REPORT No 20744
TRAFFIC IMPACT ASSESSMENT

PROPOSED MIXED USE DEVELOPMENT
ON REMAINDER OF PORTION 12 OF
THE FARM WEMMERHUIS 379-JT
AND REMAINDER OF THE FARM
BERGENDAL 981-JT,
BELFAST

SEPTEMBER 2016

# TRAFFIC IMPACT ASSESSMENT <br> PROPOSED MIXED USE <br> DEVELOPMENT ON REMAINDER OF PORTION 12 OF THE FARM WEMMERHUIS 379-JT AND REMAINDER OF THE FARM BERGENDAL 981-JT, BELFAST 

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```
WSP | Parsons Brinckerhoff
314 Glenwood Road, Lynnwood Park, Pretoria,
South Africa }008
Postnet Suite 287, Private Bag X025, Lynnwood Ridge, 0040
Tel: +27 (0) 127621200
Fax: +27 (0) 127621301
www.wspgroup.com
www.pbworld.com
```

PARSONS BRINCKERHOFF

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| Prepared by | Catherine Bilankulu <br> Technical Assistant | Catherine Bilankulu <br> Technical Assistant |  |  |
| Signature | Extakule | Extan |  |  |
| Checked by | Eben Kotze PR Tech Eng | Eben Kotze <br> PR Tech Eng |  |  |
| Signature | sterce | frese |  |  |
| Authorised by | Eben Kotze <br> PR Tech Eng | Eben Kotze <br> PR Tech Eng |  |  |
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## Certification

It is herewith certified that this Traffic Impact Assessment has been prepared according to requirements of the South African Traffic Impact and Site Traffic Assessment Manual.

Signatory : $\qquad$ Date : 21 September 2016 ECSA no :2003 70133

```
WSP Contact Person
    Name: Eben Kotze
    Address: Postnet Suite 287, Private Bag
        X025, Lynnwood Ridge, 0040
    Telephone: 0127621200
    Cellphone : 0835641563
    Email : Eben.kotze@wspgroup.co.za
```

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PRODUCTION TEAM
WSP | PARSONS BRINCKERHOFF

| Function | Name |
| :--- | :--- |
| Technical Assistant | Catherine Bilankulu |

Function Name
Regional Director Pr. Tech Eng Eben Kotze

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## LIST OF ABBREVIATIONS

| COTO | Committee of Transport Officials |
| :--- | :--- |
| ELM | Emakhazeni Local Municipality |
| GLA | Gross Leasable Area |
| LOS | Level of Service |
| LVO | Low Vehicle Ownership |
| MDoPWR\&T | Mpumalanga Department of Public Works, Roads \& Transport |
| MUD | Mixed Use Development |
| NLTA | National Land Transport Act |
| SANRAL | South African National Roads Agency Limited |
| SATGR | South African Trip Generation Rates |
| SEC | Seconds |
| SIDRA | Micro-analytical traffic evaluation |
| TMH | Technical Methods for Highways |
| V/C | Volume/Capacity ratio |
| VLVO | Very Low Vehicle Ownership |
| vph | Vehicle per hour |

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## INTRODUCTION

## $1.1 \quad$ BACKGROUND

WSP Group Africa (Pty) Ltd. (WSP) has been appointed to undertake a Traffic Impact Assessment for the proposed mixed use development situated on Remainder of Portion 12 of the Farm Wemmershuis 379-JT and the Remainder of the Farm Bergendal 981-JT, Belfast located in Mpumalanga Province. The proposed site is located approximately 510 m south of the N4/R33 interchange. The site locality is illustrated on Figure 1.

A TIA was previously submitted in November 2015 for the proposed mixed-use development. This study revises the previously submitted study as the development will now be implemented in phases.

The purpose of this traffic impact assessment is to illustrate the proposed development's traffic impact on the surrounding road network and possible mitigation of the anticipated traffic impact. This report also comments on the proposed site accesses and non-motorised and public transport aspects.

### 1.2 EXTENT OF THE DEVELOPMENT

The extent of the proposed development as well as the land-uses are indicated in Table 1 below:
Table 1: Extent of the development

| LAND USE | PHASE 1 | PHASE 2 | PHASE 3 | PHASE 4 | TOTAL |
| :--- | :---: | :---: | :---: | :---: | :---: |
| Single Residential | 31 Stands | - | 97 Stands | 33 Stands | 161 Stands |
| High Residential | - | - | - | 387 Units | 387 Units |
| Agricultural | - | - | - | $522357 \mathrm{~m}^{2}$ |  |
| Institutional - Crèche | - | - | $\wedge 200$ Pupils | - | 200 Pupils |
| - Church | - | - | - | ${ }^{*} 12796 \mathrm{~m}^{2}$ | ${ }^{*} 12796 \mathrm{~m}^{2}$ |
| Retail (Shops) | $31830 \mathrm{~m}^{2}$ GLA | - | $1460 \mathrm{~m}^{2}$ GLA | - | $33290 \mathrm{~m}^{2} \mathrm{GLA}$ |
| Business Park | - | - | $8554 \mathrm{~m}^{2} \mathrm{GLA}$ | $33790 \mathrm{~m}^{2} \mathrm{GLA}$ | $42344 \mathrm{~m}^{2} \mathrm{GLA}$ |
| Industrial | - | $14252 \mathrm{~m}^{2} \mathrm{GLA}$ | - | - | $14252 \mathrm{~m}^{2} \mathrm{GLA}$ |
| Open Space | - | $7529 \mathrm{~m}^{2}$ | - | $105068 \mathrm{~m}^{2}$ | $112597 \mathrm{~m}^{2}$ |
| Utilities | - | - | - | $1915 \mathrm{~m}^{2}$ |  |

The following assumptions were made:
^ 200 Pupils for Institutional (Phase 3)

* Assumed to be internal trips

The township layout and phasing plan are contained in Appendix A-1.

### 1.3 APPROVAL OF SUBMISSION

This traffic impact assessment report will be subject to approval from the relevant roads authorities listed below:
$\rightarrow$ Emakhazeni Local Municipality (ELM)
$\rightarrow$ Mpumalanga Department of Public Works, Roads and Transport (MDoPWR\&T)
$\rightarrow$ South African National Roads Agency Limited (SANRAL)

## 2

## DATA COLLECTION AND LIAISONS

### 2.1 SITE VISIT

On 29 October 2015 a site visit was undertaken for the development and the following was confirmed:
$\rightarrow$ Layouts of intersections considered in the study
$\rightarrow$ Appropriateness of recommended site access
$\rightarrow$ Intersection control for relevant intersections
$\rightarrow$ Presence of existing public transport and non-motorised facilities
A set of photos is included in Appendix A-2 depicting the intersections observed during the site visit.

### 2.2 LIAISON WITH THE AUTHORITIES

SANRAL
A meeting was held with SANRAL Messr. Mr. Izak van der Linde and it was requested that the Applicant should consider providing an additional access to the north of the application site to alleviate the impact of the proposed development on the N4 interchange. A limited road master plan was undertaken by WSP and is included in Appendix A-3.

## MPUMALANGA DEPARTMENT OF PUBLIC WORKS, ROAD AND TRANSPORT

A discussion was held with MDoPW\&R, Messr. Ben Viljoen and it was indicated that the proposed traffic circle at the access on Road D1477 is not supported therefore an alternative access layout was investigated in this study.

## SURROUNDING ROAD NETWORK \&

 STUDY AREA
### 3.1 SURROUNDING ROAD NETWORK

The following roads in the vicinity of the proposed development are regarded as relevant to this study and are discussed in detailed below:
$\rightarrow$ N4 Freeway: This is a Class 1 road located to the north of the site; this road follows an eastwest alignment and it abuts the application site to the north.
$\rightarrow$ Road D1477: This is a Class 2/3 road located to the west of the site; this road follows a northsouth alignment with one lane per direction. The proposed development will gain access directly from this road
$\rightarrow$ R33: This is a Class 3 road located to the west of the site; this road follows an east-west alignment with one lane per direction.
$\rightarrow$ New Link Road: As requested by SANRAL a limited road master plan was undertaken to provide an additional access to the north of the proposed development link road with Road D1477. The proposed road link (over/under the N4) will be a Class 3 road with one lane per direction. The link road is required to prevent congestion of the N4/Road D1477 interchange.

### 3.2 DETERMINATION OF THE STUDY AREA

In determining the site area TMH 16 volume 1 recommends the following:
$\rightarrow$ "Class 4 and 5 roads in the vicinity of the development up to the first Class 1 to 3 roads that can be reached by the Class 4 and 5 road network from the development, up to and including the first connection(s) on the Class 1 to 3 roads.
$\rightarrow$ The elements shall be restricted to those within a maximum distance of 1.5 km from the accesses to the site, measured along the shortest routes to the accesses, provided that there is at least one intersection within this distance. Where there is no such intersection, the distance will be extended to include at least one intersection."

TMH 16 also states that judgement should be used in selecting the intersections considered and therefore specific elements like extent of the development were also considered. A larger development will by its nature require a wider study area to be considered while for a smaller development the opposite will be true. It was decided that the following key intersections as mentioned below (refer to Figure 1) would be sufficient for analyses:
$\rightarrow$ Road D1477 and N4 Off-ramp;
$\rightarrow$ Road D1477 and N4 Off-ramp/Belfast 1Stop Access and
$\rightarrow$ Road D1477 and R33/Site access
$4.1 \quad$ SITE ACCESS
The following accesses are proposed for the development:
$\rightarrow$ Access 1 : A full access is proposed on Road D1477 directly opposite the existing R33 Road. The access is situated approximately 350 m north of the southern terminal of the N4/Road D1477 interchange. This access will be implemented in Phase 1 of the proposed development.
$\rightarrow$ Access 2 : A secondary full access is proposed on Road D1477. The access is situated approximately 290 m south of the proposed Access 1 . This access will be implemented in Phase 2 of the proposed development.
$\rightarrow$ Access 3 : A full access is proposed on the proposed link road to the north of the proposed development. This access will be implemented in Phase 3 and 4 of the proposed development.

The proposed accesses are shown conceptually on Drawings SKC 003 Rev B and SKC 004 Rev A.

The secondary access position was discussed and approved in principle by MDoPWR\&T Messr. Ben Viljoen.

### 4.2 PARKING

The South African Parking Standards was considered for the parking requirement. Table 2 shows the required parking.

Table 2: Parking requirements

| LAND USE | EXTENT | PARKING REQUIRED | REQUIREMENTS FOR LOADING | NO. BAYS REQUIRED |
| :---: | :---: | :---: | :---: | :---: |
| Industry | $14252 \mathrm{~m}^{2}$ GLA | 1 bay / $100 \mathrm{~m}^{2} \& 3$ bays / $100 \mathrm{~m}^{2}$ office floor area | 1 bay / first $1000 \mathrm{~m}^{2}$ floor area or part thereof \& 1 bay / every $1000 \mathrm{~m}^{2}$ floor area thereafter | To be confirmed during SDP submission stage |
| Dwelling House | 161 Stands | 1 bay / on-site unit | Not Applicable | 161 |
| Dwelling Units | 387 Units | 1 covered bay / Dwelling unit \& 1 uncovered bay / 2 dwelling units | 1 bay / 10 dwelling units | 622 |
| Crèche | 200 Pupils | 1 bay / 4 children | 1 bay / first $2000 \mathrm{~m}^{2}$ floor area or part thereof \& 1 bay / every $2000 \mathrm{~m}^{2}$ floor area thereafter | To be confirmed during SDP submission stage |
| Shopping Centre | 33 290m² GLA | 6 bays / 100m ${ }^{2}$ | 1 bay / first 2000m² floor area or part thereof \& 1 bay / every $2000 \mathrm{~m}^{2}$ floor area thereafter | To be confirmed during SDP submission stage |
| Business Centre | Parking ratio to | be agreed with ELM during | SDP submission stage |  |

Since the site development plan (SDP) was not yet finalized during the writing of this report, it is recommended that a parking study (Site Traffic Assessment) be undertaken during the SDP submission stage.

## 5 EXISTING TRAFFIC VOLUMES

### 5.1 GENERAL

Traffic counts were used to estimate the traffic demand and traffic volume for the development. A traffic count was commissioned on Friday 11 September 2015 at the following intersections:

```
-> Road D1477 and N4 Off-ramp;
Road D1477 and N4 Off-ramp/Belfast 1Stop Access and
-> Road D1477 and R33/Site access
```

The counted intersections are indicated on Figure 1.
From the traffic count a common peak hour was determined (the busiest hour) for each counted period and was found to be:

```
F Friday AM peak hour
08:00-09:00
-> Friday PM peak hour
16:00-17:00
```

The existing 2015 Peak Hour Traffic Volumes are shown on Figure 2. The following subheadings provide a brief overview of the existing intersections.

### 5.2 ROAD D1477 / N4 OFF-RAMP

This intersection is currently a priority side stop with the Road D1477 being the major road. Traffic counts have revealed that Road D1477 has in the order of 590vph and 970vph during the AM and PM peak hours respectively (in both directions). The intersection currently operates at LOS A or B for the southern approach, LOS A for the northern approach and LOS A and LOS C for the western approach.

### 5.3 ROAD D1477 / N4 OFF-RAMP / BELFAST 1STOP ACCESS

This intersection is currently a priority side stop with Road D1477 being the major road. Traffic counts have revealed that the N4 Off-ramp has in the order of 240 vph and 285 vph during the AM and PM peak hours respectively (in both directions). The intersection currently operates at LOS A for the north and south approaches during both the AM and PM peak hours. The east approach operates at LOS C and the west approach at LOS B during the AM and PM peak hours.

### 5.4 ROAD D1477 / R33 / SITE ACCESS

This intersection is currently an all-way stop. Traffic counts have revealed that R33 has in the order of 140 vph and 125 vph during the AM and PM peak hours respectively (in both directions). The intersection currently operates at LOS C for the north, south and east approaches during both the AM and PM peak hours. However the west approach operates at LOS D during the AM and PM peak hours.

### 5.5 ROAD D1477 / SITE ACCESS 2

This is a new intersection and it will be all-way stop controlled. Traffic counts have revealed that currently 10 vph and 13 vph pass the site during the AM and PM peak hours respectively.

## DEVELOPMENT TRIP GENERATION AND TRAFFIC VOLUMES SCENARIOS

### 6.1 TRIP GENERATION

The South African Trip Data Manual - COTO - (TMH17) was used to estimate the trip generation for the proposed development. Trip rates for the various land-uses are summarized below:

INDUSTRIAL
The recommended trip generation according to the COTO document for Industrial is:

```
-> Weekday AM Peak period 0.80 trips per 100m2 GLA
-> Weekday PM Peak period 0.80 trips per 100m2 GLA
```


## SINGLE DWELLING UNITS

The recommended trip generation according to the COTO document for Single Dwelling Units is:

```
-> Weekday AM Peak period }1.00\mathrm{ trip per Dwelling Unit
-> Weekday PM Peak period 1.00 trip per Dwelling Unit
```


## APARTMENTS AND FLATS

The recommended trip generation according to the COTO document for Apartments and Flats is:

```
-> Weekday AM Peak period 0.65 trips per Dwelling Unit
-> Weekday PM Peak period 0.65 trips per Dwelling Unit
```

PRE-SCHOOL

The recommended trip generation according to the COTO document for Pre-school is:

```
-> Weekday AM Peak period 1.00 trip per Pupil
-> Weekday PM Peak period 0.80 trips per Pupil
```


## SHOPPING CENTRE

The recommended trip generation according to the COTO document for Shopping Centre is:
$\rightarrow$ Weekday AM Peak period $\quad 0.60$ trips per $100 \mathrm{~m}^{2}$ GLA
$\rightarrow$ Friday PM Peak period $\quad 3.40$ trips per $100 \mathrm{~m}^{2}$ GLA
$\rightarrow$ Saturday Peak period $\quad 4.50$ trips per $100 \mathrm{~m}^{2}$ GLA
The size adjustment factor for Shopping Centre as per COTO TMH17 has been applied. The trips for the shopping centre were calculated by first adjusting the Gross Leasable Area (GLA) of the site, i.e. the size adjustment factor $(S)=1+A /(1+s q m$ size/B). Factor $A$ and $B$ are parameters provided in the trip rate table.

## BUSINESS CENTRE

The proposed development will also consist of small pockets of business/commercial related uses. Therefore a trip rate for Business Centre which includes a range of mixed land-uses, such as offices, banking facilities, light industrial and warehousing was considered realistic. The recommended trip generation according to the COTO document for Business Centre is:

```
-> Weekday AM Peak period
1.50 trips per 100m2 GLA
-> Weekday PM Peak period
1.50 trips per 100m2 GLA
```


### 6.2 ADJUSTMENT FACTORS

Various trip adjustment factors have been introduced into the COTO document to allow for trip reductions. These adjustment factors are discussed briefly below.

## MIXED USE DEVELOPMENT (MUD)

According to the COTO manual "mixed use development is defined as development in an area that consist of two or more single-use development between which trips can be made by means of nonmotorised modes of transport (such as walking). This has the net effect of reducing the vehicle trip generation in the area."

This study considered mixed use development reduction factor. The reduction factor which has been applied is listed in Table $\mathbf{3}$ below. Note $\mathbf{P}_{\mathbf{M}}=$ Reduction factor for mixed use development.

LOW VEHICLE OWNERSHIP (LVO) \& VERY LOW VEHICLE OWNERSHIP (VLVO)

According to COTO "the vehicle ownership in areas with high levels of vehicle ownership varies between one or two per household. In areas with a low level of vehicle ownership, the majority of households (more than $50 \%$ ) does not own a vehicle and relies on public transport for transportation. In areas with very low level of vehicle ownership, nearly all households (more than $90 \%$ ) do not own a vehicle and rely on public transportation."

This study considered low vehicle ownership and the reduction factor which have been applied is listed in Table 3 below. Note $\mathbf{P}_{\mathbf{v}}=$ Reduction factor for vehicle ownership.

## TRANSIT NODE OR CORRIDORS

According to COTO "the transit reduction factors are applicable to developments that are located within a reasonable walking distance from a major transit node or stops on a major transit corridor."

The reduction factors recommended for transit node or corridors in the manual were not applied.
Table 3: Adjustment Factors Applied for Trip Reductions

| LAND USE | $\mathbf{P M}_{\mathbf{M}}$ | $\mathbf{P v}_{\mathbf{v}}$ |
| :--- | :---: | :---: |
| Industrial | $5 \%$ | $20 \%$ |
| Single Dwelling Units | $10 \%$ | $40 \%$ |
| Apartments \& Flats | $15 \%$ | $30 \%$ |
| Pre-School | $5 \%$ | $50 \%$ |
| Business Centre | $15 \%$ | $20 \%$ |
| Shopping Centre | $10 \%$ | $30 \%$ |

### 6.3 TRIP SUMMARY

The detailed trip generation calculations are included in Appendix A-4. Using the COTO document the expected peak hour development trips generation was calculated and indicated in Tables 4 to 8 for the different Phases.

Table 4: Phase 1 - Development Generated Trips

| LAND USE | EXTENT | AM PEAK |  |  | FRIDAY PM PEAK |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | In | Out | Total | In | Out | Total |
| Single Dwelling Units | 31 Units | 4 | 13 | 17 | 12 | 5 | 17 |
| Shopping Centre |  | PRIMARY TRIPS |  |  |  |  |  |
|  |  | 125 | 67 | 192 | 315 | 315 | 630 |
|  |  | PASS-BY TRIPS |  |  |  |  |  |
|  |  | - | - | - | 96 | 96 | 193 |
|  |  | DIVERTED TRIPS |  |  |  |  |  |
|  |  | - | - | - | 99 | 99 | 198 |
| Total (Shopping Centre) | $31830 \mathrm{~m}^{2} \mathrm{GLA}$ | 125 | 67 | 192 | 510 | 510 | 1021 |
| Total Phase 1 Trips |  | 129 | 80 | 209 | 522 | 515 | 1038 |

From Table 4 it can be seen that Phase 1 of the proposed development will generate a maximum of 209 peak hour trips in the AM peak hour and a maximum of 1038 peak hour trips in the PM peak hour.

Table 5: Phase 2 - Development Generated Trips

| LAND USE | EXTENT | AM PEAK |  |  | FRIDAY PM PEAK |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | In | Out | Total | In | Out | Total |
| Industrial Area (Park) | 14 252m² GLA | 65 | 26 | 91 | 22 | 65 | 87 |
| Total Phase 2 Trips |  | 65 | 26 | 91 | 22 | 65 | 87 |

From Table 5 it can be seen that Phase 2 of the proposed development will generate a maximum of 91 peak hour trips in the AM peak hour and a maximum of 87 peak hour trips in the PM peak hour.

Table 6: Phase 3 - Development Generated Trips

| LAND USE | EXTENT | AM PEAK |  |  | FRIDAY PM PEAK |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | In | Out | Total | In | Out | Total |
| Single Dwelling Units | 97 Units | 13 | 39 | 52 | 37 | 16 | 52 |
| Pre-school (Day Care Centre) | 200 Students | 48 | 48 | 95 | 38 | 38 | 76 |
| Business Centre (Park) | $8554 \mathrm{~m}^{2} \mathrm{GLA}$ | 74 | 13 | 87 | 17 | 70 | 87 |
| Shopping Centre |  | PRIMARY TRIPS |  |  |  |  |  |
|  |  | 19 | 10 | 29 | 47 | 47 | 95 |
|  |  | PASS-BY TRIPS |  |  |  |  |  |
|  |  | - | - | - | 6 | 6 | 12 |
|  |  | DIVERTED TRIPS |  |  |  |  |  |
|  |  | - | - | - | 5 | 5 | 9 |
| Total (Shopping Centre) | $1460 \mathrm{~m}^{2} \mathrm{GLA}$ | 19 | 10 | 29 | 58 | 58 | 116 |
| Total Phase 3 Trips |  | 154 | 110 | 264 | 150 | 182 | 332 |

From Table 6 it can be seen that Phase 3 of the proposed development will generate a maximum of 264 peak hour trips in the AM peak hour and a maximum of 332 peak hour trips in the PM peak hour.

The trips for Institutional (Church) in Phase 3 and 4 was not considered in the calculations of the trip generation as it was assumed that these will be internal trips serving the community within the proposed development and will not affect the external road network.

Table 7: Phase 4 - Development Generated Trips

| LAND USE | EXTENT | AM PEAK |  |  | FRIDAY PM PEAK |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | In | Out | Total | In | Out | Total |
| Single Dwelling Units | 33 Units | 4 | 13 | 18 | 12 | 5 | 18 |
| Apartments and Flats | 387 Units | 37 | 112 | 150 | 105 | 45 | 150 |
| Business Centre (Park) | $33790 \mathrm{~m}^{2} \mathrm{GLA}$ | 293 | 52 | 345 | 69 | 276 | 345 |
| Total Phase 4 Trips |  | 335 | 177 | 512 | 186 | 326 | 512 |

From Table 7 it can be seen that Phase 4 of the proposed development will generate a maximum of 512 peak hour trips in the AM peak hour and the PM peak hour.

Table 8: Total Development Generated Trips (All Phases)

| LAND USE | TYPE OF TRIPS | AM PEAK |  |  | FRIDAY PM PEAK |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | In | Out | Total | In | Out | Total |
| Industrial | Primary Trips | 65 | 26 | 91 | 22 | 65 | 87 |
| Single Dwelling Units | Primary Trips | 22 | 65 | 87 | 61 | 26 | 87 |
| Apartments \& Flats | Primary Trips | 37 | 112 | 150 | 105 | 45 | 150 |
| Crèche | Primary Trips | 48 | 48 | 95 | 38 | 38 | 76 |
| Shopping Centre | Primary Trips | 144 | 77 | 221 | 362 | 362 | 724 |
|  | Pass-by Trips | 0 | 0 | 0 | 102 | 102 | 204 |
|  | Diverted Trips | 0 | 0 | 0 | 104 | 104 | 208 |
| Business Centre | Primary Trips | 367 | 65 | 432 | 86 | 345 | 432 |
| Total (All Phases) |  | 683 | 393 | 1076 | 880 | 1087 | 1968 |

From Table 8 it can be seen that the proposed development (All Phases) will generate a maximum of 1076 trips in the Friday AM peak hour and a maximum of 1968 trips during the Friday PM peak hour.

The TMH 16 Volume 1 requires that a traffic impact assessment be done for development which generates more than 50 peak hour trips.

### 6.4 GROWTH RATE

TMH 16 Volume 1 requires that a five year horizon be considered for development that generates more than 50 trips. TMH 17 recommends growth rates for development as shown in Table 9.

Table 9: Typical Traffic Growth Rates

| DEVELOPMENT AREA | GROWTH RATES |
| :--- | :--- |
| Low growth areas | $0-3 \%$ |
| Average growth areas | $3-4 \%$ |
| Above average growth areas | $4-6 \%$ |
| Fast growing areas | $6-8 \%$ |
| Exceptionally high growth areas | $>8 \%$ |
| Source: City Council of Pretoria (1998) |  |

A growth rate of $3 \%$ was considered appropriate for this study.

### 6.5 TRAFFIC VOLUME SCENARIOS

The existing 2015 peak hour traffic volumes (see Figure 2) were thus subjected to a $3 \%$ growth rate over five years; this is in line with an average growth rate as given in Table 9. The 2020 background peak hour traffic volumes are shown on Figure 3.

## $6.6 \quad$ TRIP DISTRIBUTION AND ASSIGNMENT

Assumptions with respect to the expected trip distribution were based on the location of the site access in relation to the surrounding road network; the existing traffic volumes, travel patterns as well as the land use nature of the proposed development.

The total development trips for Phase 1, 2, 3 and 4 are shown on Figures 4A, 4B, 4C and 4D respectively. Ultimately the expected 2020 peak hour traffic volumes plus the total development trips for Phase 1, 2, 3 and 4 are shown on Figures 5A, 5B, 5C and 5D respectively.

## TRAFFIC IMPACT AND CAPACITY ANALYSIS

### 7.1 GENERAL

In order to determine the traffic impact the proposed development will have on the surrounding road network, SIDRA traffic engineering software was used. A capacity analysis was done for the key intersections identified in Section 3.2.

The AM and PM peak hour trip generation of the development was analysed.

### 7.2 ANALYSED SCENARIOS

The different traffic scenarios considered for analysis are tabulated in Table 4.
Table 10: Traffic Scenarios Considered

| Traffic Scenario | Description |
| :--- | :--- |
| Scenario 1 | 2020 Peak Hour Background Traffic Volumes (Figure 3) <br> With Existing Intersection Layouts |
| Scenario 2 | 2020 Peak Hour Background Traffic Plus Phase 1 Development Traffic <br> (Figure 5A) With Development Upgrades if Applicable |
| Scenario 3 | 2020 Peak Hour Background Traffic Plus Phase 1 \& 2 Development Traffic <br> (Figure 5B) With Development Upgrades if Applicable |
| Scenario 4 | 2020 Peak Hour Background Traffic Plus Phase 1, 2 \& 3 Development <br> Traffic (Figure 5C) With Development Upgrades if Applicable |
|  | 2020 Peak Hour Background Traffic Plus Phase 1, 2, 3 \& 4 Development <br> Traffic (Figure 5D) With Development Upgrades if Applicable |

### 7.3 CAPACITY ANALYSIS

The existing and proposed geometric layout and traffic control of each key intersection is indicated in Appendix A-5.

The results of the SIDRA capacity analysis for the key intersections are summarised and discussed in Tables 10 to 13. Detailed SIDRA outputs are enclosed as Appendix A-6.

Table 11: Road D1477 and N4 Off-ramp

| Road D1477 \& N4 OfF-Ramp |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | TRAFFIC SCENARIO \& LAYOUT | OVERALL INTERSECTION OPERATING CONDITIONS |  |  |  |  |  |  |  |  |
|  |  |  | AM PEAK HOUR |  |  |  | PM PEAK HOUR |  |  |  |
|  |  |  | LOS | v/c | Delay(s) | COMMENTS | LOS | v/c | Delay(s) | COMMENTS |
| 1 | 2020 Peak Hour Background Traffic (With existing intersection layout) | South | NA | 0.10 | 1.0 | Acceptable operating conditions expected. | NA | 0.13 | 1.7 | Acceptable operating conditions expected. |
|  |  | East | NA | 0.00 | 0.0 |  | NA | 0.00 | 0.0 |  |
|  |  | North | NA | 0.17 | 1.4 |  | NA | 0.18 | 2.1 |  |
|  |  | West | B | 0.12 | 10.7 |  | B | 0.29 | 10.1 |  |
|  |  | OVERALL | NA | 0.17 | 3.7 |  | NA | 0.29 | 5.6 |  |
| 2 | 2020 Peak Hour | South | A | 0.19 | 4.4 | Acceptable operating conditions | A | 0.45 | 7.3 | Acceptable operating conditions |
|  | Background Traffic + Phase 1 Development | East | NA | 0.00 | 0.0 | expected. Proposed upgrades are summarized in Chapter 7.4. | NA | 0.00 | 0.0 | expected. Proposed upgrades are summarized in Chapter 7.4. |
|  | Traffic | North | A | 0.32 | 4.7 |  | A | 0.54 | 6.6 |  |
|  | (With development upgrades) | West | B | 0.32 | 15.7 |  | B | 0.53 | 13.5 |  |
|  |  | OVERALL | A | 0.32 | 7.1 |  | A | 0.54 | 8.9 |  |
| 3 | 2020 Peak Hour | South | A | 0.21 | 5.0 | Acceptable operating conditions | A | 0.49 | 7.5 | Acceptable operating conditions |
|  | Background Traffic + Phase $1 \& 2$ | East | NA | 0.00 | 0.0 |  | NA | 0.00 | 0.0 |  |
|  | Development Traffic | North | A | 0.37 | 5.2 |  | A | 0.55 | 6.7 |  |
|  |  | West | B | 0.32 | 16.3 |  | B | 0.55 | 13.7 |  |
|  |  | OVERALL | A | 0.37 | 7.5 |  | A | 0.55 | 9.0 |  |
| 4 | 2020 Peak Hour | South | A | 0.25 | 5.2 | Acceptable operating conditions | A | 0.56 | 8.1 | Acceptable operating conditions |
|  | Background Traffic + Phase 1, 2 \& 3 | East | NA | 0.00 | 0.0 | expected. | NA | 0.00 | 0.0 | expected. |
|  |  | North | A | 0.42 | 5.3 |  | A | 0.61 | 6.9 |  |
|  |  | West | B | 0.38 | 17.4 |  | B | 0.59 | 14.2 |  |
|  |  | OVERALL | A | 0.42 | 7.7 |  | A | 0.61 | 9.3 |  |


| 5 | 2020 Peak Hour Background Traffic + Phase 1, 2, 3 \& 4 Development Traffic | South | A | 0.32 | 5.7 | Acceptable operating conditions expected. | A | 0.69 | 9.4 | Acceptable operating conditions expected. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | East | NA | 0.00 | 0.0 |  | NA | 0.00 | 0.0 |  |
|  |  | North | A | 0.55 | 5.7 |  | A | 0.68 | 7.3 |  |
|  |  | West | B | 0.52 | 19.7 |  | B | 0.66 | 14.9 |  |
|  |  | OVERALL | A | 0.55 | 8.3 |  | A | 0.69 | 10.0 |  |

Note: NA = Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements

The conceptual intersection geometric layout for the above intersection is shown on Drawing SKC 001 Rev A.

Table 12: Road D1477 and N4 Off-ramp/Belfast One Stop Access

| Road D1477 \& N4 Off-Ramp/Belfast One Stop Access |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | TRAFFIC SCENARIO \& LAYOUT | OVERALL INTERSECTION OPERATING CONDITIONS |  |  |  |  |  |  |  |  |
|  |  | $\begin{aligned} & \frac{1}{0} \\ & \frac{\pi}{0} \\ & \frac{2}{4} \end{aligned}$ | AM PEAK HOUR |  |  |  | PM PEAK HOUR |  |  |  |
|  |  |  | LOS | v/c | Delay(s) | COMMENTS | LOS | v/c | Delay(s) | COMMENTS |
| 1 | 2020 Peak Hour Background Traffic (With existing intersection layout) | South | NA | 0.05 | 2.1 | Acceptable operating conditions expected. | NA | 0.06 | 2.3 | Acceptable operating conditions expected. |
|  |  | East | C | 0.29 | 20.5 |  | NA | 0.47 | 24.6 |  |
|  |  | North | NA | 0.17 | 6.5 |  | NA | 0.18 | 7.2 |  |
|  |  | West | B | 0.08 | 12.5 |  | B | 0.11 | 12.8 |  |
|  |  | OVERALL | NA | 0.29 | 9.2 |  | NA | 0.47 | 11.5 |  |
| 2 | 2020 Peak Hour <br> Background Traffic + Phase 1 Development Traffic (With development upgrades) | South | A | 0.16 | 7.3 | Acceptable operating conditions expected. Proposed upgrades are summarized in Chapter 7.4. | B | 0.49 | 10.1 | Acceptable operating conditions expected. Proposed upgrades are summarized in Chapter 7.4. |
|  |  | East | C | 0.37 | 27.5 |  | C | 0.54 | 27.4 |  |
|  |  | North | B | 0.38 | 11.6 |  | B | 0.56 | 13.7 |  |
|  |  | West | C | 0.26 | 30.7 |  | C | 0.45 | 30.3 |  |
|  |  | OVERALL | B | 0.38 | 14.9 |  | B | 0.56 | 15.6 |  |
| 3 | 2020 Peak Hour <br> Background Traffic + Phase 1\&2 Development Traffic | South | A | 0.18 | 7.4 | Acceptable operating conditions expected. | A | 0.53 | 9.8 | Acceptable operating conditions expected. |
|  |  | East | C | 0.37 | 27.5 |  | C | 0.58 | 28.7 |  |
|  |  | North | B | 0.39 | 11.1 |  | B | 0.60 | 13.5 |  |
|  |  | West | C | 0.29 | 30.9 |  | C | 0.50 | 31.5 |  |
|  |  | OVERALL | B | 0.39 | 14.4 |  | B | 0.60 | 15.4 |  |
| 4 | 2020 Peak Hour <br> Background Traffic + Phase 1, 2 \& 3 Development Traffic | South | A | 0.23 | 7.1 | Acceptable operating conditions expected. | A | 0.61 | 9.7 | Acceptable operating conditions expected. |
|  |  | East | C | 0.41 | 28.6 |  | C | 0.63 | 30.5 |  |
|  |  | North | B | 0.40 | 10.4 |  | B | 0.70 | 13.9 |  |
|  |  | West | C | 0.36 | 32.3 |  | C | 0.59 | 33.4 |  |
|  |  | OVERALL | B | 0.41 | 13.6 |  | B | 0.70 | 15.6 |  |


| South | A | 0.31 | 6.9 | Acceptable operating conditions expected. Proposed upgrades are summarized in Chapter 7.4. | A | 0.72 | 9.0 | Acceptable operating conditions expected. Proposed upgrades are summarized in Chapter 7.4. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| East | C | 0.45 | 29.8 |  | D | 0.90 | 45.4 |  |
| North | A | 0.48 | 9.9 |  | B | 0.86 | 15.3 |  |
| West | C | 0.49 | 34.2 |  | D | 0.90 | 50.1 |  |
| OVERALL | B | 0.49 | 12.9 |  | B | 0.90 | 18.0 |  |

Note: NA = Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements

The conceptual intersection geometric layout for the above intersection is shown on Drawing SKC 002 Rev A.

Table 13: Road D1477 and R33/Site Access 1

| Road D1477 \& R33/Site Access 1 |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | TRAFFIC SCENARIO \& LAYOUT | OVERALL INTERSECTION OPERATING CONDITIONS |  |  |  |  |  |  |  |  |
|  |  | $\begin{aligned} & \frac{5}{0} \\ & \frac{\pi}{0} \\ & \frac{0}{2} \\ & \frac{1}{4} \end{aligned}$ | AM PEAK HOUR |  |  |  | PM PEAK HOUR |  |  |  |
|  |  |  | LOS | v/c | Delay(s) | COMMENTS | LOS | v/c | Delay(s) | COMMENTS |
| 1 | 2020 Peak Hour Background Traffic (With existing intersection layout) | South | A | 0.01 | 8.6 | Acceptable operating conditions expected. | A | 0.00 | 9.1 | Acceptable operating conditions expected. |
|  |  | East | A | 0.02 | 9.5 |  | A | 0.02 | 9.7 |  |
|  |  | North | B | 0.16 | 10.1 |  | A | 0.12 | 10.0 |  |
|  |  | West | A | 0.05 | 8.3 |  | A | 0.05 | 8.2 |  |
|  |  | OVERALL | NA | 0.16 | 9.4 |  | NA | 0.12 | 9.2 |  |
| 2 | 2020 Peak Hour Background Traffic + Phase 1 Development Traffic (With development upgrades) | South | A | 0.01 | 9.2 | Acceptable operating conditions expected. Proposed upgrades are summarized in Chapter 7.4. | B | 0.04 | 13.7 | Acceptable operating conditions expected. Proposed upgrades are summarized in Chapter 7.4. |
|  |  | East | A | 0.10 | 8.7 |  | D | 0.88 | 26.8 |  |
|  |  | North | A | 0.14 | 8.3 |  | A | 0.24 | 7.7 |  |
|  |  | West | A | 0.04 | 7.9 |  | A | 0.05 | 7.6 |  |
|  |  | OVERALL | A | 0.14 | 8.4 |  | NA | 0.88 | 16.8 |  |
| 3 | 2020 Peak Hour Background Traffic + Phase 1 \& 2 Development Traffic | South | A | 0.03 | 9.6 | Acceptable operating conditions expected. | B | 0.08 | 14.1 | Acceptable operating conditions expected. |
|  |  | East | A | 0.10 | 8.7 |  | D | 0.88 | 26.8 |  |
|  |  | North | A | 0.14 | 8.3 |  | A | 0.24 | 7.7 |  |
|  |  | West | A | 0.04 | 7.9 |  | A | 0.05 | 7.6 |  |
|  |  | OVERALL | NA | 0.14 | 8.4 |  | NA | 0.88 | 16.5 |  |
| 4 | 2020 Peak Hour <br> Background Traffic + <br> Phase 1, 2 \& 3 <br> Development Traffic | South | B | 0.04 | 10.8 | Acceptable operating conditions expected. | C | 0.10 | 16.2 | Intersection will operate at a $\mathrm{v} / \mathrm{c}$ ratio of 1.00 but with acceptable overall LOS and delays. |
|  |  | East | A | 0.18 | 9.3 |  | E | 1.00 | 35.2 |  |
|  |  | North | A | 0.14 | 8.1 |  | A | 0.28 | 7.7 |  |
|  |  | West | A | 0.04 | 7.9 |  | A | 0.05 | 7.8 |  |
|  |  | OVERALL | NA | 0.18 | 8.6 |  | NA | 1.00 | 20.8 |  |


| South | B | 0.07 | 14.2 | Acceptable operating conditions <br> expected. | C | 0.13 | 19.6 | Intersection will operate at a $\mathrm{v} / \mathrm{c}$ <br> ratio of 1.00 but with acceptable |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| East | B | 0.38 | 11.3 |  |  |  |  |  |
| North | A | 0.22 | 7.9 |  |  |  |  |  |
| overall LOS and delays. |  |  |  |  |  |  |  |  |

Note: NA = Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements
The conceptual intersection geometric layout for the above intersection is shown on Drawing SKC 003 Rev B.

Table 14: Road D1477 and Site Access 2


| 5 | 2020 Peak Hour Background Traffic + Phase 1, 2, 3 \& 4 Development Traffic | South | NA | 0.02 | 1.3 | Acceptable operating conditions expected. | NA | 0.02 | 0.6 | Acceptable operating conditions expected. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | East | B | 0.04 | 6.9 |  | B | 0.10 | 12.8 |  |
|  |  | North | NA | 0.05 | 7.4 |  | NA | 0.04 | 3.0 |  |
|  |  | West | - | - | - |  | - | - | - |  |
|  |  | OVERALL | NA | 0.05 | 6.7 |  | NA | 0.10 | 6.5 |  |

Note: NA = Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements

The conceptual intersection geometric layout for the above intersection is shown on Drawing SKC 004 Rev A

### 7.4 ROADS UPGRADES REQUIRED

Based on the capacity analyses the following road upgrades are required as summarized in Table 14.

Table 14: Proposed Road Upgrades

| No. | Intersection | Phase 1 | Phase 2 | Phase 3 | Phase 4 |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 1. | Road D1477 and N4 Offramp | - Signalize intersection. Refer Drawing SKC 002 Rev A. | No further road upgrades required. | No further road upgrades required. | No further road upgrades required. |
| 2. | Road D1477 and N4 Offramp/Belfast 1 Stop Access | - Signalize intersection. Refer Drawing SKC 002 Rev A. | No further road upgrades required. | No further road upgrades required. | Optimize signal settings. |
| 3. | Road D1477 R33/Site Access 1 | - Provide an exclusive left turn lane (60m) and an exclusive slip lane (60m) on the northern approach. <br> - Provide an exclusive right turn lane ( 60 m ) and a shared through and left turn lane on the eastern approach. <br> - Refer Drawing SKC003 Rev B. | No further road upgrades required. | No further road upgrades required, although a traffic circle/signal can be considered in this phase. | No further road upgrades required, although a traffic circle/signal can be considered in this phase. |
| 4. | Road D1477 R33/Site Access 2 | Not Applicable | New intersection. Provide eastern leg as shown on Drawing SKC 004 Rev A. | No further road upgrades required. | No further road upgrades required. |
| 5. | Proposed link road | Not Applicable | Not Applicable | Link road required to prevent congestion of the N4/Road D1477 interchange | Link road required to prevent congestion of the N4/Road D1477 interchange |

## 8 <br> NON-MOTORISED AND PUBLIC TRANSPORT

### 8.1 BACKGROUND

In terms of the National Land Transport Act 5 of 2009, section 38, it is a requirement that an assessment of the public transport be included in a traffic impact assessment.

### 8.2 EXISTING PUBLIC TRANSPORT SERVICES AND FACILITIES

The proposed development is in close proximity to the following public transport services:

## MINIBUS TAXIS

There is an existing formalised taxi rank facility located at the corner of Fitzgerald Street and Vermooten Street in Belfast (approximately 3.3km north-west of the proposed site). The trips that are made by these minibus taxis are to surrounding towns such as Middelburg and Nelspruit.

### 8.3 PROPOSED / NEW FACILITIES

## MINIBUS TAXI LAYBYS

There are no taxi laybys available within the proximity of the proposed site. Therefore it is recommended that a pair of laybys be positioned along Road D1477 at the main access. The proposed laybys are illustrated on Drawing SKC003.

## PEDESTRIAN FACILITIES

It is proposed that surfaced pedestrian sidewalks should be provided along the site frontage to facilitate pedestrian movements to and from the site.

## 9

## CONCLUSIONS

Based on the assessment of the existing road network, traffic counts, a traffic and capacity analysis of road links in the study area, the following concluding remarks are relevant:
$\rightarrow \quad$ The proposed development is situated on Remainder of Portion 12 of the Farm Wemmerhuis 379-JT and Remainder of the Farm Bergendal 981-JR in Belfast, Mpumalanga Province (see Figure 1). The proposed development will be implemented in phases.
$\rightarrow \quad$ This study considered the following land-uses:

- Industrial $14252 \mathrm{~m}^{2}$
- Single Dwelling Units 161 Stands
- Apartments and Flats 387 Units
- Pre-school 200 Pupils
- Shopping Centre $33290 \mathrm{~m}^{2}$
- Business Centre $42344 \mathrm{~m}^{2}$
$\rightarrow \quad$ Detailed traffic surveys were carried at the following intersections (see Figure 1):
- Road D1477 and N4 Offramp;
- Road D1477 and N4 Offramp/Belfast 1Stop Access and
- Road D1477 and R33
$\rightarrow \quad$ The proposed development will generate approximately 1052 and 1887 trips during the AM and PM peak hour respectively.
$\rightarrow \quad$ It is proposed that the development be served by two full accesses off Road D1477 plus and additional access to the north of the proposed development (over/under the N4) to serve Phase 3 and 4. The main access will be via the eastern leg of the intersection of Road D1477 and R33.
$\rightarrow \quad$ The site development plans have not yet been finalized. It is proposed that a parking assessment be undertaken during the site development submission stage.
$\rightarrow \quad$ Roads upgrades required are stated in Chapter 7.4 and as shown on Drawings SKC001 to SKC004.
$\rightarrow \quad$ There are public transport services provided within the vicinity of site. There are no taxi laybys available within the proximity of the proposed site. Therefore it is recommended that a pair of laybys be positioned along Road D1477 at the main access. The proposed laybys are illustrated on Drawing SKC003.

From a traffic engineering perspective; the proposed development is supported.

## 10

## REFERENCES

$\rightarrow$ TMH 16 Volume 2, South African Traffic Impact and Site Traffic Assessment Standards and Requirements Manual, Version 1.0, Committee of Transport Officials (COTO) August 2012
$\rightarrow$ TMH 17 Volume 1, South African Trip Data Manual, Version 1.0, Committee of Transport Officials (COTO) September 2012.
$\rightarrow$ Highway Capacity Manual, Transportation Research Board, National Research Council Washington D.C., 2010.
$\rightarrow$ Manual for Traffic Impact Studies, Department of Transport, October 1995
$\rightarrow$ South African Trip Generation Rates, $2^{\text {nd }}$ edition, Department of Transport, June 1995
$\rightarrow$ South African Parking Standards, 2 ${ }^{\text {nd }}$ edition, Department of Transport, November 1985

## FIGURES

## FIGURES

FIGURE 1: LOCALITY PLAN
FIGURE 2: 2015 EXISTING TRAFFIC VOLUMES
FIGURE 3: 2020 BACKGROUND TRAFFIC VOLUMES
FIGURE 4A: PHASE 1 PEAK HOUR DEVELOPMENT TRIPS
FIGURE 4B: PHASE 2 PEAK HOUR DEVELOPMENT TRIPS
FIGURE 4C: PHASE 3 PEAK HOUR DEVELOPMENT TRIPS
FIGURE 4D: PHASE 4 PEAK HOUR DEVELOPMENT TRIPS
FIGURE 5A: 2020 BACKGROUND PLUS PHASE 1 DEVELOPMENT TRAFFIC
FIGURE 5B: 2020 BACKGROUND PLUS PHASE $1 \& 2$ DEVELOPMENT TRAFFIC FIGURE 5C: 2020 BACKGROUND PLUS PHASE 1,2 \& 3 DEVELOPMENT TRAFFIC FIGURE 5D: 2020 BACKGROUND PLUS PHASE 1, 2, 3 \& 4 DEVELOPMENT TRAFFIC




Project:
PROPOSED BELFAST MIXED USE DEVELOPMENT

Figure:

Project:
PROPOSED BELFAST MIXED USE DEVELOPMENT

Figure:

Project:
PROPOSED BELFAST MIXED USE DEVELOPMENT

Figure:

Project:
PROPOSED BELFAST MIXED USE DEVELOPMENT

Figure:





## DRAWINGS

## D R A W INGS

Drawing SKC001 Rev A: D1477/N4 Offramp Intersection Layout Plan
Drawing SKC002 Rev A: D1477/N4 Offramp/Belfast 1Stop Intersection Layout Plan
Drawing SKC003 Rev B: D1477/R33/Site Access Intersection Layout Plan
Drawing SKC004 Rev A: D1477/Site Access 2 Intersection Layout Plan





APPENDICES

## N4 Highway



Annexure A-2 - Record of site visit Photo Sheet 1 of 2


Road D1477 \& N4 Offramp (facing north)


Access to Belfast 1Stop (facing east)

Annexure A-2 - Record of site visit Photo Sheet 2 of 2



APPENDIX A-4
TRIP GENERATION CALCULATIONS


Late Land Use Vencice






$\xrightarrow[\text { Vonce }]{2}$



|  | Lenc use | Daily | m | Mipak | Tosal | m | $\xrightarrow{\text { Pumpeak }}$ | Tosal | m | Fridy pu | Toal | ${ }_{\text {lol }}$ | Mut | Toal | ${ }^{\text {m }}$ | ${ }_{\text {Everine }}$ | Toal | m | sumber | Tosal | m | sump | Total | m | $\xrightarrow{\text { anpeak }}$ | Total | m | pupeak | Total | m | Fridy oum | Total | m | , in | Toal | m | Ewome | Total | m | sumay | Total | m | sumay | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | ${ }_{6}^{63}$ | ${ }^{4}$ | ${ }_{13}^{12}$ | ${ }_{\text {c }}^{18}$ | ${ }_{10}^{12}$ | ${ }_{4}^{5}$ | ${ }_{18}^{18}$ |  |  |  |  |  |  |  |  |  | 40 | ${ }_{4}^{4}$ | ${ }_{81}$ | 40 | ${ }_{4}^{4}$ | ${ }_{81}$ | ${ }_{37}$ | ${ }^{13}$ | ${ }^{18}$ | ${ }_{10}^{12}$ | ${ }_{5}^{5}$ | ${ }_{\substack{18 \\ 150}}$ |  |  |  |  |  |  |  |  |  | ${ }_{4}$ | ${ }_{4}^{4}$ | ${ }_{81}$ | ${ }^{4}$ | ${ }_{4}^{4}$ | $\stackrel{\square}{8}$ |
| 200 | Oneme |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Appendix A-5: Existing vs. Proposed Intersection Layout and Control


Appendix A-5 (continue): Existing vs. Proposed Intersection Layout and Control

|  | EXISTING LAYOUT \& TRAFFIC CONTROL | PROPOSED LAYOUT \& TRAFFIC CONTROL |
| :---: | :---: | :---: |
|  | TRAFFIC CONTROL: SIDE STOP | TRAFFIC CONTROL: SIGNALISED |
|  | STOP |  |

Appendix A-5 (continue): Existing vs. Proposed Intersection Layout and Control

| $\begin{aligned} & \text { z } \\ & \text { ㅇㅡㅡ } \\ & \text { W } \\ & \text { un } \\ & \text { w } \\ & \underline{z} \\ & \hline \end{aligned}$ | EXISTING LAYOUT \& TRAFFIC CONTROL | PROPOSED LAYOUT \& TRAFFIC CONTROL |
| :---: | :---: | :---: |
|  | TRAFFIC CONTROL: ALL-WAY STOP | TRAFFIC CONTROL: ALL-WAY STOP |

Appendix A-5 (continue): Existing vs. Proposed Intersection Layout and Control


```
APPENDIX A-6 DETAILED SIDRA RESULTS
```

2020 AM Horizon Year Traffic Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID |  | Demand Flow vehih | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg. Sain v/C | Average Delay sec | Level of Service | 95\% Back o <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB |  |  |  |  |  |  |  |  |  |  |  |
| 2 | T | 174 | 20.0 | 0.101 | 0.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| 3 | R | 17 | 20.0 | 0.021 | 11.7 | LOS B | 0.1 | 0.7 | 0.45 | 0.71 | 45.9 |
| Approac |  | 191 | 20.0 | 0.101 | 1.0 | NA | 0.1 | 0.7 | 0.04 | 0.06 | 58.4 |
| North: Road D1477 SB |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 54 | 20.0 | 0.033 | 8.8 | LOS A | 0.0 | 0.0 | 0.00 | 0.66 | 49.0 |
| 8 | T | 288 | 20.0 | 0.167 | 0.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approac |  | 342 | 20.0 | 0.167 | 1.4 | NA | 0.0 | 0.0 | 0.00 | 0.10 | 58.0 |
| West: N4 Offramp |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 152 | 20.0 | 0.093 | 8.0 | x | x | X | X | 0.59 | 49.8 |
| 11 | T | 3 | 20.0 | 0.123 | 22.3 | LOS C | 0.4 | 3.6 | 0.66 | 0.99 | 38.6 |
| 12 | R | 33 | 20.0 | 0.123 | 22.0 | LOS C | 0.4 | 3.6 | 0.66 | 1.00 | 38.8 |
| Approac |  | 187 | 20.0 | 0.123 | 10.7 | LOS B | 0.4 | 3.6 | 0.13 | 0.67 | 47.3 |
| All Vehic |  | 720 | 20.0 | 0.167 | 3.7 | NA | 0.4 | 3.6 | 0.04 | 0.24 | 54.8 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

2020 PM Horizon Year Traffic
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg, Satn vic | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queve Distance m | Prop. Queved | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB |  |  |  |  |  |  |  |  |  |  |  |
| 2 | T | 216 | 20.0 | 0.125 | 0.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| 3 | R | 34 | 20.0 | 0.047 | 12.5 | LOS B | 0.2 | 1.5 | 0.50 | 0.76 | 45.2 |
| Approa |  | 249 | 20.0 | 0.125 | 1.7 | NA | 0.2 | 1.5 | 0.07 | 0.10 | 57.5 |
| North: Road D1477 SB |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 101 | 20.0 | 0.062 | 8.8 | LOSA | 0.0 | 0.0 | 0.00 | 0.66 | 49.0 |
| 8 | T | 314 | 20.0 | 0.182 | 0.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approac |  | 415 | 20.0 | 0.182 | 2.1 | NA | 0.0 | 0.0 | 0.00 | 0.16 | 56.9 |
| West: N4 Offramp |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 473 | 20.0 | 0.291 | 8.0 | X | X | X | $x$ | 0.59 | 49.7 |
| 11 | T | 14 | 20.0 | 0.246 | 27.6 | LOS D | 0.9 | 7.8 | 0.76 | 1.02 | 35.4 |
| 12 | R | 44 | 20.0 | 0.246 | 27.3 | LOS D | 0.9 | 7.8 | 0.76 | 1.03 | 35.6 |
| Approach |  | 531 | 20.0 | 0.291 | 10.1 | LOS B | 0.9 | 7.8 | 0.08 | 0.64 | 47.7 |
| All Vehicles |  | 1195 | 20.0 | 0.291 | 5.6 | NA | 0.9 | 7.8 | 0.05 | 0.36 | 52.5 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

2020 AM Background + Phase 1 Development Traffic
Signals - Fixed Time Cycle Time $=60$ seconds (Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow $\mathrm{veh} / \mathrm{h}$ | $\begin{aligned} & \mathrm{HV} \\ & \% \end{aligned}$ | $\begin{aligned} & \text { Deg } \\ & \text { Satn } \\ & \text { v/c } \end{aligned}$ | Average Delay sec | Level of Service | 95\% Back of Vehicles veh | Queve Distance m | Prop. Queued | Effective Stop Rate perveh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB |  |  |  |  |  |  |  |  |  |  |  |
| 2 | T | 233 | 20.0 | 0.193 | 3.3 | LOSA | 2.4 | 19.3 | 0.37 | 0.31 | 53.2 |
| 3 | R | 25 | 20.0 | 0.068 | 14.5 | LOS B | 0.3 | 2.5 | 0.41 | 0.74 | 43.3 |
| Approac |  | 258 | 20.0 | 0.193 | 4.4 | LOS A | 2.4 | 19.3 | 0.37 | 0.35 | 52.0 |
| North: Road D1477 SB |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 54 | 20.0 | 0.111 | 11.7 | LOS B | 0.5 | 4.0 | 0.32 | 0.70 | 45.9 |
| 8 | T | 383 | 20.0 | 0.317 | 3.7 | LOSA | 4.3 | 35.5 | 0.41 | 0.36 | 52.5 |
| Approac |  | 437 | 20.0 | 0.317 | 4.7 | LOS A | 4.3 | 35.5 | 0.40 | 0.40 | 51.6 |
| West: N4 Offramp |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 152 | 20.0 | 0.093 | 8.0 | X | X | $x$ | X | 0.59 | 49.8 |
| 11 | T | 3 | 20.0 | 0.323 | 29.4 | LOS C | 1.6 | 12.8 | 0.97 | 0.72 | 29.7 |
| 12 | R | 49 | 20.0 | 0.323 | 38.5 | LOS D | 1.6 | 12.8 | 0.97 | 0.75 | 29.4 |
| Approach |  | 204 | 20.0 | 0.323 | 15.7 | LOS B | 1.6 | 12.8 | 0.25 | 0.63 | 42.3 |
| All Vehicles |  | 899 | 20.0 | 0.323 | 7.1 | LOSA | 4.3 | 35.5 | 0.36 | 0.44 | 49.2 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.

2020 PM Background + Phase 1 Development Traffic
Signals - Fixed Time Cycle Time $=60$ seconds (Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID |  | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \\ & \hline \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | $95 \%$ Back of <br> Vehicles <br> veh | f Queve Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB |  |  |  |  |  |  |  |  |  |  |  |
| 2 | T | 518 | 20.0 | 0.450 | 5.1 | LOSA | 7.3 | 59.5 | 0.51 | 0.45 | 50.4 |
| 3 | R | 84 | 20.0 | 0.288 | 20.2 | LOS C | 1.6 | 13.0 | 0.63 | 0.79 | 38.9 |
| Approa |  | 602 | 20.0 | 0.450 | 7.3 | LOSA | 7.3 | 59.5 | 0.53 | 0.50 | 48.4 |
| North: Road D1477 SB |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 101 | 20.0 | 0.227 | 12.6 | LOS B | 1.0 | 8.6 | 0.37 | 0.71 | 45.1 |
| 8 | T | 621 | 20.0 | 0.540 | 5.6 | LOSA | 9.5 | 78.1 | 0.56 | 0.50 | 49.6 |
| Approa |  | 722 | 20.0 | 0.540 | 6.6 | LOS A | 9.5 | 78.1 | 0.53 | 0.53 | 48.9 |
| West: N4 Offramp |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 473 | 20.0 | 0.291 | 8.0 | $x$ | X | $x$ | x | 0.59 | 49.7 |
| 11 | T | 14 | 20.0 | 0.531 | 28.2 | LOS C | 3.4 | 28.0 | 0.98 | 0.77 | 30.2 |
| 12 | R | 102 | 20.0 | 0.531 | 37.2 | LOS D | 3.4 | 28.0 | 0.98 | 0.79 | 30.0 |
| Approach |  | 588 | 20.0 | 0.531 | 13.5 | LOS B | 3.4 | 28.0 | 0.19 | 0.63 | 44.1 |
| All Vehi |  | 1913 | 20.0 | 0.540 | 8.9 | LOSA | 9.5 | 78.1 | 0.43 | 0.55 | 47.2 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.

2020 AM Background + Phase 1\&2 Development Traffic
Signals - Fixed Time Cycle Time $=60$ seconds (User-Given Cycle Time)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn V/c | Average Delay sec | Level of Service | 95\% Back o <br> Vehicles veh | f Queve Distance m | Prop. Queved | Effective Stop Rate per veh | Average Speed km/h |
| South: Road D1477 NB |  |  |  |  |  |  |  |  |  |  |  |
| 2 | T | 252 | 20.0 | 0.213 | 3.8 | LOS A | 2.7 | 22.3 | 0.39 | 0.34 | 52.5 |
| 3 | R | 28 | 20.0 | 0.082 | 15.5 | LOS B | 0.4 | 3.0 | 0.45 | 0.74 | 42.5 |
| Approa |  | 280 | 20.0 | 0.213 | 5.0 | LOSA | 2.7 | 22.3 | 0.40 | 0.38 | 51.3 |
| North: Road D1477 SB |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 54 | 20.0 | 0.116 | 12.1 | LOS B | 0.5 | 4.2 | 0.34 | 0.70 | 45.6 |
| 8 | T | 431 | 20.0 | 0.365 | 4.3 | LOSA | 5.3 | 43.7 | 0.45 | 0.40 | 51.6 |
| Approac |  | 484 | 20.0 | 0.365 | 5.2 | LOSA | 5.3 | 43.7 | 0.44 | 0.43 | 50.8 |
| West: N4 Offramp |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 152 | 20.0 | 0.093 | 8.0 | $x$ | X | $x$ | $x$ | 0.59 | 49.8 |
| 11 | T | 3 | 20.0 | 0.321 | 28.3 | LOS C | 1.8 | 14.5 | 0.96 | 0.72 | 30.2 |
| 12 | R | 58 | 20.0 | 0.321 | 37.3 | LOS D | 1.8 | 14.5 | 0.96 | 0.75 | 29.8 |
| Approach |  | 213 | 20.0 | 0.321 | 16.3 | LOS B | 1.8 | 14.5 | 0.27 | 0.64 | 41.9 |
| All Vehic |  | 977 | 20.0 | 0.365 | 7.5 | LOSA | 5.3 | 43.7 | 0.39 | 0.46 | 48.7 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.

2020 PM Background + Phase 1\&2 Development Traffic
Signals - Fixed Time Cycle Time $=60$ seconds (User-Given Cycle Time)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow vehich | $\begin{aligned} & \text { HV } \\ & \% \\ & \hline \end{aligned}$ | Deg Satn vic | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop Queved | Effective Stop Rate perveh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB |  |  |  |  |  |  |  |  |  |  |  |
| 2 | T | 566 | 20.0 | 0.492 | 5.4 | LOSA | 8.3 | 67.8 | 0.53 | 0.48 | 50.0 |
| 3 | R | 92 | 20.0 | 0.323 | 20.5 | LOS C | 1.8 | 14.4 | 0.65 | 0.79 | 38.7 |
| Approa |  | 658 | 20.0 | 0.492 | 7.5 | LOS A | 8.3 | 67.8 | 0.55 | 0.52 | 48.1 |
| North: Road D1477 SB |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 101 | 20.0 | 0.227 | 12.6 | LOS B | 1.0 | 8.6 | 0.37 | 0.71 | 45.1 |
| 8 | T | 637 | 20.0 | 0.554 | 5.7 | LOSA | 9.9 | 81.2 | 0.57 | 0.51 | 49.5 |
| Approa |  | 738 | 20.0 | 0.554 | 6.7 | LOSA | 9.9 | 81.2 | 0.54 | 0.54 | 48.8 |
| West: N4 Offramp |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 473 | 20.0 | 0.291 | 8.0 | X | X | $x$ | X | 0.59 | 49.7 |
| 11 | T | 14 | 20.0 | 0.545 | 28.3 | LOS C | 3.5 | 28.9 | 0.98 | 0.78 | 30.2 |
| 12 | R | 105 | 20.0 | 0.545 | 37.4 | LOS D | 3.5 | 28.9 | 0.98 | 0.80 | 29.9 |
| Approach |  | 592 | 20.0 | 0.545 | 13.7 | LOS B | 3.5 | 28.9 | 0.20 | 0.63 | 44.0 |
| All Vehicles |  | 1987 | 20.0 | 0.554 | 9.0 | LOSA | 9.9 | 81.2 | 0.44 | 0.56 | 47.0 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.

2020 AM Background + Phase 1, 2 \& 3 Development Traffic
Signals - Fixed Time Cycle Time $=60$ seconds (User-Given Cycle Time)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| MovID | Turn | Demand Flow veh/h | $\begin{aligned} & \mathrm{HV} \\ & \% \end{aligned}$ | Deg, Sath V/C | Average Delay sec | Level of Service | 95\% Back <br> Vehicles <br> veh | Queve Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Road D1477 NB |  |  |  |  |  |  |  |  |  |  |  |
| 2 | T | 300 | 20.0 | 0.254 | 3.9 | LOS A | 3.4 | 27.6 | 0.41 | 0.35 | 52.3 |
| 3 | R | 35 | 20.0 | 0.107 | 16.1 | LOS B | 0.5 | 3.9 | 0.48 | 0.75 | 42.0 |
| Approac |  | 335 | 20.0 | 0.254 | 5.2 | LOS A | 3.4 | 27.6 | 0.42 | 0.39 | 51.0 |
| North: Road D1477 SB |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 54 | 20.0 | 0.116 | 12.1 | LOS B | 0.5 | 4.2 | 0.34 | 0.70 | 45.6 |
| 8 | T | 499 | 20.0 | 0.423 | 4.6 | LOSA | 6.5 | 53.5 | 0.48 | 0.42 | 51.2 |
| Approac |  | 553 | 20.0 | 0.423 | 5.3 | LOSA | 6.5 | 53.5 | 0.47 | 0.45 | 50.6 |
| West: N4 Offramp |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 152 | 20.0 | 0.093 | 8.0 | X | X | X | x | 0.59 | 49.8 |
| 11 | T | 3 | 20.0 | 0.382 | 28.5 | LOS C | 2.1 | 17.4 | 0.96 | 0.74 | 30.0 |
| 12 | R | 69 | 20.0 | 0.382 | 37.6 | LOS D | 2.1 | 17.4 | 0.96 | 0.76 | 29.7 |
| Approach |  | 224 | 20.0 | 0.382 | 17.4 | LOS B | 2.1 | 17.4 | 0.31 | 0.65 | 41.0 |
| All Vehicles |  | 1112 | 20.0 | 0.423 | 7.7 | LOSA | 6.5 | 53.5 | 0.42 | 0.47 | 48.4 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement Intersection and Approach LOS values are based on average delay for all vehicle movements
SIDRA Standard Delay Model used.

2020 PM Background + Phase 1, 2, 3 \& 4 Development Traffic
Signals - Fixed Time Cycle Time $=60$ seconds (User-Given Cycle Time)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Tuin | Demand Flow $\mathrm{veh} / \mathrm{h}$ | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg Satn v/c | Average Delay sec | Level of Service | 95\% Back of <br> Vehicles veh | Queve Distance m | Prop Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB |  |  |  |  |  |  |  |  |  |  |  |
| 2 | T | 644 | 20.0 | 0.560 | 5.8 | LOSA | 10.1 | 82.7 | 0.57 | 0.52 | 49.4 |
| 3 | R | 103 | 20.0 | 0.407 | 22.5 | LOS C | 2.2 | 18.0 | 0.71 | 0.80 | 37.4 |
| Approac |  | 747 | 20.0 | 0.560 | 8.1 | LOSA | 10.1 | 82.7 | 0.59 | 0.56 | 47.3 |
| North: Road D1477 SB |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 101 | 20.0 | 0.227 | 12.6 | LOS B | 1.0 | 8.6 | 0.37 | 0.71 | 45.1 |
| 8 | T | 700 | 20.0 | 0.608 | 6.1 | LOSA | 11.6 | 94.9 | 0.60 | 0.55 | 48.9 |
| Approac |  | 801 | 20.0 | 0.608 | 6.9 | LOSA | 11.6 | 94.9 | 0.58 | 0.57 | 48.4 |
| West: N4 Offramp |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 473 | 20.0 | 0.291 | 8.0 | $x$ | X | $x$ | x | 0.59 | 49.7 |
| 11 | T | 14 | 20.0 | 0.594 | 28.8 | LOS C | 3.9 | 32.0 | 0.99 | 0.81 | 29.9 |
| 12 | R | 116 | 20.0 | 0.594 | 37.9 | LOS D | 3.9 | 32.0 | 0.99 | 0.82 | 29.7 |
| Approach |  | 602 | 20.0 | 0.594 | 14.2 | LOS B | 3.9 | 32.0 | 0.21 | 0.64 | 43.5 |
| All Vehicles |  | 2151 | 20.0 | 0.608 | 9.3 | LOSA | 11.6 | 94.9 | 0.48 | 0.58 | 46.6 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.

2020 AM Background + Phase 1, 2, 3 \& 4 Development Traffic
Signals - Fixed Time Cycle Time $=60$ seconds (User-Given Cycle Time)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movid |  | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg <br> Satn <br> v/c | Average Delay sec | Level of Service | 95\% Back o <br> Vehicles veh | Queve Distance m | Prop. Queued | Effective Stop Rate perveh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB |  |  |  |  |  |  |  |  |  |  |  |
| 2 | T | 378 | 20.0 | 0.320 | 4.2 | LOS A | 4.5 | 36.8 | 0.43 | 0.38 | 51.9 |
| 3 | R | 46 | 20.0 | 0.166 | 18.7 | LOS B | 0.8 | 6.4 | 0.57 | 0.77 | 40.0 |
| Approac |  | 424 | 20.0 | 0.320 | 5.7 | LOS A | 4.5 | 36.8 | 0.45 | 0.42 | 50.3 |
| North: Road D1477 SB |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 54 | 20.0 | 0.116 | 12.1 | LOS B | 0.5 | 4.2 | 0.34 | 0.70 | 45.6 |
| 8 | T | 646 | 20.0 | 0.548 | 5.2 | LOSA | 9.6 | 79.0 | 0.55 | 0.49 | 50.2 |
| Approac |  | 700 | 20.0 | 0.548 | 5.7 | LOSA | 9.6 | 79.0 | 0.53 | 0.51 | 49.8 |
| West: N4 Offramp |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 152 | 20.0 | 0.093 | 8.0 | $x$ | x | x | $x$ | 0.59 | 49.8 |
| 11 | T | 3 | 20.0 | 0.515 | 29.2 | LOS C | 2.9 | 24.0 | 0.98 | 0.77 | 29.7 |
| 12 | R | 95 | 20.0 | 0.515 | 38.2 | LOS D | 2.9 | 24.0 | 0.98 | 0.78 | 29.4 |
| Approac |  | 249 | 20.0 | 0.515 | 19.7 | LOS B | 2.9 | 24.0 | 0.39 | 0.67 | 39.3 |
| All Vehic |  | 1374 | 20.0 | 0.548 | 8.3 | LOSA | 9.6 | 79.0 | 0.48 | 0.51 | 47.6 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.

2020 PM Background + Phase 1, 2, 3 \& 4 Development Traffic
Signals - Fixed Time Cycle Time $=60$ seconds (User-Given Cycle Time)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movid | Tum | Demand Flow $\mathrm{Veh} / \mathrm{h}$ | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn VIC | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB |  |  |  |  |  |  |  |  |  |  |  |
| 2 | T | 788 | 20.0 | 0.685 | 6.7 | LOS A | 14.3 | 117.0 | 0.66 | 0.60 | 48.1 |
| 3 | R | 123 | 20.0 | 0.567 | 26.6 | LOS C | 3.1 | 25.8 | 0.82 | 0.84 | 34.9 |
| Approac |  | 912 | 20.0 | 0.685 | 9.4 | LOSA | 14.3 | 117.0 | 0.68 | 0.64 | 45.8 |
| North: Road D1477 SB |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 101 | 20.0 | 0.227 | 12.6 | LOS B | 1.0 | 8.6 | 0.37 | 0.71 | 45.1 |
| 8 | T | 783 | 20.0 | 0.681 | 6.6 | LOSA | 14.1 | 115.6 | 0.66 | 0.60 | 48.1 |
| Approac |  | 884 | 20.0 | 0.681 | 7.3 | LOSA | 14.1 | 115.6 | 0.63 | 0.61 | 47.8 |
| West: N4 Offramp |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 473 | 20.0 | 0.291 | 8.0 | $x$ | X | X | $x$ | 0.59 | 49.7 |
| 11 | T | 14 | 20.0 | 0.657 | 29.6 | LOS C | 4.4 | 36.2 | 1.00 | 0.85 | 29.5 |
| 12 | R | 129 | 20.0 | 0.657 | 38.7 | LOS D | 4.4 | 36.2 | 1.00 | 0.85 | 29.4 |
| Approac |  | 616 | 20.0 | 0.657 | 14.9 | LOS B | 4.4 | 36.2 | 0.23 | 0.65 | 42.9 |
| All Vehic |  | 2412 | 20.0 | 0.685 | 10.0 | LOS B | 14.3 | 117.0 | 0.55 | 0.63 | 45.7 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.

2020 AM Horizon Year Traffic Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow $\mathrm{veh} / \mathrm{h}$ | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back o Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB mee per men km/h |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 6 | 20.0 | 0.004 | 8.8 | LOS A | 0.0 | 0.0 | 0.00 | 0.66 | 49.0 |
| 2 | T | 78 | 20.0 | 0.051 | 0.9 | LOS A | 0.3 | 2.7 | 0.34 | 0.00 | 53.7 |
| 3 | R | 6 | 20.0 | 0.051 | 9.8 | LOSA | 0.3 | 2.7 | 0.34 | 0.92 | 49.1 |
| Approac |  | 91 | 20.0 | 0.051 | 2.1 | NA | 0.3 | 2.7 | 0.32 | 0.11 | 53.0 |
| East: Belfast One Stop Access |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 13 | 20.0 | 0.288 | 20.3 | LOS C | 1.3 | 10.6 | 0.60 | 0.83 | 40.1 |
| 5 | T | 52 | 20.0 | 0.288 | 20.7 | LOS C | 1.3 | 10.6 | 0.60 | 1.03 | 39.8 |
| 6 | R | 47 | 20.0 | 0.288 | 20.3 | LOS C | 1.3 | 10.6 | 0.60 | 1.02 | 40.1 |
| Approac |  | 112 | 20.0 | 0.288 | 20.5 | LOS C | 1.3 | 10.6 | 0.60 | 1.00 | 40.0 |
| North: Road D1477 SB |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 64 | 20.0 | 0.040 | 8.8 | LOSA | 0.0 | 0.0 | 0.00 | 0.66 | 49.0 |
| 8 | T | 95 | 20.0 | 0.055 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| 9 | R | 169 | 20.0 | 0.173 | 9.4 | LOSA | 0.7 | 5.4 | 0.23 | 0.63 | 47.9 |
| Approac |  | 328 | 20.0 | 0.173 | 6.5 | NA | 0.7 | 5.4 | 0.12 | 0.45 | 51.1 |
| West: N4 Offramp |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 66 | 20.0 | 0.077 | 12.5 | LOS B | 0.3 | 2.3 | 0.22 | 0.89 | 46.0 |
| 11 | T | 1 | 20.0 | 0.077 | 12.9 | LOS B | 0.3 | 2.3 | 0.22 | 0.99 | 45.6 |
| 12 | R | 1 | 20.0 | 0.077 | 12.5 | LOS B | 0.3 | 2.3 | 0.22 | 1.00 | 46.1 |
| Approach |  | 68 | 20.0 | 0.077 | 12.5 | LOS B | 0.3 | 2.3 | 0.22 | 0.89 | 46.0 |
| All Vehic |  | 599 | 20.0 | 0.288 | 9.2 | NA | 1.3 | 10.6 | 0.25 | 0.55 | 48.2 |

Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

2020 PM Horizon Year Traffic Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movid |  | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles <br> veh | Queue <br> Distance <br> m | Prop Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 6 | 20.0 | 0.004 | 8.8 | LOS A | 0.0 | 0.0 | 0.00 | 0.66 | 49.0 |
| 2 | T | 85 | 20.0 | 0.059 | 1.0 | LOSA | 0.4 | 3.1 | 0.34 | 0.00 | 53.7 |
| 3 | R | 9 | 20.0 | 0.059 | 9.8 | LOSA | 0.4 | 3.1 | 0.34 | 0.91 | 49.0 |
| Approac |  | 101 | 20.0 | 0.059 | 2.3 | NA | 0.4 | 3.1 | 0.32 | 0.13 | 52.9 |
| East. Belfast One Stop Access |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 20 | 20.0 | 0.474 | 24.4 | LOS C | 2.8 | 22.8 | 0.66 | 0.92 | 37.4 |
| 5 | T | 71 | 20.0 | 0.474 | 24.8 | LOS C | 2.8 | 22.8 | 0.66 | 1.12 | 37.2 |
| 6 | R | 82 | 20.0 | 0.474 | 24.4 | LOS C | 2.8 | 22.8 | 0.66 | 1.10 | 37.4 |
| Approac |  | 173 | 20.0 | 0.474 | 24.6 | LOS C | 2.8 | 22.8 | 0.66 | 1.09 | 37.3 |
| North: Road D1477 SB |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 83 | 20.0 | 0.051 | 8.8 | LOSA | 0.0 | 0.0 | 0.00 | 0.66 | 49.0 |
| 8 | T | 75 | 20.0 | 0.043 | 0.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| 9 | R | 179 | 20.0 | 0.183 | 9.4 | LOSA | 0.7 | 5.8 | 0.25 | 0.63 | 47.8 |
| Approac |  | 337 | 20.0 | 0.183 | 7.2 | NA | 0.7 | 5.8 | 0.13 | 0.50 | 50.4 |
| West: N4 Offramp |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 85 | 20.0 | 0.107 | 12.8 | LOS B | 0.4 | 3.3 | 0.24 | 0.88 | 45.7 |
| 11 | T | 2 | 20.0 | 0.107 | 13.3 | LOS B | 0.4 | 3.3 | 0.24 | 0.99 | 45.3 |
| 12 | R | 3 | 20.0 | 0.107 | 12.8 | LOS B | 0.4 | 3.3 | 0.24 | 1.01 | 45.8 |
| Approach |  | 91 | 20.0 | 0.107 | 12.8 | LOS B | 0.4 | 3.3 | 0.24 | 0.89 | 45.7 |
| All Vehicles |  | 701 | 20.0 | 0.474 | 11.5 | NA | 2.8 | 22.8 | 0.30 | 0.64 | 46.1 |

Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

2020 AM Background+Phase 1 Development Traffic
Signals - Fixed Time Cycle Time $=60$ seconds (User-Given Cycle Time)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movid | Turn | Demand Fiow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | $\begin{aligned} & \text { Deg, } \\ & \text { Sath } \\ & \text { vic } \end{aligned}$ | Average Delay sec | Level of Service | 95\% Back of Vehicles veh | Queue Distance m | Prop Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB amen kern |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 16 | 20.0 | 0.034 | 14.4 | LOS B | 0.2 | 1.6 | 0.44 | 0.68 | 43.5 |
| 2 | T | 145 | 20.0 | 0.157 | 6.2 | LOSA | 2.0 | 16.7 | 0.48 | 0.40 | 49.2 |
| 3 | R | 6 | 20.0 | 0.157 | 15.0 | LOS B | 2.0 | 16.7 | 0.48 | 0.98 | 44.7 |
| Approac |  | 167 | 20.0 | 0.157 | 7.3 | LOS A | 2.0 | 16.7 | 0.48 | 0.45 | 48.4 |
| East: Belfast One Stop Access |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 13 | 20.0 | 0.373 | 31.6 | LOS C | 2.9 | 24.2 | 0.89 | 0.80 | 33.2 |
| 5 | T | 52 | 20.0 | 0.373 | 22.8 | LOS C | 2.9 | 24.2 | 0.89 | 0.71 | 33.8 |
| 6 | R | 47 | 20.0 | 0.373 | 31.5 | LOS C | 2.9 | 24.2 | 0.89 | 0.80 | 33.2 |
| Approac |  | 112 | 20.0 | 0.373 | 27.5 | LOS C | 2.9 | 24.2 | 0.89 | 0.76 | 33.5 |
| North: Road D1477 SB |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 64 | 20.0 | 0.118 | 14.6 | LOS B | 0.8 | 6.7 | 0.45 | 0.71 | 43.3 |
| 8 | T | 206 | 20.0 | 0.205 | 6.4 | LOS A | 2.9 | 23.5 | 0.50 | 0.42 | 49.0 |
| 9 | R | 169 | 20.0 | 0.385 | 16.8 | LOS B | 2.7 | 22.2 | 0.56 | 0.77 | 41.5 |
| Approac |  | 440 | 20.0 | 0.385 | 11.6 | LOS B | 2.9 | 23.5 | 0.52 | 0.60 | 45.0 |
| West: N4 Offramp |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 66 | 20.0 | 0.256 | 30.8 | LOS C | 2.1 | 17.2 | 0.87 | 0.77 | 32.7 |
| 11 | T | 1 | 20.0 | 0.256 | 22.0 | LOS C | 2.1 | 17.2 | 0.87 | 0.68 | 33.4 |
| 12 | R | 15 | 20.0 | 0.256 | 30.8 | LOS C | 2.1 | 17.2 | 0.87 | 0.77 | 32.7 |
| Approach |  | 82 | 20.0 | 0.256 | 30.7 | LOS C | 2.1 | 17.2 | 0.87 | 0.77 | 32.8 |
| All Vehic |  | 801 | 20.0 | 0.385 | 14.9 | LOS B | 2.9 | 24.2 | 0.60 | 0.61 | 42.0 |

Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.

2020 PM Background+Phase 1 Development Traffic
Signals - Fixed Time Cycle Time $=60$ seconds (User-Given Cycle Time)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movid | Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | $\begin{aligned} & \text { Deg } \\ & \text { Sath } \\ & \text { v/c } \end{aligned}$ | Average Delay sec | Level of Service | 95\% Back of Vehiclas veh | Queve Distance m | Prop Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB Ser hen kiun |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 67 | 20.0 | 0.153 | 15.6 | LOS B | 0.9 | 7.6 | 0.49 | 0.72 | 42.4 |
| 2 | T | 438 | 20.0 | 0.491 | 9.1 | LOSA | 8.2 | 67.1 | 0.66 | 0.58 | 45.6 |
| 3 | R | 9 | 20.0 | 0.491 | 17.9 | LOS B | 8.2 | 67.1 | 0.66 | 0.97 | 42.9 |
| Approac |  | 515 | 20.0 | 0.491 | 10.1 | LOS B | 8.2 | 67.1 | 0.64 | 0.61 | 45.1 |
| East: Belfast One Stop Access |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 20 | 20.0 | 0.537 | 31.0 | LOS C | 4.6 | 38.1 | 0.91 | 0.82 | 33.4 |
| 5 | T | 71 | 20.0 | 0.537 | 22.3 | LOS C | 4.6 | 38.1 | 0.91 | 0.75 | 33.9 |
| 6 | R | 82 | 20.0 | 0.537 | 30.9 | LOS C | 4.6 | 38.1 | 0.91 | 0.82 | 33.4 |
| Approac |  | 173 | 20.0 | 0.537 | 27.4 | LOS C | 4.6 | 38.1 | 0.91 | 0.79 | 33.6 |
| North: Road D1477 SB |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 83 | 20.0 | 0.161 | 15.7 | LOS B | 1.2 | 9.5 | 0.50 | 0.72 | 42.4 |
| 8 | T | 440 | 20.0 | 0.464 | 8.9 | LOSA | 7.9 | 64.5 | 0.65 | 0.57 | 45.9 |
| 9 | R | 179 | 20.0 | 0.558 | 24.8 | LOS C | 4.3 | 35.4 | 0.82 | 0.82 | 36.0 |
| Approac |  | 702 | 20.0 | 0.558 | 13.7 | LOS B | 7.9 | 64.5 | 0.67 | 0.65 | 42.5 |
| West: N4 Offramp |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 85 | 20.0 | 0.455 | 30.4 | LOS C | 3.8 | 31.5 | 0.89 | 0.80 | 32.9 |
| 11 | T | 2 | 20.0 | 0.455 | 21.7 | LOS C | 3.8 | 31.5 | 0.89 | 0.73 | 33.5 |
| 12 | R | 59 | 20.0 | 0.455 | 30.5 | LOS C | 3.8 | 31.5 | 0.89 | 0.80 | 32.9 |
| Approach |  | 146 | 20.0 | 0.455 | 30.3 | LOS C | 3.8 | 31.5 | 0.89 | 0.80 | 32.9 |
| All Vehic |  | 1536 | 20.0 | 0.558 | 15.6 | LOS B | 8.2 | 67.1 | 0.71 | 0.67 | 40.9 |

Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.

2020 AM Background+Phase 1\&2 Development Traffic
Signals - Fixed Time Cycle Time $=60$ seconds (User-Given Cycle Time)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movid | Turn | Demand Fiow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg. Sath v/c | Average Delay sec | Level of Service | 95\% Back of <br> Vehicles veh | Queue Distance m | Prop. Queved | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 20 | 20.0 | 0.043 | 14.4 | LOS B | 0.2 | 2.0 | 0.44 | 0.69 | 43.5 |
| 2 | T | 167 | 20.0 | 0.180 | 6.3 | LOS A | 2.4 | 19.5 | 0.49 | 0.41 | 49.0 |
| 3 | R | 6 | 20.0 | 0.180 | 15.1 | LOS B | 2.4 | 19.5 | 0.49 | 0.99 | 44.7 |
| Approac |  | 194 | 20.0 | 0.180 | 7.4 | LOS A | 2.4 | 19.5 | 0.49 | 0.46 | 48.3 |
| East Belfast One Stop Access |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 13 | 20.0 | 0.374 | 31.6 | LOS C | 2.9 | 24.2 | 0.89 | 0.80 | 33.2 |
| 5 | T | 52 | 20.0 | 0.374 | 22.8 | LOS C | 2.9 | 24.2 | 0.89 | 0.72 | 33.8 |
| 6 | R | 47 | 20.0 | 0.374 | 31.5 | LOS C | 2.9 | 24.2 | 0.89 | 0.80 | 33.2 |
| Approac |  | 112 | 20.0 | 0.374 | 27.5 | LOS C | 2.9 | 24.2 | 0.89 | 0.76 | 33.5 |
| North: Road D1477 SB |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 64 | 20.0 | 0.118 | 14.6 | LOS B | 0.8 | 6.7 | 0.45 | 0.71 | 43.3 |
| 8 | T | 263 | 20.0 | 0.261 | 6.6 | LOS A | 3.8 | 31.2 | 0.52 | 0.44 | 48.6 |
| 9 | R | 169 | 20.0 | 0.392 | 16.8 | LOS B | 2.7 | 22.4 | 0.57 | 0.77 | 41.4 |
| Approac |  | 497 | 20.0 | 0.392 | 11.1 | LOS B | 3.8 | 31.2 | 0.53 | 0.59 | 45.2 |
| West: N4 Offramp |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 66 | 20.0 | 0.288 | 31.0 | LOS C | 2.3 | 18.9 | 0.87 | 0.77 | 32.6 |
| 11 | T | 1 | 20.0 | 0.288 | 22.2 | LOS C | 2.3 | 18.9 | 0.87 | 0.69 | 33.3 |
| 12 | R | 22 | 20.0 | 0.288 | 31.1 | LOS C | 2.3 | 18.9 | 0.87 | 0.77 | 32.6 |
| Approac |  | 89 | 20.0 | 0.288 | 30.9 | LOS C | 2.3 | 18.9 | 0.87 | 0.77 | 32.6 |
| All Vehic |  | 892 | 20.0 | 0.392 | 14.4 | LOS B | 3.8 | 31.2 | 0.60 | 0.60 | 42.3 |

Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements. SIDRA Standard Delay Model used.

2020 PM Background+Phase 1\&2 Development Traffic
Signals - Fixed Time Cycle Time $=60$ seconds (User-Given Cycle Time)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movid | Turn | Demand fiow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | $\begin{aligned} & \text { Deg } \\ & \text { Satn } \\ & \text { v/c } \end{aligned}$ | Average Delay sec | Level of Service | 95\% Back of Vehicles veh | Queue Distance m | Prop Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 76 | 20.0 | 0.167 | 15.1 | LOS B | 1.0 | 8.3 | 0.48 | 0.72 | 42.8 |
| 2 | T | 493 | 20.0 | 0.533 | 8.8 | LOSA | 9.2 | 75.8 | 0.67 | 0.59 | 45.9 |
| 3 | R | 9 | 20.0 | 0.533 | 17.6 | LOS B | 9.2 | 75.8 | 0.67 | 0.97 | 43.2 |
| Approac |  | 578 | 20.0 | 0.533 | 9.8 | LOSA | 9.2 | 75.8 | 0.64 | 0.61 | 45.4 |
| East: Belfast One Stop Access |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 20 | 20.0 | 0.581 | 32.4 | LOS C | 4.8 | 39.5 | 0.94 | 0.84 | 32.7 |
| 5 | T | 71 | 20.0 | 0.581 | 23.6 | LOS C | 4.8 | 39.5 | 0.94 | 0.78 | 33.2 |
| 6 | R | 82 | 20.0 | 0.581 | 32.3 | LOS C | 4.8 | 39.5 | 0.94 | 0.83 | 32.8 |
| Approac |  | 173 | 20.0 | 0.581 | 28.7 | LOS C | 4.8 | 39.5 | 0.94 | 0.81 | 32.9 |
| North: Road D1477 SB |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 83 | 20.0 | 0.157 | 15.1 | LOS B | 1.1 | 9.1 | 0.48 | 0.72 | 42.8 |
| 8 | T | 459 | 20.0 | 0.469 | 8.3 | LOSA | 8.0 | 65.7 | 0.63 | 0.56 | 46.5 |
| 9 | R | 179 | 20.0 | 0.600 | 25.9 | LOS C | 4.6 | 37.3 | 0.84 | 0.84 | 35.3 |
| Approac |  | 721 | 20.0 | 0.600 | 13.5 | LOS B | 8.0 | 65.7 | 0.67 | 0.65 | 42.7 |
| West: N4 Offramp |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 85 | 20.0 | 0.500 | 31.6 | LOS C | 4.0 | 32.9 | 0.91 | 0.80 | 32.3 |
| 11 | T | 2 | 20.0 | 0.500 | 22.9 | LOS C | 4.0 | 32.9 | 0.91 | 0.75 | 32.8 |
| 12 | R | 61 | 20.0 | 0.500 | 31.7 | LOS C | 4.0 | 32.9 | 0.91 | 0.81 | 32.3 |
| Approac |  | 148 | 20.0 | 0.500 | 31.5 | LOS C | 4.0 | 32.9 | 0.91 | 0.80 | 32.3 |
| All Vehic |  | 1620 | 20.0 | 0.600 | 15.4 | LOS B | 9.2 | 75.8 | 0.71 | 0.67 | 41.1 |

Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.

2020 AM Background+Phase 1,2 \& 3 Development Traffic Signals - Fixed Time Cycle Time $=60$ seconds (User-Given Cycle Time)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg Satn v/c | Average Delay sec | Level of Service | 95\% Back c <br> Vehicles veh | Queue <br> Distance <br> m | Prop. Queued | Effective Stop Rate perveh | Average Speed km/h |
| South: Road D1477 NB |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 27 | 20.0 | 0.057 | 14.0 | LOS B | 0.3 | 2.6 | 0.42 | 0.69 | 43.8 |
| 2 | T | 223 | 20.0 | 0.231 | 6.0 | LOS A | 3.1 | 25.7 | 0.49 | 0.42 | 49.4 |
| 3 | R | 6 | 20.0 | 0.231 | 14.8 | LOS B | 3.1 | 25.7 | 0.49 | 1.00 | 45.0 |
| Approac |  | 257 | 20.0 | 0.231 | 7.1 | LOSA | 3.1 | 25.7 | 0.49 | 0.46 | 48.6 |
| East: Belfast One Stop Access |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 13 | 20.0 | 0.408 | 32.7 | LOS C | 3.0 | 24.8 | 0.91 | 0.80 | 32.7 |
| 5 | T | 52 | 20.0 | 0.408 | 24.0 | LOS C | 3.0 | 24.8 | 0.91 | 0.73 | 33.2 |
| 6 | R | 47 | 20.0 | 0.408 | 32.6 | LOS C | 3.0 | 24.8 | 0.91 | 0.80 | 32.7 |
| Approac |  | 112 | 20.0 | 0.408 | 28.6 | LOS C | 3.0 | 24.8 | 0.91 | 0.77 | 32.9 |
| North: Road D1477 SB |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 64 | 20.0 | 0.115 | 14.1 | LOS B | 0.8 | 6.4 | 0.44 | 0.71 | 43.7 |
| 8 | T | 342 | 20.0 | 0.330 | 6.5 | LOSA | 5.0 | 41.2 | 0.53 | 0.46 | 48.8 |
| 9 | R | 169 | 20.0 | 0.403 | 17.0 | LOS B | 2.8 | 22.8 | 0.58 | 0.77 | 41.3 |
| Approac |  | 576 | 20.0 | 0.403 | 10.4 | LOS B | 5.0 | 41.2 | 0.53 | 0.58 | 45.8 |
| West: N4 Offramp |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 66 | 20.0 | 0.356 | 32.4 | LOS C | 2.6 | 21.7 | 0.90 | 0.78 | 32.0 |
| 11 | T | 1 | 20.0 | 0.356 | 23.6 | LOS C | 2.6 | 21.7 | 0.90 | 0.71 | 32.5 |
| 12 | R | 32 | 20.0 | 0.356 | 32.5 | LOS C | 2.6 | 21.7 | 0.90 | 0.78 | 32.0 |
| Approach |  | 99 | 20.0 | 0.356 | 32.3 | LOS C | 2.6 | 21.7 | 0.90 | 0.78 | 32.0 |
| All Vehic |  | 1043 | 20.0 | 0.408 | 13.6 | LOS B | 5.0 | 41.2 | 0.60 | 0.59 | 42.8 |

Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.

2020 PM Background+Phase 1, 2 \& 3 Development Traffic
Signals - Fixed Time Cycle Time $=60$ seconds (User-Given Cycle Time)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | $\begin{gathered} \text { Deg }_{9} \\ \text { Satn } \\ \text { v/c } \\ \hline \end{gathered}$ | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queve Distance m | Prop Queued | Effective Stop Rate perveh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB per perven kmm |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 89 | 20.0 | 0.192 | 14.7 | LOS B | 1.2 | 9.5 | 0.46 | 0.72 | 43.2 |
| 2 | T | 582 | 20.0 | 0.610 | 8.8 | LOS A | 11.3 | 93.0 | 0.70 | 0.62 | 45.7 |
| 3 | R | 9 | 20.0 | 0.610 | 17.6 | LOS B | 11.3 | 93.0 | 0.70 | 0.97 | 43.3 |
| Approac |  | 681 | 20.0 | 0.610 | 9.7 | LOS A | 11.3 | 93.0 | 0.67 | 0.64 | 45.4 |
| East: Belfast One Stop Access |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 20 | 20.0 | 0.634 | 34.2 | LOS C | 5.0 | 41.3 | 0.96 | 0.86 | 31.9 |
| 5 | T | 71 | 20.0 | 0.634 | 25.4 | LOS C | 5.0 | 41.3 | 0.96 | 0.83 | 32.2 |
| 6 | R | 82 | 20.0 | 0.634 | 34.1 | LOS C | 5.0 | 41.3 | 0.96 | 0.86 | 31.9 |
| Approac |  | 173 | 20.0 | 0.634 | 30.5 | LOS C | 5.0 | 41.3 | 0.96 | 0.85 | 32.0 |
| North: Road D1477 SB |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 83 | 20.0 | 0.153 | 14.7 | LOS B | 1.1 | 8.8 | 0.46 | 0.72 | 43.2 |
| 8 | T | 533 | 20.0 | 0.529 | 8.2 | LOS A | 9.5 | 77.8 | 0.65 | 0.57 | 46.6 |
| 9 | R | 179 | 20.0 | 0.695 | 30.7 | LOS C | 5.3 | 43.2 | 0.90 | 0.91 | 32.8 |
| Approac |  | 795 | 20.0 | 0.695 | 13.9 | LOS B | 9.5 | 77.8 | 0.69 | 0.66 | 42.2 |
| West: N4 Offramp |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 85 | 20.0 | 0.589 | 33.5 | LOS C | 4.5 | 36.9 | 0.95 | 0.83 | 31.5 |
| 11 | T | 2 | 20.0 | 0.589 | 24.7 | LOS C | 4.5 | 36.9 | 0.95 | 0.79 | 31.8 |
| 12 | R | 71 | 20.0 | 0.589 | 33.5 | LOS C | 4.5 | 36.9 | 0.95 | 0.83 | 31.5 |
| Approach |  | 158 | 20.0 | 0.589 | 33.4 | LOS C | 4.5 | 36.9 | 0.95 | 0.83 | 31.5 |
| All Vehic |  | 1806 | 20.0 | 0.695 | 15.6 | LOS B | 11.3 | 93.0 | 0.73 | 0.69 | 40.8 |

Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.

2020 AM Background+Phase 1, 2, 3 \& 4 Development Traffic
Signals - Fixed Time Cycle Time $=60$ seconds (User-Given Cycle Time)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movid | Tum | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | $\begin{aligned} & \text { Deg } \\ & \text { Satn } \\ & \text { V/c } \end{aligned}$ | Average Delay sec | Level of Service | 95\% Back of Vehicles $\qquad$ veh | Queve Distance $\qquad$ | Prop Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
|  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 41 | 20.0 | 0.083 | 13.6 | LOS B | 0.5 | 3.8 | 0.41 | 0.70 | 44.2 |
| 2 | T | 313 | 20.0 | 0.313 | 5.9 | LOSA | 4.5 | 36.6 | 0.51 | 0.44 | 49.5 |
| 3 | R | 6 | 20.0 | 0.313 | 14.7 | LOS B | 4.5 | 36.6 | 0.51 | 1.00 | 45.1 |
| Approac |  | 360 | 20.0 | 0.313 | 6.9 | LOS A | 4.5 | 36.6 | 0.50 | 0.48 | 48.7 |
| East: Belfast One Stop Access |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 13 | 20.0 | 0.451 | 33.9 | LOS C | 3.1 | 25.6 | 0.93 | 0.80 | 32.1 |
| 5 | T | 52 | 20.0 | 0.451 | 25.2 | LOS C | 3.1 | 25.6 | 0.93 | 0.74 | 32.5 |
| 6 | R | 47 | 20.0 | 0.451 | 33.8 | LOS C | 3.1 | 25.6 | 0.93 | 0.80 | 32.1 |
| Approac |  | 112 | 20.0 | 0.451 | 29.8 | LOS C | 3.1 | 25.6 | 0.93 | 0.77 | 32.3 |
| North: Road D1477 SB |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 64 | 20.0 | 0.112 | 13.7 | LOS B | 0.7 | 6.1 | 0.42 | 0.71 | 44.1 |
| 8 | T | 516 | 20.0 | 0.485 | 6.8 | LOSA | 8.3 | 68.2 | 0.59 | 0.52 | 48.2 |
| 9 | R | 169 | 20.0 | 0.431 | 17.9 | LOS B | 3.0 | 24.4 | 0.61 | 0.78 | 40.6 |
| Approac |  | 749 | 20.0 | 0.485 | 9.9 | LOS A | 8.3 | 68.2 | 0.58 | 0.59 | 45.9 |
| West: N4 Offramp |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 66 | 20.0 | 0.494 | 34.2 | LOS C | 3.4 | 27.8 | 0.94 | 0.79 | 31.1 |
| 11 | T | 1 | 20.0 | 0.494 | 25.4 | LOS C | 3.4 | 27.8 | 0.94 | 0.76 | 31.5 |
| 12 | R | 53 | 20.0 | 0.494 | 34.3 | LOS C | 3.4 | 27.8 | 0.94 | 0.79 | 31.1 |
| Approac |  | 120 | 20.0 | 0.494 | 34.2 | LOS C | 3.4 | 27.8 | 0.94 | 0.79 | 31.1 |
| All Vehic |  | 1341 | 20.0 | 0.494 | 12.9 | LOS B | 8.3 | 68.2 | 0.62 | 0.60 | 43.2 |

Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.

2020 PM Background+Phase 1, 2, 3 \& 4 Development Traffic
Signals - Fixed Time Cycle Time $=60$ seconds (Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movid | Tum | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | $\begin{aligned} & \text { Deg } \\ & \text { Sath } \\ & \text { v/c } \end{aligned}$ | Average Delay sec | Level of Service | 95\% Back of Vehicles $\qquad$ veh | Queve <br> Distance <br> m | Prop. Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 114 | 20.0 | 0.223 | 13.4 | LOS B | 1.3 | 10.7 | 0.42 | 0.72 | 44.3 |
| 2 | T | 747 | 20.0 | 0.717 | 8.2 | LOS A | 15.2 | 124.5 | 0.73 | 0.66 | 46.2 |
| 3 | R | 9 | 20.0 | 0.717 | 17.1 | LOS B | 15.2 | 124.5 | 0.73 | 0.98 | 43.9 |
| Approac |  | 871 | 20.0 | 0.717 | 9.0 | LOS A | 15.2 | 124.5 | 0.68 | 0.67 | 45.9 |
| East: Belfast One Stop Access |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 20 | 20.0 | 0.897 | 49.0 | LOS D | 6.6 | 54.0 | 1.00 | 1.08 | 26.2 |
| 5 | T | 71 | 20.0 | 0.897 | 40.3 | LOS D | 6.6 | 54.0 | 1.00 | 1.08 | 26.3 |
| 6 | R | 82 | 20.0 | 0.897 | 48.9 | LOS D | 6.6 | 54.0 | 1.00 | 1.08 | 26.2 |
| Approac |  | 173 | 20.0 | 0.897 | 45.4 | LOS D | 6.6 | 54.0 | 1.00 | 1.08 | 26.3 |
| North: Road D1477 SB |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 83 | 20.0 | 0.140 | 13.3 | LOS B | 0.9 | 7.7 | 0.41 | 0.71 | 44.4 |
| 8 | T | 629 | 20.0 | 0.576 | 6.9 | LOS A | 10.7 | 87.9 | 0.62 | 0.56 | 48.0 |
| 9 | R | 179 | 20.0 | 0.864 | 45.6 | LOS D | 7.1 | 57.9 | 0.99 | 1.09 | 26.7 |
| Approac |  | 892 | 20.0 | 0.864 | 15.3 | LOS B | 10.7 | 87.9 | 0.68 | 0.68 | 41.1 |
| West: N4 Offramp |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 85 | 20.0 | 0.905 | 50.2 | LOS D | 6.6 | 53.9 | 1.00 | 1.09 | 25.3 |
| 11 | T | 2 | 20.0 | 0.905 | 41.5 | LOS D | 6.6 | 53.9 | 1.00 | 1.09 | 25.4 |
| 12 | R | 82 | 20.0 | 0.905 | 50.3 | LOS D | 6.6 | 53.9 | 1.00 | 1.09 | 25.3 |
| Approac |  | 169 | 20.0 | 0.905 | 50.1 | LOS D | 6.6 | 53.9 | 1.00 | 1.09 | 25.3 |
| All Vehic |  | 2104 | 20.0 | 0.905 | 18.0 | LOS B | 15.2 | 124.5 | 0.73 | 0.74 | 39.1 |

Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.

2020 AM Horizon Year Traffic
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID |  | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | $\begin{aligned} & \text { Deg } \\ & \text { Sath } \\ & \text { v/c } \end{aligned}$ | Average Delay sec | Level of Service | 95\% Back of Vehicles veh | Queve <br> Distance m | Prop. Queved | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB $0.0{ }^{\text {d }}$ |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 2 | 35.0 | 0.007 | 9.4 | LOS A | 0.1 | 0.5 | 0.15 | 0.59 | 48.4 |
| 2 | T | 6 | 35.0 | 0.007 | 8.2 | LOSA | 0.1 | 0.5 | 0.15 | 0.48 | 49.5 |
| 3 | R | 1 | 35.0 | 0.007 | 9.9 | LOSA | 0.1 | 0.5 | 0.15 | 0.69 | 48.2 |
| Approa |  | 9 | 35.0 | 0.007 | 8.6 | LOSA | 0.1 | 0.5 | 0.15 | 0.53 | 49.1 |
| East: Site Access 1 |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 1 | 35.0 | 0.024 | 9.3 | LOSA | 0.1 | 0.9 | 0.07 | 0.60 | 48.6 |
| 5 | T | 2 | 35.0 | 0.024 | 8.1 | LOS A | 0.1 | 0.9 | 0.07 | 0.49 | 49.9 |
| 6 | R | 15 | 35.0 | 0.024 | 9.7 | LOSA | 0.1 | 0.9 | 0.07 | 0.68 | 48.4 |
| Approac |  | 18 | 35.0 | 0.024 | 9.5 | LOS A | 0.1 | 0.9 | 0.07 | 0.65 | 48.5 |
| North: Road D1477 |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 11 | 35.0 | 0.156 | 10.2 | LOS B | 0.7 | 6.7 | 0.25 | 0.50 | 47.9 |
| 8 | T | 3 | 35.0 | 0.156 | 8.9 | LOSA | 0.7 | 6.7 | 0.25 | 0.41 | 48.8 |
| 9 | R | 92 | 35.0 | 0.156 | 10.2 | LOS B | 0.7 | 6.7 | 0.25 | 0.62 | 47.9 |
| Approac |  | 105 | 35.0 | 0.156 | 10.1 | LOS B | 0.7 | 6.7 | 0.25 | 0.60 | 47.9 |
| West: R33 EB |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 69 | 35.0 | 0.047 | 8.2 | x | x | x | X | 0.58 | 49.8 |
| 11 | T | 1 | 35.0 | 0.002 | 8.0 | LOSA | 0.0 | 0.1 | 0.05 | 0.52 | 50.0 |
| 12 | R | 1 | 35.0 | 0.002 | 9.7 | LOSA | 0.0 | 0.1 | 0.05 | 0.71 | 48.4 |
| Approach |  | 72 | 35.0 | 0.047 | 8.3 | LOS A | 0.0 | 0.1 | 0.00 | 0.59 | 49.8 |
| All Vehic |  | 204 | 35.0 | 0.156 | 9.4 | NA | 0.7 | 6.7 | 0.14 | 0.60 | 48.7 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

2020 PM Horizon Year Traffic
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov-ID | Tum | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | $\begin{gathered} \text { Deg, } \\ \text { Satn } \\ \text { v/c } \end{gathered}$ | Average Delay Sec | Level of Service | 95\% Back c <br> Vehicles <br> veh | Queve Distance $\qquad$ | Prop. Queved | Effective Stop Rate perveh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 1 | 35.0 | 0.004 | 9.7 | LOS A | 0.0 | 0.3 | 0.22 | 0.54 | 48.1 |
| 2 | T | 3 | 35.0 | 0.004 | 8.5 | LOS A | 0.0 | 0.3 | 0.22 | 0.44 | 49.1 |
| 3 | R | 1 | 35.0 | 0.004 | 10.2 | LOS B | 0.0 | 0.3 | 0.22 | 0.67 | 48.0 |
| Approac |  | 5 | 35.0 | 0.004 | 9.1 | LOSA | 0.0 | 0.3 | 0.22 | 0.50 | 48.6 |
| East: Site Access 1 |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 1 | 35.0 | 0.024 | 9.5 | LOSA | 0.1 | 0.9 | 0.13 | 0.56 | 48.3 |
| 5 | T | 2 | 35.0 | 0.024 | 8.3 | LOS A | 0.1 | 0.9 | 0.13 | 0.46 | 49.5 |
| 6 | R | 15 | 35.0 | 0.024 | 10.0 | LOSA | 0.1 | 0.9 | 0.13 | 0.66 | 48.1 |
| Approac |  | 18 | 35.0 | 0.024 | 9.7 | LOS A | 0.1 | 0.9 | 0.13 | 0.63 | 48.3 |
| North: Road D1477 |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 21 | 35.0 | 0.120 | 10.1 | LOS B | 0.6 | 5.8 | 0.25 | 0.50 | 47.9 |
| 8 | T | 11 | 35.0 | 0.120 | 8.9 | LOS A | 0.6 | 5.8 | 0.25 | 0.41 | 48.8 |
| 9 | R | 62 | 35.0 | 0.120 | 10.1 | LOS B | 0.6 | 5.8 | 0.25 | 0.62 | 47.9 |
| Approac |  | 94 | 35.0 | 0.120 | 10.0 | LOS A | 0.6 | 5.8 | 0.25 | 0.57 | 48.0 |
| West: R33 EB |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 76 | 35.0 | 0.051 | 8.2 | x | X | x | X | 0.58 | 49.8 |
| 11 | T | 8 | 35.0 | 0.007 | 8.0 | LOSA | 0.1 | 0.5 | 0.06 | 0.54 | 50.0 |
| 12 | R | 1 | 35.0 | 0.007 | 9.7 | LOSA | 0.1 | 0.5 | 0.06 | 0.75 | 48.4 |
| Approach |  | 85 | 35.0 | 0.051 | 8.2 | LOSA | 0.1 | 0.5 | 0.01 | 0.58 | 49.8 |
| All Vehicles |  | 202 | 35.0 | 0.120 | 9.2 | NA | 0.6 | 5.8 | 0.14 | 0.58 | 48.8 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

2020 AM Background+Phase 1 Development Traffic Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of Vehicles veh | Queve Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 2 | 20.0 | 0.013 | 9.9 | LOSA | 0.1 | 0.6 | 0.32 | 0.47 | 47.7 |
| 2 | T | 6 | 20.0 | 0.013 | 8.6 | LOSA | 0.1 | 0.6 | 0.32 | 0.38 | 48.5 |
| 3 | R | 6 | 0.0 | 0.013 | 9.4 | LOSA | 0.1 | 0.6 | 0.32 | 0.64 | 47.7 |
| Approa |  | 15 | 11.4 | 0.013 | 9.2 | LOS A | 0.1 | 0.6 | 0.32 | 0.50 | 48.0 |
| East: Site Access 1 |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 4 | 0.0 | 0.003 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.68 | 49.0 |
| 5 | T | 2 | 0.0 | 0.003 | 7.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.57 | 50.2 |
| 6 | R | 78 | 0.0 | 0.103 | 8.8 | LOS A | 0.4 | 2.7 | 0.21 | 0.62 | 48.1 |
| Approac |  | 84 | 0.0 | 0.103 | 8.7 | LOS A | 0.4 | 2.7 | 0.19 | 0.62 | 48.1 |
| North: Road D1477 |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 125 | 0.0 | 0.067 | 7.6 | LOSA | 0.0 | 0.0 | 0.00 | 0.60 | 49.8 |
| 8 | T | 3 | 20.0 | 0.002 | 7.5 | LOS A | 0.0 | 0.0 | 0.00 | 0.59 | 50.4 |
| 9 | R | 92 | 20.0 | 0.143 | 9.4 | LOSA | 0.5 | 4.3 | 0.20 | 0.62 | 48.1 |
| Approac |  | 220 | 8.6 | 0.143 | 8.3 | LOSA | 0.5 | 4.3 | 0.08 | 0.61 | 49.1 |
| West: R33 EB |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 69 | 20.0 | 0.043 | 8.0 | x | x | x | x | 0.59 | 49.8 |
| 11 | T | 4 | 0.0 | 0.004 | 7.1 | LOSA | 0.0 | 0.2 | 0.07 | 0.54 | 49.8 |
| 12 | R | 1 | 20.0 | 0.004 | 9.2 | LOSA | 0.0 | 0.2 | 0.07 | 0.73 | 48.4 |
| Approach |  | 75 | 18.9 | 0.043 | 7.9 | LOS A | 0.0 | 0.2 | 0.00 | 0.59 | 49.8 |
| All Vehic |  | 394 | 8.8 | 0.143 | 8.4 | NA | 0.5 | 4.3 | 0.10 | 0.61 | 49.0 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

2020 PM Background+Phase 1 Development Traffic
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | $\begin{gathered} \text { Deg } \\ \text { Satn } \\ \text { v/c } \end{gathered}$ | Average Delay sec | Level of Service | 95\% Back of Vehicles $\qquad$ veh | Queve Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB perveh anma |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 1 | 20.0 | 0.040 | 14.2 | LOS B | 0.2 | 1.2 | 0.54 | 0.31 | 43.6 |
| 2 | T | 3 | 20.0 | 0.040 | 13.0 | LOS B | 0.2 | 1.2 | 0.54 | 0.25 | 44.3 |
| 3 | R | 17 | 0.0 | 0.040 | 13.8 | LOS B | 0.2 | 1.2 | 0.54 | 0.78 | 43.5 |
| Approac |  | 21 | 4.0 | 0.040 | 13.7 | LOS B | 0.2 | 1.2 | 0.54 | 0.67 | 43.6 |
| East Site Access 1 |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 17 | 0.0 | 0.025 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.70 | 49.0 |
| 5 | T | 31 | 0.0 | 0.025 | 7.1 | LOSA | 0.0 | 0.0 | 0.00 | 0.58 | 50.2 |
| 6 | R | 495 | 0.0 | 0.884 | 28.7 | LOS D | 16.3 | 114.0 | 0.94 | 1.54 | 33.5 |
| Approac |  | 542 | 0.0 | 0.884 | 26.8 | LOS D | 16.3 | 114.0 | 0.86 | 1.46 | 34.5 |
| North: Road D1477 |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 441 | 0.0 | 0.237 | 7.6 | LOSA | 0.0 | 0.0 | 0.00 | 0.60 | 49.8 |
| 8 | T | 11 | 20.0 | 0.006 | 7.5 | LOS A | 0.0 | 0.0 | 0.00 | 0.59 | 50.4 |
| 9 | R | 42 | 20.0 | 0.065 | 8.8 | LOSA | 0.2 | 1.7 | 0.05 | 0.64 | 48.7 |
| Approac |  | 494 | 2.1 | 0.237 | 7.7 | LOSA | 0.2 | 1.7 | 0.00 | 0.61 | 49.7 |
| West: R33 EB |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 1 | 20.0 | 0.001 | 8.0 | x | x | X | x | 0.59 | 49.8 |
| 11 | T | 92 | 0.0 | 0.048 | 7.6 | LOSA | 0.5 | 3.5 | 0.26 | 0.44 | 48.8 |
| Approach |  | 1 | 20.0 | 0.048 | 9.7 | LOSA | 0.5 | 3.5 | 0.26 | 0.69 | 48.0 |
|  |  | 94 | 0.4 | 0.048 | 7.6 | LOSA | 0.5 | 3.5 | 0.25 | 0.45 | 48.8 |
| All Vehic |  | 1151 | 1.0 | 0.884 | 16.8 | NA | 16.3 | 114.0 | 0.44 | 1.00 | 41.1 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

2020 AM Background+Phase 1 \& 2 Development Traffic Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movid | Tufn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | $\begin{gathered} \text { Deg. } \\ \text { Satn } \\ \text { v/C } \\ \hline \end{gathered}$ | Average Delay sec | Level of Service | 95\% Back of Vehicles veh | Queue <br> Distance <br> m | Prop. Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB Sec perven $\mathrm{km} / \mathrm{m}$ |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 3 | 20.0 | 0.030 | 10.6 | LOS B | 0.2 | 1.8 | 0.43 | 0.40 | 47.2 |
| 2 | T | 32 | 20.0 | 0.030 | 9.4 | LOSA | 0.2 | 1.8 | 0.43 | 0.33 | 47.9 |
| 3 | R | 6 | 0.0 | 0.030 | 10.2 | LOS B | 0.2 | 1.8 | 0.43 | 0.68 | 47.2 |
| Approa |  | 41 | 16.9 | 0.030 | 9.6 | LOSA | 0.2 | 1.8 | 0.43 | 0.39 | 47.8 |
| East: Site Access 1 |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 4 | 0.0 | 0.003 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.68 | 49.0 |
| 5 | T | 2 | 0.0 | 0.003 | 7.1 | LOSA | 0.0 | 0.0 | 0.00 | 0.57 | 50.2 |
| 6 | R | 78 | 0.0 | 0.103 | 8.8 | LOSA | 0.4 | 2.7 | 0.21 | 0.62 | 48.1 |
| Approa |  | 84 | 0.0 | 0.103 | 8.7 | LOSA | 0.4 | 2.7 | 0.19 | 0.62 | 48.1 |
| North: Road D1477 |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 125 | 0.0 | 0.067 | 7.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 49.8 |
| 8 | T | 66 | 20.0 | 0.038 | 7.5 | LOSA | 0.0 | 0.0 | 0.00 | 0.59 | 50.4 |
| 9 | R | 92 | 20.0 | 0.144 | 9.8 | LOSA | 0.6 | 4.6 | 0.26 | 0.63 | 47.8 |
| Approac |  | 283 | 11.2 | 0.144 | 8.3 | LOS A | 0.6 | 4.6 | 0.08 | 0.61 | 49.3 |
| West: R33 EB |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 69 | 20.0 | 0.043 | 8.0 | x | x | x | X | 0.59 | 49.8 |
| 11 | T | 4 | 0.0 | 0.005 | 7.1 | LOSA | 0.0 | 0.2 | 0.07 | 0.53 | 49.8 |
| 12 | R | 2 | 20.0 | 0.005 | 9.2 | LOSA | 0.0 | 0.2 | 0.07 | 0.72 | 48.4 |
| Approach |  | 76 | 18.9 | 0.043 | 7.9 | LOS A | 0.0 | 0.2 | 0.01 | 0.59 | 49.7 |
| All Vehic |  | 484 | 10.9 | 0.144 | 8.4 | NA | 0.6 | 4.6 | 0.12 | 0.59 | 49.0 |

X: Not applicable for Continuous movement
Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

Processed: 26 September 2016 03:14:25 PM SIDRA INTERSECTION 5.1.2.1953
Project: Z:L200001-Witbankl20744.R_Belfast Mall 17 - Sidras\Phasing TIAL20744_Belfast_Sidra.sip 8000993, WSP SA CIVIL \& STRUCTURAL ENGINEERS (PTY) LTD, LIMITED

2020 PM Background+Phase 1 \& 2 Development Traffic Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | $\begin{aligned} & \text { Deg, } \\ & \text { Sath } \\ & \text { V/C } \end{aligned}$ | Average Delay sec | Level of Service | 95\% Back o <br> Vehicles veh | of Queue Distance m | Prop. Queved | Effective Stop Rate per veh | Averege Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB per men kmin |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 3 | 20.0 | 0.080 | 15.2 | LOS C | 0.8 | 6.1 | 0.63 | 0.26 | 43.1 |
| 2 | T | 66 | 20.0 | 0.080 | 13.9 | LOS B | 0.8 | 6.1 | 0.63 | 0.21 | 43.7 |
| 3 | R | 17 | 0.0 | 0.080 | 14.7 | LOS B | 0.8 | 6.1 | 0.63 | 0.85 | 43.1 |
| Approac |  | 86 | 16.1 | 0.080 | 14.1 | LOS B | 0.8 | 6.1 | 0.63 | 0.34 | 43.6 |
| East: Site Access 1 |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 17 | 0.0 | 0.025 | 8.2 | LOSA | 0.0 | 0.0 | 0.00 | 0.70 | 49.0 |
| 5 | T | 31 | 0.0 | 0.025 | 7.1 | LOSA | 0.0 | 0.0 | 0.00 | 0.58 | 50.2 |
| 6 | R | 495 | 0.0 | 0.884 | 28.7 | LOS D | 16.3 | 114.0 | 0.94 | 1.54 | 33.5 |
| Approac |  | 542 | 0.0 | 0.884 | 26.8 | LOS D | 16.3 | 114.0 | 0.86 | 1.46 | 34.5 |
| North: Road D1477 |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 441 | 0.0 | 0.237 | 7.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.60 | 49.8 |
| 8 | T | 33 | 20.0 | 0.019 | 7.5 | LOSA | 0.0 | 0.0 | 0.00 | 0.59 | 50.4 |
| 9 | R | 42 | 20.0 | 0.066 | 9.7 | LOSA | 0.2 | 2.0 | 0.25 | 0.62 | 47.9 |
| Approac |  | 516 | 2.9 | 0.237 | 7.7 | LOS A | 0.2 | 2.0 | 0.02 | 0.60 | 49.7 |
| West: R33 EB |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 1 | 20.0 | 0.001 | 8.0 | $x$ | X | X | x | 0.59 | 49.8 |
| 11 | T | 92 | 0.0 | 0.048 | 7.6 | LOSA | 0.5 | 3.5 | 0.26 | 0.44 | 48.8 |
| 12 | R | 1 | 20.0 | 0.048 | 9.7 | LOS A | 0.5 | 3.5 | 0.26 | 0.69 | 48.0 |
| Approac |  | 94 | 0.4 | 0.048 | 7.6 | LOS A | 0.5 | 3.5 | 0.26 | 0.44 | 48.8 |
| All Vehic |  | 1238 | 2.4 | 0.884 | 16.5 | NA | 16.3 | 114.0 | 0.45 | 0.95 | 41.3 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.
+Phase 1,2 \& 3 Development
Traffic
2020 AM Background+Phase 1,2 \& 3 Development Traffic Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | $\begin{gathered} \text { Deg, } \\ \text { Satn } \\ \text { V/C } \end{gathered}$ | Average Delay $\qquad$ sec | Level of Service | 95\% Back of Vehicles $\qquad$ veh | f Queue Distance $\qquad$ | Prop. Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
|  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 3 | 20.0 | 0.040 | 11.7 | LOS B | 0.3 | 2.4 | 0.49 | 0.35 | 46.1 |
| 2 | T | 32 | 20.0 | 0.040 | 10.5 | LOS B | 0.3 | 2.4 | 0.49 | 0.29 | 46.9 |
| 3 | R | 12 | 0.0 | 0.040 | 11.3 | LOS B | 0.3 | 2.4 | 0.49 | 0.72 | 46.1 |
| Approac |  | 46 | 15.0 | 0.040 | 10.8 | LOS B | 0.3 | 2.4 | 0.49 | 0.40 | 46.7 |
| East: Site Access 1 |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 7 | 0.0 | 0.006 | 8.2 | LOSA | 0.0 | 0.0 | 0.00 | 0.69 | 49.0 |
| 5 | T | 4 | 0.0 | 0.006 | 7.1 | LOSA | 0.0 | 0.0 | 0.00 | 0.57 | 50.2 |
| 6 | R | 141 | 0.0 | 0.185 | 9.4 | LOSA | 0.8 | 5.6 | 0.31 | 0.64 | 47.6 |
| Approac |  | 153 | 0.0 | 0.185 | 9.3 | LOSA | 0.8 | 5.6 | 0.29 | 0.64 | 47.8 |
| North: Road D1477 |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 216 | 0.0 | 0.116 | 7.6 | LOSA | 0.0 | 0.0 | 0.00 | 0.60 | 49.8 |
| 8 | T | 66 | 20.0 | 0.038 | 7.5 | LOSA | 0.0 | 0.0 | 0.00 | 0.59 | 50.4 |
| 9 | R | 92 | 20.0 | 0.144 | 9.8 | LOSA | 0.6 | 4.6 | 0.26 | 0.63 | 47.8 |
| Approac |  | 374 | 8.5 | 0.144 | 8.1 | LOSA | 0.6 | 4.6 | 0.06 | 0.61 | 49.4 |
| West: R33 EB |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 69 | 20.0 | 0.043 | 8.0 | $x$ | x | x | $x$ | 0.59 | 49.8 |
| 11 | T | 7 | 0.0 | 0.007 | 7.2 | LOS A | 0.0 | 0.3 | 0.10 | 0.52 | 49.7 |
| 12 | R | 2 | 20.0 | 0.007 | 9.3 | LOSA | 0.0 | 0.3 | 0.10 | 0.72 | 48.3 |
| Approac |  | 79 | 18.1 | 0.043 | 7.9 | LOSA | 0.0 | 0.3 | 0.01 | 0.59 | 49.7 |
| All Vehic |  | 652 | 8.1 | 0.185 | 8.6 | NA | 0.8 | 5.6 | 0.14 | 0.60 | 48.8 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

2020 PM Background+Phase 1,2 \& 3 Development Traffic
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Tuin | Demand Flow $\mathrm{veh} / \mathrm{h}$ | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg Satn v/c | Average Delay sec | Level of Service | 95\% Back of <br> Vehicles <br> veh | Queue Distance m | Prop Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB sec mim ker men kmin |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 3 | 20.0 | 0.100 | 17.2 | LOS C | 1.0 | 7.5 | 0.66 | 0.24 | 41.4 |
| 2 | T | 66 | 20.0 | 0.100 | 16.0 | LOS C | 1.0 | 7.5 | 0.66 | 0.20 | 42.0 |
| 3 | R | 22 | 0.0 | 0.100 | 16.7 | LOS C | 1.0 | 7.5 | 0.66 | 0.91 | 41.4 |
| Approac |  | 92 | 15.2 | 0.100 | 16.2 | LOS C | 1.0 | 7.5 | 0.66 | 0.37 | 41.8 |
| East: Site Access 1 |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 22 | 0.0 | 0.077 | 8.2 | LOSA | 0.0 | 0.0 | 0.00 | 0.68 | 49.0 |
| 5 | T | 123 | 0.0 | 0.077 | 7.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.57 | 50.2 |
| 6 | R | 515 | 0.0 | $1.000{ }^{3}$ | 43.1 | LOSE | 21.3 | 149.1 | 1.00 | 1.76 | 27.4 |
| Approac |  | 660 | 0.0 | 1.000 | 35.2 | LOSE | 21.3 | 149.1 | 0.78 | 1.50 | 30.5 |
| North: Road D1477 |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 525 | 0.0 | 0.283 | 7.6 | LOSA | 0.0 | 0.0 | 0.00 | 0.60 | 49.8 |
| 8 | T | 33 | 20.0 | 0.019 | 7.5 | LOS A | 0.0 | 0.0 | 0.00 | 0.59 | 50.4 |
| 9 | R | 41 | 20.0 | 0.065 | 9.7 | LOSA | 0.2 | 1.9 | 0.25 | 0.62 | 47.9 |
| Approac |  | 599 | 2.5 | 0.283 | 7.7 | LOS A | 0.2 | 1.9 | 0.02 | 0.60 | 49.7 |
| West: R33 EB |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 1 | 20.0 | 0.001 | 8.0 | X | x | X | x | 0.59 | 49.8 |
| 11 | T | 99 | 0.0 | 0.052 | 7.7 | LOSA | 0.5 | 3.8 | 0.29 | 0.42 | 48.7 |
| 12 | R | 1 | 20.0 | 0.052 | 9.8 | LOSA | 0.5 | 3.8 | 0.29 | 0.69 | 47.9 |
| Approac |  | 101 | 0.4 | 0.052 | 7.8 | LOSA | 0.5 | 3.8 | 0.29 | 0.43 | 48.7 |
| All Vehic |  | 1452 | 2.0 | 1.000 | 20.8 | NA | 21.3 | 149.1 | 0.42 | 0.98 | 38.3 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.
$3 x=1.00$ due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions,

2020 AM Background+Phase 1,2,3\&4 Development Traffic Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Tum | Demand Fiow vehih | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | $\begin{array}{r} \text { Deg, } \\ \text { Sath } \\ \text { V/c } \end{array}$ | Average Delay sec | Level of Service | 95\% Back of Vehicles veh | f Queue Distance $\qquad$ | Prop. Queved | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB pee men kion |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 3 | 20.0 | 0.072 | 15.1 | LOS C | 0.5 | 3.9 | 0.58 | 0.29 | 43.0 |
| 2 | T | 32 | 20.0 | 0.072 | 13.8 | LOS B | 0.5 | 3.9 | 0.58 | 0.24 | 43.7 |
| 3 | R | 22 | 0.0 | 0.072 | 14.6 | LOS B | 0.5 | 3.9 | 0.58 | 0.83 | 43.0 |
| Approa |  | 57 | 12.2 | 0.072 | 14.2 | LOS B | 0.5 | 3.9 | 0.58 | 0.47 | 43.4 |
| East: Site Access 1 |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 14 | 0.0 | 0.012 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.69 | 49.0 |
| 5 | T | 8 | 0.0 | 0.012 | 7.1 | LOSA | 0.0 | 0.0 | 0.00 | 0.57 | 50.2 |
| 6 | R | 244 | 0.0 | 0.375 | 11.6 | LOS B | 2.1 | 14.6 | 0.50 | 0.77 | 45.5 |
| Approa |  | 266 | 0.0 | 0.375 | 11.3 | LOS B | 2.1 | 14.6 | 0.46 | 0.76 | 45.8 |
| North: Road D1477 |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 411 | 0.0 | 0.221 | 7.6 | LOSA | 0.0 | 0.0 | 0.00 | 0.60 | 49.8 |
| 8 | T | 66 | 20.0 | 0.038 | 7.5 | LOS A | 0.0 | 0.0 | 0.00 | 0.59 | 50.4 |
| 9 | R | 92 | 20.0 | 0.144 | 9.8 | LOSA | 0.6 | 4.6 | 0.26 | 0.63 | 47.8 |
| Approac |  | 568 | 5.6 | 0.221 | 7.9 | LOSA | 0.6 | 4.6 | 0.04 | 0.61 | 49.5 |
| West: R33 EB |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 69 | 20.0 | 0.043 | 8.0 | X | x | X | $x$ | 0.59 | 49.8 |
| 11 | T | 14 | 0.0 | 0.010 | 7.3 | LOSA | 0.1 | 0.6 | 0.15 | 0.50 | 49.4 |
| 12 | R | 2 | 20.0 | 0.010 | 9.4 | LOS A | 0.1 | 0.6 | 0.15 | 0.71 | 48.2 |
| Approac |  | 85 | 16.8 | 0.043 | 7.9 | LOS A | 0.1 | 0.6 | 0.03 | 0.58 | 49.7 |
| All Vehi |  | 977 | 5.4 | 0.375 | 9.2 | NA | 2.1 | 14.6 | 0.19 | 0.64 | 48.1 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

2020 PM Background+Phase 1,2,3\&4 Development Traffic
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow vehih | $\begin{aligned} & \text { HV } \\ & \text { \% } \end{aligned}$ | Deg Satn v/c | Average Delay sec | Level of Service | 95\% Back of Vehicles veh | Queve Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 1 | 20.0 | 0.130 | 20.6 | LOS C | 1.2 | 9.5 | 0.76 | 0.17 | 38.9 |
| 2 | T | 66 | 20.0 | 0.130 | 19.3 | LOS C | 1.2 | 9.5 | 0.76 | 0.14 | 39.4 |
| 3 | R | 27 | 0.0 | 0.130 | 20.1 | LOS C | 1.2 | 9.5 | 0.76 | 0.93 | 38.9 |
| Approac |  | 95 | 14.2 | 0.130 | 19.6 | LOS C | 1.2 | 9.5 | 0.76 | 0.37 | 39.2 |
| East: Site Access 1 |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 33 | 0.0 | 0.215 | 8.2 | LOSA | 0.0 | 0.0 | 0.00 | 0.68 | 49.0 |
| 5 | T | 369 | 0.0 | 0.215 | 7.1 | LOSA | 0.0 | 0.0 | 0.00 | 0.56 | 50.2 |
| 6 | R | 464 | 0.0 | $1.000{ }^{3}$ | 50.5 | LOS F | 21.3 | 149.1 | 1.00 | 1.91 | 25.1 |
| Approac |  | 866 | 0.0 | 1.000 | 30.4 | LOS D | 21.3 | 149.1 | 0.54 | 1.29 | 32.7 |
| North: Road D1477 |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 635 | 0.0 | 0.342 | 7.6 | LOSA | 0.0 | 0.0 | 0.00 | 0.60 | 49.8 |
| 8 | T | 33 | 20.0 | 0.019 | 7.5 | LOS A | 0.0 | 0.0 | 0.00 | 0.59 | 50.4 |
| 9 | R | 41 | 20.0 | 0.065 | 9.7 | LOS A | 0.2 | 1.9 | 0.24 | 0.62 | 47.9 |
| Approac |  | 708 | 2.1 | 0.342 | 7.7 | LOS A | 0.2 | 1.9 | 0.01 | 0.60 | 49.7 |
| West: R33 EB |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 1 | 20.0 | 0.001 | 8.0 | X | x | X | X | 0.59 | 49.8 |
| 11 | T | 103 | 0.0 | 0.054 | 7.9 | LOS A | 0.6 | 4.1 | 0.33 | 0.40 | 48.4 |
| 12 | R | 1 | 20.0 | 0.054 | 10.0 | LOS B | 0.6 | 4.1 | 0.33 | 0.69 | 47.8 |
| Approac |  | 105 | 0.4 | 0.054 | 8.0 | LOSA | 0.6 | 4.1 | 0.33 | 0.40 | 48.5 |
| All Vehic |  | 1775 | 1.6 | 1.000 | 19.4 | NA | 21.3 | 149.1 | 0.33 | 0.91 | 39.2 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.
$3 x=1.00$ due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

2020 AM Background+Phase 2 Development Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movid Tum | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | $\begin{gathered} \text { Deg. } \\ \text { Sath } \\ \text { v/c } \end{gathered}$ | Average Delay sec | Level of Service | 95\% Back of <br> Vehicles <br> veh | Queue Distance m | Prop. Queued | Effective Stop Rate perveh | Average Speed $\mathrm{km} / \mathrm{h}$ |
|  |  |  |  |  |  |  |  |  |  |  |
| 2 T | 16 | 20.0 | 0.012 | 0.4 | LOSA | 0.1 | 0.5 | 0.21 | 0.00 | 55.9 |
| 3 R | 3 | 20.0 | 0.012 | 9.5 | LOSA | 0.1 | 0.5 | 0.21 | 0.96 | 48.7 |
| Approach | 19 | 20.0 | 0.012 | 1.9 | NA | 0.1 | 0.5 | 0.21 | 0.16 | 54.6 |
| East: Site Access 2 |  |  |  |  |  |  |  |  |  |  |
| 4 L | 1 | 20.0 | 0.039 | 12.4 | LOS B | 0.1 | 1.2 | 0.21 | 0.82 | 46.0 |
| 6 R | 26 | 20.0 | 0.039 | 12.3 | LOS B | 0.1 | 1.2 | 0.21 | 0.88 | 46.2 |
| Approach | 27 | 20.0 | 0.039 | 12.3 | LOS B | 0.1 | 1.2 | 0.21 | 0.88 | 46.2 |
| North: Road D1477 SB |  |  |  |  |  |  |  |  |  |  |
| 7 L | 64 | 20.0 | 0.044 | 8.8 | LOSA | 0.0 | 0.0 | 0.00 | 0.69 | 49.0 |
| 8 T | 7 | 20.0 | 0.044 | 0.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 72 | 20.0 | 0.044 | 7.9 | NA | 0.0 | 0.0 | 0.00 | 0.62 | 49.9 |
| All Vehicles | 118 | 20.0 | 0.044 | 7.9 | NA | 0.1 | 1.2 | 0.08 | 0.61 | 49.7 |

Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

2020 PM Background+Phase 2 Development Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg. Sato v/c | Average Delay sec | Level of Service | 95\% Back o Vehicles veh | Queue <br> Distance <br> - m | Prop. Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB sec veh marven km/n |  |  |  |  |  |  |  |  |  |  |  |
| 2 | T | 22 | 20.0 | 0.014 | 0.3 | LOSA | 0.1 | 0.6 | 0.17 | 0.00 | 56.7 |
| 3 | R | 1 | 20.0 | 0.014 | 9.4 | LOSA | 0.1 | 0.6 | 0.17 | 1.08 | 48.8 |
| Approac |  | 23 | 20.0 | 0.014 | 0.7 | NA | 0.1 | 0.6 | 0.17 | 0.05 | 56.3 |
| East: Site Access 2 |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 3 | 20.0 | 0.097 | 12.5 | LOS B | 0.4 | 3.2 | 0.23 | 0.82 | 45.9 |
| 6 | R | 65 | 20.0 | 0.097 | 12.4 | LOS B | 0.4 | 3.2 | 0.23 | 0.89 | 46.1 |
| Approac |  | 68 | 20.0 | 0.097 | 12.4 | LOS B | 0.4 | 3.2 | 0.23 | 0.88 | 46.1 |
| North: Road D1477 SB |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 22 | 20.0 | 0.029 | 8.8 | LOSA | 0.0 | 0.0 | 0.00 | 0.86 | 49.0 |
| 8 | T | 27 | 20.0 | 0.029 | 0.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approac |  | 49 | 20.0 | 0.029 | 3.9 | NA | 0.0 | 0.0 | 0.00 | 0.38 | 54.5 |
| All Vehic |  | 141 | 20.0 | 0.097 | 7.5 | NA | 0.4 | 3.2 | 0.14 | 0.57 | 50.3 |

Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

2020 AM Background+Phase 2 \& 3 Development Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow $\mathrm{veh} / \mathrm{h}$ | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | $\begin{aligned} & \text { Deg } \\ & \text { Satn } \\ & \text { v/c } \end{aligned}$ | Average Delay $\qquad$ sec | Level of Service | 95\% Back Vehicles $\qquad$ veh | f Queve Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB |  |  |  |  |  |  |  |  |  |  |  |
| 2 | T | 20 | 20.0 | 0.014 | 0.4 | LOSA | 0.1 | 0.6 | 0.22 | 0.00 | 55.8 |
| 3 | R | 3 | 20.0 | 0.014 | 9.5 | LOSA | 0.1 | 0.6 | 0.22 | 0.98 | 48.8 |
| Approac |  | 23 | 20.0 | 0.014 | 1.6 | NA | 0.1 | 0.6 | 0.22 | 0.13 | 54.7 |
| East: Site Access 2 |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 1 | 20.0 | 0.039 | 12.6 | LOS B | 0.2 | 1.2 | 0.23 | 0.81 | 45.9 |
| 6 | R | 26 | 20.0 | 0.039 | 12.4 | LOS B | 0.2 | 1.2 | 0.23 | 0.88 | 46.1 |
| Approac |  | 27 | 20.0 | 0.039 | 12.4 | LOS B | 0.2 | 1.2 | 0.23 | 0.88 | 46.1 |
| North: Road D1477 SB |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 64 | 20.0 | 0.046 | 8.8 | LOS A | 0.0 | 0.0 | 0.00 | 0.71 | 49.0 |
| 8 | T | 12 | 20.0 | 0.046 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approac |  | 76 | 20.0 | 0.046 | 7.4 | NA | 0.0 | 0.0 | 0.00 | 0.60 | 50.4 |
| All Vehic |  | 126 | 20.0 | 0.046 | 7.4 | NA | 0.2 | 1.2 | 0.09 | 0.57 | 50.1 |

Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

2020 PM Background+Phase 2 \& 3 Development Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mavid |  | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | $\begin{aligned} & \text { Deg, } \\ & \text { Sath } \\ & \text { v/c } \end{aligned}$ | Average Delay sec | Level of Service | 95\% Back c <br> Vehicles <br> veh | Queve Distance m | Prop. Queued | Effective Stop Rate perveh | Average Speed $\mathrm{km} / \mathrm{h}$ |
|  |  |  |  |  |  |  |  |  |  |  |  |
| 2 | T | 25 | 20.0 | 0.016 | 0.3 | LOSA | 0.1 | 0.7 | 0.19 | 0.00 | 56.5 |
| 3 | R | 1 | 20.0 | 0.016 | 9.4 | LOSA | 0.1 | 0.7 | 0.19 | 1.08 | 48.9 |
| Approac |  | 26 | 20.0 | 0.016 | 0.7 | NA | 0.1 | 0.7 | 0.19 | 0.04 | 56.1 |
| East: Site Access 2 |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 3 | 20.0 | 0.099 | 12.7 | LOS B | 0.4 | 3.3 | 0.25 | 0.81 | 45.8 |
| 6 | R | 65 | 20.0 | 0.099 | 12.5 | LOS B | 0.4 | 3.3 | 0.25 | 0.88 | 46.0 |
| Approac |  | 68 | 20.0 | 0.099 | 12.5 | LOS B | 0.4 | 3.3 | 0.25 | 0.88 | 46.0 |
| North: Road D1477 SB |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 22 | 20.0 | 0.033 | 8.8 | LOSA | 0.0 | 0.0 | 0.00 | 0.88 | 49.0 |
| 8 | T | 34 | 20.0 | 0.033 | 0.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach |  | 56 | 20.0 | 0.033 | 3.5 | NA | 0.0 | 0.0 | 0.00 | 0.35 | 55.1 |
| All Vehicles |  | 151 | 20.0 | 0.099 | 7.1 | NA | 0.4 | 3.3 | 0.15 | 0.54 | 50.7 |

Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

2020 AM Background+Phase 2, 3 \& 4 Development Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg Satn V/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB |  |  |  |  |  |  |  |  |  |  |  |
| 2 | T | 31 | 20.0 | 0.020 | 0.4 | LOSA | 0.1 | 1.0 | 0.23 | 0.00 | 55.6 |
| 3 | R | 3 | 20.0 | 0.020 | 9.6 | LOSA | 0.1 | 1.0 | 0.23 | 1.01 | 48.8 |
| Approac |  | 34 | 20.0 | 0.020 | 1.3 | NA | 0.1 | 1.0 | 0.23 | 0.09 | 54.9 |
| East: Site Access 2 |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 1 | 20.0 | 0.040 | 12.8 | LOS B | 0.2 | 1.3 | 0.26 | 0.80 | 45.7 |
| 6 | $R$ | 26 | 20.0 | 0.040 | 12.6 | LOS B | 0.2 | 1.3 | 0.26 | 0.87 | 45.9 |
| Approac |  | 27 | 20.0 | 0.040 | 12.6 | LOS B | 0.2 | 1.3 | 0.26 | 0.87 | 45.9 |
| North: Road D1477 SB |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 64 | 20.0 | 0.049 | 8.8 | LOS A | 0.0 | 0.0 | 0.00 | 0.72 | 49.0 |
| 8 | T | 17 | 20.0 | 0.049 | 0.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach |  | 81 | 20.0 | 0.049 | 6.9 | NA | 0.0 | 0.0 | 0.00 | 0.57 | 50.9 |
| All Vehicles |  | 142 | 20.0 | 0.049 | 6.7 | NA | 0.2 | 1.3 | 0.10 | 0.52 | 50.7 |

Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

2020 PM Background+Phase 2, 3 \& 4 Development Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movid | Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | $\begin{gathered} \text { Deg, } \\ \text { Sath } \\ \text { V/c } \end{gathered}$ | Average Delay sec | Level of Service | 95\% Back o <br> Vehicles <br> veh | queve Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Road D1477 NB ${ }^{\text {a }}$ |  |  |  |  |  |  |  |  |  |  |  |
| 2 | T | 32 | 20.0 | 0.019 | 0.3 | LOSA | 0.1 | 0.9 | 0.20 | 0.00 | 56.1 |
| 3 | R | 1 | 20.0 | 0.019 | 9.5 | LOSA | 0.1 | 0.9 | 0.20 | 1.07 | 48.9 |
| Approac |  | 33 | 20.0 | 0.019 | 0.6 | NA | 0.1 | 0.9 | 0.20 | 0.03 | 55.9 |
| East: Site Access 2 |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 3 | 20.0 | 0.102 | 12.9 | LOS B | 0.4 | 3.4 | 0.28 | 0.80 | 45.6 |
|  | R | 65 | 20.0 | 0.102 | 12.8 | LOS B | 0.4 | 3.4 | 0.28 | 0.88 | 45.8 |
| Approach |  | 68 | 20.0 | 0.102 | 12.8 | LOS B | 0.4 | 3.4 | 0.28 | 0.88 | 45.8 |
| North: Road D1477 SB |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 22 | 20.0 | 0.039 | 8.8 | LOSA | 0.0 | 0.0 | 0.00 | 0.91 | 49.0 |
| 8 | T | 43 | 20.0 | 0.039 | 0.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approac |  | 65 | 20.0 | 0.039 | 3.0 | NA | 0.0 | 0.0 | 0.00 | 0.31 | 55.7 |
| All Vehic |  | 166 | 20.0 | 0.102 | 6.5 | NA | 0.4 | 3.4 | 0.16 | 0.49 | 51.2 |

Level of Service (LOS) Method: Delay (HCM 2000).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

