

**PROPOSED TOWNSHIP, SITUATED IN REMAINDER OF THE FARM  
DWARSLOOP 248 KU, MPUMALANGA PROVINCE**

**1:100 RETURN PERIOD FLOODLINE DETERMINATION REPORT**

**DECEMBER 2020, Rev 0**

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## 1.0 INTRODUCTION

### 1.1 Study Request

Dalimede Projects (PTY) LTD was appointed by Nkanivo Development Consultants to undertake floodline assessment relating to the proposed township situated on remainder of the farm Dwarloop 248 KU, Mpumalanga Province.

### 1.2 Locality

The proposed township is situated in Dwarloop, 11km north of Bushbuckridge town along the R40 highway. Bushbuckridge is in turn 100km north from Nelspruit the capital of Mpumalanga Province. The area is administered by Bushbuckridge Local Municipality, under the Ehlanzeni District Municipality. GPS coordinates of site are 24°46'35.20"S 31° 5'20.21"E.

The locality map is shown on the figures below.

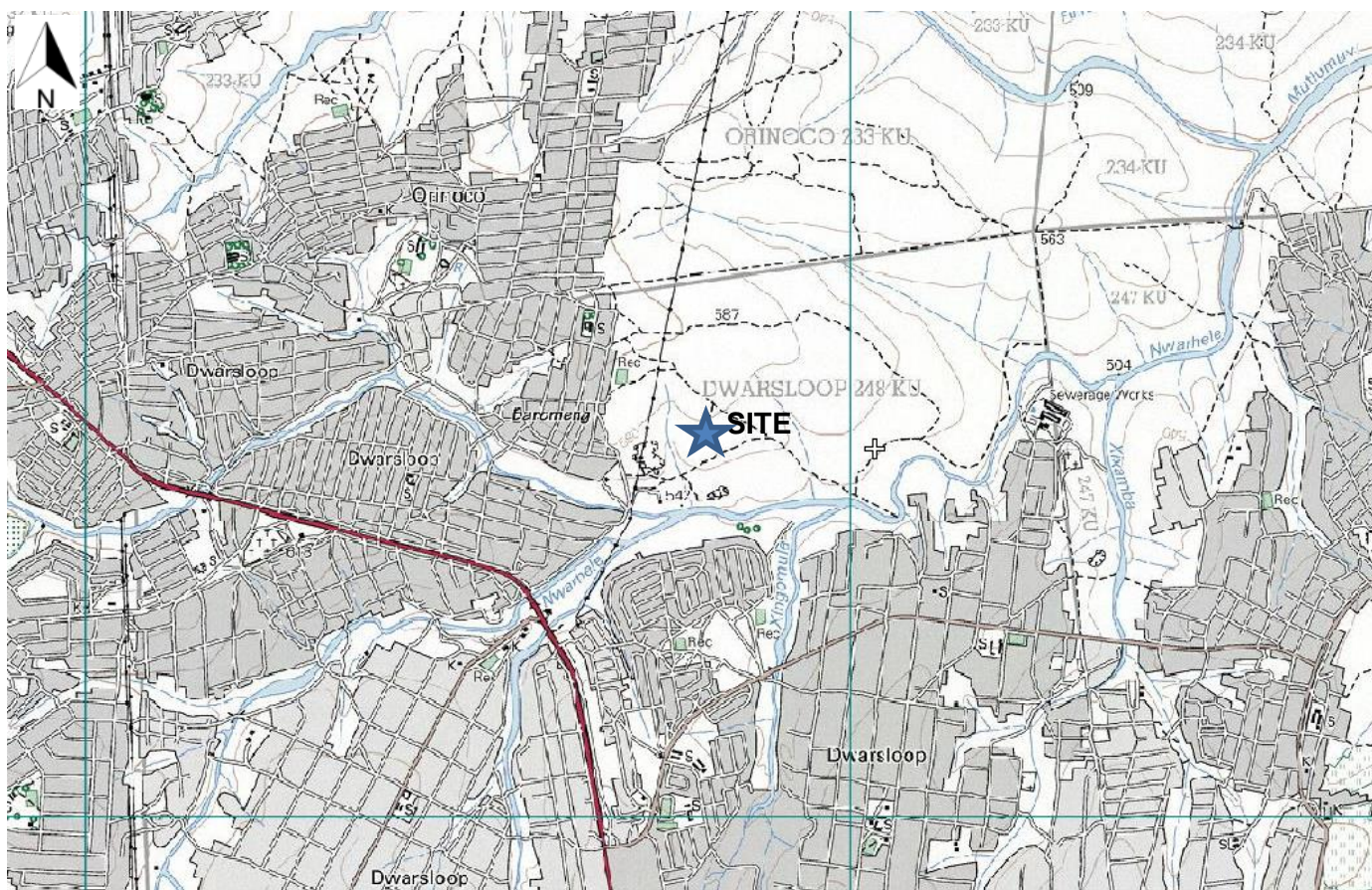


Figure 1 Location of development site

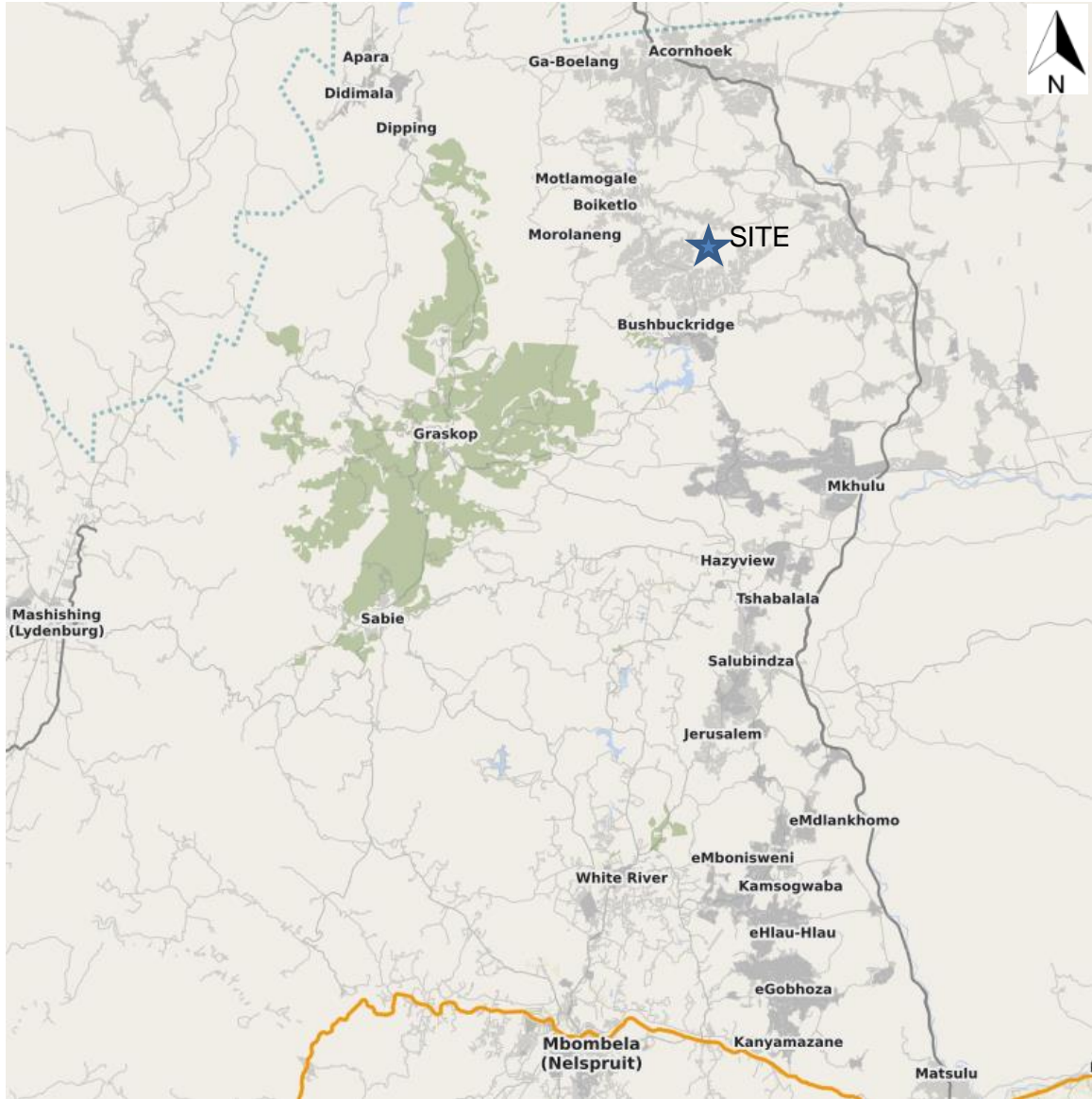


Figure 2 Project site

### 1.3 Background

A flood line analysis must be conducted along the streams traversing the site of proposed development.



Figure 3 Streams onsite

## 1.4 Methodology

### 1.4.1 general

The study consists of 2 major components:

- a flood analysis to determine the flood peak flow and,
- a surface water profile analysis to determine the flood line.

The magnitude of a flood is dependent on many factors, such as catchment size, slope and rainfall intensity. There are several different methods for determining floods and in general, different methods arrive at different estimates of the peak flow rate. The accepted approach is therefore to use several methods and then make a judgment call as to which method is the most applicable to the catchment under consideration. For this study, the Rational, Alternative Rational, Standard Design Flood (SDF) methods were used to determine the peak flow rate for the 1:100 return flood.

The reason for choosing these methods was because they are applicable to the catchment and to show the variance in the flood line between the method that produced the highest flood peak and the method that produced the lowest flood peak. Hence a flood line has been produced to take cognisance of the uncertainty related to estimating floods and flood lines.

The area of the catchment in which the adjacent stream is located was determined using GIS software as were additional properties applicable to the catchment, such as the length of the watercourse and the centroid of the catchment. The flood peak flows provide the flow used in the flood line analysis using the *HEC-RAS* software.

Other input required for *HEC-RAS* is channel geometry and roughness parameters. Channel cross-sections were taken at points along the river / stream course, within the area that contour lines were provided. The stream reaches that were analysed are in a natural state. See the figure below for the streams in catchment area.



Figure 4 Stream 1 onsite

## 1.4.2 Flood Modelling Methodology

Flood peaks for the catchments selected for flood modelling were estimated by the following methods using the Utility Programs for Drainage (UPD) software, 2007 with the methods detailed in SANRAL, 2013:

- Rational Method (RM).
- Alternative Rational Method (ARM).
- Standard Design Flood (SDF).

### 1.4.2.1 Rational Method

This method is based on the conservation of mass and is applicable for catchment areas below 15 km<sup>2</sup>. Aerial and time distributions of rainfall in this method are assumed to be uniform throughout the catchment. Flood peaks and empirical hydrographs can be determined by this method.

Where: The peak flow is obtained from the following relationship:

$$Q = \frac{CIA}{3.6}$$

Where: Q = peak flow (m<sup>3</sup>/s)

C = runoff coefficient (dimensionless)

I = average rainfall intensity over the catchment (mm/hour)

A = effective runoff area of the catchment (km<sup>2</sup>)

3.6 = conversion factor

### 1.4.2.2 Alternative Rational Method

This method is based on the rational method with the point precipitation being adjusted using the Design Rainfall Estimation Methodology developed by Smithers and Schulze (2003) to consider local South African conditions.

Design rainfall values for the study area were extracted from the database of six closest to site South African Weather Service stations, using the Design Rainfall Utility developed by Smithers and Schulze (2000).



Table 1 Design Rainfall Values for the site

Duration		Return Period (Years) Design Rainfall Depth (mm)						
		1:2	1:5	1:10	1:20	1:50	1:100	1:200
5	m	7.6	10.5	12.8	15.2	18.7	21.7	25
10	m	13.8	19.1	23.2	27.6	34.1	39.5	45.5
15	m	19.5	27.2	33	39.2	48.3	56.1	64.6
30	m	29.6	41.1	49.9	59.3	73.1	84.8	97.7
45	m	37.6	52.3	63.5	75.5	93.1	108	124.4
1	h	44.7	62.1	75.4	89.6	110.5	128.1	147.7
1.5	h	56.9	79.1	96	114.1	140.7	163.2	188
2	h	67.5	93.9	114	135.5	167	193.7	223.2
4	h	81.1	112.8	136.9	162.8	200.6	232.7	268.1
6	h	90.3	125.5	152.4	181.2	223.3	259.1	298.5
8	h	97.4	135.4	164.5	195.5	241	279.6	322.1
10	h	103.4	143.7	174.5	207.4	255.7	296.6	341.7
12	h	108.5	150.8	183.1	217.7	268.3	311.2	358.6
16	h	117.1	162.7	197.6	234.9	289.5	335.8	387
20	h	124.2	172.6	209.6	249.2	307.1	356.3	410.5
24	h	130.3	181.1	220	261.5	322.3	373.9	430.8
1	h	108.1	150.3	182.5	216.9	267.4	310.2	357.4
2	h	138.5	192.5	233.8	277.9	342.6	397.4	457.9
3	h	160.1	222.6	270.2	321.3	396	459.3	529.3
4	h	176.7	245.5	298.2	354.5	436.9	506.8	583.9
5	h	190.7	265	321.8	382.6	471.5	547	630.2
6	h	202.9	282	342.5	407.2	501.9	582.1	670.7
7	h	213.9	297.3	361	429.2	529	613.6	707

## 2.0 PROPOSED DEVELOPMENT

### 2.1 Flood Analysis

To make the analysis possible, properties of the catchments that influence the runoff relating to the 1:100 return flood event need to be determined. These properties are described in the following sections.

#### 2.1.1 Catchment Properties

The catchment topography is composed of flat, hilly, and mountainous areas. The topographic elevation ranges from 540m to 1152m above sea level. The landscape soils mostly ranging from high infiltration rates with rapid permeability well drained, to having slow infiltration rates with restricted permeability (Schulze, 2010). The soils are classified to have a range from low to high runoff potential.

The Mean Annual Precipitation (MAP) of the catchment was determined from weather stations gridded from in the vicinity of the site. The MAP for the catchment is estimated to be 1278mm.

Table 2 Rainfall data

Station Name	SAWS	Distance (km)	Record (Years)	Latitude		Longitude		MAP (mm)
	Number			(°)	(')	(°)	(')	
Onverwag (BOS)	0594828_W	0	35	24	50	31	0	1340
Bosbokrand (POL)	0595110_W	7.2	92	24	50	31	4	1024
Dunnottar	0594802_W	8	49	24	52	30	56	1582
Modderspruit	0594896_W	9.2	39	24	55	30	59	1157
Marite "Mariti"(BOS)	0594715_W	13	46	24	54	30	54	1307
Wilgeboom (BOS)	0594806_W	13	64	24	56	30	56	1331

The climate is characterised by hot and rainy summers for a long period as well as cool and dry winters over a short period.

There is an existing but non-functioning river gauging station X3H022 on the catchment, 1.2km upstream of township site. Data from this river gauge could not be obtained.

### 2.1.2 Catchment Delineation

There were three catchments that were delineated.

The catchment areas are within the Inkomati Water Management Area.

Catchments in the table below was delineated to cover the streams within the project boundary and was utilised to determine the flood peaks for 1:100 return extreme events. The catchment information is listed in the table below.

Table 3 Catchment area

Catchment Site	Catchment area (km <sup>2</sup> )	Remark	Quaternary catchment
C1	57.041	Catchment	X32E
C2	12.500	Catchment	X32E
C3	0.594	Catchment	X32E

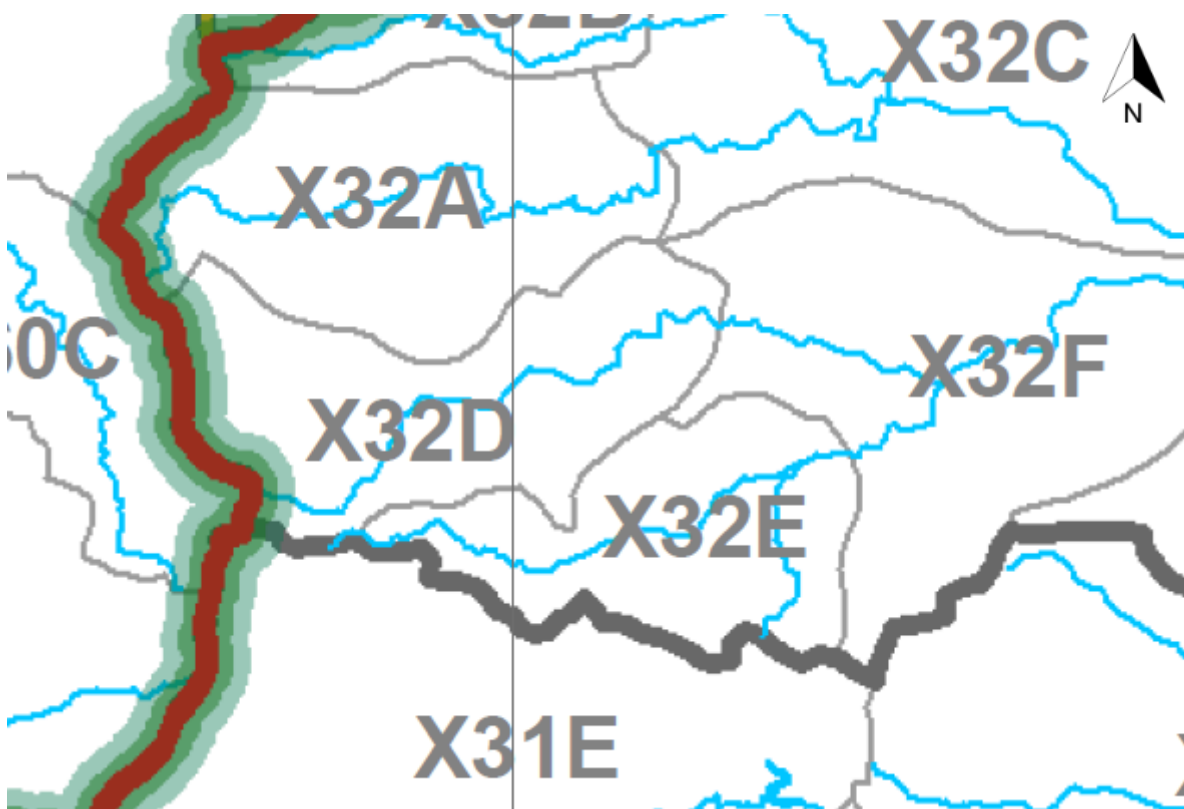


Figure 5 Quaternary catchments

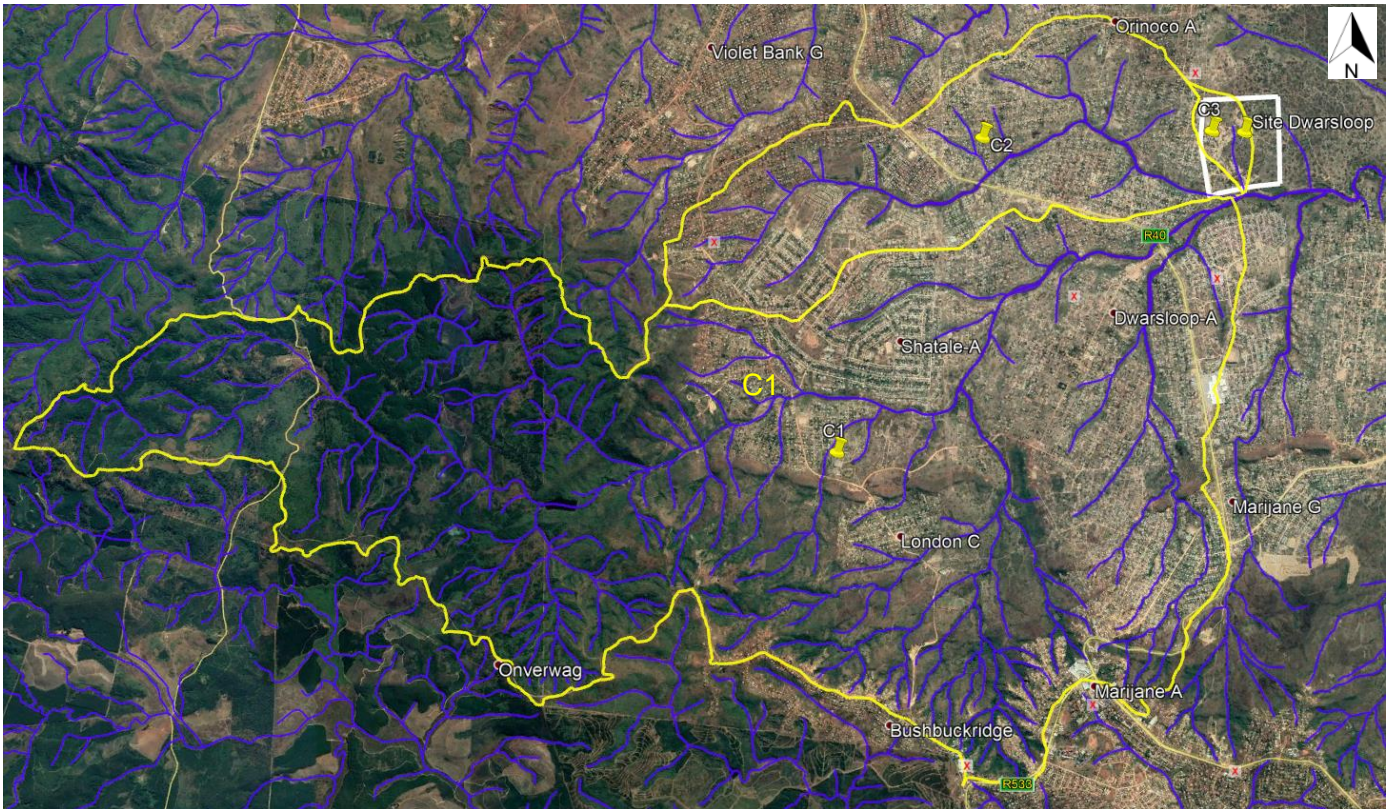


Figure 6 Catchments (yellow line boundary)

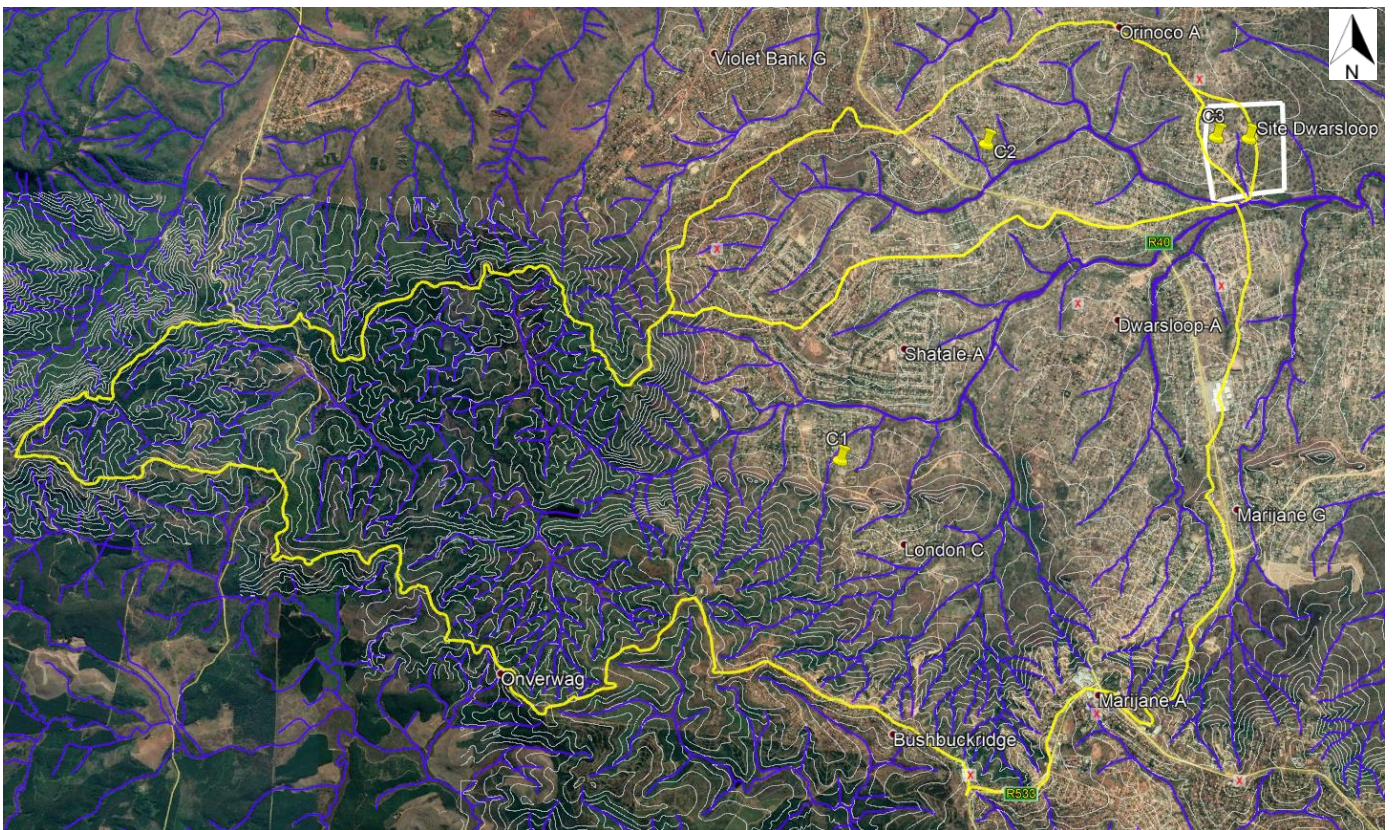


Figure 7 Catchment delineation (yellow line boundaries)

Table 4 Catchment Characteristic

Characteristic	Rural	Urban	Lakes	Total
	Distribution	Distribution	Distribution	
Catchment	%	%	%	(%)
C1	89%	11%	0%	100%
C2	82%	18%	0%	100%
C3	100%	0%	0%	100%

Table 5 Rural area - Surface slope

Rural area - Surface slope	Lakes and pans (<3%)	Flat area (3 to 10%)	Hilly (10 to 30%)	Steep areas (>30%)	Total
	Distribution	Distribution	Distribution	Distribution	
Catchment	(%)	(%)	(%)	(%)	(%)
C1	5%	34%	45%	16%	100%
C2	8%	74%	18%	0%	100%
C3	9%	91%	0%	0%	100%

Table 6 Rural area – Permeability

Rural area - Permeability	Very permeable	Permeable	Semi-permeable	Impermeable	Total
	Distribution	Distribution	Distribution	Distribution	
Catchment	(%)	(%)	(%)	(%)	(%)
C1	8%	42%	45%	5%	100%
C2	12%	47%	40%	1%	100%
C3	15%	60%	25%	0%	100%

Table 7 Rural area - Vegetation

Rural area - Vegetation	Thick bush & forests	Light bush & cultivated land	Grasslands	Bare	Total
	Distribution	Distribution	Distribution	Distribution	
Catchment					(%)
C1	50%	6%	6%	38%	100.0%
C2	3%	12%	10%	75%	100.0%
C3	7%	48%	7%	38%	100.0%

Table 8 Urban

Urban	C1	C2
<u>Lawns</u>		
Sandy, flat	12%	11%
Sandy, steep		
Heavy soil, flat	6%	7%
Heavy soil, steep		
<u>Residential areas</u>		
Houses	55%	54%
Flats	4%	3%
<u>Industry</u>		
Light industry		
Heavy industry		
<u>Business</u>		
City centre		
Suburban	3%	1%
Streets	20%	24%
Maximum flood		
<b>Total</b>	<b>100%</b>	<b>100%</b>

Table 9 Run-off factors

Catchment	Run-off factor			
	Rural (C <sub>R</sub> )	Urban (C <sub>U</sub> )	Lakes (C <sub>L</sub> )	Combined (C)
C1	0.492	0.536	0	<b>0.497</b>
C2	0.527	0.549	0	<b>0.531</b>
C3	0.429	0	0	<b>0.429</b>

Table 10 Hydrological input data

Catchment	Catchment Area (km <sup>2</sup> )	Longest water course (km)	Height difference 1085 method (m)	Days thunder was heard (No.)	Area Dolomite (%)	Mean Annual Precipitation (mm)	SDF Basin no. (No.)
C1	57.041	19.067	366.9	45	0	1278	29
C2	12.500	8.414	91.4	45	0	1278	29
C3	0.594	1.534	56.4	45	0	1278	29

Table 11 Catchment characteristics

Catchment Site	Catchment area (km <sup>2</sup> )	Longest water course, L (km)	Height difference along 10-85 slope (m)	Average slope S <sub>av</sub> (m/m)	Time of concentration, T <sub>c</sub> (hours)	% Slope	MAP (mm)	Run-off factor C
C1	57.041	19.067	366.9	0.02565613	2.6302737	2.57%	1278	0.497
C2	12.500	8.414	91.4	0.01447869	1.74609857	1.45%	1278	0.531
C3	0.594	1.534	56.4	0.04904112	0.294370559	4.90%	1278	0.429

## Flood magnitudes

The flood magnitudes from the 1:2 return up to 1:100 return floods are presented in the Tables below.

Table 12 Estimated stormwater flow (m<sup>3</sup>/s)

	Rational method						Alternative rational method					
Return	1:2	1:5	1:10	1:20	1:50	1:100	1:2	1:5	1:10	1:20	1:50	1:100
Catchment												
C1	158.86	224.94	295.12	375.89	497.3	620.99	110.53	197	271.48	352.44	462.57	557.57
C2	54.08	76.98	101.59	130.42	175.31	222.67	38.24	67.8	92.99	120.22	157.18	188.8
C3	6.317	9.168	12.32	16.09	22.06	28.54	4.761	8.567	11.91	15.58	20.58	24.97

Table 13 Estimated stormwater flow (m<sup>3</sup>/s)

	Standard design flood method					
Return	1:2	1:5	1:10	1:20	1:50	1:100
Catchment						
C1	23.94	74.34	120.52	172.43	249.42	313.79
C2	7.584	23.56	38.19	54.64	79.03	99.43
C3	1.241	3.854	6.248	8.939	12.93	16.27



The applications and limitation of flood calculation methods are shown in the table below.

Table 14 Applications and limitation of flood calculation methods

Method	Recommended maximum area (km <sup>2</sup> )	Return period of floods that could be determined
Statistical method	No limitation (larger areas)	1:2 to 1:200
Rational method	Usually less than 15km <sup>2</sup>	1:2 to 1:200
Unit Hydrograph method	15km <sup>2</sup> to 5,000km <sup>2</sup>	1:2 to 1:100
Standard Design Flood method	No limitation	1:2 to 1:200
SCS-SA method	Less than 30km <sup>2</sup>	1:2 to 1:100
Empirical methods	No limitation (larger areas)	1:10 to 1:100

### Flood magnitudes for the 1:100-year floods

The Rational, Alternative Rational (AR), and Standard Design Flood (SDF) methods were used to select the flood peak.

The method with the highest magnitude of the peak flow was used for the 1:100 return flood for a sub-catchment. However, for C1 the Rational methods were not used as the catchment area is beyond the recommended maximum area, hence the SDF method flow was selected.

The selected maximum peak flows are shown in the table below.

Table 15 Catchment generated estimated 1:100 peak flow

<b>C1</b>	Catchment, estimated 100year peak flow =	313.79	m <sup>3</sup> /s
<b>C2</b>	Catchment, estimated 100year peak flow =	222.67	m <sup>3</sup> /s
<b>C3</b>	Catchment, estimated 100year peak flow =	28.54	m <sup>3</sup> /s

The estimated 1:100 stream flow is listed in the table below.

Table 16 Stream Peak flows estimates

Stream - Reach	Flow (m <sup>3</sup> /s)
Stream 1	313.79
Stream 2	222.67
Stream 4	536.46
Stream 3	28.54
Stream 5	565.00

## 2.2 Flood line Modelling

The HEC-RAS model was used to determine the flood line during the event of a flood for any return period, and in this case the 1:100-year floods were modelled.

### 2.2.1 Cross section profile

Cross sectional data was generated using GIS and CAD software, as well as the contour lines that were obtained from Messers Windus M & Associates Surveys. Sections shown in Annexure 5 were used to approximate the geometry for the river.

### 2.2.2 Flood profiles

Annexure 4 shows the longitudinal profile for the 1:100 return peak flow.

### 3.0 CONCLUSION

The determination of the 1:100 return period floodlines was undertaken for the site of the proposed development. The results of this determination provide an indication as to the extent of the areas that will be inundated by the 1:100 return design flood.

It is recommended that a buffer zone of 20m should be provided between the 1:100 flood line and any proposed development.

The lateral extent of the 1:100 return flood line is shown in Annexure 2. These flood lines have also been provided as Gauss Conform WGS84 LO31 coordinated CAD dwg softcopy files.

## 4.0 REFERENCES

- Smithers J.C. and Schulze R.E. (2002): Drainage rainfall and flood estimation in South Africa, WRC project KS/1060.
- The South African National Roads Agency Limited (2013): Drainage manual, 6<sup>th</sup> Edition.

## ANNEXURE 1: FLOODLINE CERTIFICATE



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## FLOOD LINE CERTIFICATE

Dalimede Projects (PTY) LTD was appointed by Nkanivo Development Consultants to undertake floodline assessment relating to the proposed township situated on remainder of the farm Dwarsloop 248 KU, Mpumalanga Province.

This will entail to delineate the 1:100 return flood line.

**Site:** *Remainder of the farm Dwarsloop 248 KU*  
**Township Name:** *Dwarsloop, Bushbuckridge*  
**Co-ordinates:** *24°46'35.20"S 31° 5'20.21"E*  
**Municipality:** *Bushbuckridge Local Municipality, in Ehlanzeni District Municipality*

In terms of section 114 of the National Water Act, Act 36 of 1998 the above-mentioned property is affected by flood water within the 1:100 period from the stream / river as indicated in the floodline report. Development must be done outside of the floodline.

It is recommended that a buffer zone of 20m should be provided between the 1:100 flood line and any proposed development.

Thus, done and signed in ..... on this day.....

Signature: .....

Engineer: Litmos Mthunzi  
Pr Tech Eng

Pr no.: .....




---

## ANNEXURE 2: FLOODLINE DELINEATION



**NOTES**

**KEY**

-  FLOODLINE
-  CROSS SECTION
-  STREAM / RIVER

**REVISIONS**

REV	DATE	SIGN	DESCRIPTION
0	.....	...	.....

**CLIENT**



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**DRAWING STATUS**

FOR INFORMATION

**PROJECT TITLE**

Proposed township situated on remainder of the farm Dwarsloop 248 KU, Mpumalanga Province.

**PROJECT LOCATION**

The proposed township is situated in Dwarsloop, 11km north of Bushbuckridge town along the R40 highway. Bushbuckridge is in turn 100km north from Nelspruit. GPS coordinates of site are 24°46'35.20"S 31° 5'20.21"E

**DRAWING DESCRIPTION**

FLOODLINE DELINEATION 1:100 RETURN PERIOD

SCALE	DATE	DESIGNED	DRAWN	CHECKED
As Shown	Dec 2020	LM	HM	CM

<b>DRAWING No.</b> DWARSLOOP/FL/01	<b>REVISION</b> A
---------------------------------------	----------------------

SCALE 1:2 000

A1





**NOTES**

**KEY**

- FLOODLINE
- CROSS SECTION
- STREAM / RIVER

**REVISIONS**

REV	DATE	SIGN	DESCRIPTION
0	.....	...	.....

**CLIENT**



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**DRAWING DESCRIPTION**

FLOODLINE DELINEATION 1:100 RETURN PERIOD

SCALE	DATE	DESIGNED	DRAWN	CHECKED
As Shown	Dec 2020	LM	HM	CM

SCALE 1:2 000

A1

<b>DRAWING No.</b> DWARLOOP/FL/02	<b>REVISION</b> A
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## ANNEXURE 3: HEC-RAS PROGRAMME MODELLING RESULTS

HEC-RAS Plan: Current mode River: Stream Reach: 1 Profile: 1:100

Reach	River Sta	Profile	Q Total (m3/s)	Min Ch El (m)	W.S. Elev (m)	Crit W.S. (m)	E.G. Elev (m)	E.G. Slope (m/m)	Vel Chnl (m/s)	Flow Area (m2)	Top Width (m)	Froude # Chl
1	108	1:100	313.79	538.50	542.76		542.84	0.001271	1.60	295.77	185.25	0.33
1	107	1:100	313.79	538.50	542.73		542.83	0.001596	1.71	275.26	184.50	0.36
1	106	1:100	313.79	538.76	542.66		542.79	0.001835	1.90	255.34	185.55	0.39
1	105	1:100	313.79	538.52	542.61		542.75	0.001949	1.96	249.55	184.79	0.40
1	104	1:100	313.79	538.08	542.61		542.71	0.001085	1.68	278.47	155.71	0.31
1	103	1:100	313.79	538.02	542.59		542.69	0.000957	1.62	275.10	134.25	0.29
1	102	1:100	313.79	538.31	542.59		542.67	0.000744	1.46	306.47	135.87	0.26
1	101	1:100	313.79	538.50	542.35		542.62	0.002187	2.47	156.63	69.68	0.45

HEC-RAS Plan: Current mode River: Stream Reach: 2 Profile: 1:100

Reach	River Sta	Profile	Q Total (m3/s)	Min Ch El (m)	W.S. Elev (m)	Crit W.S. (m)	E.G. Elev (m)	E.G. Slope (m/m)	Vel Chnl (m/s)	Flow Area (m2)	Top Width (m)	Froude # Chl
2	210	1:100	222.67	541.00	543.11		543.42	0.008946	3.03	103.11	91.19	0.80
2	209	1:100	222.67	541.00	543.03		543.29	0.005928	2.72	115.25	95.79	0.67
2	208	1:100	222.67	540.63	542.65	542.58	543.09	0.015440	3.54	84.55	84.61	1.02
2	207	1:100	222.67	540.09	542.61		542.88	0.005080	2.73	116.22	90.21	0.63
2	206	1:100	222.67	539.97	542.59		542.79	0.003238	2.38	140.44	105.18	0.51
2	205	1:100	222.67	539.50	542.62		542.72	0.001392	1.74	204.85	128.36	0.34
2	204	1:100	222.67	539.50	542.58		542.69	0.001589	1.84	179.12	87.41	0.36
2	203	1:100	222.67	539.50	542.58		542.65	0.000988	1.58	219.86	105.32	0.30
2	202	1:100	222.67	539.25	542.55		542.63	0.000810	1.51	197.51	68.95	0.27
2	201	1:100	222.67	538.85	542.48		542.61	0.001047	1.85	158.06	48.69	0.32

HEC-RAS Plan: Current mode River: Stream Reach: 3 Profile: 1:100

Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
			(m3/s)	(m)	(m)	(m)	(m)	(m/m)	(m/s)	(m2)	(m)	
3	316	1:100	28.54	551.09	552.27	552.27	552.35	0.010556	1.66	37.21	243.59	0.72
3	315	1:100	28.54	550.31	551.12	551.12	551.30	0.012743	2.05	19.61	73.98	0.83
3	314	1:100	28.54	549.39	550.01	550.01	550.26	0.041051	2.59	14.13	43.02	1.36
3	313	1:100	28.54	548.65	549.27	549.27	549.40	0.025171	2.02	22.56	95.11	1.06
3	312	1:100	28.54	546.98	548.34	548.34	548.49	0.012558	2.00	23.93	90.51	0.81
3	311	1:100	28.54	545.87	547.30	547.30	547.42	0.008424	1.75	29.37	142.57	0.68
3	310	1:100	28.54	544.89	546.15	546.15	546.51	0.018185	2.67	10.68	14.78	1.00
3	309	1:100	28.54	543.93	545.10	545.10	545.47	0.018119	2.69	10.61	14.48	1.00
3	308	1:100	28.54	542.96	544.14	544.14	544.51	0.017634	2.69	10.67	15.21	0.99
3	307	1:100	28.54	542.00	543.17	543.17	543.53	0.017872	2.66	10.79	15.73	0.99
3	306	1:100	28.54	541.04	542.19	542.19	542.56	0.017982	2.68	10.79	16.14	0.99
3	305	1:100	28.54	540.28	541.84		541.87	0.001145	0.88	48.12	68.14	0.27
3	304	1:100	28.54	539.28	541.85		541.86	0.000185	0.50	86.64	71.77	0.12
3	303	1:100	28.54	538.65	541.85		541.85	0.000051	0.33	119.98	60.09	0.07
3	302	1:100	28.54	538.20	541.85		541.85	0.000040	0.30	119.70	54.36	0.06
3	301	1:100	28.54	538.00	541.85		541.85	0.000022	0.27	153.63	63.58	0.05

HEC-RAS Plan: Current mode River: Stream Reach: 4 Profile: 1:100

Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
			(m3/s)	(m)	(m)	(m)	(m)	(m/m)	(m/s)	(m2)	(m)	
4	415	1:100	536.46	538.89	541.92		542.46	0.005205	3.55	189.83	90.15	0.68
4	414	1:100	536.46	538.50	542.05		542.31	0.002156	2.50	275.29	114.54	0.45
4	413	1:100	536.46	538.50	542.06		542.25	0.001673	2.18	324.92	140.63	0.39
4	412	1:100	536.46	538.33	542.07		542.21	0.001389	1.93	395.80	193.42	0.36
4	411	1:100	536.46	538.04	542.05		542.18	0.001338	1.88	401.91	192.97	0.35
4	410	1:100	536.46	537.99	542.03		542.15	0.001153	1.83	426.03	201.42	0.33
4	409	1:100	536.46	537.58	542.01		542.13	0.000939	1.78	448.07	199.09	0.30
4	408	1:100	536.46	537.50	541.99		542.11	0.000982	1.83	436.22	193.37	0.31
4	407	1:100	536.46	537.50	541.95		542.09	0.001073	1.93	416.81	191.36	0.32
4	406	1:100	536.46	537.50	541.91		542.06	0.001155	2.00	414.68	205.71	0.33
4	405	1:100	536.46	537.50	541.89		542.04	0.001131	2.04	423.06	199.91	0.33
4	404	1:100	536.46	537.50	541.85		542.01	0.001303	2.16	398.00	196.24	0.36
4	403	1:100	536.46	537.50	541.83		541.98	0.001196	2.07	411.29	206.51	0.34
4	402	1:100	536.46	537.50	541.77		541.95	0.001382	2.17	340.36	146.99	0.36
4	401	1:100	536.46	537.50	541.48		541.89	0.002670	2.95	213.95	92.40	0.50

HEC-RAS Plan: Current mode River: Stream Reach: 5 Profile: 1:100

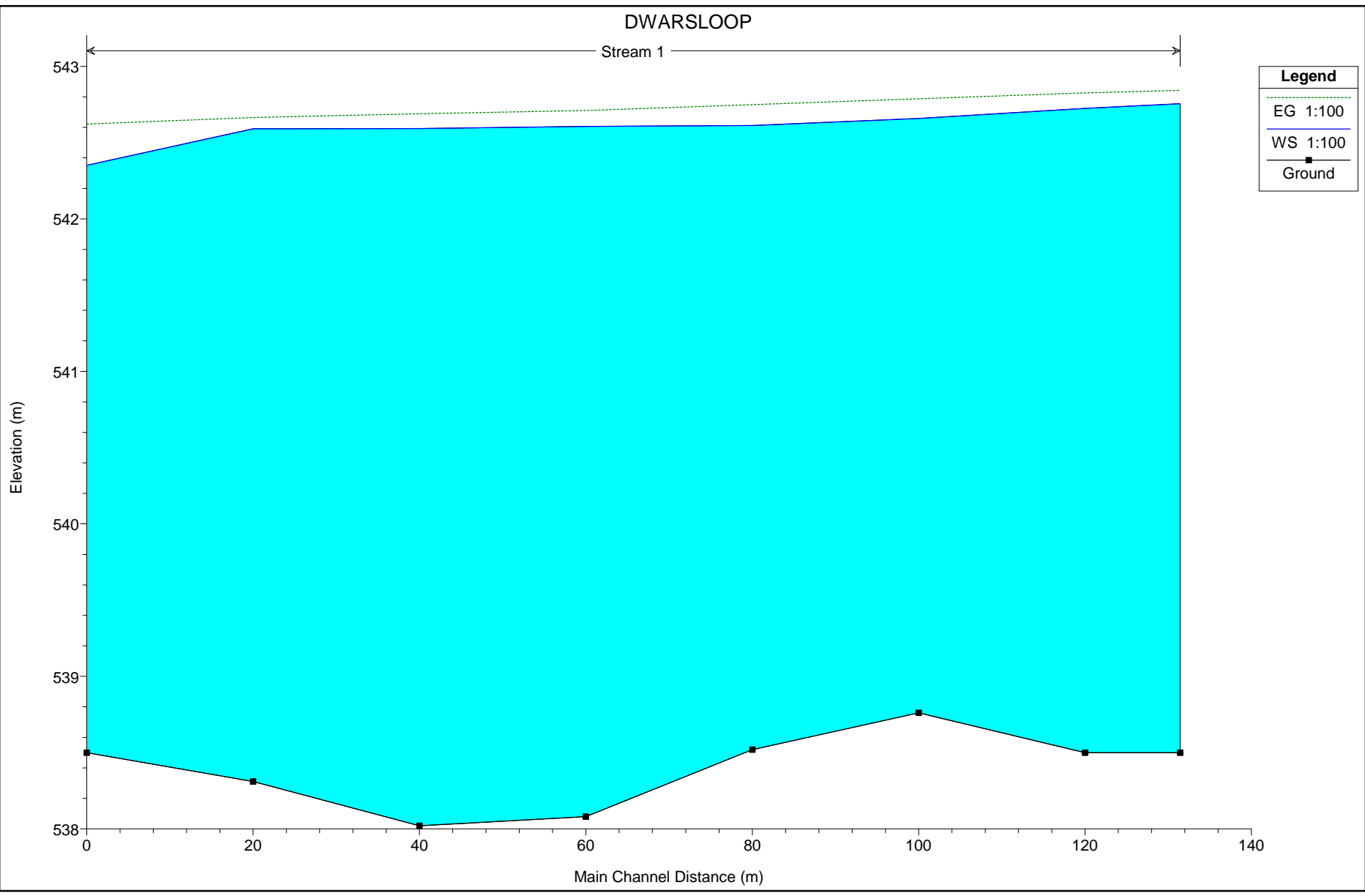
Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
			(m3/s)	(m)	(m)	(m)	(m)	(m/m)	(m/s)	(m2)	(m)	
5	511	1:100	565.00	537.30	541.46		541.78	0.002287	2.67	276.61	142.33	0.46
5	510	1:100	565.00	537.09	541.43		541.75	0.002228	2.66	271.43	148.82	0.46
5	509	1:100	565.00	537.00	541.33		541.70	0.002718	2.79	254.09	152.12	0.50
5	508	1:100	565.00	537.58	540.78	540.78	541.56	0.009773	4.07	168.32	129.68	0.89
5	507	1:100	565.00	537.29	540.42	540.42	541.20	0.010167	4.09	165.98	123.37	0.90
5	506	1:100	565.00	536.86	540.16	539.90	540.78	0.006839	3.62	186.76	123.24	0.75
5	505	1:100	565.00	536.50	540.23		540.61	0.003504	2.90	242.79	141.33	0.55
5	504	1:100	565.00	536.50	540.15		540.54	0.003842	2.90	238.71	138.85	0.57
5	503	1:100	565.00	536.57	540.06		540.46	0.004255	2.93	231.80	138.43	0.60
5	502	1:100	565.00	536.50	539.76	539.49	540.33	0.007104	3.47	189.06	121.55	0.75
5	501	1:100	565.00	536.30	539.44	539.36	540.15	0.010007	3.85	166.98	116.46	0.88

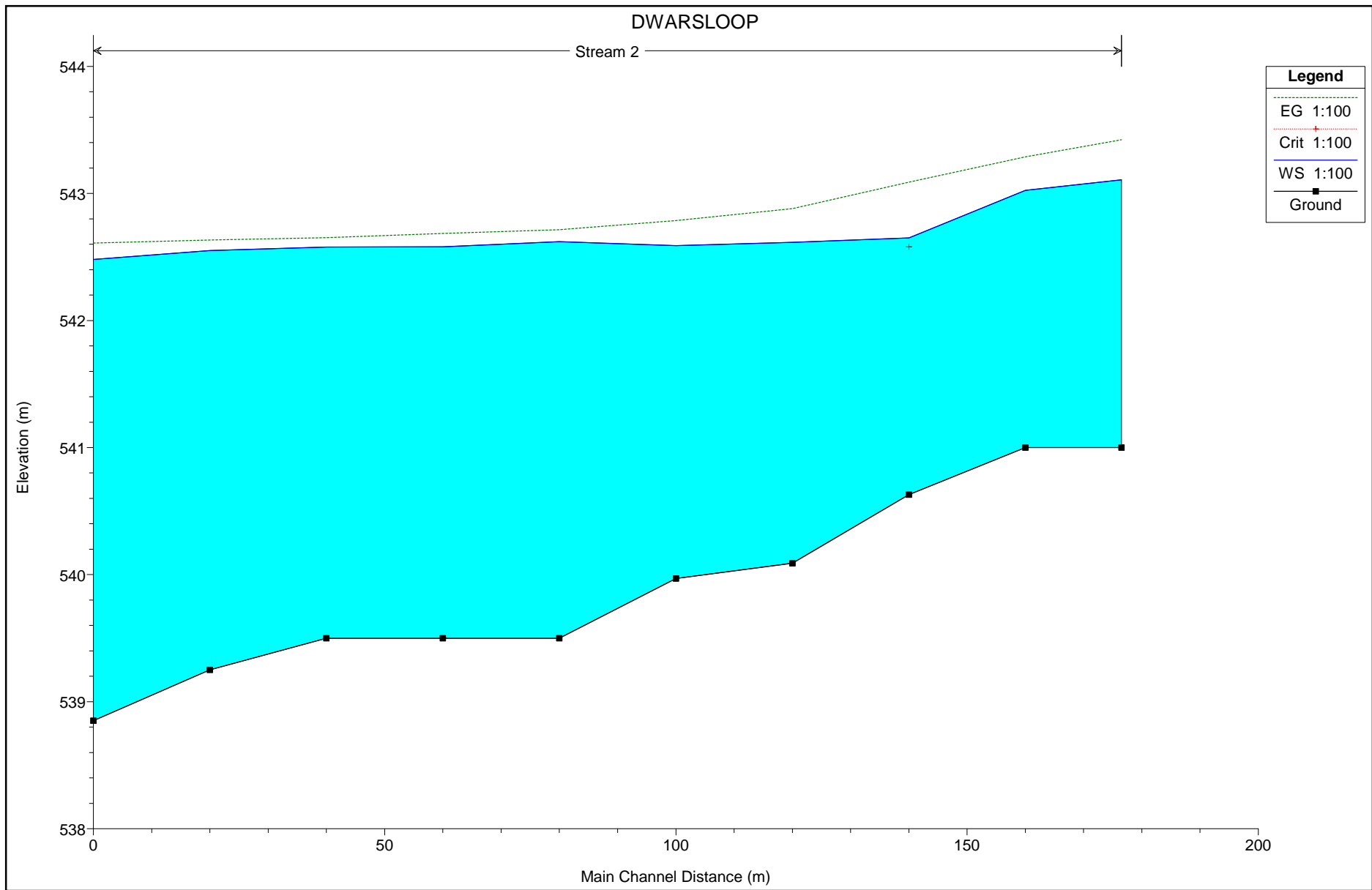
**ANNEXURE 4: LONGITUDINAL FLOW PROFILE FOR THE FLOOD PEAK**



# DWARSLOOP

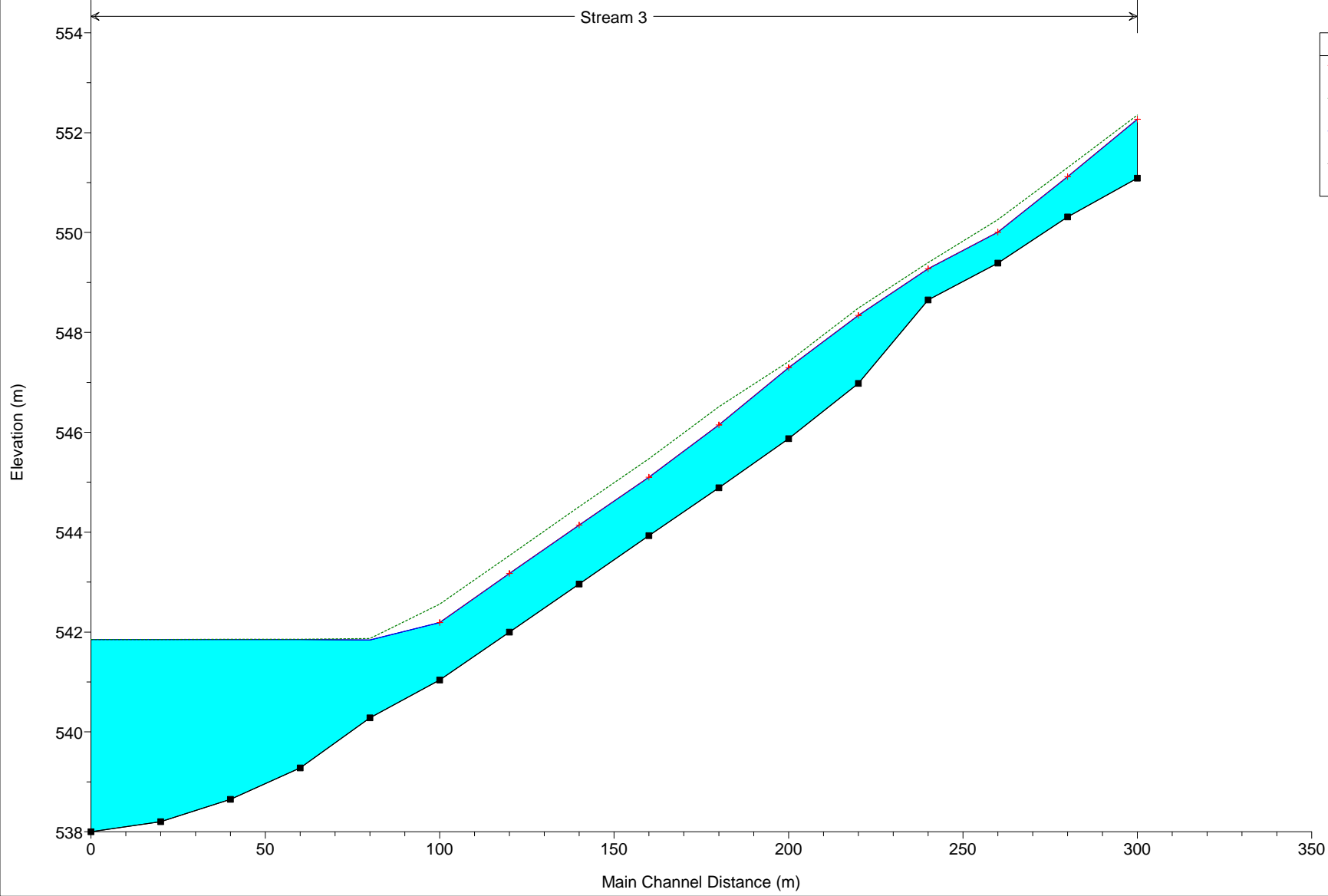
Stream 1



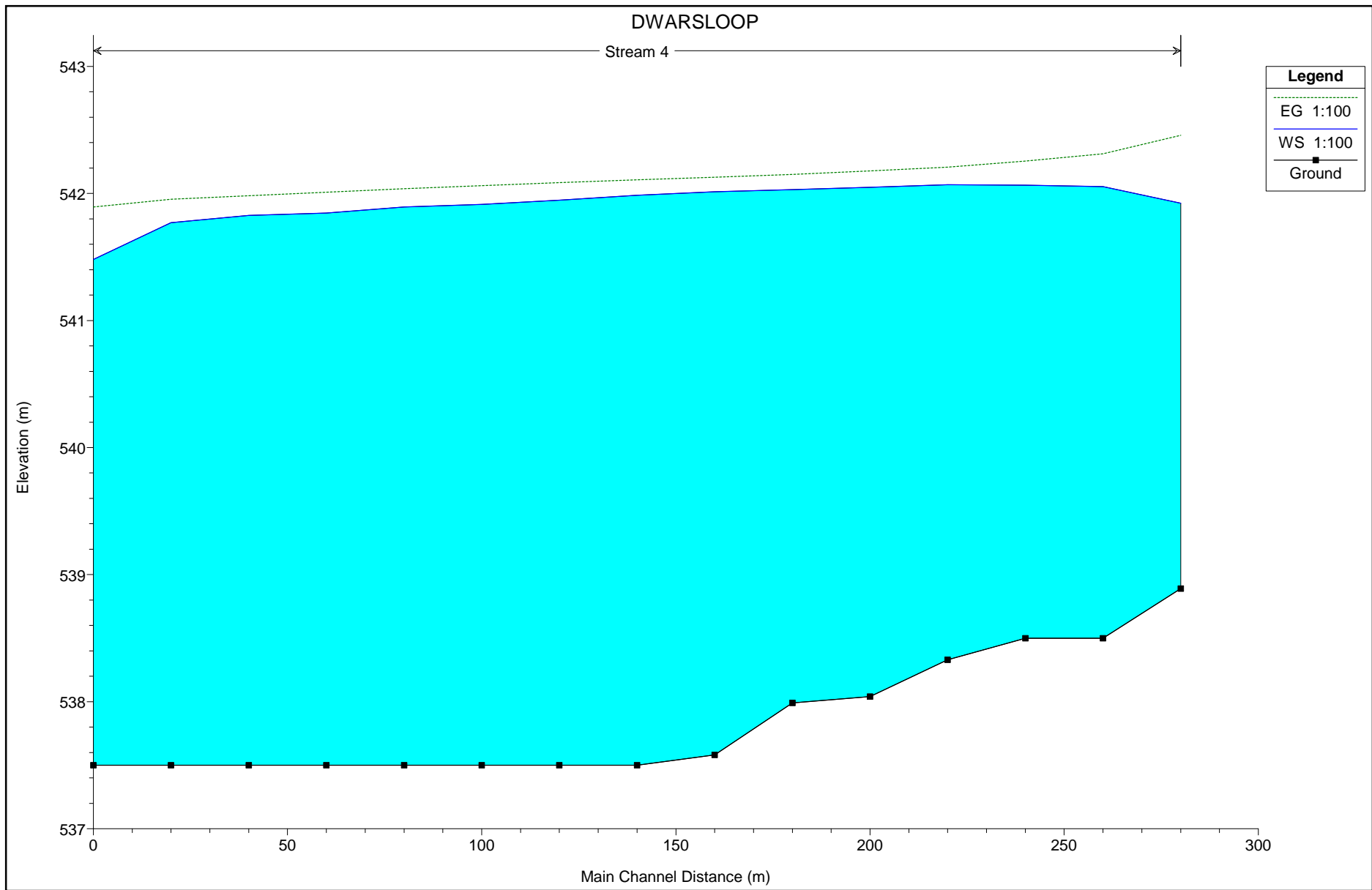


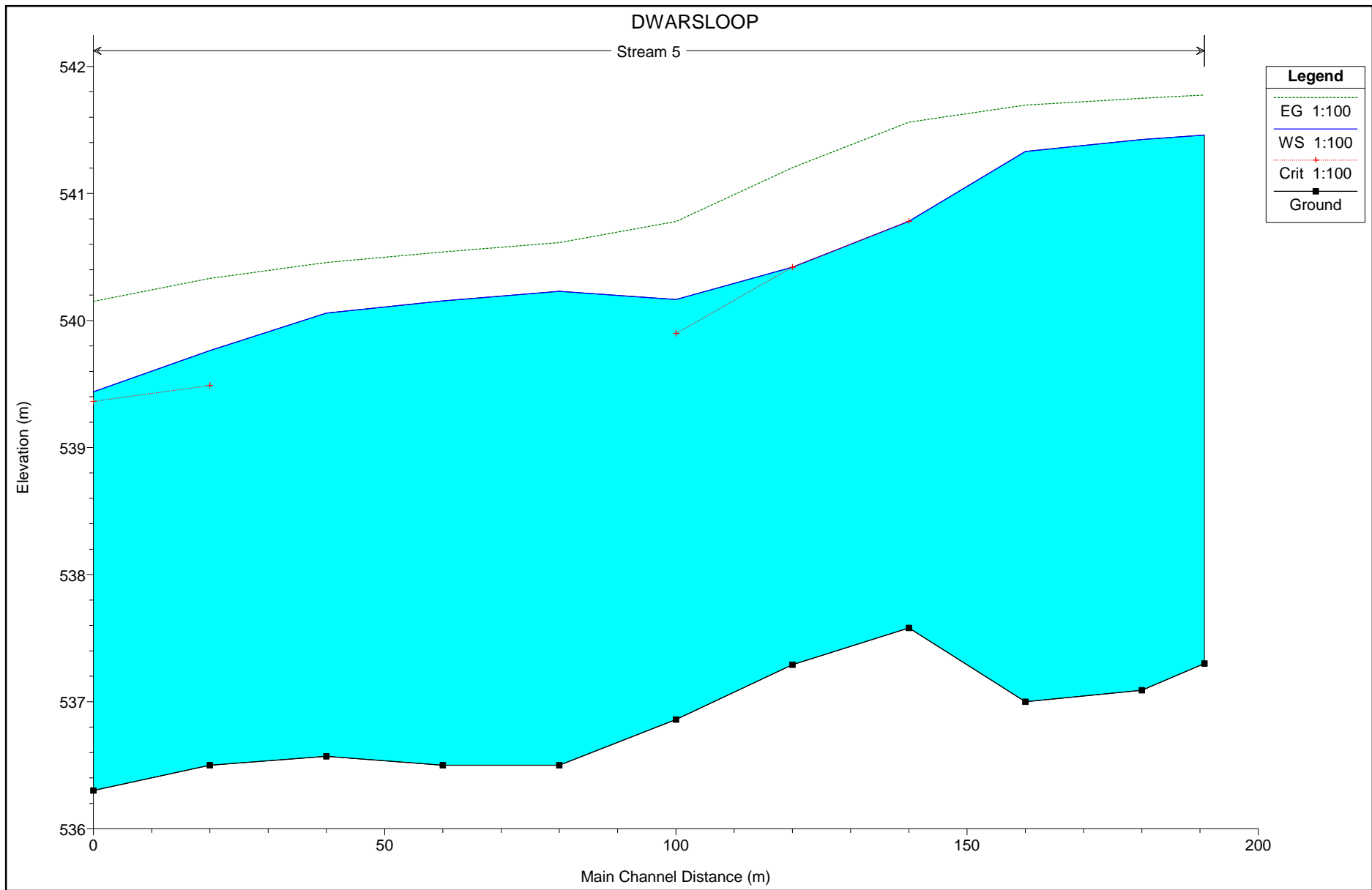
# DWARSLOOP

Stream 3



Legend	
—+—	Crit 1:100
-.-.-	EG 1:100
—	WS 1:100
■	Ground





## ANNEXURE 5: FLOW CROSS SECTIONS FOR THE FLOOD PEAK

