

CONTRACT NO NKT 176/2018

FOR

GEOTECHNICAL INVESTIGATION REPORT OF 20KM RAW WATER PIPELINE FROM ARLINGTON TO LINDLEY

GEOTECHNIAL INVESTIGATION REPORT



PREPARED FOR:

NSVT CONSULTANTS 1 FOURTH STREET OFFICE 1A, ARBORETUM BLOEMFONTEIN, 9301

CONTRACT NKT 176/2018

FOR

GEOTECHNICAL INVESTIGATION REPORT OF A 20KM RAW WATE PIPELINE FROM ARLINGTON TO LINDLEY

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THIS DOCUMENT WAS COMPILED BY

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TABLE OF CONTENTS

EXE	CUTIVE SUMMARY 1
1.	INTRODUCTION AND TERMS OF REFERENCE
2.	DESCRIPTION OF THE SITE AND ACCESS
3.	INVESTIGATION PROCEDURE
3.1	DESK STUDY
3.2	FIELD-WORK
3.3	LABORATORY TESTING
4. 4.1	SITE GEOLOGY AND CLIMATE
4.2	TOPOGRAPHY, DRAINAGE and SITE CLIMATE
4.3	GEOHYDROLOGY
5.	SUMMARY OF LABORATORY RESULTS
5.1	POTENTIAL EXPANSIVENESS
5.2	EXCAVATION CLASSIFICATION
5.3	ERODABILITY
5.4	GROUND SLOPE STABILITY
5.5	CALIFORNIA BEARING RATIO TEST11
6.	ENGINEERING PROPERTIES OF SOILS
7.	GEOTECHNICAL CONSIDERATIONS
7.1	CLASSIFICATION OF SOILS
7.2	EXCAVATABILITY
7.3	SOIL CLASSIFICATION
7.4	GROUND WATER13
7.5	STABILITY OF SLOPES AND EXCAVATIONS
7.6	FLOOD LINE
8.	CONCLUSIONS AND RECOMMENDATIONS
8.1	EXCAVATABILITY
8.2	GEOHYDROLOGY14
8.3	CONSTRUCTION MATERIAL
8.4	STABILITY OF EXCAVATIONS
	OF REFERENCES

TABLE OF FIGURES

Fig 1: Site Access	.3
Fig 2: Geological Map of the Study	5
Fig 3: Macro-Climatic Regions of Southern Africa (Adapted from Weinert, 1980)	6
Fig 4: Regional Distribution of Expansive Clays	10

TABLE OF TABLES

Table 1: Reference Summary	.2
Table 2: Summary of Laboratory Results	7
Table 3: Estimated Potential Heave	.9
Table 4: Estimated Ultimate Bearing Capacity	11
Table 5: COLTO/TRH14 Classification of Materials	.12

EXECUTIVE SUMMARY

Magareng Civil Laboratory was appointed by **NSVTConsultants** to compile a geotechnical engineering report based on the conditions on site. The Investigation was carried out in March 2019.

The following activities were carried out to finalize this report

- Desktop Study
- Site Visit
- Field mapping
- Soil profiling
- Laboratory testing (in progress)

The coordinates of the site is 27°57'27.80"S 27°52'45.24"E.

The study area encompasses approximately 20 kilom etersbetween Arlington and Lindley in the Free State Province. Forty (40) test pits were excavated to a depth of 2 meters or shallower refusal and the soil profiles were described according to the standard procedure.

Disturbed samples of the most prominent soil horizons were taken and submitted for indicator, CBR, pH and Conductivity tests. According to the test pits dug, some excavatibility constraints are expected on this site with 24 refusals shallower than 1.5m. Several rock outcrops (dolerite) were identified on site between test pits 34 and 40 near Lindley.

The geological map from the Council for Geosciences indicates that the site is underlain by fine- to medium-grained, yellow and khaki-coloured sandstone, red, purple and green mudstone of the Tarkastad Subgroup as part of the Beaufort Group.Post-Karoo dolerite intrusions may be encountered throughout the study area, especially near Lindley and Arlington. The soil horizons consisted of sity, clayey and sandy materials with ferricrete, sandstone, mudstone and dolerite ancountered in places. The profiles were recorded in the attached soil profiles included as Annexure A.

No ground water occurred in any of the test pits. This may be due to the extreme dry conditions that currently exists in the country.

The potential expansiveness of the material encountered on the site was calculated according to the method proposed by Van der Merwe (1964). The following material characteristics are considered when applying this method:

- Clay content
- Plasticity index
- Liquid limit
- Linear shrinkage

Low expansive in almost ALL the test pits

The following geotechnical considerations that could influence the proposed development were identified:

Engineering properties of soils:

The clayey materials encountered on site should be cut to spoil insofar economically feasible while materials with a plasticity index below 10 being considered suitable for backfilling of the pipe line. The



bedding and blanket materials should comply with the relavent specifications as set by SANS 10200 or the relevant project specifications.

Generally speaking it could be summarised that the geotechnical conditions of the site are **FAVOURABLE** for the proposed pipe line, provided that cognisance is taken of the expected excavatability constraints.

1. INTRODUCTION AND TERMS OF REFERENCE

Magareng Civil Laboratory was appointed by NSVT to compile a materials report on the 20km raw water pipeline project between Arlington and Lindey in the Free State Province. The site field investigation was undertaken according to the normal requirements for a pipeline project.

The following aspects were addressed in this report:

- 1.1 Geology and soil profiles
- 1.2 Geohydrology
- 1.3 Engineering properties of soil samples taken

The schedule of services include trial pits (40 for this project), with material classifications (classified according to COLTO), grading analysis, Atterberg limits and potential expansiveness of the in-situ material. For the purpose of this study, 20 foundation indicators were sampled with 20 maximum dry density, optimum moisture content and California Bearing Ratio samples.

Table 1: Reference Summary

Description	Quantity	Relevant method or specification
Test Pits Excavated	40 test pits	As per quotation, excavated by TLB.
Fieldwork and Sampling	20 samples	Sampled according to TMH 5 with relevance to SAICE Geotechnical Investigations Manual. No deviations were recorded.
Analysis of samples	20 samples	Subjected to analy sis according to SANS 3001:2011 GR1, GR3, GR10, GR20, GR30 and GR40
Dynamic Cone Penetration Tests (DCP)	78 tests	As per quotation.
Material Classifications	20 classifications	According to COLTO 1998 and TRH14
pH and Conductiv ity	0 samples	Subjected to analy sis according to TMH1 1986: Methods A20 and A21

Phase 1: Fieldwork, which includes the excavation of 40 Test pits, profiled to at least 2m deep or to shallower refusal for soil profiling and sampling purposes as part of the contract.

<u>Phase 2:</u> Laboratory testing to establish the characteristics of the in-situ materials on site done by **MAGARENG CIVIL LABORATORY (PTY) LTD**

The testing includes:

- Sieve Analysis and Grading
- Hydrometer Analysis and Moisture content testing
- Atterberg Limits
- Moisture Density Relationship and Californian Bearing Ratio

Phase 3: Assessment Reporting done by NKIaas, which includes the following:



- Geotechnical assessment of the site conditions and recommendations thereon
- Any Precautions to be taken with regards to the geotechnical conditions for the proposed development.
- Other requirements

This report outlines the method of the investigation and describes the geological conditions encountered. The results of the investigation are evaluated and conclusions drawn with regard to the above objectives.

2. DESCRIPTION OF THE SITE AND ACCESS

Arlington is a very small town in the north eastern Free State on the R707 between the towns of Lindley and Senekal. Arlington's 'reason for being' is primarily as an agricultural rail-link, at the rail jundure between the Lindley – Senekal Line and the Bethlehem – Steynsrus Line.

Arlington form part of Nketoana Local Municipality (NLM) jurisdiction situated within the Thabo Mofutsanyana District. The site geographical coordinates are 28°2'0" South, 27°51'0" East.

The study area encompasses approximately 20km between Arlington and Lindey in the Free State Province. Forty (40) test pits were excavated to a depth of 2 meters or shallower refusal and the soil profiles were described according to the standard proposed by Jennings, Brink and Williams (1973).

Access to the site is obtained as follows: (Figure 1)



Figure 1: Site Access



3. INVESTIGATION PROCEDURE

3.1 DESK STUDY

A desk study involving the perusal of the 1:250000 geological maps as well as a detailed geological description of the area by Brink (1979) was undertaken to establish broad geological boundaries Geological information obtained from the Council of Geoscience is depicted in Figure 2 within section 4.1.

3.2 FIELD-WORK

The field-work included the excavation of 40 test pits, TP1 to TP40, across the site, in order to determine the soil formations of the underlying soil and to obtain samples for possible laboratory testing.

The test pitswere excavated by TLB to a depth of 3 meteror refusal. The test pits positions are indicated on Figure 1. The soil profiling of the 40 test pits was carried out according to the guidelines proposed by Jennings et al (1973). The profile logs of the test pits are given in Appendix A. Soil samples were taken from strategic horizons along the sides of the test pits for laboratory testing (Appendix B).

3.3 LABORATORY TESTING

Soil samples taken during the field-work stage were submitted to the laboratory for the following testing:

- a) Foundation Indicator Test: SANS 3001 GR1, GR10
- b) Optimum Moisture Content and Maximum Dry Density Test: SANS 3001: GR20 and GR30
- c) Californian Bearing Ratio of a Soil Sample: SANS 3001 GR40
- d) pH Test: TMH1 Test Method A20
- e) Conductivity Test: TMH1 Test Method A21T

The test results are included in Appendix B at the back of the report.

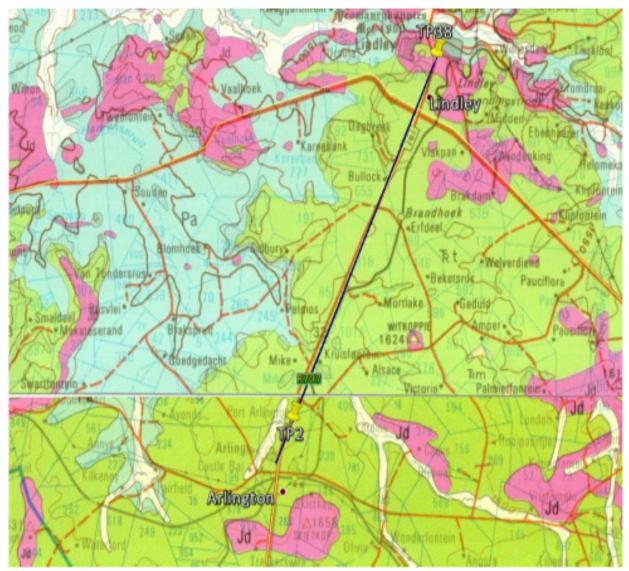


4. SITE GEOLOGY AND CLIMATE

4.1 GENERAL GEOLOGY

The geological map from the Council for Geosciences indicates that the site is underlain by fine to medium-grained, yellow and khaki-coloured sandstone, red, purple and green mudstone of the Tarkastad Subgroup as part of the Beaufort Group. Post-Karoo dolerite intrusions may be encountered throughout the study area, especially near Lindley and Arlington. The soil horizons consisted of silty, clayey and sandy materials with ferricrete, sandstone, mudstone and dolerite ancountered in places The profiles were recorded in the attached soil profiles included as Annexure A.

Figure 2: Geological Map of the Study Area





4.2 TOPOGRAPHY, DRAINAGE and SITE CLIMATE

The study area is next to Route R707 between Arlington and Lindley. The drainage of Route 707 may lead to an influx of moisture into the soil profile, drainage measures may be required if problem materials exists in the soil profile. The study area has a flattish gradient and appears to drain well. The area is located in a moderate climatic zone of South Africa with a Weinert N-value between 2 and 5. The region is a moderate one with warm summers, summer rainfall and cold dry winter months.

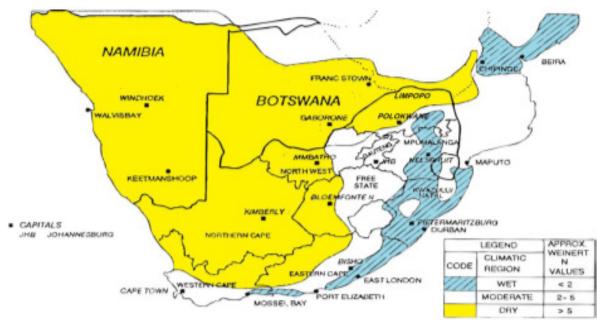


Figure 3: Macro-Climatic Regions of Southern Africa (Adapted from Weinert, 1980)

4.3 GEOHYDROLOGY

No ground water occurred in any of the test pits. This may be due to the extrem e dry conditions that currently exists in the country.



5. SUMMARY OF LABORATORY RESULTS

Table 2: Summary of Laboratory Results

TEST PIT	DEPTH (mm)	MATERIAL DESCRIPTION	PLASTICITY INDEX	PASSING 5mm	PASSING 0.425mm	PASSING 0.075mm	GRADING MODULUS	LIQUID LIMIT	MAXIMUM DRY DENSITY	MOISTURE	CBRAT 95% MOD AASHTO	CLASSIFICATION COLTO/TRH14
1	1740	Reddish clayey sand with mudstone	10.3	98	93		1.11	31.52	1940	11.3	23	
2	2000	Shale plus clay	15.8	65	40	25	1.82	37.8	1952	10.7	25	
3	1250	Dark brown, shale plus clay	8	93	83		1.28	32.97	1892	10.5	25	
4	1150	Dark brown, clay soil + ferrecrete	16.2	98	88		1.2	34.56				
6	2070	Sandstone plus clay	17.2	99	90		1.13	37.39	1876	12.7	25	
7	2070	Dark brown, sandstone plus clay	16	90	84		1.29	42.29	1743	14.2	24	
8	1140	Reddish sandstone plus clay	14.9	92	86		1.24	35.84	1827	14.2	25	
9	2055	Reddish clayey sand with sandstone	9.9	100	99	61	0.41	28.5	1886	13.6	24	
10	2150	Dark brown silty sandstone plus clay	14.8	92	96		1.31	36.47	1843	15.5		
11	1160	Dark brown clay soil and ferrecrete	14.23	99	96		1.06	36.93				
12	1100	Shale plus clay	16.2	71	61		1.71	40.44	1977	10.6	25	
13	1640	Shale plus clay	16.2	65	50	18.8	1.722	39.9	1897	11.2	24	
14	1000	Yellowish soft sandstone	9.3	98	93		1.13	27.65				



16	800	Yellowish soft sandstone	6.3	43	38		2.22	31.57				
18	1420	Dark brown sandstone plus clay		67	53	23	1.65	26.20	1956	11.7	25	
19	410	Grey Calcrete silty stone plus clay		98	95		1.09	34.72	1865	12.8	24	
20	1550	Light brown calcrete silty sand with clay	9.9	87	73	51	0.94	28.2	1953	12.1	25	
21	2010	Light brown calcrete silty stone with clay		92	82		1.29	38.26	1914	13.2	23	
22	1740	Clayey sand with sandstone	9.1	89	77	48	0.9	26.5	1894	12.2	25	
23	1500	Dark brown clay ey sand with sandstone	11.7	82	69	44	1.09	29.4	1941	14.7	25	
24	1440	Dark brown sandstone	18.2	97	87		1.2	39.11	1831	15.7	26	
25	1510	Light brown silty calcrete plus silty sandstone		70	63	21	1.5	24.6	2000	10.4	23	
26	2020	Reddish brown sandstone plus clay	23.26	92	80		1.37	56.59	1875	13.7	25	
27	2010	Grey calcrete plus clay	20.16	95	87		1.21	45	1845	15.5	25	
31	1440	Light brown clay ey sand with calcrete	10.7	70	61	43	1.31	28.1				
32	1100	Yellowish soft clay soil plus ferrecrete	14.3	97	87		1.21	36.01				
33	2040	Dark brown clay ey sand with sandstone	18	95	91	60	0.57	39	1677	17.7	26	



The relevant engineering characteristics of the materials encountered have been evaluated by visual assessment during profiling and from the results of the field and laboratory testing; these may be summarized as follows:

5.1 POTENTIAL EXPANSIVENESS

The potential expansiveness of the materials encountered on the site was calculated according to the method proposed by Van der Merwe (1964). The following material characteristics are considered when applying this method:

- > Clay content
- Plasticity index
- ➤ Liquid limit
- Linear shrinkage

The method of Van der Merwe (1964) was used to determine the potential heave of soil samples In addition to Van der Merwe's method, the plasticity index and linear shrinkage of soil samples were used to indicate the soils potential expansiveness. From the laboratory test results the potential expansiveness of all soils on the site is as follows:

Test Pit	Depth (mm)	Plastic Index	Passing 0.045mm	Heave Potential	Estimated Heave (mm)
1	720 – 1740	10.3	93	Low	0
2	370 – 2000	15.8	40	Low	0
3	630 - 1250	8	83	Low	0
4	510 - 1150	16.2	88	Medium	8.6
6	650 – 2070	17.2	90	Medium	15.7
7	600 - 1140	16	84	Low	0
8	1220 – 2055	14.9	86	Low	0
9	860 - 2150	9.9	99	Low	0
10	820 - 1650	14.8	96	Low	0
11	520 - 1160	14.2	96	Low	0
12	220 - 1100	16.2	61	LOW	0
13	250 - 1640	16.2	50	Low	0
14	100 - 1000	9.3	93	Low	0
16	300 - 800	6.3	38	Low	0
18	510 - 1420	6.7	58	Low	0
19	410 - 1210	11.8	95	LOW	0
20	660 - 1550	9.9	73	Low	0
21	670 – 2010	15.56	82	Low	0
24	650 - 1440	18.2	87	Low	0
25	610 - 1510	5.1	63	Low	0
26	820 – 2020	23.26	80	Low	0
27	450 - 2010	26.26	87	Low	0
31	750 - 2040	10.6	61	Low	0
32	+700	14.3	87	Low	0

Table 3 : Estimated Potential Heave

Low expansive in almost all the test pits Medium expansive was experienced in test pits 4 and 6.

Based on van der Merwe's method (1964) heaving clays are considered to be a geotechnical constraint on site. Heaving clays may lead to significant upwards forces which may have an adverse effect on the proposed pipelines. Clayey materials should not be used for backfilling, bedding or blanket materials and should be cut to spoil in-so-far economically feasible.

The site classes are indicated on the soil profiles.



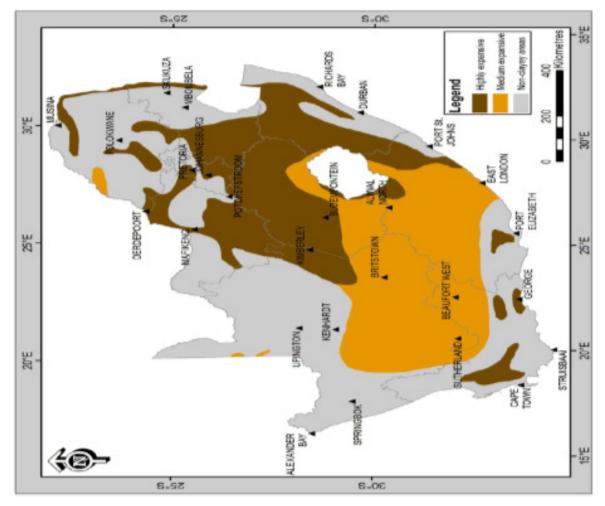


Figure 4: Regional Distribution of Expansive Clays



5.2 EXCAVATION CLASSIFICATION

Excavatability is defined as the ease with which the ground can be dug to a depth of 1,5m. This is of importance for urban development as increased costs are associated with installing services or foundations in areas where difficulty is experienced during the investigation stage.

According to the test pitsdug, some excavatibility constraints are expected on this site with 24 refusals shallower than 1.5m. Several rock outcrops (dolerite) were identified on site between test pits 5, 15, 17, 28,29,30,34 to 40 near Lindley.

In terms of the SABS 1200 the excavations can be classified as soft to intermediate to 1 meterin depth where after it becomes intermediate to hard.

5.3 ERODABILITY

There were no signs of piping (erosion) visible on site.

5.4 GROUND SLOPE STABILITY

No unstable geological materials that can move either gradually (creep) or suddenly as a slump or a slide are visually present.

5.5 CALIFORNIA BEARING RATIO TEST

California Bearing Ratio (CBR) Tests were conducted to determine the estimated ultimate bearing capacity of the saturated material. This serves as a relatively conservative estimation of the bearing capacity of the in-situ material under the worst expected conditions with the assumption that naturally consolidated materials, especially those with overburden in excess of 500mm, will have the same (or higher) degree of consolidation than a MOD AASHTO of 95%.

A paperby W.P.M Blacktitled "The Calculation of Laboratory and In-situ Values of California Bearing Ratio from Bearing Capacity Data" indicates that the CBR values of material are roughly 10% of the ultimate bearing capacity (q_u) of the material. In the paper W.P.M Black suggests using a lower factor in order to obtain more conservative values.

The CBR values can be summarised as follows

Test Pit	Depth (mm)	CBR v alue at 95% MOD AASHTO	ESTIMATED BEARING CAPACITY (kPa)
1	1740	23	195.5
2	2000	25	212.5
3	1250	25	212.5
4	1150		
6	2070	25	212.5
7	2070	24	204
8	1140	25	212.5
9	2055	24	204
10	2150		
11	1160		
12	1100	25	212.5
13	1640	24	204

Table 4: Estimated Ultimate Bearing Capacity (qu)



14	1000	23	195.5
16	800		
18	1420	25	212.5
19	410	24	204
20	1550	25	212.5
21	2010	23	195.5
22	1740	25	212.5
23	1500	25	212.5
24	1440	26	221
25	1510	23	195.5
26	2020	25	212.5
27	2010	25	212.5
31	1440		
32	1100		
33	2040	26	221

6. ENGINEERING PROPERTIES OF SOILS

The clayey materials encountered on site should be cut to spoil insofar economically feasible while materials with a plasticity index below 10 being considered suitable for backfilling of the pipe line. The bedding and blanket materials should comply with the relevant specifications as set by SANS 10200 or the relevant project specifications.

7. GEOTECHNICAL CONSIDERATIONS

7.1 CLASSIFICATION OF SOILS

The materials were classified interms of COLTO and TRH14 for road construction purposes, as shown in the summary of the test pit data. The engineering properties can be summarised as follows:

Test Pit	Layer (mm)	Classification
1	720 – 1740	G6/G7
2	370 – 2000	<g9 <g10<="" td=""></g9>
3	630 – 1250	G6/G7
4	510 – 1150	<g9 <g10<="" td=""></g9>
6	650 – 2070	<g9 <g10<="" td=""></g9>
7	600 - 1140	<g9 <g10<="" td=""></g9>
8	1220 - 2055	< <i>G</i> 9/ <g10< td=""></g10<>
9	860 – 2150	G6⁄G7
10	820 – 1650	<g9 <g10<="" td=""></g9>
11	520 - 1160	< <i>G</i> 9/ <g10< td=""></g10<>
12	220 – 1100	< <i>G</i> 9/ <g10< td=""></g10<>
13	250 – 1640	< <i>G</i> 9/ <g10< td=""></g10<>
14	100 - 1000	G6/G7
16	300 – 800	G6/G7
18	510 - 1420	G6/G7
19	410 – 1210	G6⁄G7
20	660 – 1550	G6⁄G7
21	670 – 2010	<g9 <g10<="" td=""></g9>
24	650 – 1440	<g9 <g10<="" td=""></g9>
25	610 – 1510	G6/G7
26	820 - 2020	< <i>G</i> 9/ <g10< td=""></g10<>
27	450 - 2010	<g9 <g10<="" td=""></g9>
31	750 – 2040	G6/G7

Table 5: COLTO/TRH14 Classification of Materials

Typically material classified as a G5, G6, G7 or G8 can be used for road construction and material classified as G5, G6 and G7 can be used as backfill material, depending on the engineering design and specifications supplied by the consulting engineer.



7.2 EXCAVATABILITY

Several rock outcrops were identified between test pits 34 and 40 near Lindley. Twenty-four (24) test pits refused at depths shallower than 1.5 meters with 9 test pits refusing at depths shallower than 1.0 meter. Excavatibility constraints are expected.

7.3 SOIL CLASSIFICATION

The materials are predominantly clayey, silty and sandy materials with building rubble, organic matter and plastic encountered between 100-600mm.

7.4 GROUND WATER

No ground water was encountered in any of the test pits. Test pit 28 was located in a large pond of water and was not excavated. Water draining from the R707 may ingress into the soil profile.

7.5 STABILITY OF SLOPES AND EXCAVATIONS

All side walls were stable.

7.6 FLOOD LINE

An exact flood-line should be determined, but in this report it is suggested that 1:50 year flood-line is adopted.



8. CONCLUSIONS AND RECOMMENDATIONS

It is important to note that the recommendations are based primarily on the profiling of test pits and the interpolation of information betweentest pits. It is therefore possible that variations from the expected conditions can occur.

8.1 EXCAVATABILITY

Several refusals shallower than 800mm indicate that excavatibility may be a constraint on site.

8.2 GEOHYDROLOGY

Excavations are to be adequately drained should rain water fill trenches during construction or if the water tables rise.

8.3 CONSTRUCTION MATERIAL

The clayey materials encountered on site should be cut to spoil insofar economically feasible while materials with a plasticity index below 10 being considered suitable for backfilling of the pipe line. The bedding and blanket materials should comply with SABS 1200LB.

8.4 STABILITY OF EXCAVATIONS

Excavations were all stable and no side walls collapsed.

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Pr Tech Eng (Civil) ECSA Reg nr 201770095



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LIST OF APPENDICES

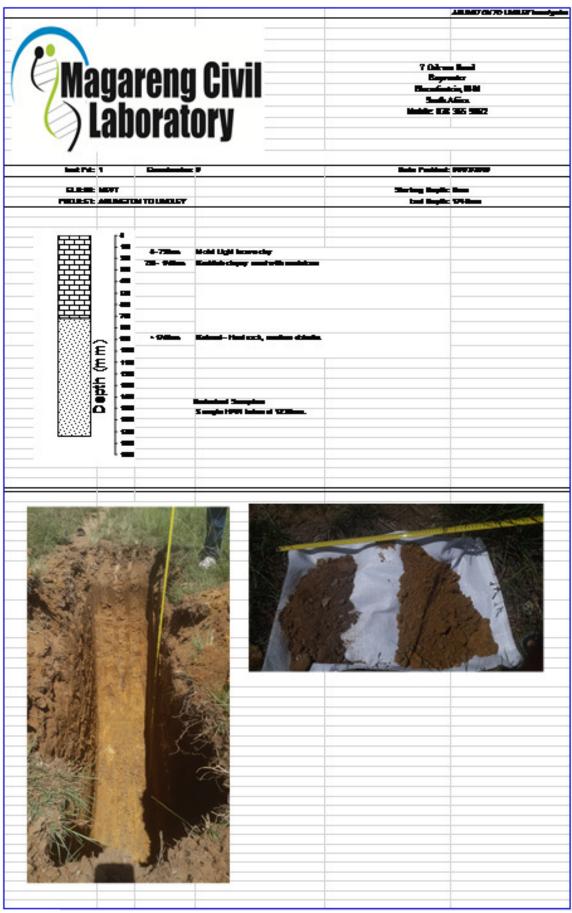
- APPENDIX A: Soil Profile Sheets
- APPENDIX B: Laboratory Test Results
- APPENDIX C: Dynamic Cone Petrometer



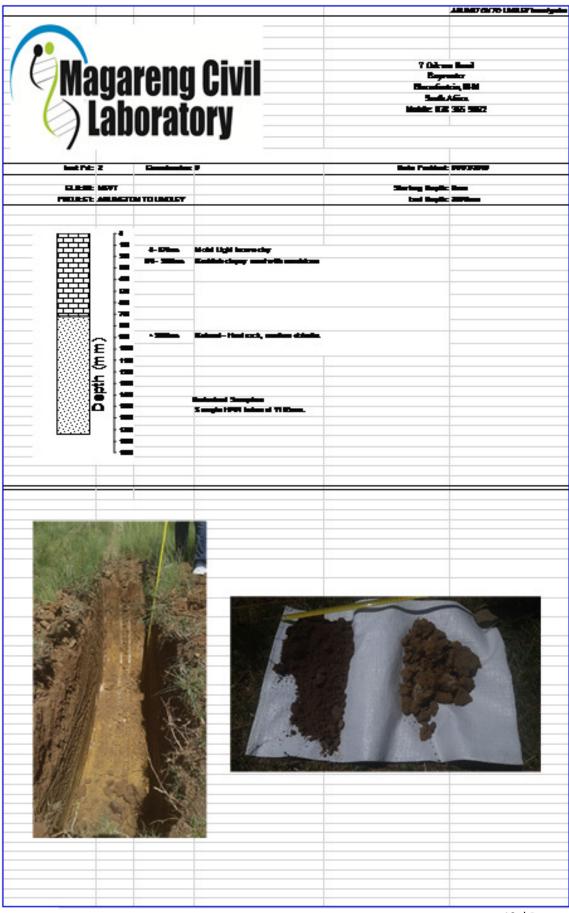
APPENDIX A:

Soil Profile Sheets

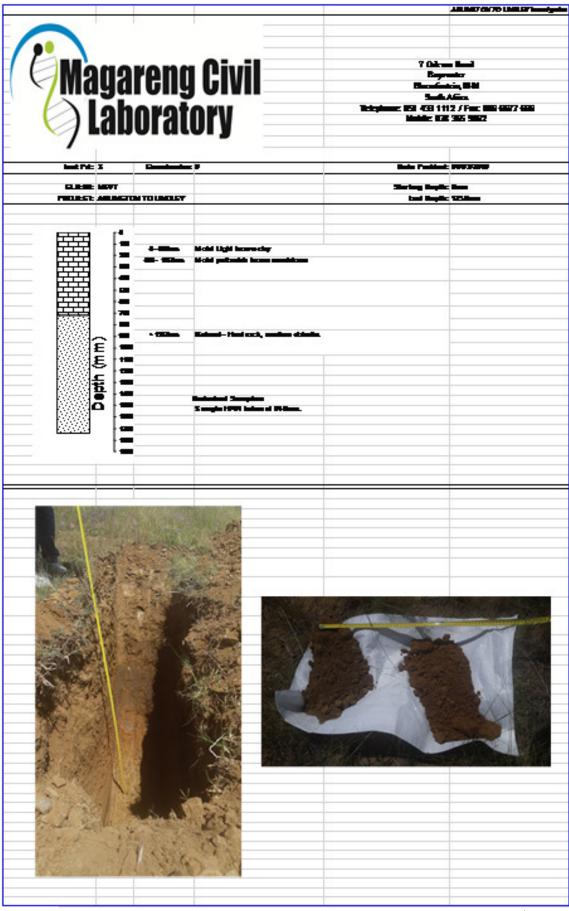




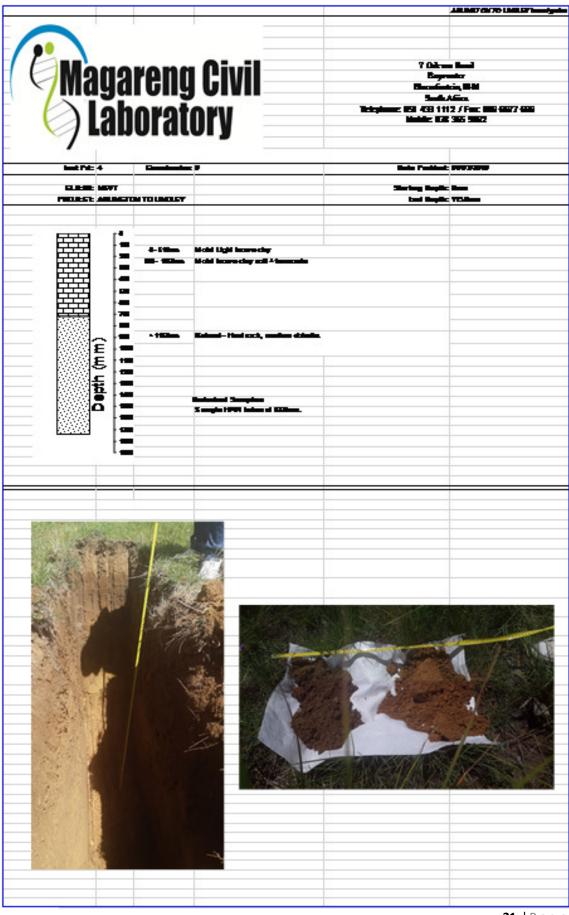




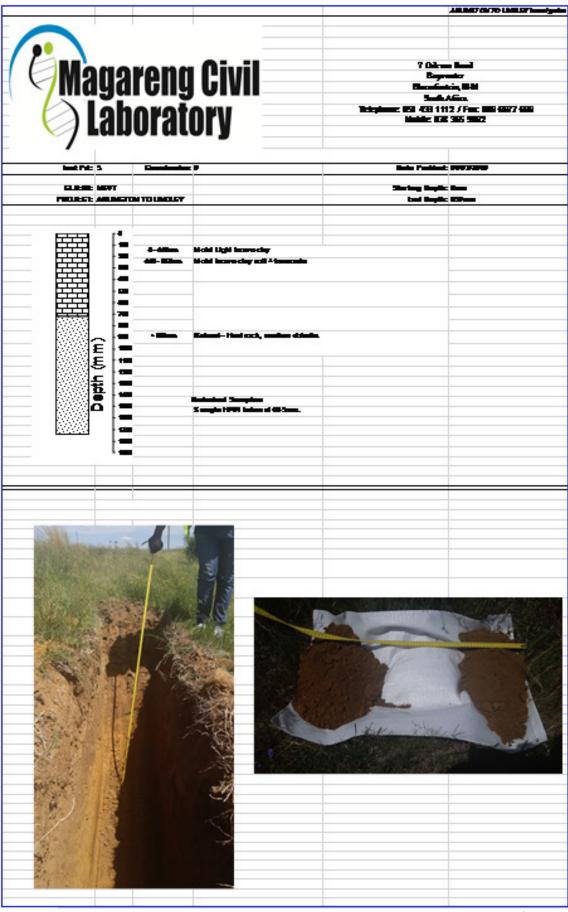






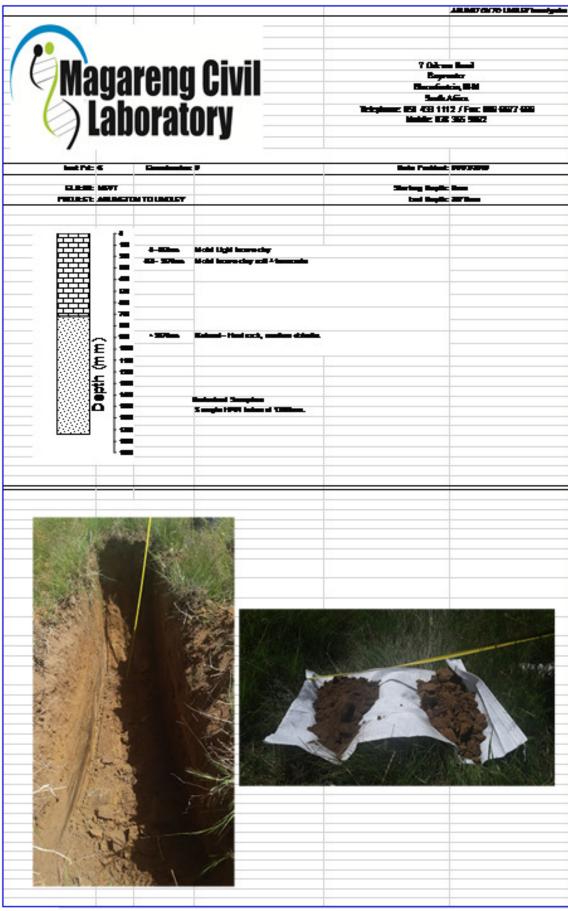




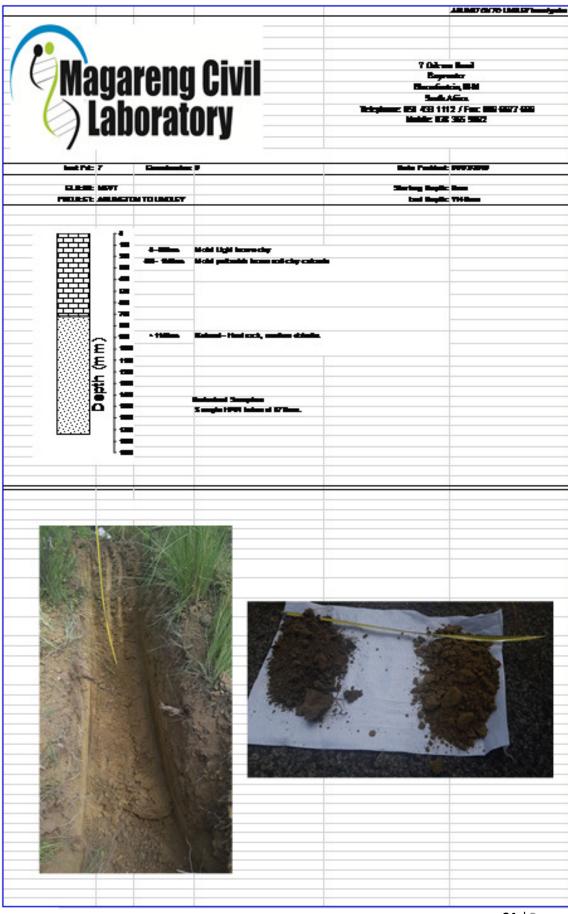




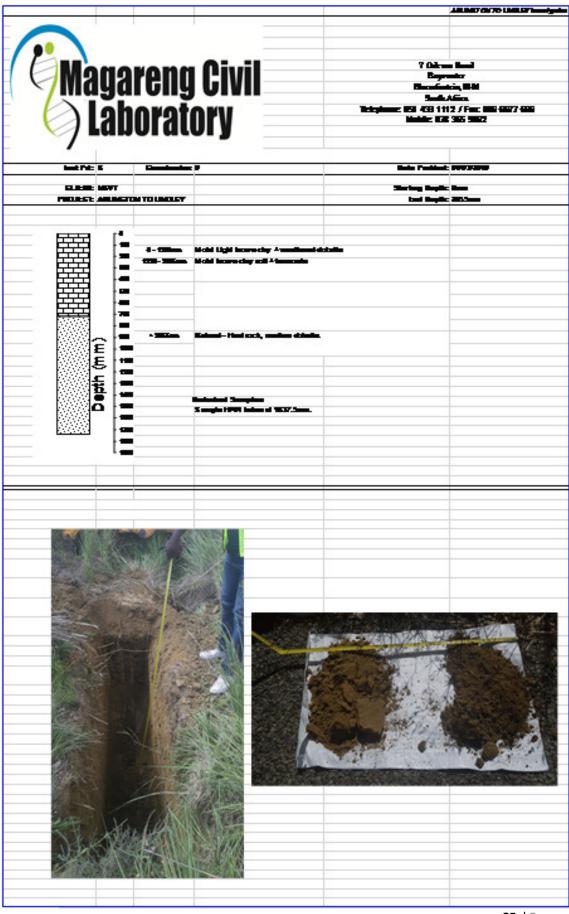
7 Orleans Road, Bay swater, BLOEMFONTEIN, 9301 Tel: 078 365 9862 Email: patric@mcivilab.co.za



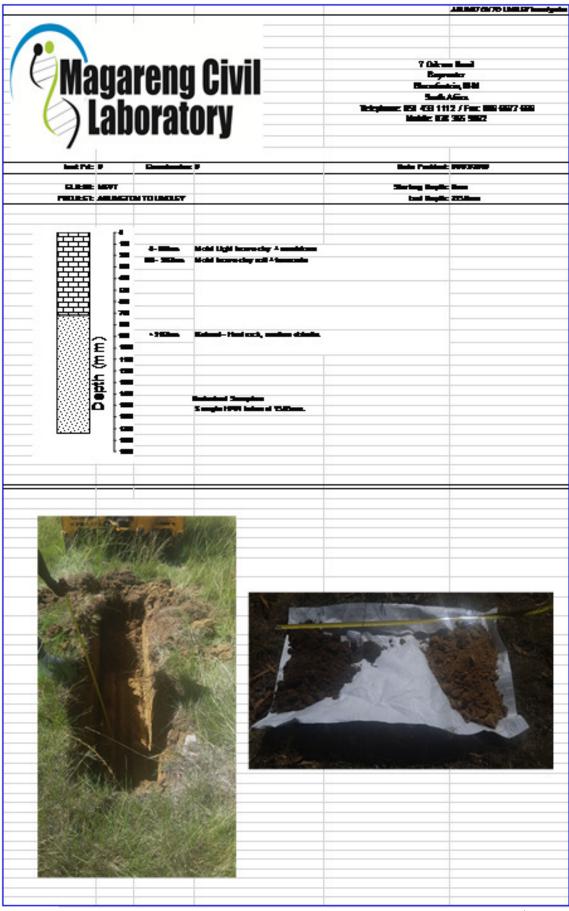




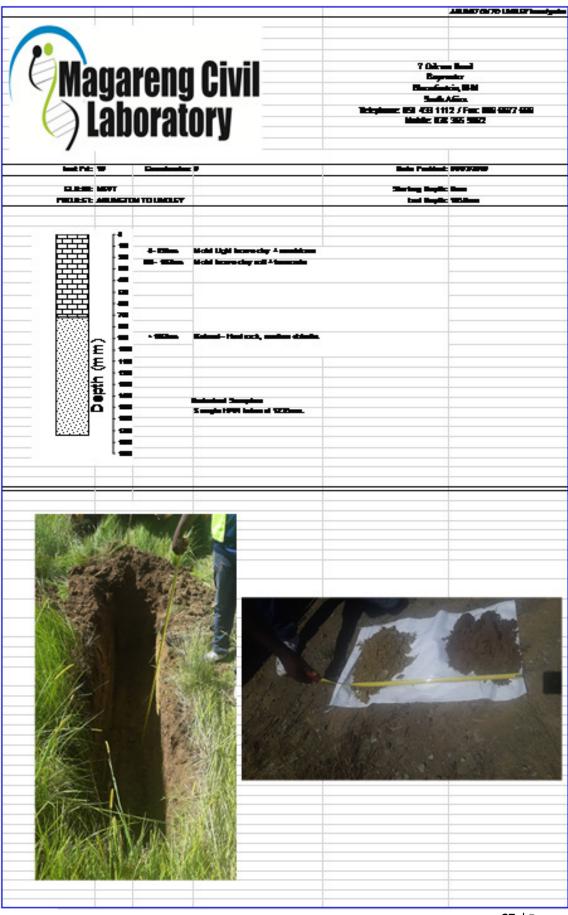




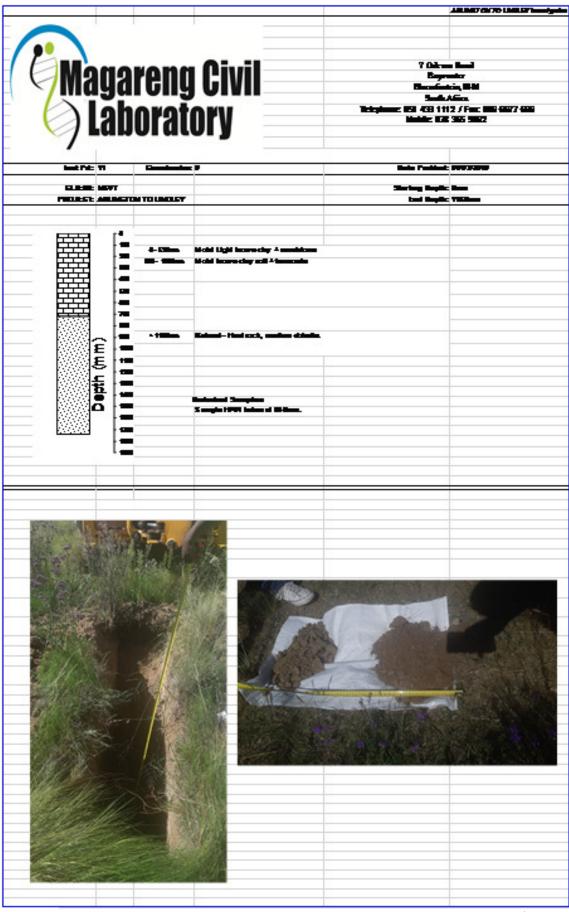




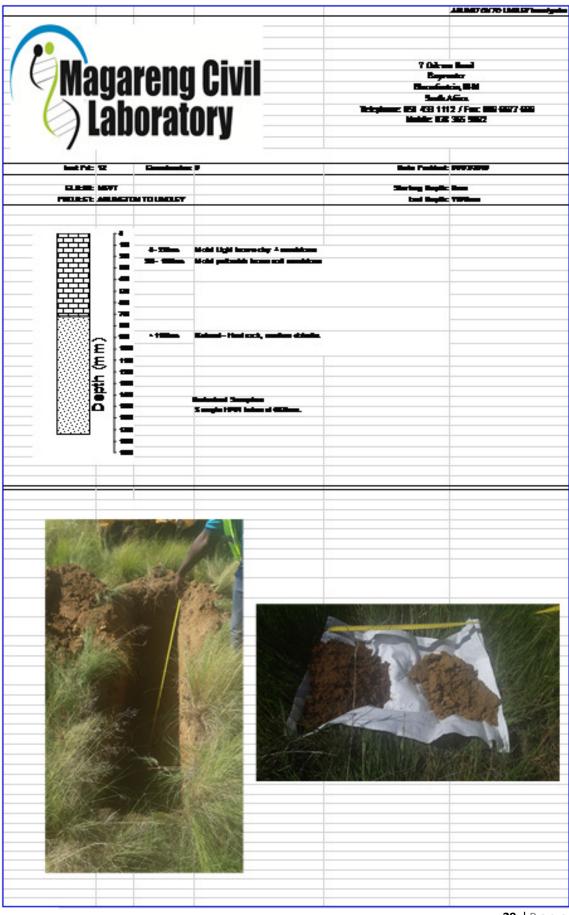




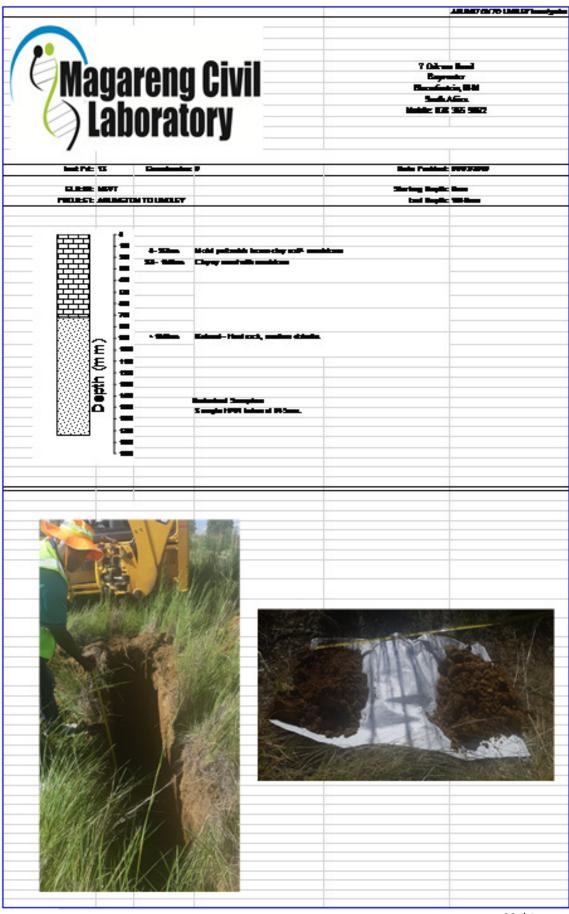




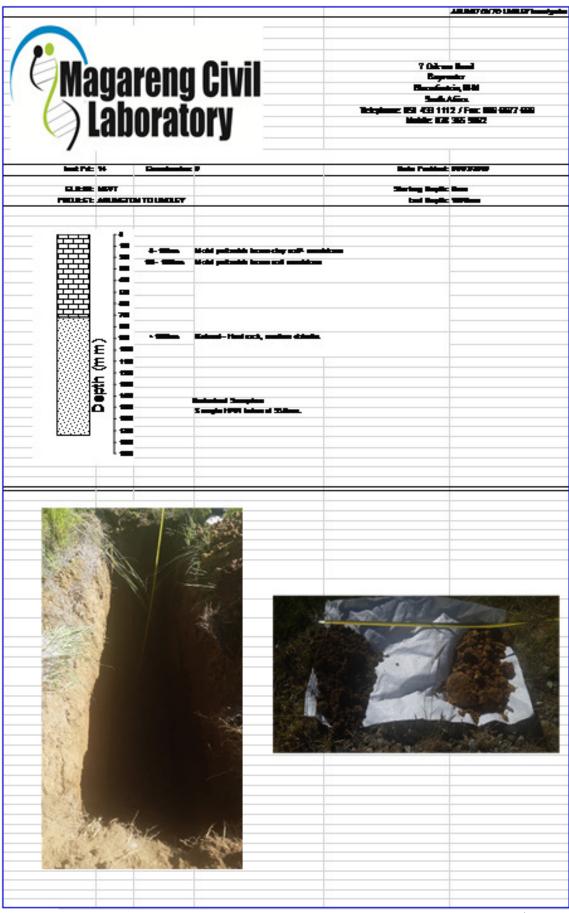




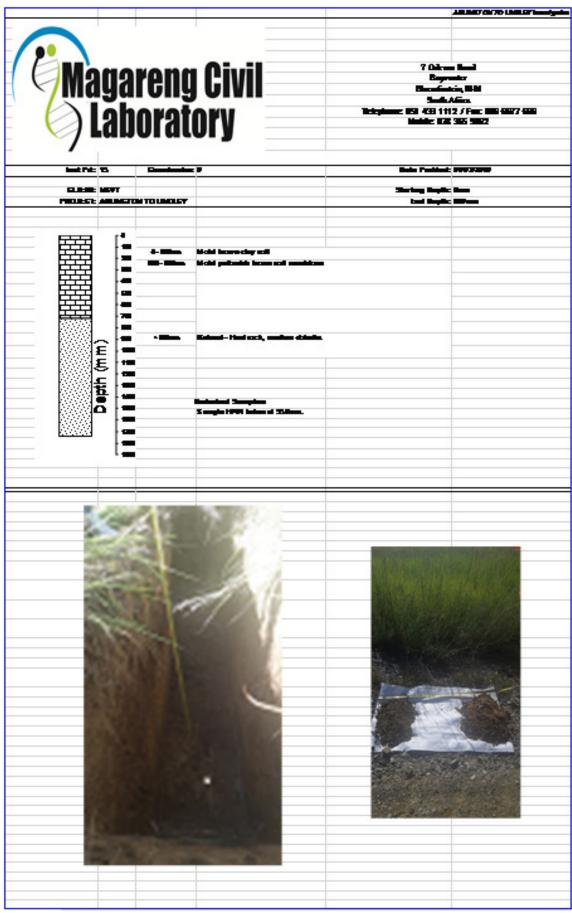




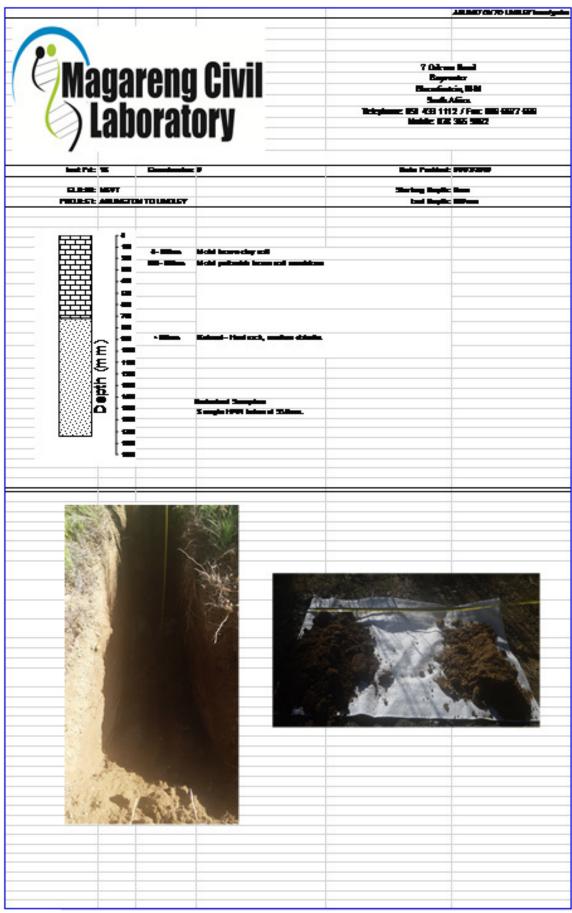




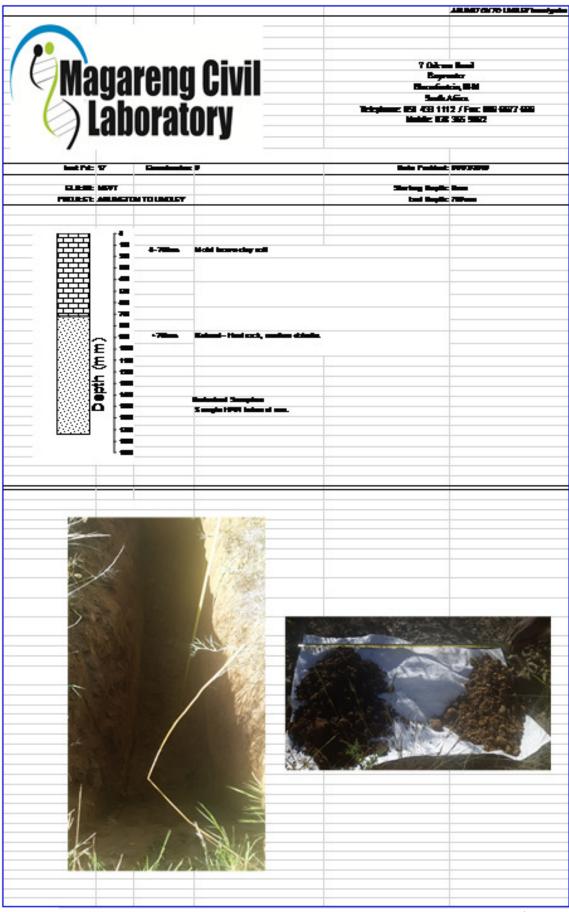




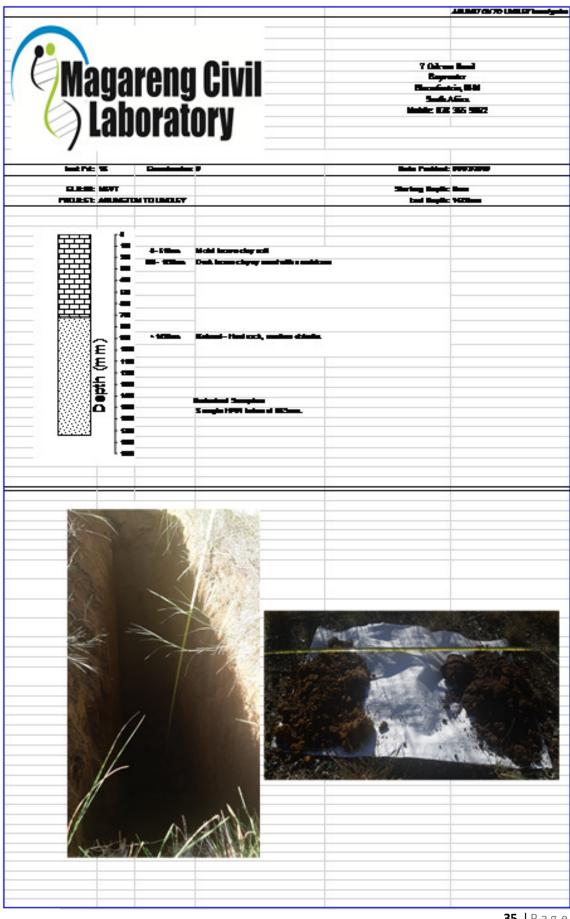




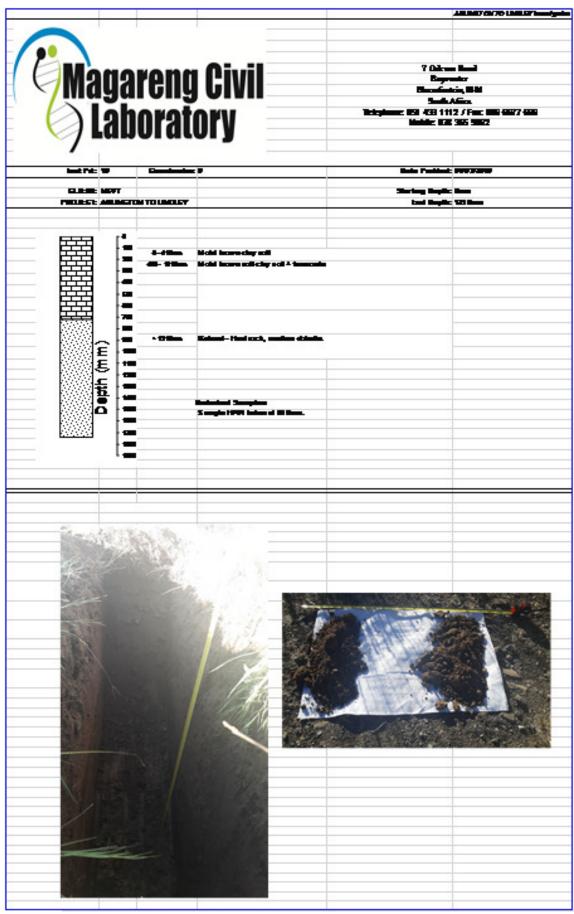




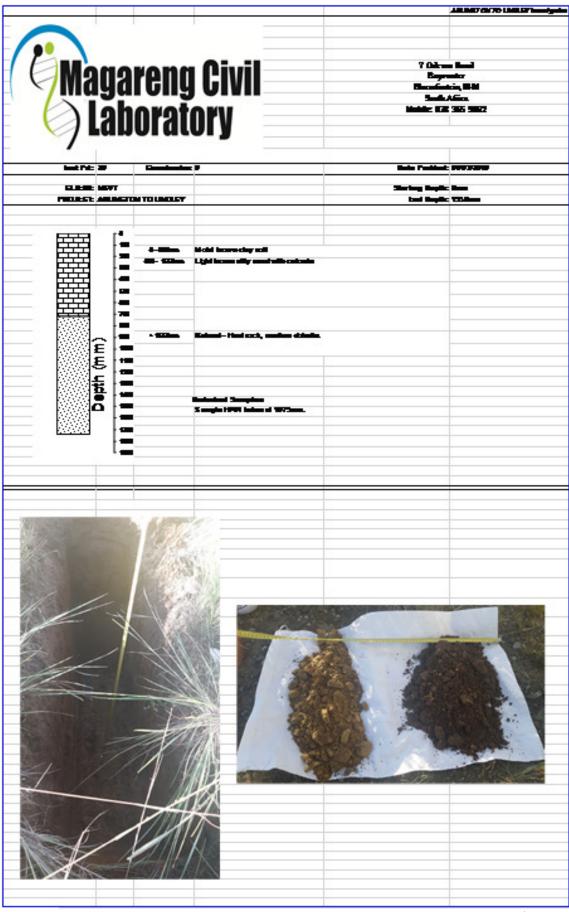




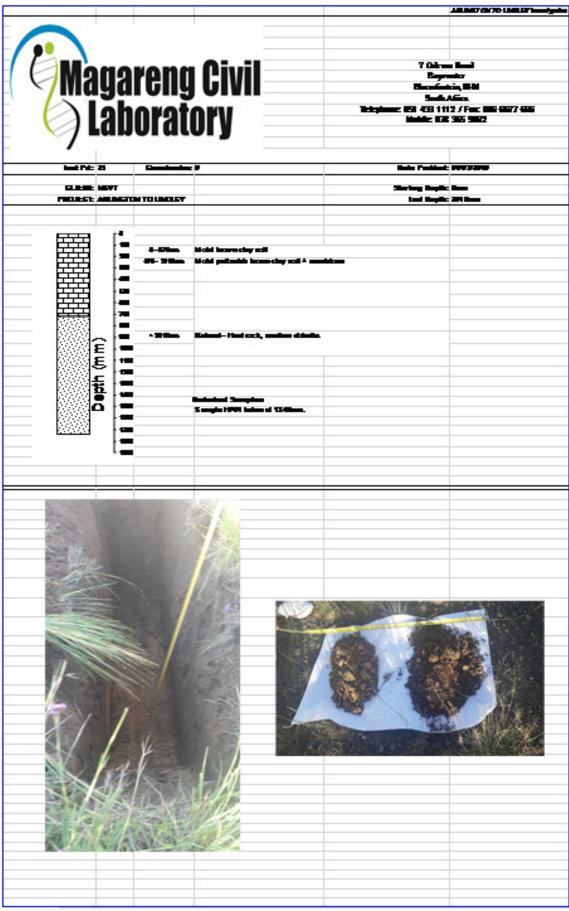




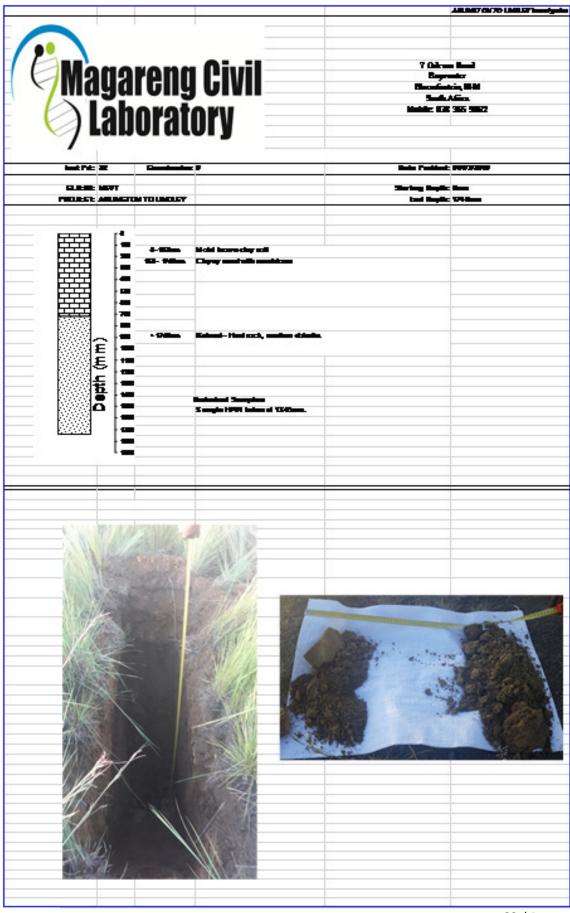




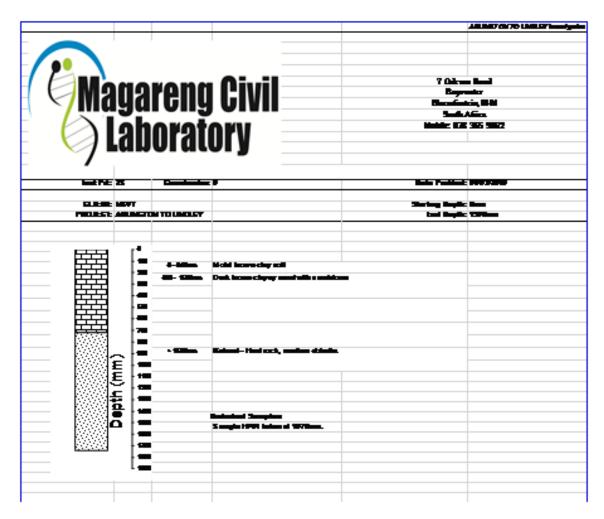








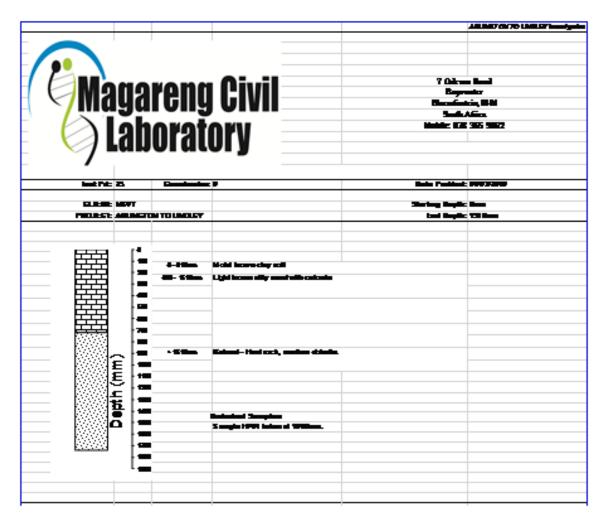






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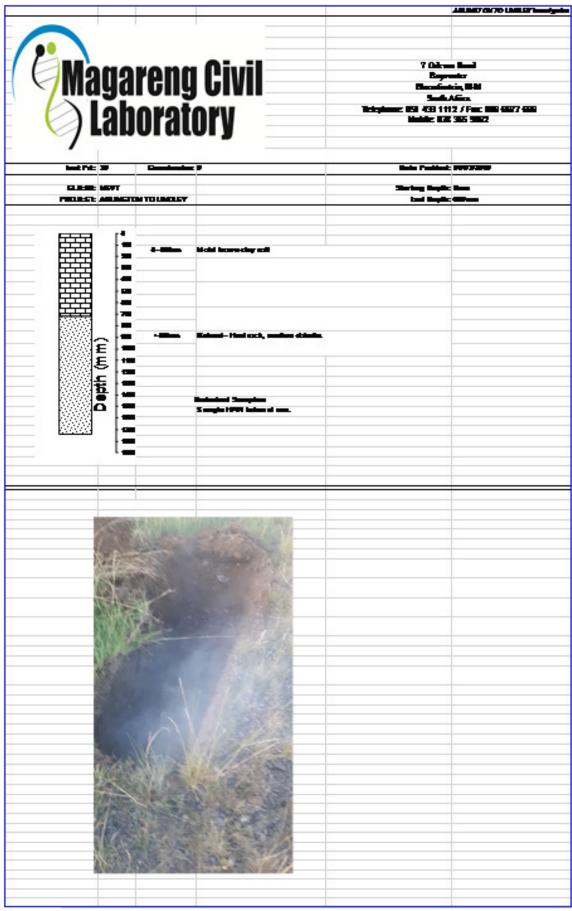


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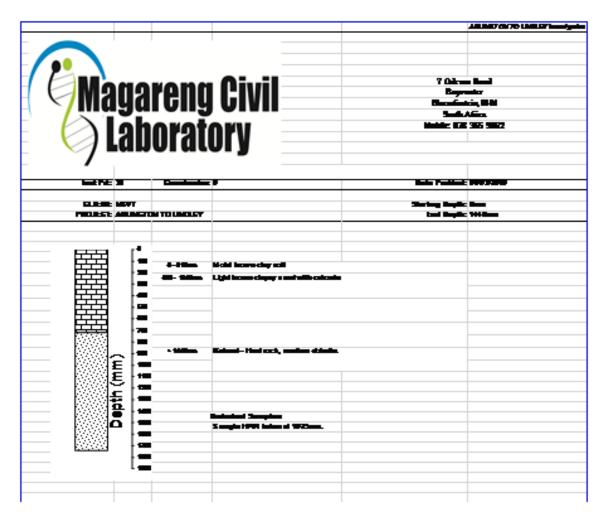


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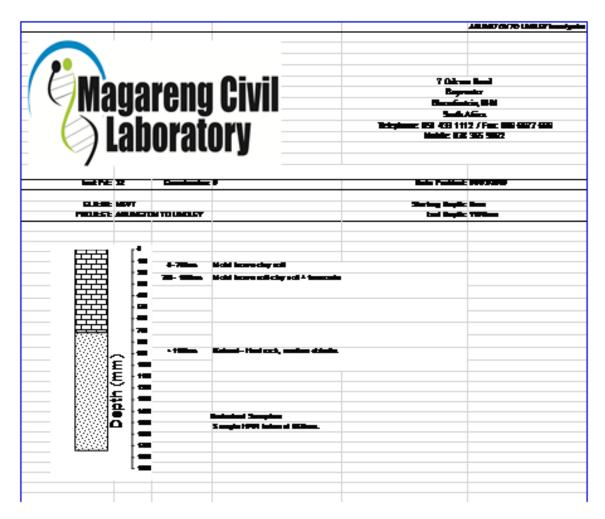




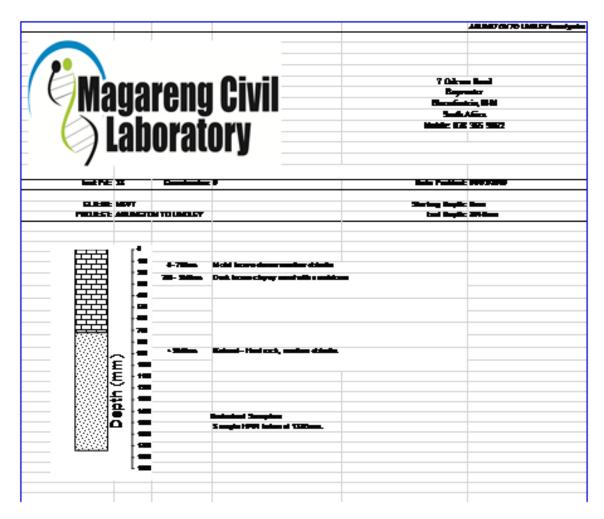




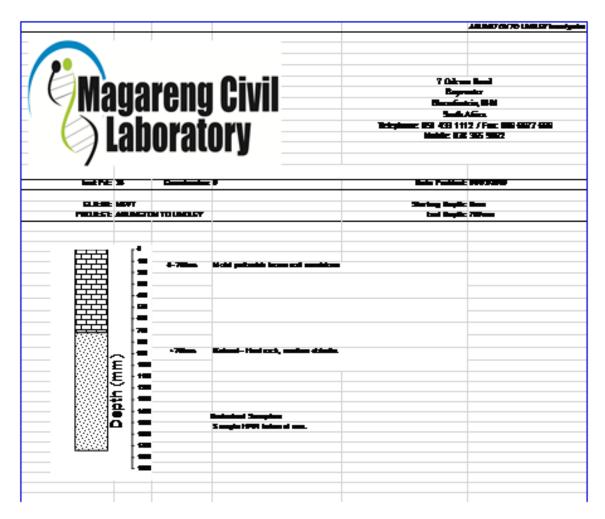




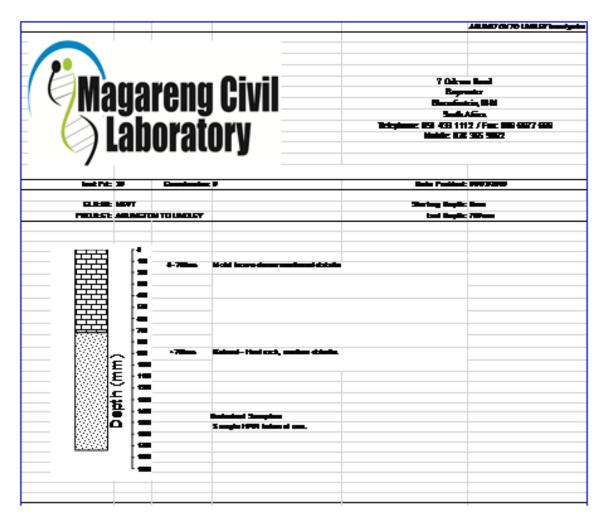




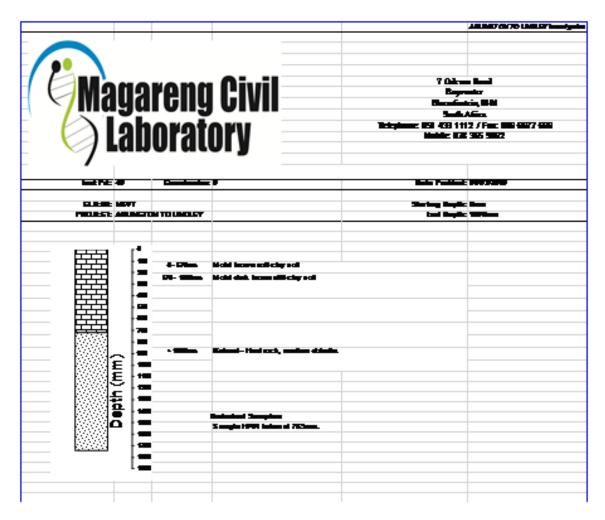














APPENDIX B:

Laboratory Test Results



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			25				
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_	8.158-8.875	52		55.74	49.63		69
	40.75						
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in is	ш	31.52	37.3	25	34.56		1
-	LB	53	13	3.6	32		8.3
BAR 8 2001	-						
	PI	103	15.3	3	162		17.2
	1076	50	53	55			55
California	386	37	38	4D			4D
beering	576	32	34	34			34
Ratio	3736	73	75	25			25
		17	13	13			13
		11	11	12			11
	0 11 1	3.24	3.36	446			457
	051	3.52	3.15	3.87			443
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cA	576						
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112	556						
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Remarks :						Chile :	25-82-15
						Technician :	T. MAGLE



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l	2.00	2	50		36	5	
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	1.75	[1	£ 1			
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	0.150-0.075	52.89	51_54	£1.44	5051	61.63	55
	4075						
Additional of the second se	EM	1.2	1.24	D41	1.31	1.05	1.31
5	ш	42.25	3 .M	78.5	36.47	36.03	40.44
	LE	21	7.4	43	7.3	7.1	8.1
BRIE	R	16	14.9	9.9	143	14.23	16.2
	1076	51	55	52	50		57
California	386	33	ē	38	37		41
beering	576	32	34	<u></u> 88	32		35
Rrito	75	24		 Z4	23		25
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		11	11	11	11	t	11
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						Technician :	T. MAGLE



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in its	<u> </u>	34.72		3.5		29.4	38.11
-		57	<u>782</u> 52	11	<u>265</u> 45	53	9.1
			3.2		4.3		
BRM	PI	11.3	9.9	15.55	9 1	113	18.2
	1075	53	54	5D	56	5	55
California				36	41		43
bearing	576	39	34	31	35	35	36
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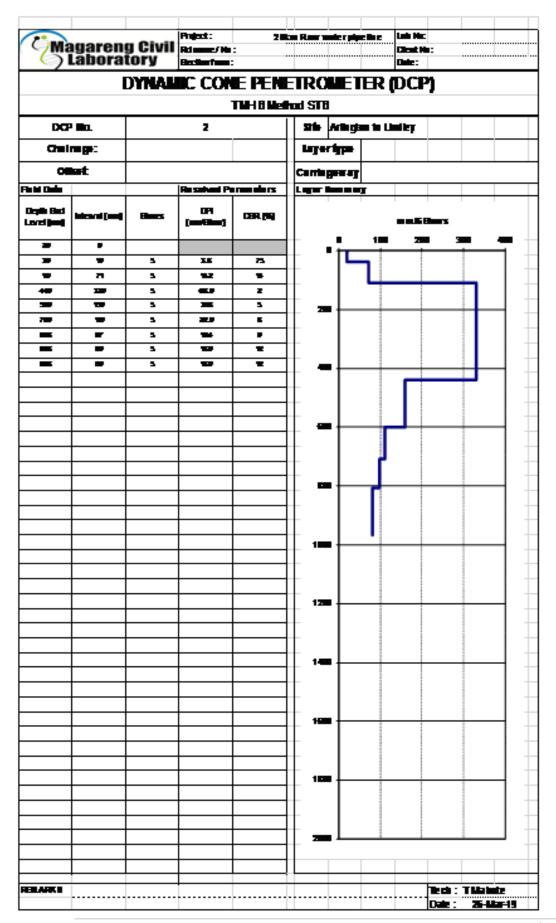
APPENDIX C:

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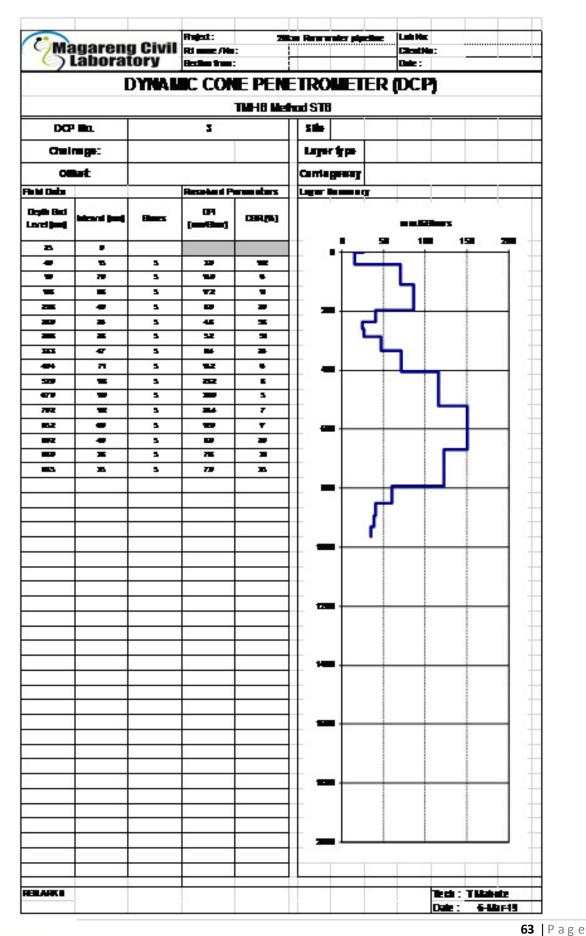


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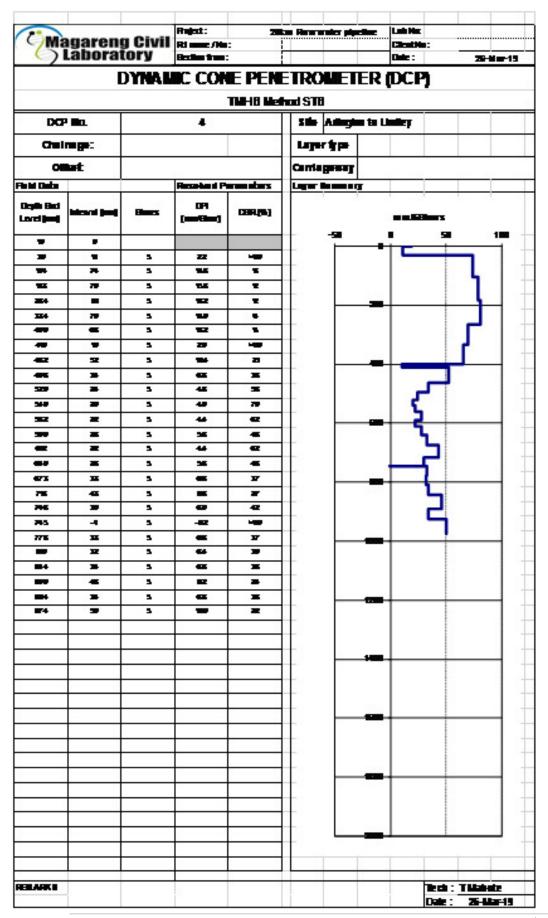




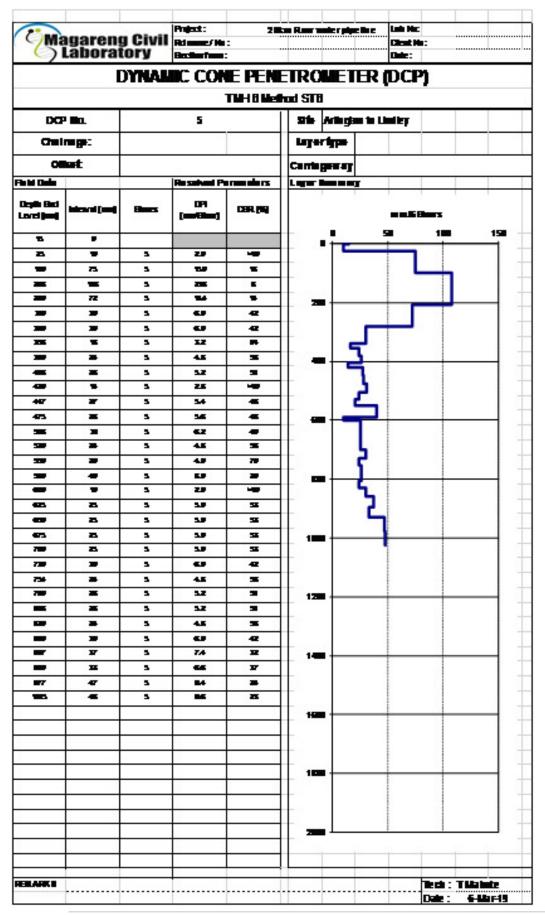














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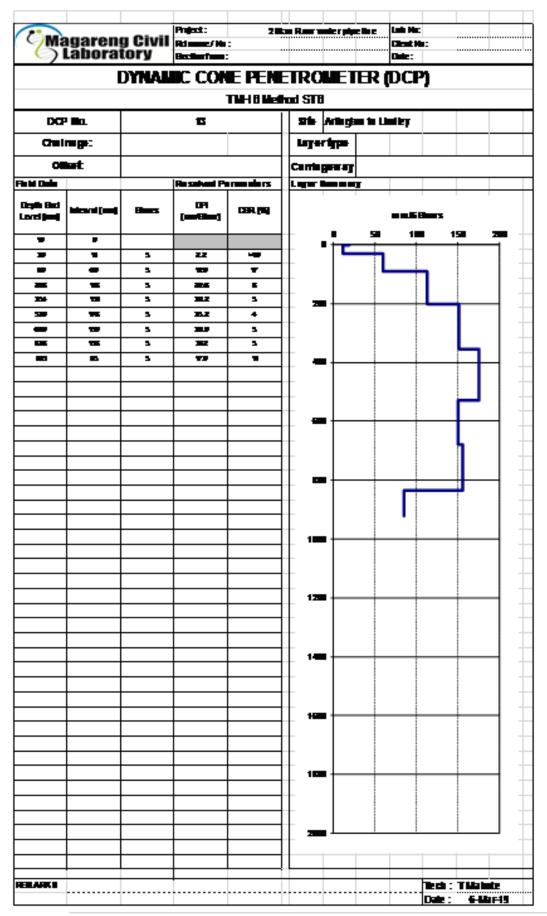


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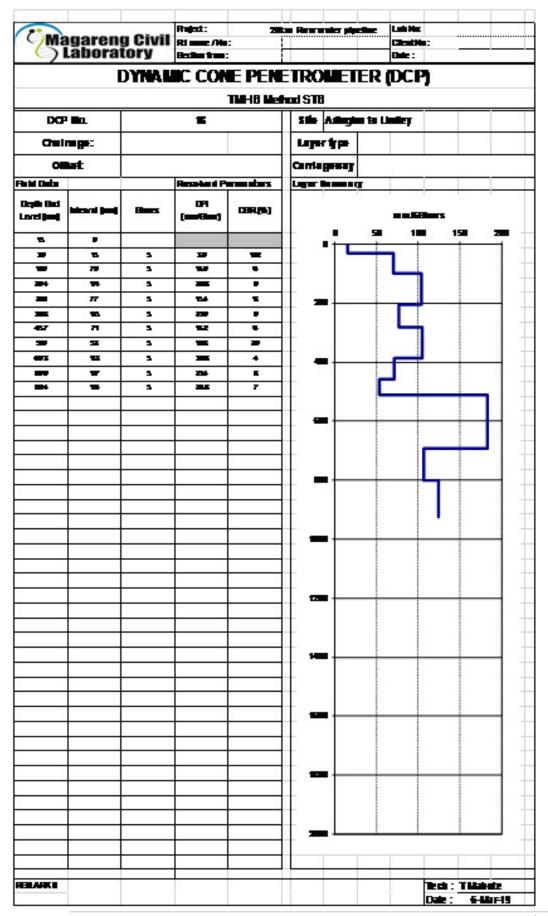


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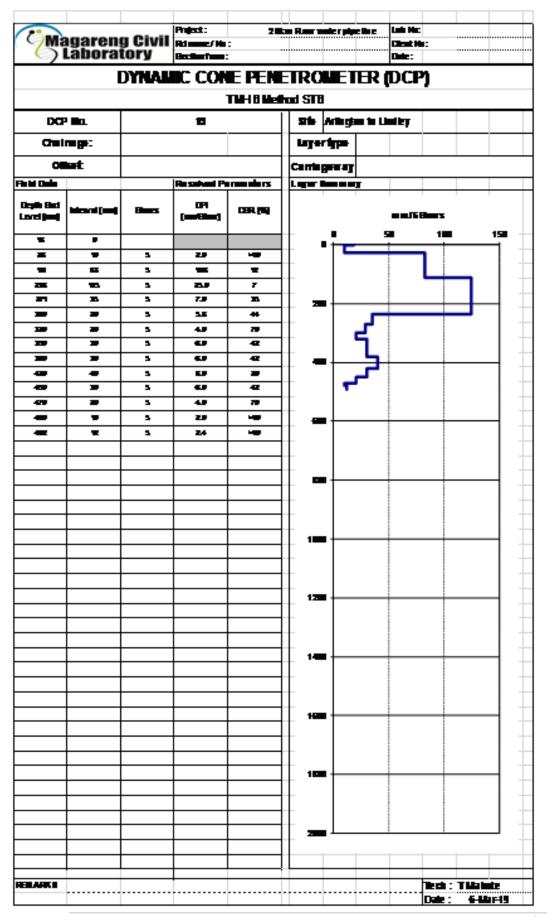


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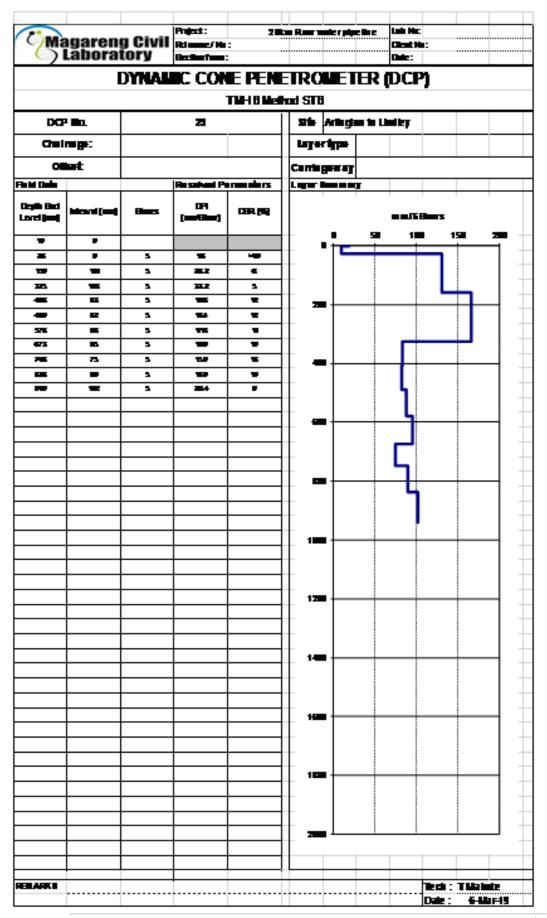


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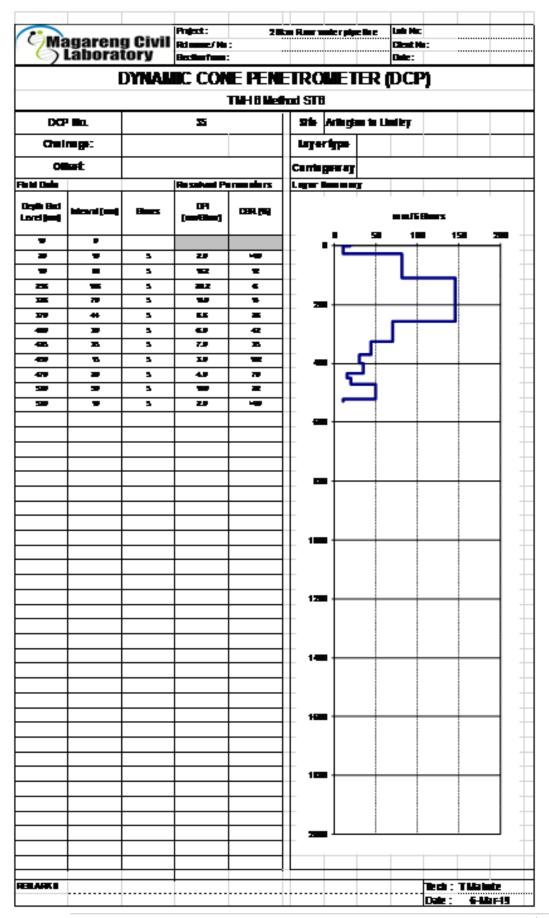


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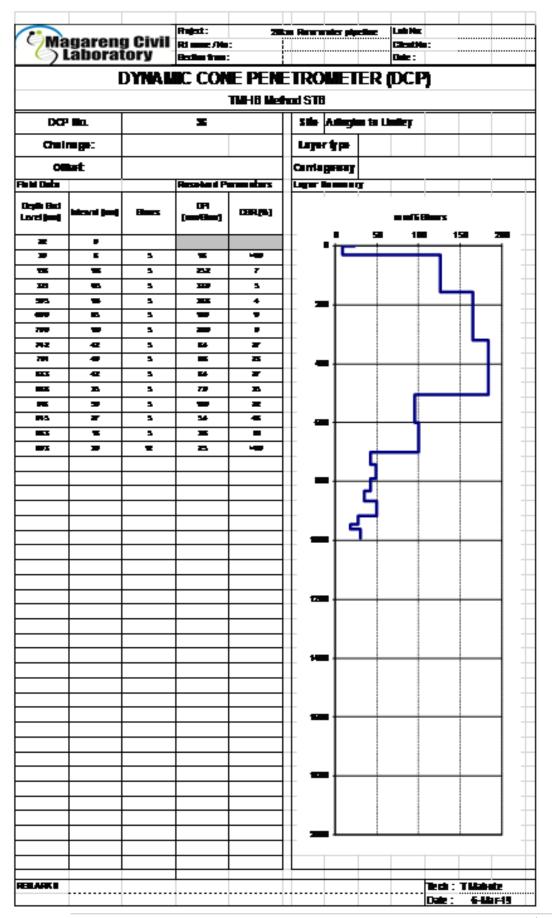


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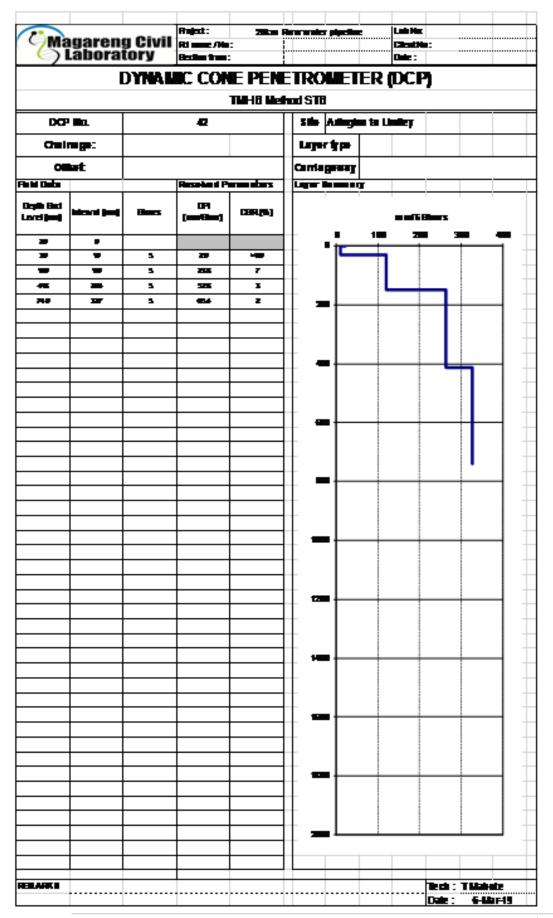


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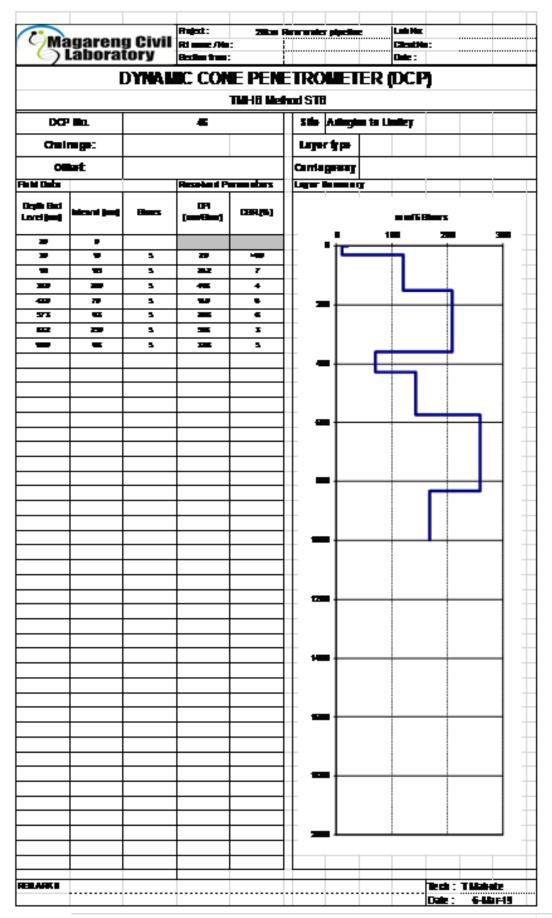


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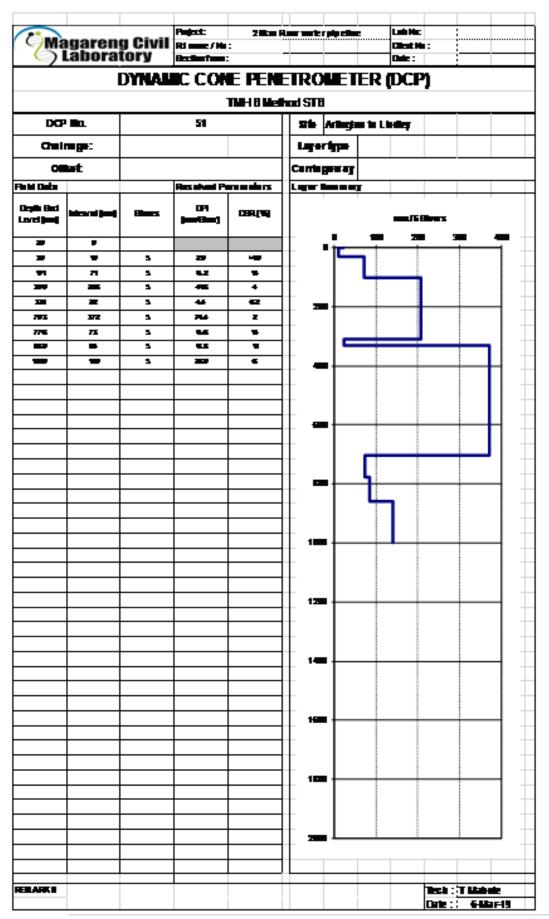


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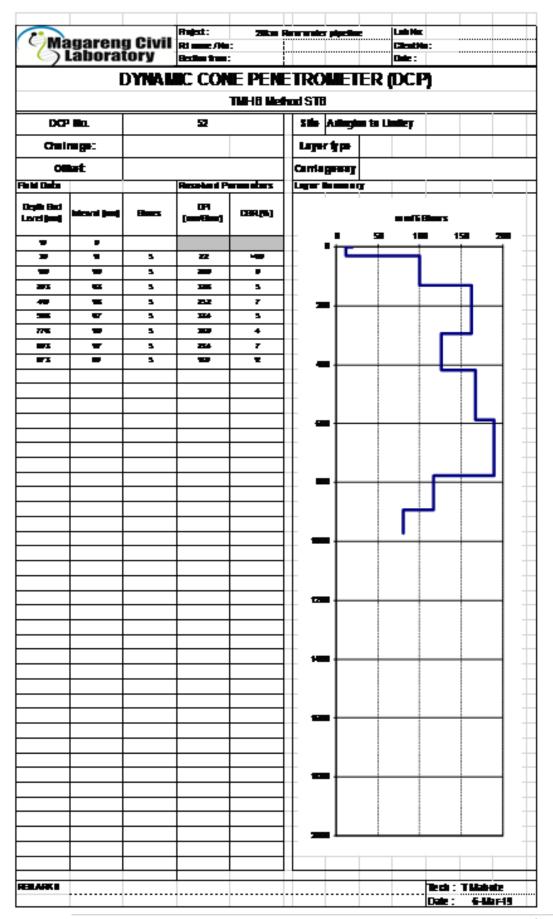


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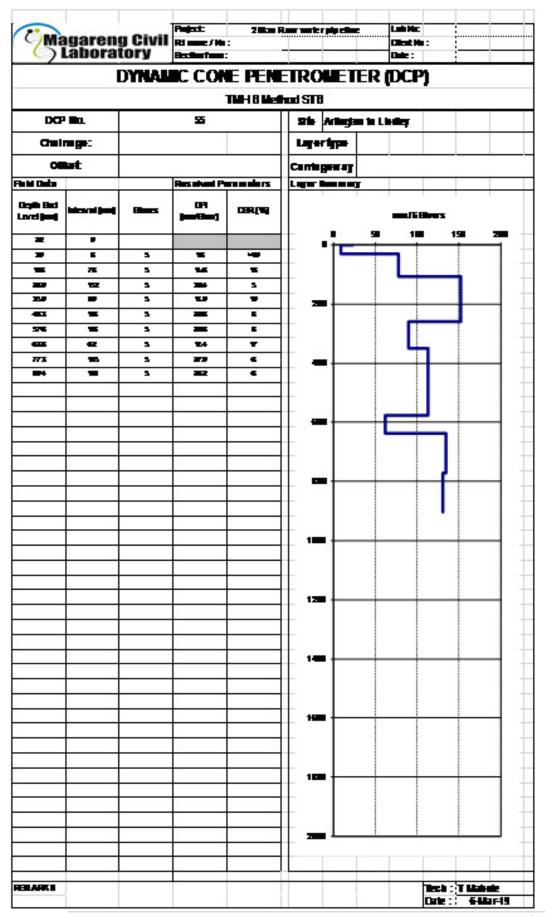
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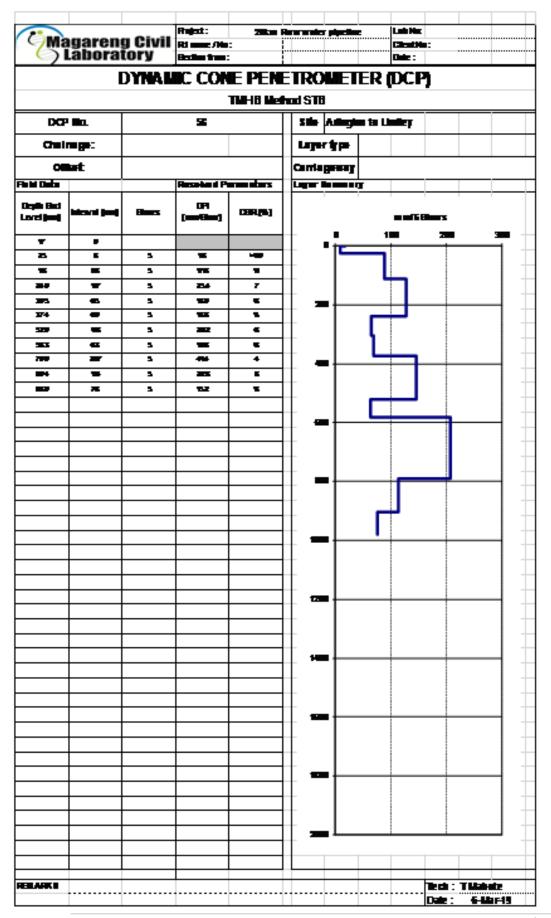


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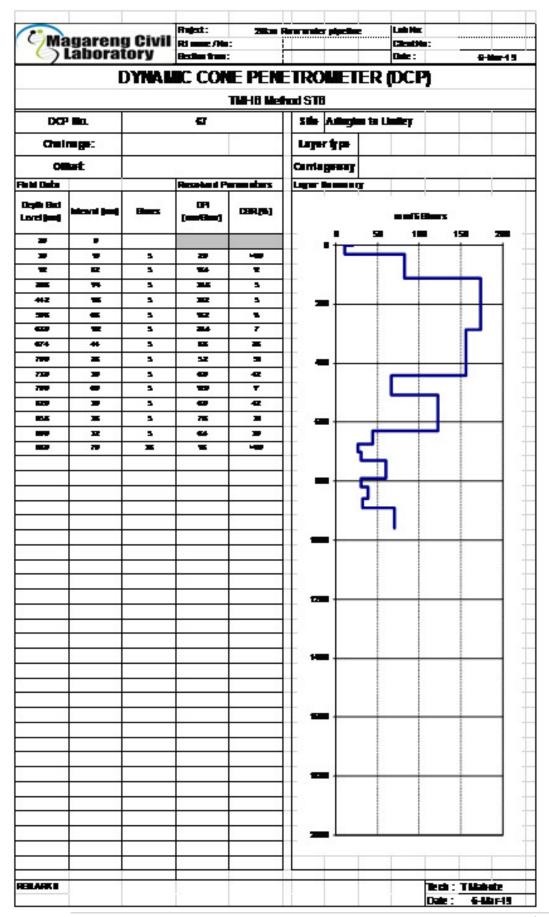
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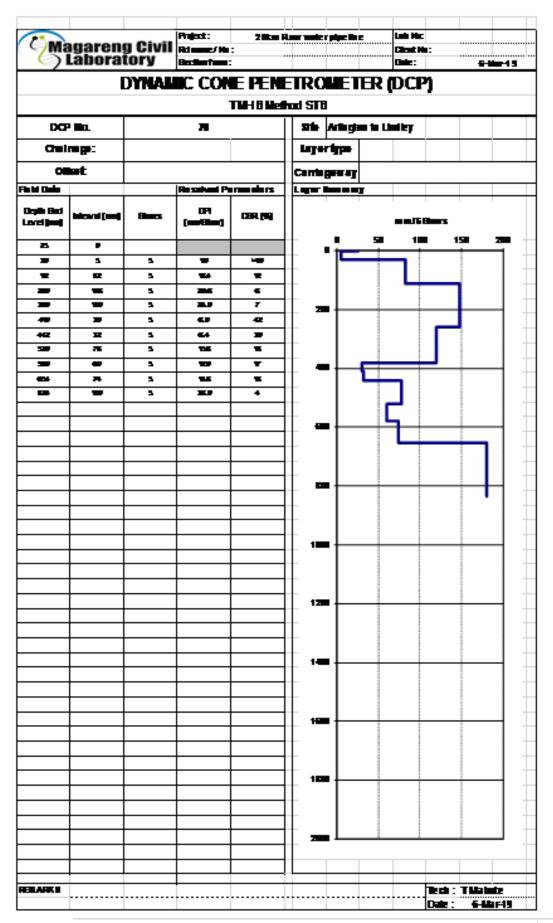


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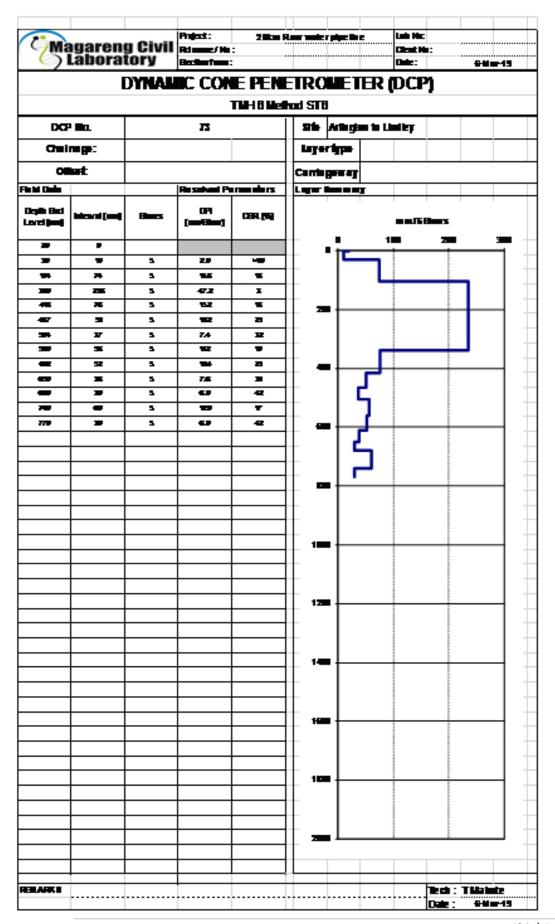


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Geotechnical Investigation Report: Alington to Lindley

