0.00		3.43	1.37	0.00	23.25	
Road Servitudes	Soil Description:	Vryheid Formation	Hyrological soil group:	D							
			Develo	oped Area			Road Servitudes		Onen Space	Cane Areas	-

		Develo	Developed Area			Road Servitudes		Onen Space Cane Areas	Cano Areas	
Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
		20%	15%	35%	60'0	20%	20%	0.00	1.18	
1		0.0	0.0	0.0	20	0.04	0.04			
2		0.0	0.0	0.0		0.00	0.00			
m		0.0	0.0	0.0		0.00	0.00			
4		0.0	0.0	0.0		0.00	0.00			
2		0.0	0.0	0.0		0.00	0.00			
9		0.0	0.0	0.0		0.00	0.00			
7		0.0	0.0	0.0		00'0	0.00			
8	0.0	0.0	0.0	0.0		0.00	00.0			
Area Totals	0.0	0.0	0.0	0.0	0.1	0.0	00	00	1.3	
Total Soil Type 2 Catchment Area	1.27								717	
% Split to Total		0.00%	0.00%	0.00%		3 50%	3 50%	/2000	2000	
CN for use Class		84	80	32		86	80	80	93.00%	*00.001
Composite CN:		0.00	0.00	0.00		3.43	2.80	000	77.19	83.42
Total Catchment Area	7.20									
	700	Comp								37.83

kmZ

HC1 - PRE DEVELOPMENT - SPLIT OF AREA FOR CN CALCULATION: Sub Soil Description: Berea Red Formation Hyrological soil group:

		Develo	Developed Area			Road Servitudes		Open Space Cane Areas	Cana Areas	
Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
		%09	10%	30%	0.54	20%	20%	000	7.13	-
-	000	00:0	0.00	0.00	0.54	0.27	0.27			1
2	0.00	0.00	0.00	0.00		000	000			
23	100	0.00	0.00	0.00		000	000			
4	900	0.00	0.00	0.00		000	00.0			
Ŋ	Also.	0.00	0.00	0.00		00'0	000			
9	22	0.00	0.00	0.00		000	00.0			
7	- 900	0.00	0.00	0.00		00.0	000			
8	DO-O	0.00	0.00	0.00		000	00.0			
Area Totals	0.00	0.00	0.00	0.00	0.54	0.77	0.37	00.0	7.13	
Total Soil Type 1 Catchment Area	7.66			e e				2000	CT.	7
% Split to Total		0.00%	0.00%	0.00%		3,50%	3.50%	0.00%	93.00%	
CN for use Class		7.1	39	25		886	36	30	25	
Composite CN:		0.00	0.00	0.00		3,43	1,37	0.00	23.25	
Sub Soil Description:	Vryheid Formation	Hyrological soil group:	up: D							
		Devel	then Alea			Road Servitudes		Open Space	Cane Areas	-
Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	_
		%09	10%	30%	0.15	20%	20%	00'0	2.02	-
н.	0.0	0.0	0.0	0.0	6.0	0.08	0.08	0.11	0.1	
2	40	0.0	0.0	0.0		0.0	0.0			
m	-20	0.0	0.0	0.0		0.0	0.0			
4	100	0.0	0.0	0.0		0.0	0.0			
Ŋ	100	0.0	0.0	0.0		0.0	0.0			
9	77	0.0	0.0	0.0		0.0	0.0			
7	-09	0.0	0.0	0.0		0.0	00			
80	0.0	0.0	0.0	0.0		0.0	0.0			
Area Totals	0.0	0.0	0.0	0.0	0.2	0.1	0.1	0.0	2.0	
Total Soil Type 2 Catchment Area	2.17									7
% Split to Total		0.00%	0.00%	0.00%		3,50%	3.50%	0.00%	93.00%	
CN for use Class		25	80	32		86	08	80	83	100.00%
Composite CN:		000								

93.00% 83 77.19

km2

9.83 0.10

Total Catchment Area

83.42

40.27

HS1 - PRE DEVELOPMENT - SPLIT OF AREA FOR CN CALCULATION:
Sub Soil Description: Berea Red Formation Hyrological soil group: A
Developed Area

3.50% 50% 50% 0.00 0.00 0.00 0.00 0.00 0.00 0.07 3.50% 3.50% 0.0 0.0 0.0 0.0 0.0 0.0 0.0			Develo	Developed Area			Road Servitudes		Open Space Cane Areas	Cane Areas	
Series S	Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
Care Control		Service of the service of	%09	10%	30%	1,33	20%	20%	0.00	17.72	
Continuent Con	T	600	0.00	0.00	0.00	-	0.67	29.0	010	41/11	
Carcin Companies	2	200	0.00	00'0	0.00		0.00	0.00			
Continuent Con	3	000	0.00	0.00	0.00		0.00	0.00			
13.06 0.000 0.00	4	000	0.00	0.00	0.00		0.00	0.00			
13.06	2	00'0	0.00	0.00	0.00		0.00	0.00			
19-06 0.000 0.00	9	001	0.00	0.00	00'0		0.00	0.00			
19-06 0.000 0.00	7	010	0.00	0.00	0.00		0.00	0.00			
19.06 0.00 0.00 0.00 0.00 1.33 0.67 0.67 0.00 1.772 1.976 1.97	8	000	0.00	00.00	0.00		0.00	0.00			
19.06 0.000% 0.000% 0.000% 3.50% 3.50% 3.50% 93.00	Area Totals	0.00	0.00	0.00	00'0	1.33	0.67	0.67	0.00	17.77	
Catchment(ha) Final Columnia Final	Total Soil Type 1	19.06									
The property of the property	% Split to Total		0.00%	0.00%	7 U U		2 5000	2 500	2000	7000 00	400 000
Cool 0.00 0.00 3.33 1.37 0.00 23.25 Vryhelid Formation Hyrological soli group: Developed Area Developed Area Promosphe Ar	CN for use Class		71	OF.	35		000	9/000	50000 SE	33,00%	100.00%
Carcinment(has) Ayrological soft group: Developed Area Developed A	Composite CN:		0.00	0.00	0.00		3.43	1.37	0.00	23.25	28.05
Carchiment(ha) Building Area (ha) Developed Area Carchiment(ha)	ib Soil Description:	Vryheid Formation	Hyrological soil group:	۵							
Gross Area in Carchment(ha) Building Area (ha) Soft Landscaping (ha) Permeable Paving (ha) Gross Area (ha) Impermeable Sufface (ha) Soft Landscaping (ha) Permeable Paving (ha) Gross Area (ha) Impermeable Sufface (ha) Soft Landscaping (ha) Permeable Paving (ha) Gross Area (ha) Impermeable Sufface (ha) Soft Landscaping (ha) Chan			Develo	ped Area			Road Servitudes		Onen Snare	Cana Amaze	
60% 10% 30% 0.71 50% 50% 0.00 9.45 0.0 <td>Sub-Precinct</td> <td>Gross Area in Catchment(ha)</td> <td>Building Area (ha)</td> <td>Soft Landscaping (ha)</td> <td>Permeable Paving (ha)</td> <td>Gross Area (ha)</td> <td>Impermeable Suface (ha)</td> <td>Soft Landscaping (ha)</td> <td>(ha)</td> <td>(ha)</td> <td></td>	Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
0.00 0.00 <th< td=""><td></td><td></td><td>%09</td><td>10%</td><td>30%</td><td>0.71</td><td>20%</td><td>20%</td><td>00:00</td><td>9.45</td><td></td></th<>			%09	10%	30%	0.71	20%	20%	00:00	9.45	
10.16	1	0.0	0.0	0.0	0.0		0.36	0.36	1/0		
0.00 0.00 <th< td=""><td>2</td><td>970</td><td>0.0</td><td>0.0</td><td>0.0</td><td></td><td>0.0</td><td>0.0</td><td></td><td></td><td></td></th<>	2	970	0.0	0.0	0.0		0.0	0.0			
0.0 0.0 <td>3</td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td></td> <td>0.0</td> <td>0.0</td> <td></td> <td></td> <td></td>	3	0.0	0.0	0.0	0.0		0.0	0.0			
0.0 0.0 <td>4</td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td></td> <td>0.0</td> <td>0.0</td> <td></td> <td></td> <td></td>	4	0.0	0.0	0.0	0.0		0.0	0.0			
0.0 0.0 <td>5</td> <td>D.D.</td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td></td> <td>0.0</td> <td>0.0</td> <td></td> <td></td> <td></td>	5	D.D.	0.0	0.0	0.0		0.0	0.0			
0.0 0.0 <td>9</td> <td>90</td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td></td> <td>0.0</td> <td>0'0</td> <td></td> <td></td> <td></td>	9	90	0.0	0.0	0.0		0.0	0'0			
0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 9.5 10.16 0.00 0.0 0.0 0.0 0.0 0.0 0.0 9.5 10.16 0.00% 0.00% 0.00% 3.50% 0.00% 93.00% 84 80 32 98 80 80 83 29.22 km2 0.00 0.00 77.19	7	d'a	0.0	0.0	0.0		0.0	0.0			
10.16 10.16 0.00 0.00 0.00% 0.000%	8	90	0.0	0.0	0.0		0.0	0.0			
10.16 0.00% 0.00% 0.00% 3.50% 0.00% 93.00% 84 80 87 80 83 0.00 0.00 0.00 77.19 29.22 km2 0.29 km2	Area Totals	0.0	0.0	0.0	0.0	0.7	0.4	0,4	0.0	9,5	
0.00% 0.00% 0.00% 3.50% 3.50% 0.00% 93.00% 84 80 32 98 80 80 83 80 83 80 80 83 80 80 83 80 80 80 80 80 80 80 80 80 80 80 80 80	Total Soil Type 2 Catchment Area	10.16									
84 80 32 98 80 83 0.00 0.00 0.00 3.43 2.80 0.00 77.19 29.22 km2	% Split to Total		0.00%	0.00%	0.00%		3.50%	3 50%	20000	03 00%	100 000
29.22 km2 km2	CN for use Class		84	80	32		86	80	80	83	100.001
29.22 0.29 km2	Composite CN:		0.00	0.00	0.00		3.43	2.80	0.00	77.19	83.42
0.29 кm2	tal Catchment Area	29.22									
		0.29	km2								47.31

HS2 - PRE- DEVELOPMENT - SPLIT OF AREA FOR CN CALCULATION:
Sub Soil Description: Berea Red Formation Hyrological soil group: A

3.50% 0.000 (ha) 50% 0.000 0.00 0.00 0.00 0.00 0.00 0.00	Sub-Precinct						Road Servitudes		Open Space	Cane Areas	
Color Colo		Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
Columentary			909	10%	30%	0.53	20%	20%	0.00	7.08	
Continue	•		0.00	0.00	0.00	610	0.27	0.27			
Composition	7	207	0.00	0.00	0.00		0.00	0.00			
Continue	8		0.00	0.00	0.00		0.00	0.00			
Control Cont	4		000	0.00	0.00		0.00	00'0			
Continue	N)		00'0	0.00	0.00		0.00	0.00			
1,000 0,00	9		000	0.00	0.00		0.00	0.00			
1,000	7		0.00	000	0.00		0.00	0.00			
7.51 2.000 0.000	8		0.00	0.00	0.00		0.00	0.00			
1,000 1,00	Area Totals	0.00	00:0	0.00	0.00	0.53	0.27	0.27	0.00	7.08	
Catciment(ha) Building Area (ha) Soft Landscaping (ha) Soft Landscaping (ha) Soft Landscaping (ha) Soft Landscaping (ha) Catciment(ha) Soft Landscaping (ha) Soft Landscaping (ha) Catciment(ha) Soft Landscaping (ha) Soft Landscaping (ha) Catciment(ha) Soft Landscaping (ha) Soft Landscapin	Total Soil Type 1	7.61			The state of the s						
Catchment(ha) Profogical soil group; Catchment(ha) Cat	of Califor Tatal		10000		-						
Viryheid Formation Hyrological soli groups Aural	% Split to lotal		0.00%	0.00%	%00.0		3.50%	3.50%	%000	93.00%	100.00%
Catchment(ha) Edition Formation Fo	CN for use Class		71	39	25		86	39	33	25	
Carchiment(ha) Pyrological soil group: Developed Area Carchiment(ha) Carchiment(h	Composite CN:		0.00	0.00	0.00		3.43	1.37	0.00	23.25	28.05
Carchiment(ha) Building Area (ha) Soft Landscaping (ha) Permeable Paving (ha) Gross Area (ha) Impermeable Surface (ha) Soft Landscaping (ha) Gross Area (ha) Impermeable Surface (ha) Soft Landscaping (ha) Gross Area (ha) Impermeable Surface (ha) Soft Landscaping (ha) (ha) (ha) (ha)	Sub Soil Description:	Vryheid Formation	Hyrological soil group:	۵							
Gross Area in Catchment(ha) Building Area (ha) Soft Landscaping (ha) Permeable Paving (ha) Gross Area (ha) Impermeable Surface (ha) Soft Landscaping (ha) (ha) (ha) (ha) Catchment(ha) 60% 10% 30% 0.01 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.06 0.00			Develo	ped Area			Road Servitudes		Open Space	Cane Areas	
60% 10% 30% 0.11 50% 50% 50% 152 0.0 0.0 0.0 0.0 0.06 0.06 0.00 1.52 0.0 <td>Sub-Precinct</td> <td>Gross Area in Catchment(ha)</td> <td>Building Area (ha)</td> <td>Soft Landscaping (ha)</td> <td>Permeable Paving (ha)</td> <td>Gross Area (ha)</td> <td>Impermeable Suface (ha)</td> <td>Soft Landscaping (ha)</td> <td>(ha)</td> <td>(ha)</td> <td></td>	Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
0.00 0.00 <th< td=""><td></td><td></td><td>%09</td><td>10%</td><td>30%</td><td>0.11</td><td>20%</td><td>20%</td><td>00'0</td><td>1.52</td><td></td></th<>			%09	10%	30%	0.11	20%	20%	00'0	1.52	
Color Colo	1		0.0	0.0	0.0		0.06	90.0			
0.00 0.00 <th< td=""><td>7</td><td></td><td>0.0</td><td>0.0</td><td>0.0</td><td></td><td>0.0</td><td>0.0</td><td></td><td></td><td></td></th<>	7		0.0	0.0	0.0		0.0	0.0			
0.0 0.0 <td>33</td> <td></td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td></td> <td>0.0</td> <td>0.0</td> <td></td> <td></td> <td></td>	33		0.0	0.0	0.0		0.0	0.0			
0.0 0.0 <td>4</td> <td></td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td></td> <td>0.0</td> <td>0.0</td> <td></td> <td></td> <td></td>	4		0.0	0.0	0.0		0.0	0.0			
0.0 0.0 <td>52</td> <td>- 46</td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td></td> <td>0.0</td> <td>0.0</td> <td></td> <td></td> <td></td>	52	- 46	0.0	0.0	0.0		0.0	0.0			
0.0 0.0 <td>9</td> <td></td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td></td> <td>0.0</td> <td>0.0</td> <td></td> <td></td> <td></td>	9		0.0	0.0	0.0		0.0	0.0			
0.0 0.0 0.0 0.0 0.0 0.0 0.1 0.0 1.5 1.64 0.00 0.0 0.0 0.0 0.0 0.0 0.0 1.5 1.64 0.00% 0.00% 0.00% 0.00% 3.50% 0.00% 93.00% 84 80 37 98 80 80 83 9.25 3.43 2.80 0.00 77.19	7		0.0	0.0	0.0		0.0	0.0			
0.0 0.0 0.0 0.1 0.1 0.1 0.1 0.1 0.1 0.1 0.1 0.1 0.1 0.1 0.1 0.1 0.1 0.1 0.1 0.1 0.0 <td>8</td> <td>3</td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td></td> <td>0.0</td> <td>0.0</td> <td></td> <td>-</td> <td></td>	8	3	0.0	0.0	0.0		0.0	0.0		-	
1.64 0.00% 0.00% 0.00% 3.50% 3.50% 0.00% 93.00% 80.00% 83 0.00 0.00 0.00 0.00 77.19	Area Totals	0.0	0.0	0.0	0.0	0.1	0.1	10	0.0	1.5	
0.00% 0.00% 0.00% 3.50% 3.50% 0.00% 93.00% 84 80 32 98 80 83 0.00 0.00 0.00 3.43 2.80 0.00 77.19 9.25 1.00 1.00 1.00 1.00 1.00 1.00	Total Soil Type 2	1.64									
0.00 0.00% 3.50% 3.50% 0.00% 93.00% 9	a Calif to Tatal		, , ,	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,							
98 80 83 0.00 0.00 0.00 3.43 80 83 9.25	ra spill to lotal		0.00%	0.00%	0.00%		3.50%	3.50%	%00.0		100.00%
9.25 8.43 0.00 77.19	CN tor use Class		84	80	32		86	80	80		
9,25 0.09 km3	Composite CN:		0.00	0.00	0.00		3.43	2.80	0.00	77.19	83.42
km3	Total Catchment Area	9.25									
		90.0	Pm2								37.86

WH1 - PRE DEVELOPMENT - SPLIT OF AREA FOR CN CALCULATION:
Sub Soil Description: Berea Red Formation Hyrological soil group:

Conciouration Building Area (bit) Soft Landscaping (bit) Permeable Paving (bit) Gross Area (bit) Impermeable Safetic (bit) Conciouration Conci			Devel	Developed Area			Donel Complements				
Catchment(ba) Ruilliff Area (ba) Soft Landscaping (ba) Gross Area (ba) Impermebble Softiae (ba) Soft Landscaping (ba) Gross Area (ba) Impermebble Softiae (ba) Soft Landscaping (ba) Gross Area (ba) Catchment(ba) Cat		Gross Area in					Road Servicades		Open Space	Cane Areas	
Continue	Sub-Precinct	Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
6-46 6-46			60%	10%	30%	0.45	20%	20%	00.0	5.01	
Color Colo	-		0.00	0.00	0.00	0.45	0.23	0.23	200	100	
Control Cont	2	200	00'0	0.00	0.00		00.0	000			
Continue	e		0.00	00.00	0.00		00'0	00.0			
Charle C	4		0.00	00'0	00'0		0.00	0.00			
6.46	in		0.00	0.00	0.00		0.00	000			
Column C	9		00.00	0.00	0.00		0.00	000			
6.46 0.000	7		00.00	0.00	0.00		0.00	00.0			
6.46 0.000	80	000	0.00	0.00	00'0		0.00	000			
6.46 0.000% 0.000% 3.50% 3.50% 3.50% 9.8 3.50% 9.00 9.00 9.00 9.00 9.00	Area Totals	0.00	0.00	00'0	00'0	0.45	0,23	0.23	000	F.M.	
Cartchment(ha) Foundation Hymological soil groups 25 25 25 25 25 25 25 2	Total Soil Type 1 Catchment Area	6.46								70.0	
The color of the	% Split to Total		0.00%	0.00%	%00.0		3.50%	3 50%	7600	93.00%	10000
Carcimentinal Hypological soil group: D Carcimentinal Hypological soil group: D Carcimentinal Hypological soil group: D Carcimentinal Coros Area (na)	CN for use Class		7.1	39	25		86	39	39	75	00:001
Gross Area In Catchment(ha) Building Area (back) Soft Landscaping (ha) Permeable Paving (ha) Gross Area (ha) Road Servitudes Gop (ha) Catchment(ha) Condition (ha) Change (ha)	Composite CN:		0.00	0.00	00.00		3.43	1.37	0.00	23.25	28.05
Cart-Information Building Area (ha) Soft Landscaping (ha) Permeable Paving (ha) Impermeable Surface (ha) Impermeable Surface (ha) Soft Landscaping (ha) Cart-Information	b Soil Description:	Vryheid Formation	Hyrological soil group:	a							
Gross Area in Catchment(ha) Building Area (ha) Soft Landscaping (ha) Permeable Paving (ha) Gross Area (ha) Impermeable Sufface (ha) Soft Landscaping (ha) Catchment(ha) Catchment(ha) Soft Landscaping (ha) Catchment(ha) <			Develo	oped Area			Road Servitudes		Onen Spare	Cana Areas	
60% 10% 30% 0.15 50% 50% 50% 105 105 0.0	Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
0.00 0.00 <th< td=""><td></td><td></td><td>60%</td><td>10%</td><td>30%</td><td>0.15</td><td>20%</td><td>20%</td><td>0.00</td><td>1 95</td><td></td></th<>			60%	10%	30%	0.15	20%	20%	0.00	1 95	
0.0 0.0 <td>1</td> <td></td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td></td> <td>0.07</td> <td>0.07</td> <td></td> <td>-</td> <td></td>	1		0.0	0.0	0.0		0.07	0.07		-	
0.00 0.00 <th< td=""><td>2</td><td></td><td>0.0</td><td>0.0</td><td>0.0</td><td></td><td>0.0</td><td>0.0</td><td></td><td></td><td></td></th<>	2		0.0	0.0	0.0		0.0	0.0			
0.0 0.0 <td>m</td> <td></td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td></td> <td>0.0</td> <td>0.0</td> <td></td> <td></td> <td></td>	m		0.0	0.0	0.0		0.0	0.0			
0.00 0.00 <th< td=""><td>4</td><td></td><td>0.0</td><td>0.0</td><td>0.0</td><td></td><td>0.0</td><td>0.0</td><td></td><td></td><td></td></th<>	4		0.0	0.0	0.0		0.0	0.0			
0.00 0.00 <th< td=""><td>15</td><td></td><td>0.0</td><td>0.0</td><td>0.0</td><td></td><td>0.0</td><td>0.0</td><td></td><td></td><td></td></th<>	15		0.0	0.0	0.0		0.0	0.0			
0.0 0.0 <td>9</td> <td></td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td></td> <td>0.0</td> <td>0.0</td> <td></td> <td></td> <td></td>	9		0.0	0.0	0.0		0.0	0.0			
0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 2.0 2.0 2.10 0.00% 0.00% 0.00% 0.00% 0.00% 3.50% 0.00% 93.00% 84 80 32 98 80 80 83 0.00 0.00 0.00 3.43 2.80 0.00 77.19	1		0.0	0.0	0.0		0.0	0.0			
0.0 0.0 0.0 0.0 0.1 0.1 0.1 0.0 2.0 2.10 0.00% 0.00% 0.00% 0.00% 3.50% 3.50% 0.00% 93.00% 84 80 32 98 80 83 8.56 3.43 2.80 0.00 77.19	80	- SAID	0.0	0.0	0.0		0.0	0.0			
2.10 0.00% 0.00% 0.00% 3.50% 3.50% 93.00% 84 80 32 98 80 83 0.00 0.00 0.00 0.00 77.19 8.56	Area Totals	0.0	0.0	0.0	0.0	0.1	0.1	0.1	0.0	2.0	
84 80 3.50% 3.50% 3.50% 93.00% 93.00% 83.00% 80.00 0.00 0.00 0.00 3.43 2.80 0.00 77.19 8.56	Catchment Area	2.10									
84 80 32 5.50% 0.00% 93	% Split to Total		0.00%	%00'0	0.00%		2 500%	1001	,,,,,,,		
8.56	CN for use Class		84	80	32		86	3.30% 80	800%	33.00%	100.00%
8.56	Composite CN:		0.00	00'0	0.00		3.43	2.80	0.00	77.19	83.42
0.006 Local	tal Catchment Area	8.56									
		9000									41.63

WH1A - PRE DEVELOPMENT - SPLIT OF AREA FOR CN CALCULATION:
Sub Soil Description: Berea Red Formation Hyrological soil group:

		Develo	Developed Area			Road Servitudes		Open Space Cane Areas	Cana Areac	
Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
		%09	10%	30%	0.19	20%	20%	000	2.49	
-		0.00	0.00	0.00	11.0	0.09	0.09	202		
2	-	0.00	0.00	0.00		0.00	0.00			
m		0.00	0.00	0.00		0.00	0.00			
4		00.0	0.00	0.00		0.00	0.00			
ın		0.00	0.00	0.00		0.00	000			
9	000	0.00	0.00	0.00		0.00	000			
7		0.00	0.00	0000		00:00	00.0			
88		0.00	0.00	0.00	Section 1	00:0	000			
Area Totals	00'0	00'0	00'0	0.00	0.19	60.0	000	000	2.40	
Total Soil Type 1 Catchment Area	2.68									
% Split to Total		0.00%	0.00%	0.00%		3.50%	3 50%	2000	93 00%	200000
CN for use Class		7.1	39	23		86	30	30	25	100.001
Composite CN:		0.00	0.00	0.00		3.43	1.37	0.00	23.25	28.05
and son describtions	vryneid Formation	riyrological soil group:	Operational Area							
Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	Open Space (ha)	Cane Areas (ha)	
		%09	10%	30%	0.08	20%	20%	000	1.03	
F		0.0	0.0	0.0		0.04	0.04		Jan.	
7		0.0	0.0	0.0		0.0	0.0			
8		0.0	0.0	0.0		0.0	00			
4	+	0.0	0.0	0.0		0.0	0.0			
ı,	*	0.0	0.0	0.0		0.0	0.0			
9		0.0	0.0	0.0		0.0	0.0			
7		0.0	0.0	0.0		0.0	0.0			
80	TO .	0:0	0.0	0.0		0.0	0.0			
Area Totals	0.0	0.0	0.0	0.0	0.1	0.0	00	00	10	
Total Soil Type 2 Catchment Area	1.09							200	0.1	
% Split to Total		0.00%	0.00%	0000		3 500/	202.0	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		1
CN for use Class		84	80	32		9/20/2	5,30%	0.00%	93.00%	100.00%
Composite CN:		0.00	0.00	000			00 0	20	83	
				2000		5,43	7.80	0.00	77.19	83,42
Total Catchment Area	3.77									
										AA 08

WH2 - PRE DEVELOPMENT - SPLIT OF AREA FOR CN CALCULATION: Sub Soil Description: Berea Red Formation Hyrological soil group:

		Devel	Developed Area			Road Servitudes		Onen Snaco	Cana Asses
	Gross Area in	Desilydian Asses (Las)						about share	Calle Aleas
Sub-Precinct	Catchment(ha)	onlining Area (na)	Sort Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)
		60%	10%	30%	0.23	20%	20%	00.0	0.10
1	000	0.00	00:00	0.00	211	012	0.13	0010	2776
2		0.00	0.00	0.00		000	200		
e		0.00	0.00	0.00		00.0	000		
4	- 000	0.00	0.00	0.00		0.00	00.0		
S		0.00	0.00	0.00		0.00	00.0		
9		0.00	0.00	0.00		0.00	000		
1		0.00	0.00	00'0		0.00	000		
8	200	0.00	0.00	00.0		0.00	900		
Area Totals	0.00	00:0	0.00	0.00	0.23	0.12	0.12	000	213
Total Soil Type 1	3.35						2410	200	2776
% Solit to Total		7600.0	/000 C	,000					
			2000	0,0078		3.30%	3.50%	0.00%	93.00%
CN for use Class		7.1	39	25		98	39	39	25
Composite CN:		0.00	0.00	0.00		3.43	1.37	0.00	23.25
Sub Soil Description:	Vryheid Formation	Vryheld Formation Hyrological soil group:	0						

our son pescription:	- 1	vryneid Formation Hyrological soil group:	D							
		Devek	Developed Area			Road Servitudes		Onen Chare Cana Areas	Cana Areas	
Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	_	(ha)	
		%09	10%	30%	0.15	20%	20%	0.00	2.02	
1		0.0	0.0	0.0	10	0.08	0.08			
2		0.0	0.0	0.0		0.0	0.0			
8		0.0	0.0	0.0		0.0	0.0			
4		0.0	0.0	0.0		0.0	0.0			
50		0.0	0.0	0.0		0.0	0.0			
9		0.0	0.0	0.0		0.0	0'0			
7		0.0	0.0	0.0		0.0	0.0			
80	100	0.0	0.0	0.0	200	0.0	0.0			4
Area Totals	0.0	0.0	0.0	0.0	0.2	10	0.1	000	0.0	
Total Soil Type 2 Catchment Area	2.17							25	0.2	
% Split to Total		%00.0	0.00%	0.00%		3.50%	3.50%	%00.0	93.00%	100.00%
CIN IOI USE CIASS		48	80	32		86	80	80	83	
composite CN:		0.00	0.00	0.00		3.43	2.80	0.00	77.19	83.42
	653									
Total Catchment Area	30.0									- BRAN
		The second secon								40.04

49.84

km2

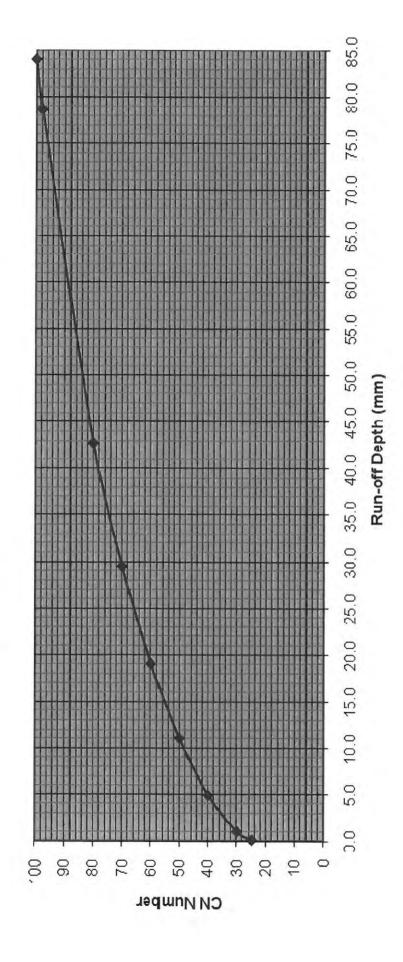
5.52 90.0

WH2A - PRE DEVELOPMENT - SPLIT OF AREA FOR CN CALCULATION: Sub Soil Description: Berea Red Formation Hyrological soil group:

		Develo	Developed Area			Road Servitudes		Open Space Cane Areas	Cane Areas	_
Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	_
		%09	10%	30%	1.03	20%	2005		1.00	
1		0.00	0.00	0.00		0.51	20.0	on'n	13,05	
2	9.6	0.00	0.00	0.00		000	0.00	1000	200	
m		0.00	0.00	0.00		900	000			
4		0.00	0.00	0.00		000	00.00			
2	- 900	0.00	0.00	0.00		000	0.00			
9		0.00	0.00	00.00		00.0	00.0			
7		0.00	0.00	000		00:0	0.00			
8	- 0.0	0.00	0.00	0.00		00.0	0.00			
Area Totals	0.00	0.00	0.00	0.00	1.03	0.00	0.00			
Total Soil Type 1 Catchment Area	14.68					100	15.0	0.00	13.65	_
% Split to Total		0.00%	0.00%	7600 0		200				
CN for use Class		71	330	25		3.50%	3.50%	0.00%	93.00%	100.00%
Composite CN:		0.00	0.00	3 6		0 6	£ .	39	23	
) ;		0.00		3.43	1.37	0.00	23.25	28.05
Sub Soil Description:	Vryheid Formation	Hyrological soil group:	D							
		Develo	Developed Area			Road Servitudes		Open Space	Cane Areas	-
Sub-Precinct	Gross Area in Catchment(ha)	Building Area (ha)	Soft Landscaping (ha)	Permeable Paving (ha)	Gross Area (ha)	Impermeable Suface (ha)	Soft Landscaping (ha)	(ha)	(ha)	
		%09	10%	30%	0.07	20%	20%	0.08	400	
1		0.0	0.0	0.0	10	0.03	0.03	a)		
2		0.0	0.0	0.0		0.0				
9		0.0	0.0	0.0		000	000			
4		0.0	0.0	0.0		0.0	00			
יע		0.0	0.0	0.0		000	000			
9		0.0	0.0	0.0		0.0	0.0			
7		0.0	0.0	0.0		0.0	00			
80	10	0.0	0.0	0.0		0.0	00			
Area Totals	0.0	0:0	0.0	0.0	0.1	0.0	0.0	1.0	0.1	
Catchment Area	1.12									
% Split to Total		0.00%	0.00%	7000		-				
CN for use Class		84	80	32		5.07%	3.07%	87.72%	6.14%	100.00%
Composite CN:		0.00	000	000		96	08	80	83	
0000				0.00		3.01	2.46	70.18	5,10	80.74
Total Catchment Area	15.80									

ANNEXURE E5 Typical Run-Off Depth for Various CN Numbers

Typical Run-off Depth for Various CN Numbers



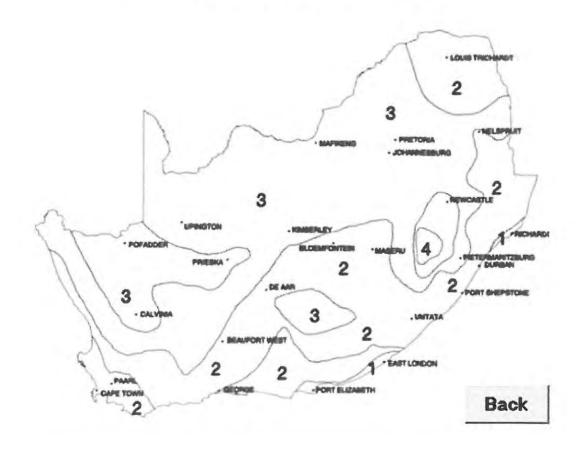
ANNEXURE E6

RAINFALL STATION DATA AND DESIGN RAINFALL INTENSITY DISTRIBUTION MAP

F1: Rainfall Station Data

General Rainfall	Station Information	on Please Choose	e from the list	of closest	Rainfall Statio	ons			0
Station Number	SAWB Code	Station Name	DG MN D	37.5	MAP (mm)	Altitude (n	n) Ye	ars D	istance (km)
@ 1	241302	FRASERS	29.32.31	111	971	76	46	7	.6
C 2	241068	NO NAME	29 38 31	13	854	122	44	1	0.5
C 3	241131	BLACKBURN	29 41 31	5	922	15	62	1	21
C 4	241163	NO NAME	29 43 31	6	1043	15	27	1	4.8
C 5	241103	CORNUBIA	29 43 31	4	998	91	62	1	6.1
			, 1-D	ay Design	Rainfall Dept	h (mm) for the f	ollowing	Return Pe	riods
THE CATCHM MAP = 970 m	ENT ALTITUDE	= 85 m and	2	5	10	20	50	100	200
			84	122	153	188	241	287	340
seculed location	ves the 5 closes 1. When choosin	g a station, you	86	133	173	218	290	354	428
	onsider the dista rd, the MAP and		85	12€	161	201	262	316	378
			80	114	141	170	215	253	296
			89	132	168	208	270	326	389

F2: Design Rainfall Intensity Distribution Map



ANNEXURE E7

SUMMARY OF THE INPUT DATA IN THE ANALYSIS AND OUTPUT RESULTS USING SWMM MODEL

Part	on crement Descripti	Area Ura	inage W	Area Urainage Weighted Conductivity Drying Average Equivalent Impervious Impervious	ductivity D	rying A	verage E	quivalent In	pervions In	npervious	Impervious	Impervious	Pervious	Pervious	Curb &	Rain Gage	Total	Total	Total	Total	Total	Total	Peak	Time
Minipary Minipary Minipary Minipary Dept. Roughings De		No		Curve		Time	Slope	Width	Area	Area	Area	Area	Area	Area			ecipitation	Zunon E	vaporation	Infiltration	nfiltration	Runoff	Runoff	
				lumber						No L	Depression	Manning's	Depression	Manning's	_									Concentration
Columbial Carrello										epression	Depth			Roughness										
Column C		(ha)					(%)	(m)	(%)	(%)	(mm)		(mm)		(m)		(mm)	(mm)	(mm)	(mm)	(ma)	ı	femel fd	and physical
13 4927 1000 700 28900 23129 0.00 0.0156 38000 0.00 0.0 FRASER 25:00 0.00 0.00 23740 23757 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0	Ξ	17.33	90	69.12	4.0000		8.2700	174.40	00:0	00'0	2,0000	0.0150	3 8000	0.1000	000	FRASER	25.00	000	00000	23 5500	AD CRON		000	00.00.0
07 69 44 4 10000 7 10 2 85 04 4 0000 7 10 2 85 04 0 00 2 60 00 0 0000 2 34 72 2 80 0 0 00 2 60 00 0 0000 2 74 67 0 00 0 0000 2 34 72 2 80 0 0 0 0 0000 2 74 67 0 00 0 0000 2 74 60 0 00 0 0000 2 74 60 0 00 0 0000 2 74 60 0 00 0 00000 0 0000 0 00000	HCI	10.01	13	40.27	4.0000		9,2600	231.29	0.00	00.0	2,0000	0.0150	3 8000	0.1000	000	FRASER	25.00	000	00000	23 7340	227 3766	000	000	0 03.49.
08 44 67 4 0000 700 200 100 FRASER 25 00 0.00 0.00 23 7450 45 00 0.00 23 7450 45 00 0.00 23 7450 45 00 0.00 20 00 0.00 0.00 20 00 0.00 <td>HN1</td> <td>11.93</td> <td>20</td> <td>69.44</td> <td>4.0000</td> <td></td> <td>2.8900</td> <td>285.04</td> <td>00.00</td> <td>00.0</td> <td>2 0000</td> <td>0.0150</td> <td>3 8000</td> <td>0 1000</td> <td>000</td> <td>FRASER</td> <td>25.00</td> <td>000</td> <td>00000</td> <td>93 4720</td> <td>2000 24</td> <td>00.0</td> <td>00.0</td> <td>0 02 143</td>	HN1	11.93	20	69.44	4.0000		2.8900	285.04	00.00	00.0	2 0000	0.0150	3 8000	0 1000	000	FRASER	25.00	000	00000	93 4720	2000 24	00.0	00.0	0 02 143
03 46.56 4 0000 700 94.30 16.68 0 00 0.015 25.00 0 00 0.00 23.731 25.00 0.00 23.731 25.00 0.00 0.00 23.731 25.00 0.00 0.00 23.731 25.00 0.00 0.00 23.731 25.00 0.00 0.00 23.731 25.00 0.00 <th< td=""><td>HN2</td><td>19.00</td><td>80</td><td>44.67</td><td>4.0000</td><td></td><td>4.0100</td><td>219.18</td><td>00.0</td><td>00.00</td><td>2.0000</td><td>0.0150</td><td>3 8000</td><td>0.1000</td><td>000</td><td>FRASER</td><td>25.00</td><td>000</td><td>00000</td><td>23.4120</td><td>4511 55</td><td>070</td><td>000</td><td>0 03.07</td></th<>	HN2	19.00	80	44.67	4.0000		4.0100	219.18	00.0	00.00	2.0000	0.0150	3 8000	0.1000	000	FRASER	25.00	000	00000	23.4120	4511 55	070	000	0 03.07
04 28 05 4 0000 700 6 5400 140 54 0.00 0.00 2 000 0.00 2 000 0.00 2 000 0.00 2 000 0.00	HN3	5.27	03	46.36	4.0000		9.4300	166,83	00.00	00.00	2 0000	0.0150	3.8000	0 1000	000	FRASER	25.00	000	00000	23.7310	1250.62	000	0000	0 04.52
17 37.83 4 0000 7.00 56400 32111 0.00 0.00 2 0000 0.0150 3.8000 0.1000 0.00 0.0 FRASER 25.00 0.00 0.0000 237.440 247.64 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0	HN4A	3.65	8	28.05	4.0000		6.5400	140.54	00:00	00.00	2.0000	0.0150	3,8000	0.1000	000	FRASER	25.00	000	00000	24 0760	20,0021		000	
1.5 1.5	HN4B	10,14	01	37.83	4.0000		5,6400	321,11	00.00	00'0	2.0000	0 0 0 1 5 0	3.8000	0 1000	00.0	FRASER	25.00	000	0.0000	23 7440	2407 64		000	0 02:00
17 3786 40000 700 113500 21795 0.00 25 00 2 0000 0.0150 38000 0.1000 0.00 FRASER 25 00 0.00 0.0000 237420 27420 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0	HS1	19.94	90	47.31	4.0000		7.2600	271.90	00'0	00:0	2.0000	0.0150	3.8000	0.1000	000	FRASER	25.00	000	00000	23 7310	4731 96	000		0 02.03.
102 36.01 01500 7.00 8.0000 186.00 0.00 0.0150 3.8000 0.0160 0.00 0.00 0.00 0.00 0.00 0.00	HS2	9.21	17	37.86	4.0000		1.3500	217.95	00'0	25.00	2,0000	0.0150	3.8000	0.1000	000	FRASER	25.00	000	00000	23 7420	2186 64	800	000	0 03 16
92 36 01 0150 70 8 0000 32730 000 000 0150 38000 0100 000 FRASER 25 00 000 0000 237470 4375 26 00 00 0000 23 7470 4375 26 00 00 0000 23 7470 4375 26 00 00 0000 23 7470 7180 00 00 00 00 0000 23 7470 7180 00 00 00 00 00 00 00 00 00 00 00 00 0	Sub-15	6.52	02	36.01	0.1500		8.0000	186.00	00'0	00.0	2 0000	0.0150	3.8000	0.1000	00.0	FRASER	25.00	000	00000	23.7470	1548 30	000	000	0 02:04:0
02 36 01 01500 7.00 8 0000 154 00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	HN4	20.53	02	36.01	0.1500		8,0000	327.00	00:00	00.00	0.0500	0.0150	3 8000	0.1000	000	FRASER	25.00	000	00000	23 7470	4875.26	000	000	0 00 56
02 36 01 01500 709 8 0000 136 02 0.000 23.470 778.90 0.000 23.477 778.90 0.00 0.000 23.477 778.90 0.0	Sub-18	2.66	02	36.01	0.1500		8.0000	154.00	00.00	00.0	0.0500	0.0150	3 8000	0.1000	000	FRASER	25.00	000	00000	23 7470	631.67	000	000	0 04.34
10 41.63 4.0000 7.00 3.7000 261.74 0.00 0.00 0.0160 3.8000 0.1000 0.00 0.00 0.00 0.00 0.00 0.	Sub-19	3.28	02	36.01	0.1500		8.0000	136.02	0.00	00:00	0.0500	0.0150	3.8000	0 1000	000	FRASER	25.00	000	00000	23.7470	778 00	8 6	000	0 00 20
94 44 08 4,000 700 92400 114,06 0.00 0.00 2,000 0.0150 3,8000 0.1000 0.00 FRASER 25,00 0.00 0.0000 23,7310 885,17 0.00 0.00 0.00 17,800 0.00 0.00 0.00 0.00 0.00 0.00 0.00	WH1	8.58	10	41.63	4.0000		3.7000	261.74	00:00	00:00	2.0000	0.0150	3.8000	0.1000	00:00	FRASER	25.00	000	0.0000	23 7440	2037.24	0000	000	
12 49.84 4.0000 7.00 8.0400 151.33 0.00 0.00 2.0000 0.0150 3.8000 0.1000 0.00 FRASER 25.00 0.00 0.0000 23.7400 1329.44 0.00 0.00 0.00 178.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00	WH1A	3.73	60	44.08	4.0000		9.2400	114.06	00:00	00:00	2.0000	0.0150	3.8000	0.1000	00:00	FRASER	25.00	0.00	0,0000	23.7310	885 17	000	000	0 01:54:0
11 31.77 4.0000 7.00 2.1000 259.29 0.00 0.00 2.0000 0.0150 3.8000 0.1000 0.00 FRASER 25.00 0.00 0.0000 23.9040 378.78 0.00 0.00 0.00 0.00 0.00 0.00 0.00	WH2	2.60	12	49.84	4.0000	Ü	8.0400	151.93	00.0	00.00	2.0000	0.0150	3.8000	0.1000	00.00	FRASER	25.00	00.0	00000	23.7400	1329.44	000	000	0.02.07
22 72.00 0.1500 7.00 9,0000 150.00 25.00 0.0500 0.0100 5,0000 0.1000 0.00 FRASER 25.00 0.00 0.0000 17,8000 662.16 6.26 0.10 1.0 72.00 0.1500 7.00 88800 134.00 25.00 25.00 0.0500 0.0100 5,0000 0.1000 0.00 FRASER 25.00 0.00 0.0000 17,8000 96.25 0.13 0.13 0.13 0.10 72.00 0.1500 7.00 88800 134.00 25.00 25.00 0.0500 0.0100 5,0000 0.1000 0.00 FRASER 25.00 0.00 0.0000 17,8000 642.68 6.26 0.13 0.13 0.10 72.00 0.1500 7.00 88800 134.00 25.00 25.00 0.0500 0.0100 5,0000 0.1000 0.00 FRASER 25.00 0.000 0.0000 17,8000 642.68 6.26 0.09 0.13 0.13 0.13 0.13 0.13 0.13 0.13 0.13	WHZA	15.85	11	31.77	4.0000	7.00	2.1000	259.29	0.00	00.00	2.0000	0.0150	3.8000	0.1000	00.00	FRASER	25.00	000	00000	23 9040	3788 78	000	000	0 04:18:
21 72.00 0.1500 7.00 7.8000 208.00 25.00 25.00 0.0500 0.0100 5.0000 0.1000 0.00 FRASER 25.00 0.00 0.000 17.8000 906.02 2.56 0.13 0 10 72.00 0.1500 7.00 8.8000 134.00 25.00 25.00 0.0500 0.0100 5.0000 0.1000 0.00 FRASER 25.00 0.00 0.0000 17.8000 642.58 6.28 0.09 0 10 72.00 0.1500 1.15000 134.00 25.00 0.0500 0.0100 5.0000 0.1000 0.00 FRASER 25.00 0.00 0.0000 17.8000 642.58 6.28 0.09 0 10 10 10 10 10 10 10 10 10 10 10 10 1	WH3	3.72	22	72.00	0.1500		000006	150.00	25.00	25.00	0.0500	0.0100	5.0000	0.1000	0.00	FRASER	25.00	000	00000	17 8000	662.16		0.00	0 04-24-6
10 72.00 0.150 7.00 8.8000 134.00 25.00 25.00 0.0500 0.0100 5.0000 0.1000 0.00 FRASER 25.00 0.000 17.8000 642.58 6.26 0.09 0	WH4	5.09	21	72.00	0.1500		7.8000	208.00	25.00	25.00	0.0500	0.0100	5.0000	0.1000	0.00	FRASER	25.00	000	00000	17 8000	906.02		2,0	0 01:24:
Volume infiltration = 43312 m ³	WH5	3.61	10	72.00	0.1500		8.8000	134.00	25.00	25.00	0.0500	0.0100	5.0000	0.1000	00.0	FRASER	25.00	000	0.000	17.8000	642 FB	6.26	00.0	0 01.24
		185.65																	Volume int	iltration =		I'E	00.0	0.01.20.0

25mm Rainfall Pre-Development Runoff

Time Slope Width Area (days) (m) (m) (m) 700 82700 17440 0.00 700 92600 23129 0.00 700 4,0400 285.04 0.00 700 4,0400 285.04 0.00 700 4,0400 219.18 0.00 700 5,400 32.11 0.00 700 5,6400 32.11 0.00 700 5,6400 32.11 0.00 700 6,5400 24.79 0.00 700 8,0000 24.79 0.00 700 8,0000 186.00 0.00 700 8,0000 154.00 0.00 700 8,0000 156.02 0.00 700 8,0000 136.02 0.00	Area Man Depth Rour (mm) 2 0000 2.0000 2.0000	Area								
Number (mm/h) (days) (%) (m) (%) (m) (%) (m) (days) (%) (m) (days) (%) (m) (days) (days	Depth F (mm) 2 0000 2 0000 2 0000 2 0000		Area Gutter	ID Pr	Precipitation Runon	non Evapora	Evaporation infiltration infiltration Runoff Runoff	n infiltration	Runoff Run	off
(6912 40000 700 82700 17440 0000 4027 40000 700 82700 17440 0000 8244 40000 700 92600 23129 000 9244 40700 700 92600 23129 000 92600 700 92600 23129 000 92600 700 92600 23129 000 92600 700 92600 23129 000 92600 700 94000 700 94300 16683 000 92600 700 92600 221719 000 92600 700 700 92600 221719 000 92601 01500 700 13600 13600 000 92601 01500 700 13000 134000 000 92601 01500 700 80000 13500 000 92601 01500 700 80000 13600 000 92601 01500 700 80000 13600 13600 000 92601 01500 700 80000 13600 13600 000 92601 01500 700 80000 13600 13600 000 92601	2 0000 2 0000 2 0000	Depression	Manning's Length							Concentration
Control Cont	2 0000 2 0000 2 0000	s Depth	Roughness							
6912 4 0000 7 00 82700 17440 0 00 69 44 4 0000 7 00 9 2600 23129 0 00 69 44 4 0000 7 00 9 2600 2313 0 00 4 4 67 4 0000 7 00 9 4300 168 0 00 2 8 63 4 0000 7 00 9 4300 166 83 0 00 2 8 63 4 0000 7 00 9 4300 140 54 0 00 3 7 83 4 0000 7 00 5 6400 37.11 0 00 3 7 8 4 0000 7 00 7 2600 27.75 0 00 3 7 8 4 0000 7 00 7 2600 27.75 0 00 3 6 01 0 1500 7 00 8 0000 217.95 0 00 3 6 01 0 1500 7 00 8 0000 154 00 0 00 3 6 01 0 1500 7 00 8 0000 154 00 0 00 3 6 01 0 1500 7 00 8 0000 154 00 <	2 0000	(mm)	(m)		(mm)	mm) (r	mm) (mm)	I'm)	(mm)	care if days his marse
40.27 40.000 700 92.600 231.29 0.00 69.44 40.000 700 28900 285.04 0.00 46.36 4.0000 700 94.300 166.83 0.00 28.05 4.0000 700 94.300 166.83 0.00 37.83 4.0000 7.00 5.400 37.11 0.00 47.31 4.0000 7.00 5.400 271.90 0.00 37.86 4.0000 7.00 13.500 271.90 0.00 35.01 0.1500 7.00 140.50 271.90 0.00 36.01 0.1500 7.00 80000 186.00 0.00 36.01 0.1500 7.00 80000 154.00 0.00 36.01 0.1500 7.00 80000 154.00 0.00 36.01 0.1500 7.00 8.0000 154.00 0.00 36.01 0.1500 7.00 8.0000 154.00 0.00	2 0000	0.0150 3.8000	0.1000 0.00	FRASER			57	917	30.04	37 n n2:21:22
69 44 4 0000 700 285 04 0 00 44 67 4 0000 700 4 0100 219 8 000 46 84 4 0000 700 4 4010 168 8 000 28 05 4 0000 700 65400 140 54 000 37 83 4 0000 700 65400 32111 0.00 47 31 4 0000 700 17860 271 9 0.00 37 86 4 0000 700 113500 271 90 0.00 36 01 0 1500 700 80000 186 00 0.00 36 01 0 1500 700 80000 154 00 0.00 36 01 0 1500 700 80000 154 00 0.00 36 01 0 1500 700 80000 154 00 0.00	2 0000		_	FRASER	84 00				10.52	0.06 0.01.22.59
4467 4000 700 4100 219.18 0.00 4636 40000 700 94300 166.83 0.00 28.05 40000 700 56400 140,54 0.00 37.83 40000 70 7560 6700 277.90 0.00 37.86 40000 70 7260 277.90 0.00 36.01 0.1500 70 80000 247.95 0.00 36.01 0.1500 70 80000 247.95 0.00 36.01 0.1500 70 80000 327.90 0.00 36.01 0.1500 70 80000 154.00 0.00 36.01 0.1500 70 80000 154.00 0.00 36.01 0.1500 70 80000 154.00 0.00				FRASER	84 00	000			30.71	0 0
46.36 4,0000 7.00 9,4300 166.83 0.00 2.8.05 4,0000 7.00 6,5400 140.54 0.00 47.31 4,0000 7.00 7,2600 271.90 0.00 47.31 4,0000 7.00 7,2600 277.90 0.00 37.86 4,0000 7.00 11.3500 217.36 0.00 36.01 0.1500 7.00 80000 186.00 0.00 36.01 0.1500 7.00 80000 154.00 0.00 36.01 0.1500 7.00 80000 154.00 0.00 36.01 0.1500 7.00 8.0000 154.00 0.00 36.01 0.1500 7.00 8.0000 154.00 0.00 36.01 0.1500 7.00 8.0000 136.02 0.00	2.0000			FRASER	84 00			-	11 70	0 0
28 05 4,0000 700 65400 140,54 0.00 37.83 4,0000 700 65400 32713 0.00 47.31 4,0000 7.00 72600 27130 0.00 37.86 4,0000 7.00 113500 27130 0.00 36.01 0,1500 7.00 80000 186,00 0.00 36.01 0,1500 7.00 80000 154,00 0.00 36.01 0,1500 7.00 80000 154,00 0.00 36.01 0,1500 7.00 8,0000 154,00 0.00	2.0000			FRASER	84 00				14.02	
37 83 4,0000 7 00 5,6400 32.111 0.00 47.31 4,0000 7 00 7.2600 27.19 0.00 37 86 4,0000 7 00 11.3500 217.95 0.00 36 01 0.1500 7 00 8,0000 247.95 0.00 36 01 0.1500 7 00 8,0000 247.95 0.00 36 01 0.1500 7 00 8,0000 154.00 0.00 36 01 0.1500 7 00 8,0000 154.00 0.00 36 01 0.1500 7 00 8,0000 136.02 0.00	2.0000			FRASER	84 00				5 10	o C
47.31 4.0000 7.00 7.2600 271.90 0.00 37.86 4.0000 7.00 11.3500 277.95 0.00 36.01 0.1500 7.00 80.000 186.00 0.00 36.01 0.1500 7.00 80.000 327.70 0.00 36.01 0.1500 7.00 80.000 154.00 0.00 36.01 0.1500 7.00 8.0000 154.00 0.00 36.01 0.1500 7.00 8.0000 136.02 0.00	2.0000			FRASER	84.00	00.0		-		0 0
37.26 4.0000 7.00 11.3500 217.35 0.00 36.01 01500 7.00 8.0000 186.00 0.00 36.01 01500 7.00 8.0000 32.700 0.00 36.01 01500 7.00 8.0000 154.00 0.00 36.01 01500 7.00 8.0000 136.02 0.00	2.0000			FRASER	84.00			-	13.80 0	0.00.00 0 0.00.00
36.01 0.1500 7.00 8,0000 186.00 0.00 36.01 0.1500 7.00 8,0000 32.700 0.00 36.01 0.1500 7.00 8,0000 154.00 0.00 36.01 0.1500 7.00 8,0000 136.02 0.00	2.0000	0.0150 3.8000		FRASER	84.00				0.30	0 0
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36.01 0.1500 7.00 8.0000 154.00 0.00 36.01 0.1500 7.00 8.0000 136.02 0.00	0.0500	0.0150 3.8000		FRASER				-		0 0
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72.00 0.1500 7.00 8.8000 134.00 25.00	0.0500	10		FRASER						
						Volu	infilt		m ₃	

1:5 Yr Pre-Development Runoff

	Concentration	1	days nn mm.ss	3 6	0 01 39 20	0 02-19-24		0 00.36.31		0 01 08 38						0 00:32.31						0 00.44.39	Time	jo	Concentration		avs hh:mm.ss)	0 01:51:12	0 01:05:17	0 01:30:43	0 02:07:17	0 00 53 45	0 00:53:21	0 01:02:41	0 01:30:22	0 01:00:31	0 01 25 16	0 00:39:19	0 00:47:58	0 01:12:45		0 01:01:52	0 02:05:18	00.00.00
	unoff		O OB	0.00	0.84	030	0.03	000	0.04	0.40	0.43	0.21	0.14	670	90.0	0.03	0.14	0.05	0.43	000	1 10	0.13	Peak	House			(cms) (d.	1.64	0.51	1.39	0.58	0.43	0.11	0.47	0.04	0.28	0.62	0.18	0.18	0.42	0.27	0.47	0.24	4 00
	Kunoff F	1	57.24	24 70	58 14	27.46	30 90	14 51	30.00	20.02	20.00	56.22	50.38	20.44	20 00	25.83	28.50	34.33	15.94	76.00	76.87	76.85	Total	a Hount			(mm)	81.78	38.85	82.81	43.03	47.34	24.17	20.78	35.87	33.56	32.91	34.09	33.86	40,44	44.17	52.04	56.69	 100 00
	Itration	freed.	11009 75	9617.41	7474 74	7730 42	4738 31	3878 56	00.0700	70.405	1920.00	3041.33	6504.61	0004.93	2265 90	8144 22	37.44.02	4842 49	6620 94	1647.63	2255 23	1600.08	Total	traffon R			(m)		1307.80					0570469										1000 10 1
The second	ration inti	(minu)	5300	0780									٠	И					-				Total	ration Infi			(mm)		_				127.6120		4 6								-	48 2020
	Evaporation intilifation intilifation Runoff Runoff	(mm)	00000					-											-				Total	Precipitation Runon Evaporation Infiltration Infiltration Runoff Runoff			(mm)						0,000,0	- '	- •			-			-		0.000	
		luc.										000			000			0.00				0.00	lai	on Evapor			11)	0.00																
	ON KUN	(mm) (mm)			122 00 0																		Fotal Total	on Rund			m) (m						00 00				00 0 00			00.00			00.0	
The second	Precipitation Kunon	, m	122	122	122	122	122	122	199	122	122	122.00	122.00	122.00	122.00	122.00	122.00	122.00	122 00	122 00	122 00	122.00	To	Precipitati			(mm)	153.00	153	153 00	153 00	153.00	153,00	153.00	153.00	153.00	153	153.00	153	153.00	153.	153.00	153.00	3.4
3	2		FRASER	FRASER	FRASER	FRASER	FRASER	FRASER	FRASER	FRASER	FRASER	FDACED	EDACED	EDASED	FRASER	FRASER	FRASER	FRASER	FRASER	FRASER	FRASER	FRASER	Rain Gage	9				FRASER	FRASER	FRASER	FRASER	FRASER	FRASER	FDASER	FRASER	FRASER	FRASER	FRASER	FRASER	FRASER	FRASER	TRASER	Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z	エスダンアス
	Length	(m)	000	00.0	000	000	000	00.0	000	000	000	000	000	000	000	0.00	0.00	00.00	0.00	000	0.00	0.00	Curb & F	Gutter	Length		(m)	00.00	000	0000	00.0	00.0	000	000	000	000	00.00	0.00	000	0.00	0.00	0.00	00.0	1000
		Koughness	0.1000	0.1000	0.1000	0.1000	0.1000	0.1000	0.1000	0 1000	0.1000	0 1000	0 1000	00010	0 1000	0.1000	0.1000	0.1000	0.1000	0 1000	0.1000	0.1000	Pervious C	Area	-	Roughness		0.1000	0.1000	0 1000	0.1000	0 1000	0.1000	0.1000	0.1000	0 1000	0.1000	0.1000	0.1000	0.1000	0.1000	0.1000	0.1000	
		(mm)	3.8000	3.8000	3.8000	3,8000	3.8000	3.8000	3,8000	3 8000	3 8000	3 8000	3 8000	3 8000	3 8000	3.8000	3.8000	3.8000	3.8000	5.0000	5.0000	5.0000	Pervious	Area	Depression M		(mm)	3.8000	3.8000	3.8000	3,8000	3,0000	3.8000	3 8000	3,8000	3,8000	3.8000	3.8000	3.8000	3.8000	3.8000	3.8000	0.0000	OOOT C
Annual	Manning's Depression	ssaul	0.0150	0.0150	0.0150	0.0150	0.0150	0.0150	0.0150	0.0150	0.0150	0.0150	0.0150	0.0150	0.0150	0.0150	0.0150	0.0150	0.0150	0.0100	0,0100	0.0100		Area	Manning's Depi			0.0150	0.0150	0.0150	0.0150	00150	0.0130	0.0150	0150	0.0150	0.0150	0.0150	0.0150	0.0150	0.0150	0.0130	00100	0010
		in Koughness nl														ı							s Impervious			h Roughness																		
Assess	Depression	(mm)	2,0000	2,0000	2,0000	2.0000	2,0000	2.0000															Impervious	Area	Depression	Depth	(mm)	2.0000	2,0000	2.0000	2,0000	2,0000	2 0000	2,000	2 0000	2.0000	0.0500	0 0200	0.0500	2.0000	2,0000	2,0000	0.0500	30.5
Acces	No	Depression (%)	0.00	00'0	00'0	00.00	00:00	00.0	00.00	00.0	25.00	000	000	000	00.0	00:00	0.00	000	00:0	25.00	25.00	25.00	npervious	Area	No	Depression	(%)	000	000	0.00	000	000	900	000	25.00	0.00	00.0	00.0	00.0	0.00	0.00	0.00	25.00	 20.00
Acres		(%)	00'0	00'0	00.00	00.00	00'0	00'0	00.00	00.0	00.0	00.0	000	000	00:00	0.00	00'0	00.00	00:00	25.00	25.00	25.00	Impervious Impervious	Area			(%)	000	00.0	000	00.0	000	000	000	0.00	0.00	00.0	00'0	00.00	0.00	00.0	000	25.00	20.00
Wilseles		(m)	174.40	231 29	285.04	219.18	166.83	140.54	321.11	271.90	217.95	186.00	327.00	154.00	136,02	261.74	114.06	151.93	259.29	150.00	208.00	134.00	Equivalent Im	Width			(E)	174.40	231.29	285.04	166.83	140 54	321 11	271 90	217.95	186.00	327.00	154.00	136.02	261.74	151.00	259.29	150.00	20.00
Slone		(%)	8,2700	9.2600	2.8900	4.0100	9.4300	6.5400	5.6400	7.2600	1 3500	8.0000	8 0000	8.0000	8.0000	3.7000	9.2400	8.0400	2.1000	9.0000	7.8000	3.8000	Average Equ	Slope			(%)	8.2700	9.2600	2.8900	9.4300	5400	5.6400	7 2600	11.3500	0000	8 0000	3,0000	8 0000	3.7000	8.0400	1000	9 0000	00000
Time		(days)	Ų.		2.00					7.00						7.00							Drying Av	Time			J			00.7											7.00			
		mm/hr) (c	4,0000	4.0000	4.0000	4.0000	4.0000	4 0000	4.0000	4.0000	4.0000	0.1500	0.1500	0.1500	0.1500	4.0000	4.0000	4.0000	4.0000	0.1500	0.1500	0.1500						4.0000	4.0000	4.0000										4.0000				
Curve	Number		69.12	40.27	69.44	44.67	46.36	28.05	37.83	47.31	7.86	6.01	6.01	6.01	36.01	41.63	44.08	49.84	1,77	2.00	72.00	2.00	Weighted Conductivity	Curve	Number			69 12	40.27	69.44	46.36	28.05	37.83	7.31			36 01			41.63				
	Z															10 4									Nun																			
Modell																							Area Drainage	Node 1D			ľ	٥,	- 0		5 0	0	0	Ö	-	0	0	6	Ö,	č	12	+	2	
		(ha)	17.33	10.01	11.93	19.00	5.27	3.66	10.14	19.94	9.21	6.52	20.53	2.66	3.28	8.58	3.73	5.60	15.85	3.72	5.09	3.61					(ha)	17.33	10.01	19000	527	3.65	10.14	19.94	9.21	6.52	20.53	2.66	3.28	3.73	5.60	15,85	3.72	
0			H1	달	21	HNZ	HN3	4A	4B	S1	S2	15	74	18	19	F	1A	42	2A	43	-14	45	SN Element Descripti	0				F 2	3.2	- (1	5 2	IA.	18	1.0	32	15	14	20 (19	- 4	12	Y.	13	
				Ĭ	Ī:	Í.	Ī	HN4A	ŽI	HS1	HS2	Š			S	14 WH1	_	WHZ	WHZA	WH3	WH4	WHE	Eleme					Ē	2 1	L	HN3	HNA	HNA	HS1	HS2	Sup-15	HN4	Sub-18	Sup-19	WHIA	WHZ	WHZA	WH3	

1:50 Yr Pre-Development Runoff

NO	uo					Building					snowladim snowladim	enois ladim	S S S S S S S S S S S S S S S S S S S	Spointer	curo &	Pervious curb & Rain Gage	Total Total	lotal	Total	Total	Total	Fotal Total	Peak	Time
=	0	-	Node ID	Curve		Time	Slope	Width	Area	Area	Area	Area	Area	Area	Gutter	0	Precipitation Runon		Evaporation infiltration infiltration Runoff Runoff	ifiltration	nilliration	Runoff	Sunoff	
				Number					-	No No	Depression	Manning's		Manning's	Length									Concentration
		(ha)			(mm/hr)	(days)	(%)	(m)	1%)	(%)		Rouginess	mdan,	Kondnness	Anna A		No. of Contract of		Character	No.				
I	1	17.33	90	69.12	4.0000		8 2700	174.40	000	000	00000	COAEC	00000	04000	000	010407	(mm)	(mm)	[mm]	(mm)	(m ²)	(mm)	(cms) (d	cms) (days hh:mm.ss
H	-	10.01	13	40.27	4 0000	200	00920	224 20	000	0000	2,0000	0.0130	3.6000	0.1000	000	FRASER	241 00	000	0.0000	82,1060	14228.97	157.76	4.05	0 01:32:43
Z	-	11 03	20	60 44	4 0000		9.2000	201.23	0.00	000	2.0000	00100	3.8000	0001.0	00.00	FKASEK	241.00	00.00	00000	151,6330	15178,46	88.26	1.61	0 00:54:26
LINI		11.33	5 6	## NO	4,000		2.8900	50°C87	000	000	2.0000	0.0150	3.8000	0.1000	0000	FRASER	241.00	000	00000	80.8570	9646.24	159.06	3.34	0 01 15 38
NE .	7 (19:00	80	44.67	4,0000	_	4.0100	219.18	00'0	00'0	2.0000	0.0150	3,8000	0.1000	00'0	FRASER	241.00	0.00	0.0000	143.1300	27194.70	96.72	191	0 01.46.07
ZI.	5	5.27	63	46.36	4.0000		9.4300	166,83	00'0	00.00	2.0000	0.0150	3.8000	0.1000	00'0	FRASER	241.00	0.00	00000	136,6400	7200.93	103 32	1.20	0 00 44 48
FVI.	*	3.65	04	28.05	4.0000	00'2	6.5400	140.54	0000	00:0	2.0000	0.0150	3,8000	0.1000	00.0	FRASER	241.00	0.00	0.0000	180 2200	6578 03	59 71	0.40	0 00 44 20
IN4	m	10.14	01	37.83	4,0000	_		321.11	00'0	00.00	2.0000	0.0150	3.8000	0.1000	00.00	FRASER	241.00	000	0.0000	157 3910	15959 45	82.52	1 54	0 00-52-46
HS		19.94	90	47,31	4.0000			271.90	000	00.0	2,0000	0.0150	3.8000	0.1000	00.00	FRASER	241.00	0000	0.000	135 7000	27058 58	104 17	2 89	0 01-20-21
HS	2	9.21	17	37.86	4.0000	00'2 0	11.3500	217.95	00'0	25.00	2.0000	0.0150	3.8000	0.1000	00.00	FRASER	241 00	0.00	00000	157 2300	14480 88	82.67	1 44	0 00-50-27
Sup-1	5	6.52	05	36.01	0.1500		8.0000	186,00	00'0	00.0	2.0000	0.0150	3.8000	0.1000	00.0	FRASER	241 00	0.00	00000	161 6180	10537 49	78 28	0.05	0 00 50 06
ŻI	4	20.53	02	36.01	0.1500		8.0000	327.00	00'0	00.00	0.0500	0.0150	3.8000	0 1000	000	FRASER	241 00	000	0 0000	162 4300	33346 88	77.40	2 14	0 00 30.00
Sub-1	8	2.66	02	36.01	0.1500		8.0000	154.00	0.00	00.0	0.0500	0.0150	3.8000	0.1000	000	FRASER	241 00	0.00	00000	160 8970	4279 BE	20.07	0.54	77.55.00
Sub-1	0	3.28	02	36.01	0.1500	00.7 0	8.0000	136,02	00'0	00.00	0.0500	0.0150	3.8000	0.1000	000	FRASER	241 00	000	00000	161 1680	5286 31	78 74	0.56	0 00 32.47
MM	1	8.58	10	41.63	4.000C	00'7 0		261.74	0.00	0.00	2.0000	0.0150	3.8000	0.1000	000	FRASER	241 00	000	00000	148 6380	12753 14	04.26	1 22	0 04:00:30
WH1	A	3.73	60	44.08	4.0000			114,06	00:00	00.00	2.0000	0.0150	3.8000	0.1000	000	FRASER	241 00	000	00000	142 1830	5203 30	07.70	20.1	0 00.00.39
WH	2	5.60	12	49.84	4.0000		8.0400	151.93	0.00	0.00	2.0000	0.0150	3.8000	0.1000	000	FRASER	241.00	00.0	0.000	128 4890	7105 38	111 47	1 20	0 00:48:02
WH2A	A	15.85	11	31.77	4.0000	00'2 (2.1000	259.29	0.00	0.00	2.0000	0.0150	3.8000	0.1000	0.00	FRASER	241 00	000	00000	173 8860	27560 93	65.06	0.01	0 04:44:36
MM	3	3.72	22	72.00	0.1500	00.7	9.0000	150.00	25.00	25.00	0.0500	0.0100	5.0000	0.1000	0.00	FRASER	241 00	000	00000	55 9340	2080.23	184 52	2.36	0 00:33:03
WH4	4	5.09	21	72.00	0.1500	00'2 0	7.8000	208.00	25.00	25.00	0.0500	0.0100	5.0000	0.1000	0.00	FRASER	241 00	000	00000	55 9610	2848 41	184.48	2 10	0 00:33:03
WHS	5	3.61	10	72.00	0.1500	00'2 (8.8000	134,00	25.00	25.00	0.0500	0.0100	5,0000	0.1000	0.00	FRASER	241.00	000	00000	55 9750	2020 70	184 46	20.0	0 00:34:13

Time	lo political por		(days hh:mm:ss)	0 01:26:27	0 00-50-45	0 01 10 32	0 0138.57	0 00 41 47	0 00.41.28	0 00.48.44	0 01-14-55	0 00-47-03	0 00 46 43	0 00 46 43	0 00 30 34	0 00-37-17	000000	0 00.35	0 00:42:55	0 00:48:05	0 01:37:25	0 00:30:49	11 10 00 0
Peak	noff		eveb) (em	5.55	2.35	151	85	89	161	900	126	2.10	40	3.50	22	181	0.00	40.	01.1	.80	.44	3.00	00
Total P	Inoff Ru		(mm) (c	199.65		1 05	128.84	136.51	2.23	1 10	137.69	111 28	105.81			00 901	124.00	000	129.80	146.29	90.63	228.09	20 000
Total	Infiltration Runoff Runoff		(m)	14950.76 19						7730 60 11			746 63 10	37161 76 10						1825.12 14	941.58	176.39 22	
Total	Infiltration Infi		(mm)	86.2710 1.		_	- 2	_	0			-			, 				136.2490	- 1	195.2150 30		58 5190 °
Total			(mm)	8 0000	-		_	_	-	-							ľ				0000		25 0000 5
<u>=</u>	in Evaporation			0 00 0	0 00.0	U)	0	_	0 000					0 000								
Total Total	on Runon		n) (mm)		_	٠	_	_	_		Ĭ		_	_	_	_					,		000
Tot	Precipitation		(mm)	287.00	287.00	287.00	287.00	287.00	287.00	287.00	287.00	287.00	287.00	287 00	287.00	287.00	287.00	00.780	00.182	7.107	287.00	287.00	287 00
Rain Gage	Ō			FRASER	FRASER	FRASER	FRASER	FRASER	FRASER	FRASER	FRASER	FRASER	FRASER	FRASER	FRASER	FRASER	FRASER	FRASED	FDACED	TO A OT D	TRASER	LYASEK	FRASER
Curb & R	Gutter		(m)	0.00	00.0	00.0	00'0	00.0	000	00.0	0.00	0000	00.0	00'0	00'0	000	000	000	00.0	000	00.0	0.00	0.00
Pervious C	Area (Manning's L	Roughness		0.1000	0.1000	0.1000	0.1000	0.1000	0.1000	0 1000	0.1000	0 1000	0.1000	0.1000	0.1000	0 1000	0.1000	0.1000	0 1000	0.1000	0.1000	0.1000	0.1000
Pervious	Area epression M	ж.	(mm)	3.8000	3.8000	3.8000	3.8000	3.8000	3.8000	3.8000	3,8000	3.8000	3.8000	3.8000	3,8000	3.8000	3.8000	3 8000	3 8000	3 8000	3.9000	5.0000	5.0000
	Area Manning's Dep	hness		0210	0.0150	0.0150	0.0150	0.0150	0.0150	0.0150	0150	0150	0150	0.0150	0150	.0150	.0150	0150	0150	0150	0400	00100	00100
pervious Impervious		th Roughness					_		_			-	_	3				_					_
Ξ	Depres			2	2	2	2	2						0.0500									
mpervious	Area	Depression	(%)	00.0	00.0	00.0	00.0	00'0	00'0	00.00	00.0	25.00	000	00.0	00.00	00'0	0.00	0.00	0.00	000	25.00	22.00	75.00
snowadi	Area		(%)	00 0	0.00	00.00	00'0	00'0	0.00	00.00	00.0	00'0	00.00	00.0	00.00	00.0	00.00	00.00	00'0	000	25.00	25.00	25.00
urvalent in	Width		(m)	174.40	231.29	285 04	219.18	166.83	140.54	321.11	271.90	217.95	186.00	327.00	154.00	136.02	261.74	114.06	151,93	259 29	150.00	200.00	208.00
verage Eq	Slope	1,000	(1/0)	8 2700	9.2600	2.8900	4.0100	9.4300	6.5400	5.6400	7.2600	1.3500	8.0000	8.0000	8.0000	8.0000	3,7000	9.2400	8.0400	2,1000	0000	2 8000	0008.
nying A	Time			2.00								-		2.00							2 00		
ductivity D				4.0000	4.0000	4,0000	4,0000	4.0000	4.0000	4.0000	4.0000	4,0000	0.1500	0.1500	0.1500	0.1500	4.0000	4.0000	4.0000	4.0000	0.1500	0.1500	0.1300
Area Drainage Weighted Conductivity Drying Average Equivalent Impervious Impervious	Curve			69.12	40.27	69.44	44.67	46.36	28.05	37.83	47.31	37.86	36.01	36.01	36.01	36.01	41.63	44.08	49.84	31.77	72.00	72 00	12.00
age Wer	Node ID Nu		0.0	90	13	20								05				60	12	11	22	21	7
a Drain	Nod			2	1	33	0	2.	22	4	7	1	25	33	9	80	8	3	0	5	2	6	0
		1	(na)	17.	10.0	11.5	19 (2.5	3.6	10.1	19.5	5.6	9	20.53	2.6	3.2	8.5	3.7	5.6	15.8	3.7	50	2 1
SN Element Descripti							5	2	-	m's		2	15	**	60	6		1					
V Elemen	TI.				HC	3 HN1	HN,	NH S	HN4A	7 HN4E	HS HS	HS.	1-qnS 0	1 HN4	5 Sup-11	3 Sub-18	4 WH1	5 WH1A	S WHZ	7 WH2A	3 WH3	WH4	
ŝ			ľ		4		7	47.				J.	ĭ	F	1,	-	14	4)	16	17	18	19	

Time	Concentration	dawn of commend	0 01:51:18	0 01:36:52	0 01:09:44	0 01:43:56	0 00:00:00	12:15:00 0	0 00-40-52	0 02:37:51	0 04:54:16	0 03:50:40	0 02:59:46	0 03:08:55	0 01:02:48	0 01:31:32	0 02:28:53	0 02:31:59	0 02:20:03	0 01:28:12	0 01:53:12	0 01.48.16	0 02:20:09	0 01:02:10	0 01:35:40	0 01:39:24	0 01:34:07	0 05:10:49	0 01:55:28	0 01:56:45	0 01:20:44	0 01:23:52	0 02:39:09	0 02:57:24	0 02:11:29	0 02:35:10	0 01:32:59	0 02:08:46	0 05:15:43	0 02:35:53	0 01:38:47	0 02:19:57	0 07:04:27	0 02-17-58	0 01:56:34	0 01:14:39	0 03:42:38	0 03:31:37	0 04:08:24	0 02:58:54
Riteratif		Comment of	00.00	0.00	0.00	000	00.0	00.0	000	000	00.00	00.00	00.0	0000	000	0.00	0.00	0.00	0.00	0.00	000	0000	00.00	0.05	0.00	000	00.0	0.00	0.00	000	000	0.00	0.00	0.00	0.00	0.00	0.00	0.00	000	0.00	00'0	0.00	00.0	000	000	0.00	0.00	0.00	0.00	1
Sant Ruest		from	0.63	0.68	0.78	0.00	2000	000	000	000	000	00.0	0.00	000	0.60	0.00	0.00	0.00	0.55	0.00	0.65	0.64	0.00	15.66	0.68	0.67	0.69	0.26	0.62	0.86	00.0	0.00	0000	0.00	00.0	0.00	00.00	0.00	1.70	1.44	1.67	1.50	00.0	1.51	1.59	1.77	00'0	0000	0.00	N. A. C. A.
Total	Road						400 30	117.80	289.80	256 20	840.00	483.00	604.80																		214.20	231.00	138.60	189.00	113.40	340.20	105.00	130.20	3733.80 2511.60	2217.60	533.40	1050.00	705.60	200			563.80	367.00	453.60	1000000
Millerwice 1	Mood	Table 1	283.50	286.50	1/2.50	103 50	00.001							22 50	415.50	205.50	391.50	513.00	1117.50	340.00	237.00	151,50	270.00	207.00	505.50	1519.50	316.50	1330.50	670.50	133.50																				
fillestion II	Second	1001	426.95	431.47	67.602	155.87	64.70	86.45	163 74	144.76	474.62	272.91	341.73	1495.05	625.74	325.11	619.38	811.60	1682.96	524.00	356.92	228.16	427.21	61.82	761.28	2288.37	476.65	2003.73	1009.77	201.05	121.03	130.52	78.34	106.79	64.07	313.25	59.33	73.57	1267 10	1118.78	269.10	529.73	398 93	517.01	294.53	531.84	329.86	201.71	256.29	
Therman is		Distanti	22.5900	22,5900	0086.22	22.590n	23 7310	23 7310	23.7310	23.7310	23,7310	23.7310	23.7310	23.7310	22,5900	23.7310	23.7310	23.7310	22.5900	22.7310	22.5900	22.5900	23.7340	4.4800	22.5900	22.5900	22.5900	22,5900	22.5900	22.5900	23.7310	23.7310	23 /310	23.7310	23,7310	23.7310	23,7310	23.7310	21 1890	21.1890	21.1890	21,1890	23.7340	21,1890	21.1890	21.1890	23.7310	23.7310	23.7310	
oralism 100			0.0000																						0,000,0		0.0000					0.0000							0,000				0.0000		0.0000				0.0000.0	
ton Sens		and i	00.0	00.0	200	200	00	00	00	000	00.0	00.0	00.0	00.0	00.0	00.0	00.0	00.0	00.0	9.6	00	00'	00.0	00	00.00	00	00	00'	8.0	00.	00	0.00	000	00	00.	8 8	00	00.	00	00.	00	00.00	0.00	8	00.00	00	00.0	00	00	
Marrier Por		п	25.00																																															
St. Punny			E	¥ 6	200	ER	ER	E	ER	ER	ER	ER	E E	H 4	E E	ER	ER	H (X 0		E E	ER	ER	H 1	2 2	ER	ER	2 6	X a	3 22	ER	E 6	X 22	8	Z 0	5 6	8	2 0	<u> </u>	2	E.	2 0	2 00	E.	2	e :	Y a	1	K	
			FRASER																						FRASER								FRASER		FRASER		_	FRASER			FRASER			_				1	FRASER	
1	1		00.00																						0000							0.00												0			0000		00.00	
-	Roughest Fourtheese		0.1000	001.00	0100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100	0.100														0.1000				0.1000		0.1000	2		0.1000				0.1000	0.1000	0.1000	0.1000	0.1000	0.1000	0.1000	0.1000	-
Amen	Denth	freezel	5.0000	5,0000	5 0000	5.0000	5.0000	5.0000	5,0000	5.0000	5.0000	5.0000	5.0000	5 0000	5.0000	5.0000	5,0000	5.0000	5,0000	5 0000	5.0000	5.0000	5.0000	5,0000	5,0000	5.0000	5.0000	5.0000	5,0000	5.0000	5.0000	5.0000	5.0000	5.0000	5.0000	5.0000	5.0000	5.0000	5.0000	5.0000	5.0000	2,0000	5.0000	5.0000	5.0000	5.0000	5 0000	5,0000	5.0000	
Brisi	Townson I		0.0100	0.0100	00100	0.0100	0.0100	0,0100	0.0100	0.0100	0.0100	0.0100	0.0100	0.0100	0.0100	0.0100	0.0100	0.0100	00100	00100	0,0100	0.0100	0.0100	0.0100	0.0100	0.0100	0.0100	0.0100	00100	0.0100	0.0100	0.0100	0.0100	0.0100	0.0100	0,0100	0.0100	0,0100	0.0100	0.0100	0,0100	0.0100	0.0100	0.0100	0.0100	0.0100	0.0100	0.0100	0.0100	00000
Asses	State R	(IIIII)	0.0500	0.0300	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0900	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	0.0500	00000
hope	Me Per	IWI	00.0	000	000	00.0	00'0	00.00	0.00	0.00	00.00	0.00	0.00	0000	00.0	00.00	0.00	0.00	000	000	00.00	0.00	0.00	00.0	0.00	00'0	0.00	00.00	000	0.00	00.00	0.00	0.00	00'0	0.00	0.00	0.00	000	0.00	0.00	0.00	000	0000	00.00	0.00	000	00.00	0.00	0.00	000
i	Dhun	No.	0.00	000	00.00	0.00	00.0	0.00	0.00	00.00	0.00	0.00	000	0.00	0.00	000	0.00	0.00	000	000	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0000	0.00	0.00	00.0	0.00	00.0	0.00	0.00	00.0	0.00	000	0.00	0.00	0.00	000	00.0	0.00	0.00	0.00	0.00	0.00	0.00	000
· Allen		Tall I	9.15	00 6	9.76	5,11	0.58	1.79	3.00	3.00	8.20	3.00	2.00	2.61	90.0	3.15	2.83	57.6	174	3.73	1.66	1.55	7.33	000	333	00.0	3.93	000	250	00.0	3.00	33,00	00.1	00.1	00.1	00.0	00.8	200	1.93	141	00	76.	303.97	86"	.39	24	34	.94	.32	20
		J	0000								1.0000 3		5000		.9000																																			
mo Steam			7.00 2.0) LC	ູເລ								7.00 0.5		2		2,8000						7.00 0.5		7.00 2.5		7,00 5,0		7.00 2.0			7.00 6.6000													7.00 2.7800		7.00 4.4000		7.00 2.0000	
Time		3	0.1500 7																													0.1500 7											0.1500 7.		0.1500 7.				1500 7.	
		1																																															0 0	_
D CHINE			76.40 B 76.40																3 69.40				1 72.00		76.40				6 76.40					1 65.00		7 65.00		80.00			80.00		1 56.00					65	65.00	55
Here ID			B STR4B																													5 5															32		39	
		THE STATE OF THE S	1.91	1.15	2.47	0.66	0.26	0.28	0.68	0.61	2.00	0.5	6.30	0.46	2.77	1.37	2.6	7.45	2.27	2.32	1.58	1.01	1.80	3.37	18.76	10.13	2.11	4.87	2.38	0,89	0,51	0.55	0,33	0.45	0.27	1.32	0.25	8.89	5.98	5.28	7.27	3.90	1.68	2.44	1.39	1.39	1.42	0.85	1.08	7411
- 01		000	18B-1	3B-2	RFP5	RFP9A	WR1	RMR2	MR3	MR4	RMR5	MRO	RPF1	RPF10	RPF11	RPF12	714	RPE16A	16B	17A	RPF17B	18A	RPF2A	F34	RPF4A	F4B	RPF6	RPFR	F9A	RPF9B	108	RSR5A	R5B	RSR6A	SR9	R10	10A	348	45A	TR6	/H/	388	RSTR9	D1A	100	, K.	SRZ	SR3	244	ì
			2 RFP18	3 RFP18B-2	4 R	5 RF	6 R	7 8	8	2	20	2 6	200	RP RP	RP	4 E	¥ 6	RPE	RPF	1 RPF	RPF	3 RPF	4 G	T d	RP	RP	oc ö	r ir	RP	RP	RSR10B	RSF	RSI	RS	ž ž	RST	RSTR	RSTR4B	RST	RS	POTE	RSTR8B	RS	RUNDIA	RUND18	0)			76	

RF3B 189 21 RFP16B-1 1.91 RSTR4B RFP5 2.47 38 RFP5 2.47 38 RFP5 0.26 28 RMR3 0.69 28 RMR4 0.61 28 RMR4 0.61 28 RMR6 1.15 32 RMR7 1.44 31		THE REAL PROPERTY.						AL DIV		Service Day									Infill Titles			Thirties Talence		
1.18 1.18 0.08 0.08 0.08 1.14 1.14 1.14 0.63 0.63 0.63 0.63 0.63 0.63 0.63 0.63									Thomas of the		Washin M.	S. Periodical	though .											Concontration
7.89 1.15 2.47 0.28 0.28 0.08 1.44 1.44 1.44 0.63 0.63 0.63 0.64 0.64 0.64 0.64 0.64 0.64 0.64 0.64			in property (570	(m)	CNA Disease	100	Depth Res	Adimenta.	(mm)	opposite	000		- January	l manual and a second	- Janes	The same of	lleed!					Ī
2.47 2.47 0.08 0.08 0.08 0.08 1.44 1.44 0.63 0.63 0.63 0.63 0.63 0.63 0.63 0.63		76.40	0.1500	7.00	2.0000	129.15	0.00	00'0	0.0500	0.0100	5.0000	0.1000	0.00 F	FRASER		0.00	0.0000	45.1370	853.09	283.50			12 0	01:08:32
			0.1500		5 0000	108.00	0.00	0.00	0.0500	0.0100	5.0000	0.1000		RASER	84.00	0.00	0.0000	45.0340	860.15	286.50		37.77 0.	0.14 0	00:59:38
	36		0.1500		5.0000	119.76	00.0	000	0.0200	0.0100	5,0000	0.1000		KASEK		0.00	0.0000	44.8270	515.51	172.50			11	00:42:56
			0.1500		6,1500	86.11	0.00	00'0	0.0500	0.0100	5 0000	0.1000		FRASER		200	0.0000	44 7030	38888	370.50			60	01:04:00
			0.1500		1,0000	89.06	0.00	0.00	0.0500	0.0100	5.0000	0.1000		RASER		000	00000	56 2400	146 45	103.50			/0	00:33:57
	28		0.1500		1.0000	91.79	00.00	0.00	0.0500	0.0100	5.0000	0.1000		FRASER		000	0,000	56 2100	157.30		108.20		20 00	00:31:37
			0.1500	2.00	1.0000	33.00	0.00	00:0	0.0500	0.0100	5.0000	0.1000		RASER		0.00	0.0000	57 2030	394.70				7 6	04:44:36
			0.1500	2.00	1.0000	33.00	0.00	0.00	0.0500	0.0100	5.0000	0.1000		FRASER		0.00	00000	57 1180	348 42					04.97.44
			0.1500	7.00	1.0000	38.20	0.00	00.0	0.0500	0.0100	5.0000	0.1000		FRASER		000	0 0000	58 3940	1167 88				500	03.04.44
			0.1500	2.00	1.0000	33.00	00'0	00'0	0.0500	0.0100	5.0000	0.1000		FRASER		00.00	0.000	57.7700	664.36				200	03:01:11
			0.1500		1,0000	62.84	00.0	0.00	0.0500	0.0100	5.0000	0.1000		RASER		000	00000	57 3160	825.35				200	02.22.20
			0.1500		0.5000	357.00	0.00	0.00	0.0500	0.0100	5.0000	0.1000		FRASER		000	00000	53 3000	335B 47	045.00			20 4	4.00.10
			0.1500		2.9000	72,61	0.00	0.00	0.0500	0.0100	5.0000	0.1000		FRASER		000	00000	44 7750	219.40	73.50	•		2 6	61.00.10
	43		0.1500		2.9000	150.06	0.00	0.00	0.0500	0.0100	5.0000	0.1000		RASER		000	00000	45 1720	1251 26	415.50			200	00.30.40
RPF12 1.37			0.1500		2.9000	108.15	0.00	00'0	0.0500	0.0100	5.0000	0.1000		RASER		000	00000	56 5220	77/ 35	205.50			3 0	07:01:10
	09		0.1500		2.8000	92.83	0.00	00.00	0.0500	0.0100	5.0000	0.1000		FRASER		000	0.000	57 0330	1400 58	202.202			000	12.90.00
			0.1500		2.9000	115.73	0.00	0.00	0.0500	0.0100	5.0000	0.1000		RASER		000	00000	57 0840	1051.40	513.00			90	01:31:40
			0.1500		4.2000	239.57	00.00	00.0	0.0500	0 0100	5 0000	0.1000		BASEB		000	00000	46 2640	95.0766	313.00			808	01.33.34
168 2.27			0.1500		5.0000	144.74	00.0	0.00	0.0500	0.0100	5 0000	0.1000		FDACED		000	00000	45.3010	9278.28	05.7111	8.1		3/	01:26:14
			0.1500		2.3000	143.73	0.00	00.0	0.0500	00100	5.0000	0 1000		RASED		000	0.0000	32,4400	1047.60	340.50	(5,3		5	00:54:18
			0.1500		2.3000	111.66	0.00	0.00	0.0500	0.0100	5 0000	0 1000		RASER		000	0,0000	45 0980	747.90	346.00			0.	01:09:42
RPF18A 1.01	52		0.1500		2.5000	64.55	0.00	0.00	0.0500	0.0100	5 0000	0 1000		FRASER		000	0,000,0	45 1200	455 74	454.50			0.11	01:04:19
			0.1500		0.5000	167.33	0.00	0.00	0.0500	0.0100	5.0000	0.1000		RASER		000	0.0000	50.2540	904 57	270.00			200	04.26.47
1.38 1.38	21 8		0.1500		0.5000	500.00	0.00	00.00	0.0500	0.0100	5.0000	0.1000		FRASER		0.00	0.0000	4.9540	68.37	207.00			30	01.20.17
			0.1500		2.0000	296.24	0.00	0.00	0.0500	0.0100	5.0000	0.1000		RASER		00'0	0.0000	45.0170	1517.07	505.50	17		25	00-58-54
			98.0000		2.5000	339.33	0.00	00.0	0.0500	0.0100	5.0000	0.1000		RASER		00.00	0.0000	45.5170	8538.99	2814.00			54	00.00.00
(PF4B 10.13			30.0000		5.6000	500.00	0.00	0.00	0.0500	0.0100	5.0000	0.1000	0.00 F	RASER		0.00	0,0000	45.0510	4563.67	1519.50			72 0	01:01:12
			0001.0		00000	120.93	0.00	0.00	0.0500	0.0100	5.0000	0.1000		RASER		0.00	0.0000	45.0170	949.86	316.50			16 0	75:75:00
	20		0.1500		5,000	183.00	0.00	000	0.0500	0.0100	5.0000	0.1000		FRASER		0.00	0.0000	45.5170	4037.36	1330.50			18 0	03:11:22
			0.1500	7.00	2,0000	150.22	00.0	000	0.0500	0.0100	5.0000	0.100	000	KASEK		000	0.0000	45,1720	2019.19	670.50			27 0	01:11:06
			0.1500		0.5000	500.00	00'0	00'0	0.0500	0.0100	5 0000	0.100		RASER		200	0.000	44 6650	1075.50	357.00			0 0	01:11:53
RSR10B 0.51			0.1500		6.6000	33.00	0.00	00.00	0.0500	0,0100	5.0000	0.1000	D 00 0	RASER		000	00000	56.4370	287.83				0 0	00:29:20
	54 6		0.1500	7.00	6,6000	33.00	00'0	00.00	0.0500	0.0100	5.0000	0.1000		RASER		000	0.0000	56.4650	310.56				7 6	00.49.42
5A 0.50			0.1500		2.0000	18.80	0.00	00.00	0.0500	0.0100	5.0000	0.1000		FRASER		0.00	0,000	57,1180	285.59		210.00	25.65 0.0	0.01	01:37:59
	42		0.1500		2.0000	14.00	00.0	00.00	0.0500	0,0100	5.0000	0.1000	0.00 F	RASER		0.00	0.0000	57,0040	188.11				0 10	01:30:36
RSR6R 0.45	40		0.1500		2.0000	14.00	0.00	00.0	0.0500	0.0100	5.0000	0,1000		RASER		0.00	0.0000	57.2880	257.80				0 10	01:49:13
	47		0.1500		4 3000	14.00	0.00	0.00	0.0500	0.0100	5.0000	0.1000		RASER		0.00	0.0000	56.8620	153,53				0 10	01:20:57
	27 6		0 1500		6 6000	200.005	000	00.0	0.0500	0.0100	9,000	0.000	00.0	KASEK		000	0.0000	57.0890	462.42				0 20	01:35:32
	47 €		0.1500	7.00	1,0000	33.00	0.00	000	0.0500	00100	5 0000	0,1000		CASER		000	0.0000	56.0110	739.35				0	00-17:10
	47 6		0.1500		0.5000	33.00	00.00	0.00	0.0500	0.0100	5,0000	0.1000		RASER		000	00000	56.8620	176 97				5 5	00:57:14
			0.1500		0.5000	214.02	00'0	00:00	0.0500	0.0100	5.0000	0.1000		FRASER		8 11	00000	37 5880	3341 57			78.62		71.18.17
			0.1500		2,0000	188.93	00.00	00.00	0.0500	0.0100	5.0000	0.1000		RASER		00.0	0.0000	40 0950	2397.68				2 0	01.48.53
			0.1500		0.5000	412.11	00.00	0.00	0.0500	0.0100	5.0000	0.1000		RASER		0.00	0.0000	40,0950	2117.02				0 0	01-35-58
KSIK!	24 8		0.1500		0.5000	212.00	0.00	0.00	0.0500	0.0100	5.0000	0.1000		FRASER		0,00	0.0000	40.0950	509.21		-13		1	01:00:49
			0.1500		2,6600	101.32	00.00	000	0.0500	0.0100	5.0000	0,1000		RASER		0.00	0.0000	40.0950	1002.38				15 0	01:26:10
Design to the second se			0.1500		0.5000	17.75	0.00	0.00	0.0500	0.0100	5.0000	0.1000		RASER		0.00	0.0000	51.5920	2012.09			29.90 0.04	0 0	04:21:20
	48		0.1500	2007	0.5000	303.97	0000	0.00	0.0500	0.0100	5.0000	0.1000		FRASER		0.00	0.0000	63.6210	1068.83				0 0	00:57:54
			0.1500		2 7800	74.30	00.0	000	0.0900	0.0100	5.0000	0001.0		KASEK		0.00	0.0000	40,0950	978.32		4		5 0	01:24:56
	46 8		0.1500		1,0000	103 00	000	000	0.0500	0.0100	9.0000	0,1000	0.00	FKASEK		0.00	0.0000	40.0950	557.32		4		0	01:11:46
	33 6		0.1500		4.4000	20.24	00.0	00.0	0.0200	0.0100	5.0000	0.000		KANEK	84.00	0.00	00000	40.0950	1006.38			42.42 0.26	0 9	00:45:57
	32 6		0.1500		4,4000	22.34	000	000	0.0500	0.0100	5 0000	0 1000		TANGE OF THE PARTY	84.00	000	0,0000	57.7130	802.21				0 0	02:17:04
	40 6		0.1500		2.0000	19.94	0.00	000	0.0500	0.0100	5.0000	0.1000		RASER	84.00	000	00000	57 8000	190.60			25.16 0.0	05 0	02:10:26
SR4 1.08		65.00	0.1500	7.00 2	2.0000	19.32	0.00	0.00	0.0500	0.0100	5.0000	0.1000	ш	RASER		00.00	0.000	57 9400	625.75			24 84 0.0	5 6	11:01:20
	44		0.1500		2.0000	19.25	00'0	0.00	0.0500	0.0100	5.0000	0.1000		RASER		00.0	0.0000	57.2880	355.19			· c	2 5	04.50.07
			0.1500	1	2.0000	17.54	0.00	0.00	0.0500	0.0100	5.0000	0.1000		FRASER		00.0	0.0000	56.8620	181.96		134.40 2	0 0		01.30.07

21 7640 RSTR4B 7640 61 7640 36 65.00 48 76.0 28 65.00 28 65.00 28 65.00 28 65.00 28 65.00	ī			Medition	and .	Man De	America A	Amountaine Sa	Arica Management	- Service	South	ad an	Presentation Numer		Eventorellon II	of metter in	Mirriton		-	John Peak Scentif Romot	
26 26 26 26 26 26 26 26 26 26 26 26 26 2		3 70000	200	Dod	an and	1931	Death Re	anipuene	Duestin Re	THE STATE OF THE S	1		famel	1	1	leaned.	1				Constitution
25 55 55 55 55 55 55 55 55 55 55 55 55 5	40 0.1500	0 7.00	2.0000	129.15	00.00	0.00	0.0500	0.0100	5.0000	0.1000	100	FRASER	122.00	0.00	0.0000	52.3900	990.17	283.50	39	1	0
8 6 8 8 8 8				108.00	0.00	0.00	0.0500	0.0100	5,0000	0.1000	0.00	FRASER	122.00	0.00	0.0000	52.2740	998.43	286.50	99	68.59 0.30	0 00:51:22
26 26 26 26 26 26 26 26 26 26 26 26 26 2				119.76	0.00	00'0	0.0500	0.0100	5 0000	0.1000		FRASER	192.00	000	0,0000	52.0610	238.70	172.50	8 7		0 0
8 8 8 8				86.11	00.00	00'0	0.0500	0.0100	5.0000	0,1000		FRASER	122.00	000	0,0000	51.9290	358.31	103.50	5 6		0 0
6 6 6				90.58	0.00	0.00	0.0500	0.0100	5.0000	0.1000		FRASER	122.00	00'0	0.0000	68.7110	178.65				0 0
60 00				91.79	0.00	0.00	0.0500	0.0100	5.0000	0.1000		FRASER	122.00	0.00	0.0000	68.7410	192.47		117.60 52		0
				33.00	00.0	00'0	0.0500	0.0100	5.0000	0.1000		FRASER	122.00	00'0	0.0000	69.9100	482.38				0
				33.00	0.00	0.00	0.0500	0.0100	5.0000	0.1000		FRASER	122.00	00.0	0.0000	69.7900	425.72				0
				33.00	0.00	0.00	0.0500	0.0100	5.0000	0.1000		FRASER	122.00	0.00	0,000	71.2300	1424.60				0
				00.00	00.0	0.00	0.0200	0.0100	5.0000	0.1000		FRASER	122.00	0.00	0.0000	70.5400	811.21				0
				267.00	800	00.0	0.0000	0.0100	5.0000	0,1000		FRASER	122.00	0.00	0.0000	70.0000	1008.00				٥
				72.61	00.0	00.0	0.0500	0.0100	5,0000	0.4000		FKASER	122.00	0.00	0.0000	63.9260	4027.34	945.00	99		0
3 76				150.08	000	000	0.0500	00100	2.0000	0.1000		COACIO	122.00	000	0.0000	51.9950	254.78	73.50	89		0
65				108 15	000	00.0	0.000	00100	2000	0.100		A DO A CO	122.00	00.00	0.0000	52.4230	1452.12	415.50	99		0
90 65				92.83	000	000	0.0500	00100	2.0000	0.1000		FRASER	122.00	00.00	0.0000	69.1310	947.09	205.50	5		0
44 65				115 73	000	000	0.0500	00100	5,0000	0.1000		CDAGG	122.00	00.0	0.0000	0007.69	1819.17	391.50	51		0
				230.57	000	000	00000	00100	20000	0.1000		A LOS COL	122.00	000	0.0000	69.7300	2384.77	513.00	51		0
				144.74	00.0	000	0.0000	0.0100	5.0000	0.1000		FRASEK	122.00	0.00	0.0000	52.4720	3909.16	1117.50	99		0
				143.73	000	00.0	0.0300	00100	5.0000	0.1000		FRASER	122.00	0,00	0.0000	62.9420	1428.78	340.50	22		0
				444 56	00.0	0.00	0.0000	00100	5.0000	0.1000		FKASER	122.00	0.00	0.0000	52.4060	1215.82	348.00	89		0
				04.66	000	0.00	0.0500	0.0100	5.0000	0.1000		FRASER	122.00	00.00	0.0000	52.3400	826.97	237,00	89		0
				04.33	0.00	000	0.0500	00100	2.0000	0.1000		FRASER	122.00	0.00	0.0000	52.3730	528.97	151.50	89		0
				167,33	0.00	0.00	0.0500	0.0100	5.0000	0.1000		FRASER	122.00	00.00	0.0000	59,5920	1072.66	270.00	61		0
				200.00	000	0,00	0.0500	0.0100	5.0000	0.1000	0,00	FRASER	122.00	0.00	0.0000	5.0240	69.33	207.00	112		0
				230.33	0.00	0.00	0.0500	0.0100	5.0000	0.1000		FRASER	122.00	00.00	0.0000	52.2740	1761,63	505.50	89		0
33 76				500.00	000	0.00	0.0500	0.0100	5.0000	0,1000		FRASER	122.00	0.00	0.0000	52.4720	9843.75	2814.00	67	67.37 1.37	0
				200.00	00.0	0.00	0.0000	0.0100	5.0000	0.1000		FKASEK	122.00	00.00	0.0000	52,2910	5297.08	1519.50	68		0
				210.00	00.0	000	0.0300	0,0100	9.0000	0.1000	00.0	FRASER	182.00	0.00	0.0000	52.2580	1102.64	316.50	89		0
				182.00	000	000	0.0500	00100	2,0000	0.1000		COASED	122.00	00.00	0.0000	52.4720	4654.27	1330.50	99		0
				150.22	0.00	0.00	0.0500	00100	5 0000	0.1000		FRASER	122 00	00.0	0.000	52.4390	1240 05	06,079	89		0 0
				500.00	00.00	0.00	0.0500	0,0100	5.0000	0.1000		FRASER	122.00	000	00000	51 8800	461 72	133 50	80 8	66.40 0.33	0 0
				33.00	00.00	00.0	0.0500	0,0100	5.0000	0.1000	000	FRASER	122.00	000	00000	69 0110	351.06				0 00:25:16
				33.00	0.00	00.00	0.0500	0.0100	5.0000	0 1000		FRASER	122 00	000	00000	80.0410	370.72			51.65	0 0
				18.80	00.0	0.00	0.0500	0.0100	5.0000	0.1000		FRASER	122,00	00.0	0.0000	69 7900	348 95		210.00 50		0.0
				14.00	00.00	00.00	0,0500	0.0100	5.0000	0.1000		FRASER	122.00	000	0.0000	69.6700	229.91			51 12 0 02	0 0
				14.00	00'0	00.00	0.0500	0.0100	5.0000	0.1000		FRASER	122.00	000	0.0000	70.0000	315.00				0 0
42 65.				14,00	00.0	00'0	0.0500	0.0100	5.0000	0.1000	00.00	FRASER	122,00	00.00	0,0000	69.5210	187.71			51.29 0.02	0 01:09:43
97 65.				41.00	00.0	00.0	0.0500	0.0100	5.0000	0.1000		FRASER	122.00	0.00	0.0000	69.7600	565.06				0
47 65.			6.6000	900,000	0.00	0.00	0.0500	0.0100	5.0000	0.1000		FRASER	122.00	00.00	0.0000	68.5010	904.21				0
47 65				33.00	000	0.00	0.0500	0.000	5.0000	0.1000		FRASER	122,00	0.00	0.0000	69.1310	172.83			51.71 0.03	0
38 80				214 02	000	0000	0.0000	00100	2.0000	0000		TRASER	122.00	0.00	0.0000	69.4910	215.42				0
				188.93	00.0	00.0	0.0500	0.0100	5,0000	0.1000		S S S S S S S S S S S S S S S S S S S	122.00	5/4	0.0000	43.3620	3854.88	m			0 1
				412 11	000	0000	0.000	00100	5,0000	0.1000		TO A CELO	122.00	00.0	0.0000	45.3740	2713.37	2 0			0
			0.5000	212.00	0.00	000	0.0500	00100	5 0000	0.1000		FRASED	132 00	000	00000	45.3740	C1.CE52	N			0
51 80.				101,32	0.00	000	0.0500	00100	5 0000	0 1000		FRASED	122.00	000	00000	45.3740	310,23	1			0 0
			0.5000	57.27	0.00	00'0	0.0500	0 0100	5 0000	0 1000		FRASER	122.00	000	00000	80.7530	2360.33				
				303.97	0.00	0000	0.0500	0.0100	5.0000	0 1000	000	FRASER	122.00	000	0000	80.4290	1351 21			40.30	0 0
				98.96	00.00	00.00	0.0500	0.0100	5.0000	0.1000		FRASER	122.00	0.00	00000	45 3740	1107 13				0 04:43:00
				74.39	00.00	0.00	0.0500	0,0100	5.0000	0.1000		FRASER	122.00	0.00	0.000	45 3740	630 70		12	ĺ	0 0
			**	103.00	0.00	0.00	0.0500	0.0100	5,0000	0.1000		FRASER	122.00	000	0,000	45.3740	1138 89		74	74 88 051	0 00:30:35
				20.24	0.00	0.00	0.0500	0.0100	5.0000	0,1000		FRASER	122.00	0.00	0,000	70.4500	979.26				0 01-58-03
				22.34	0.00	0.00	0.0500	0.0100	5.0000	0.1000	0.00	FRASER	122.00	0.00	0.0000	70,3600	999.11				0 01-55-36
40 65.00	00,1500		2	19.94	0.00	0.00	0,0500	0.0100	5.0000	0.1000		FRASER	122.00	0.00	0.0000	70.3600	598.06		357.00 50		0 01-5
			2,0000	19.32	00.00	000	0.0500	0.0100	5.0000	0.1000	0.00	FRASER	122.00	0.00	0,0000	70.7500	764.10		453.60 50.03	0.05	0 02:1
			4 0	19,20	0.00	0.00	0.0500	0.0100	5.0000	0.1000		FRASER	122.00	000	0.0000	70.0000	434.00		260.40 50.78	78 0.04	0 01:34:50

0		Mente 10	Pilaminar			Siene	- Marie	Ł	-	THE REAL PROPERTY.	STATE OF THE PARTY	The same of the sa	THE REAL PROPERTY.	- Contract		potentialization	funor =	Poete fallen Runar Evaturalitza faffinakan		Hillipation		Some	Russill Remott	
	1								(Opensous)	Cheette	Conditions	Chelle R	SHEWREN											Cerrosente
RE3B				0.4500	4		IMI OCA	1000	911	I IIIIII		Imm)		IWI		(mm)	louni	- January	Ammil	limit.		100	d (cmc)	(days Humm
RFP18B-1	-	1.91 RSTR4B	76.40	0.1500	2.00	5.0000	104,00	000	0.00	0.0500	0.0100	5.0000	0.1000	0000	FRASER	153.00	0.00	0.0000	56.3050	1064.16	283.50	95.37	37 0.41	0 00:53:54
RFP18B-2	1	1,15 61		0.1500		5.0000	108.00	00'0	00.00	0.0500	0.0100	5.0000	0.1000	000	FRASER	153.00	000	0.0000	56 1810	646 OR	172.50	CS O		0.00.0
KFP5	oi i			0.1500		5,0000	119.76	00.00	00'0	0.0500	0.0100	5.0000	0,1000	0.00	FRASER	153.00	0.00	0.0000	77.0450	1903.01	370.50	74		0 00 0
AFFINA	o c			0.1500		6.1500	86,11	00.00	0.00	0.0500	0.0100	5.0000	0,1000	0.00	FRASER	153.00	00.00	0.0000	56.0570	386.79	103.50	96		0 00
NINK	5 6	0.20	65.00	0.1500	7.00	1.0000	90.58	0.00	0.00	0.0500	0.0100	5.0000	0.1000	0.00	FRASER	153.00	0.00	0.0000	76.5050	198,91				0 00
PMP3	5 6	80 00	65.00	0.1500	00.7	0000		0.00	00.0	0.0500	0.0100	5,0000	0.1000	0.00	FRASER	153.00	0.00	0.0000	76.5050	214.21		117.60 75.		0 00:3
PMPA	o c	0.03	65.00	0.1500	1,00	0000		0.00	00.00	0.0500	0.0100	5.0000	0,1000	00'0	FRASER	153.00	00'0	0.0000	77.7650	536.58				0 01:2
DMDE	0 0	2000	00.00	0.1500		0000		0.00	0.00	0.0500	0.0100	5.0000	0.1000	00.0	FRASER	153.00	00'0	0.0000	77.6450	473.63				0 01:1
RMR	. +	15 33	65.00	0.1500		0000		0.00	0.00	0.0500	0.0100	5.0000	0.1000	000	FRASER	153.00	00.0	0.0000	79.1740	1583,48				0 02:22:31
RMR7		31	85.00	0.1500		0000		000	0.00	0.0500	0.0100	5.0000	0.1000	00'0	FRASER	153.00	0.00	0.0000	78.4550	902.23		483.00 73.34		0 01:5
BDE-1	· «	830 28	80.40	0.1500		0.5000		0.00	0.00	0.0200	0.0100	5.0000	0.1000	0.00	FRASER	153.00	000	0.0000	77.8850	1121.54				0 01:2
DDE 40	0 0	0.30	78.40	0.1500	2007	0,000		0.00	0.00	0.0500	0.0100	5.0000	0.1000	0.00	FRASER	153.00	0.00	0.0000	70.2900	4428.27	945.00			0 01:3
RDE11	o' c	77 49	76.40	0.1500	2.00	2.9000		0.00	0.00	0.0500	0.0100	5.0000	0.1000	0.00	FRASER	153.00	0.00	0.0000	56.1190	274.98	73.50	95.94		0 00:3
00510	v +	27	16.40	0.1500	3.6	2.9000	150.06	0.00	000	0.0500	0.0100	5.0000	0.1000	0.00	FRASER	153.00	0.00	0.0000	56.3050	1559.65	415.50	95.		0 00:5
7 1 1 1	- 6	10.1	00.00	0.1500		2,9000	108.15	0.00	000	0.0500	0.0100	2.0000	0.1000	000	FRASER	153.00	0.00	0.0000	76.9250	1053.87	205.50	74.		0 00:4
PDE 16		247	95.00	0.1500		2.8000	92.83	0.00	0.00	0.0500	0.0100	2,0000	0.1000	0.00	FRASER	153.00	0.00	0.0000	77.5550	2024.19	391.50	74.		0 01:1
DDE46A	9 1	150	76.40	0.1500		2.9000	115,73	0.00	0.00	0.0500	0.0100	2.0000	0.1000	000	FRASER	153,00	0.00	0.0000	77,5850	2653.41	513.00	74.		0 01:1
No. 100		1.45	70.40	0.1500	00.7	4.2000	239.57	00.00	00'0	0.0500	0,0100	5.0000	0.1000	000	FRASER	153.00	000	0.0000	56.3050	4194.72	1117.50	95.		0 01:0
KFF 100	N C		69.40	0.1500	00.7	5.0000		0.00	0.00	0.0500	0.0100	5.0000	0.1000	0.00	FRASER	153.00	0.00	0.0000	69.2800	1572.66	340.50	82.		0 00:4
KPT1/A	, iv	32 64	76.40	0.1500	2.00	2.3000		0.00	0,00	0.0500	0.0100	5.0000	0.1000	0.00	FRASER	153.00	0.00	0.0000	56,3050	1306.28	348.00	95.		0 00:5
KPF1/B	-		76.40	0.1500		2.3000		0.00	0.00	0.0500	0.0100	5.0000	0.1000	0.00	FRASER	153.00	0.00	0.0000	56.3050	889.62	237.00	95.		0 00 5
KPF18A	1	.01 52	76.40	0.1500		2.5000		0.00	0.00	0,0500	0.0100	5,0000	0.1000	000	FRASER	153.00	0.00	0.0000	56.3050	568.68	151.50	95		0 00:5
RPF2A	-	80 21	72.00	0.1500		0.5000		0.00	0.00	0.0500	0.0100	5,0000	0.1000	0.00	FRASER	153.00	0.00	0,0000	65.0820	1171,48	270.00	86		0 01:0
RPF2B	-		98.00	0.1500		0.5000		0.00	00.0	0.0500	0.0100	5.0000	0.1000	0.00	FRASER	153.00	0.00	0.0000	5.0550	92 69	207.00	143		0 00-3
KPESA	3.37		76.40	0.1500		2.0000		0.00	0.00	0.0500	0.0100	5.0000	0.1000	0.00	FRASER	153.00	0.00	0.0000	56.3050	1897,48	505,50	95.		0 00-4
KPF4A	18.76	76 41	76.40	98.0000		2.5000		0.00	0.00	0.0500	0.0100	5,0000	0.1000	0.00	FRASER	153.00	00'0	0.0000	56.3050	10562.82	2814.00	94.24		0.015
27.77	0.0		76.40	80.0000		5.6000		0.00	0.00	0.0500	0.0100	5.0000	0.1000	00'0	FRASER	153.00	00'0	0.0000	56.3050	5703.70	1519.50	95.8		0 00:4
0100	2.17		70.40	0.1500		5.0000		0.00	0.00	0.0500	0.0100	5.0000	0.1000	0.00	FRASER	153.00	0.00	0.0000	56.3050	1188.04	316.50	95.5		0 00:4
PDES	ò		76.40	0.1500		00000		0.00	0.00	0.0500	0.0100	5.0000	0.1000	0.00	FRASER	153.00	00'0	0.0000	56.3050	4994.25	1330.50	93.6		0 02:3
RPF9A		38 46	76.40	0.1500		20000		00.0	00.0	0.0000	0.0100	00000	0.1000	0.00	FKASEK	153.00	0.00	0.0000	56.3050	2516.83	670,50	95.3		0 00:5
RPF9B	0	39 46	76.40	0.1500		0.5000		000	0000	0.0500	0.0100	5.0000	0.1000	0.00	X TO S C I	153.00	00'0	0.0000	56.3050	1340.06	357.00	95.0		0 00:5
RSR 10B	0		65.00	0.1500		6 8000		000	00.0	0.0500	0.0100	9.0000	0.1000	000	TRACE L	153.00	0.00	0.0000	55.9950	498.36				0 00:5
RSR11B	0		65.00	0.1500		6.6000		000	000	0.0500	00100	5,0000	0.1000	000	FOASER	153.00	000	0.0000	76.8050	391.71				0 00:3
RSR5A	0		65.00	0.1500		2.0000		00.0	000	0.0500	00100	5 0000	0.100	000	FDASED	153.00	000	0,000	77 6450	86.224				0 00 4
RSR5B	0.33	33 42	65.00	0.1500	7	2.0000		0.00	00'0	0.0500	00100	5 0000	0.1000	200	FDASER	153.00	000	0.0000	77 5750	386.23				0 01:1
RSR6A	0.		65.00	0.1500		2.0000		0.00	00.00	0.0500	0.0100	5 0000	0.1000	000	FPASED	153.00	000	0,0000	77.9550	250.05				0 01:1
RSR6B	0		65.00	0.1500		2.0000		000	00.0	0.0500	0 0100	5 0000	0.1000	000	FRASER	153.00	000	00000	77 2450	200.33		142 40 74 40		0 012
RSR9	0,1	31 47	92.00	0.1500		1,2000		00.00	00.0	0.0500	0.0100	5 0000	0 1000	000	FRASER	153.00	000	00000	77 8150	626.63				0.01.0
RSTR10	4	32 27	65.00	0.1500		6.6000		00'0	00'0	0.0500	0.0100	5.0000	0.1000	00.0	FRASER	153.00	000	00000	76 2650	1006.70				000
RSTR10A	0	75 47	65.00	0,1500		1.0000		00.00	00.00	0.0500	0.0100	5.0000	0,1000	000	FRASER	153.00	000	00000	76 9250	102.31				0000
RSTR11A	0		65.00	0.1500		0.5000		00.00	00'0	0.0500	0.0100	5.0000	0.1000	000	FRASER	153.00	00.00	0.0000	77.3150	239 68				4.00
RSTR4B	ထ်	38	80,00	0.1500	7.00	0.5000		0.00	00'0	0.0500	0.0100	5.0000	0.1000	00.0	FRASER	153.00	20.52	0.0000	46.4870	4132.69			000	0 00 3
HSTR5A	S.		80.00	0.1500		2.0000		00'0	00.0	0.0500	0.0100	5.0000	0.1000	000	FRASER	153.00	00.0	0.0000	48.2000	2882.36				0 01-2
RSIKe	Ď.	20 20	80.00	0.1500		0.5000		0.00	00.00	0.0500	0,0100	5.0000	0.1000	0.00	FRASER	153,00	0.00	0.0000	48.2000	2544.96				0 01:1
XOLY O	127		80.00	0.1500		0.5000	212.00	0.00	00'0	0.0500	0.0100	5.0000	0.1000	0.00	FRASER	153,00	00.00	0,0000	48.2000	612.14		533.40 102.6	1 0.32	0 00:4
ROLLOG	7 0	000	30.00	0.1500		2,6600		0.00	000	0.0500	0.0100	5.0000	0.1000	0.00	FRASER	153.00	0.00	0.0000	48,2000	1205.00				0.110
BOTTE	9.		72.00 F8.00	0.1500		00000		0.00	0.00	0.0500	0.0100	2.0000	0.1000	0.00	FRASER	153.00	0.00	0.0000	65.9730	2572.95				0 03:2
RUNDIA	2.6		80.00	0.1500		2 7000		0.00	0.00	0.0500	0.0100	5.0000	0.1000	0.00	FRASER	153.00	00'0	0.0000	91.4370	1536.14				0 00:4
RUNDIB		97	BD DD	0.1500		2 7800		000	000	0.0000	0.0100	5.0000	0.1000	0.00	FKASEK	153.00	0.00	0.0000	48,2000	1176.08		102.2		0 01:0
RUNDZ	2.51	1 46	80,00	0.1500	7.00	21,0000	103 00	000	00.0	0.0200	0.0100	20000	0.1000	00.00	TRACER	153.00	0.00	0.0000	48.2000	669.98		102.44		0 00:2
SR1	1.2	33	65.00	0.1500		4.4000		0.00	000	0 0 200	0.0100	5,0000	0.1000	000	FDASER	153.00	000	0.0000	48.2000	1209.82				0 00:3
SR2	1.42	32	65.00	0,1500		4.4000	22.34	00.00	0.00	0.0500	00100	5 0000	0.1000	000	FRASER	153.00	200	00000	70.3030	1088.27				0 01:4
SR3	3.0	15 40	65.00	0.1500		2.0000	19.94	00.00	00'0	0.0500	0.0100	5.0000	0,1000	00.0	FRASER	153.00	000	00000	78 2450	665 DB			21.0	0 00.4
SK4	1,1	39	65,00	0.1500	7.00	2.0000	19,32	00.00	0.00	0.0500	0.0100	5.0000	0.1000	0.00	FRASER	153.00	000	0,000	78.6350	849.26		453 60 73 14		0 00:00:42
YY C	0.0	52 44	65.00	0.1500	2.00	2,0000	19 25	000	000	00200	00000	40000	1977	4000	110	44 44	1	0000						0.20
N. XX			200	-			2	2	00.00	20000	0.0100	00000	0.1000	0.00	LYASEK	153.00	0.00	0,0000	77.8850	482.89				0 01-28-37