## APPENDIX D ROUTE POSITION INFORMATION

KANTEY \& TEMPLER CONSULTING ENGINEERS

ESTABLISHED 1953

## PROPOSED FILLING STATION

## ON

THE FARM 751 REGISTRATION DIVISION I.Q. PROVINCE OF GAUTENG

BOLENG (PTY) LTD

## TRAFFIC AND ACCESS ASSESSMENT

JUNE 2021

K\&T PROJECT REFERENCE: 8101

REVISION 0

KANTEY \& TEMPLER
CONSULTING ENGINEERS
ESTABLISHED 1953

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## THE FARM 751

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## 1. BACKGROUND

Kantey \& Templer Consulting Engineers (K\&T) were appointed by Boleng (Pty) Ltd (referred to herein as "the client") to obtain approval of the proposed access to a filling station which will be situated on The Farm 751 Registration Division I.Q, province of Gauteng.

The previous zoning of the site allowed for commercial and residential land-uses. In 2008, PD Naidoo \& Associates (Pty) Ltd prepared a Traffic Impact Assessment (TIA) in support of a proposed warehouse development on Aeroton Extensions 32, 33 and the site. The TIA was approved by the Johannesburg Roads Agency (JRA) in 2010. A proposed full access off Southgate Road to the site was approved by the JRA. In 2013, Moyeni Professional Engineering (MPE) prepared an Addendum Traffic Impact Assessment which at the time addressed a residential development (worst-case trip generator) and a proposed additional access via the existing access point to SAB which is situated to the east of the site.

Since then, Shell SA has purchased the property to develop a filling station. FJC Consulting have been appointed as the Town Planners to undertake the proposed rezoning of the site which will permit rights for a filling station. Details of the proposed rezoning application was not available at the time of preparing this study since approval of the application was still pending.

## 2. PURPOSE

The purpose of this report is to obtain approval of the proposed accesses to the filling station development on The Farm 751 Registration Division I.Q, province of Gauteng, hereafter referred to as "the site".

## 3. THE SITE

### 3.1 LOCALITY

The site falls within jurisdiction of the City of Johannesburg (COJ) Metropolitan Municipality and Johannesburg Roads Agency (JRA).

The locality and aerial plans of the site are shown on figures 8101 - TIA - FIG 1 and 8101 - TIA - FIG 2 in Annexure A.

The land use surrounding the site is a mix of Residential and Commercial. The site boundaries are described in Table 1 below.

Table 1: Site Boundaries

| Boundary | Boundary Description |
| :--- | :--- |
| North | N12 road reserve |
| East | Baragwanath Extension 1 |
| South | Chris Hani Road (M68) |
| West | Aerodrome Road |

Southgate Road passes through the site on the north. The site area for the filling station will therefore be approximately $12885 \mathrm{~m}^{2}$ in extent.

Two (2) accesses are proposed to the site. A marginal (left-in and left-out only) access is proposed off Chris Hani Road (Access 1) and a full access is proposed of Southgate Road (Access 2).

### 3.2 PROPOSED DEVELOPMENT

The intention is to develop a filling station with a convenience store and fast food with a floor area limited to $30 \%$ of the total built floor area or $150 \mathrm{~m}^{2}$, whichever is the lesser as per COJ Land-Use Scheme (2018).

## 4. ROAD NETWORK

### 4.1 EXISTING ROAD NETWORK

The classification of the roads in the study area was obtained from the RISFSA Road Master Plan ${ }^{(2)}$.
The filling station will be bound by Chris Hani Road to the south which is a class 2 road. The road is currently a dual carriageway with three lanes per direction and a posted speed limit of $70 \mathrm{~km} / \mathrm{h}$.

Southgate Road will border the filling station to the north. Southgate Road is a class 5 road with a speed limit of $60 \mathrm{~km} / \mathrm{h}$.

Aerodrome Road is a class 3 road with a posted speed limit of $60 \mathrm{~km} / \mathrm{h}$ which will border the filling station to the west.

The aforementioned roads fall within the jurisdiction of the JRA. The site is not affected by any future roads planning.

The aerial plan showing the existing road network is included in Annexure A as figure 8109 - TIA FIG 2.

## 5. LATENT DEVELOPMENTS

A meeting was held with Mr P Peska from the JRA on 3 June 2021 where the proposed conceptual access drawing was discussed. Future developments in the area were also discussed. The proposed road upgrades for Aeroton Extensions 53 to 56 at the intersections of Chris Hani Road with Aerodrome Road and Chris Hani Road with Southgate Road were obtained from Moyeni Professional Engineering (MPE). The latent road upgrades (as shown in Annexure B) were considered to be in place before the filling station is developed.

## 6. SITE ACCESSES

The filling station will have a marginal (left-in, left out only) access off Chris Hani Road and a full access off Southgate Road. The proposed accesses are indicated in the accompanying drawing 8101 - TIA FIG 3 attached in Annexure A of this report.

### 6.1 DESIGN CRITERIA

The Guidelines for Access to Filling Stations in the Greater Pretoria Area ${ }^{(1)}$ by ITS (March 2001) were followed in the conceptual design. It was confirmed in the meeting with JRA that the aforementioned guidelines are also used for designing filling station accesses off JRA roads as well.

### 6.2 CLASSIFICATION OF ROADS AND DESIGN SPEED

Chris Hani Road is a class 2 road with a posted speed limit of $70 \mathrm{~km} / \mathrm{h}$ and Southgate Road is class 5 road with a posted speed limit of $60 \mathrm{~km} / \mathrm{h}$. The ITS guidelines uses a Level Of Access (LOA) classification when designing filling station accesses. Chris Hani Road will have a LOA of 2 and Southgate Road will have a LOA of 9. It also supported in guidelines that the posted speed limit should be used as the design speed of the filling station accesses.

### 6.3 MARGINAL ACCESS OFF CHRIS HANI ROAD (ACCESS 1)

A median barrier is recommended for roads with LOA 1,2,3,4 and 5 . It should be noted that Chris Hani Road is a dual carriageway road which is already separated by a median barrier. According to the guidelines, the criteria for determining a filling station access downstream of an intersection is based on the deceleration length and shoulder sight distance. The marginal access will be located to east of the signalised Chris Hani Road / Aerodrome Road intersection.

### 6.3.3 SHOULDER SIGHT DISTANCE

According to Table 8.3 from the ITS Guidelines, the minimum shoulder sight distance required on a road with a design speed of $70 \mathrm{~km} / \mathrm{h}$ is 138 m which is considered adequate in this case.

### 6.3.4 ACCELERATION \& DECELERATION LANES

Provision of deceleration lanes at access points to reduce speed differential is an acceptable principle to reduce conflict A detailed survey for the relevant longitudinal section of Chris Hani Road was not available at the time of this study, therefore the typical standard was used for the design. Table 9.3 from the ITS guidelines was used to determine a taper length of 40 m for the deceleration lane. It is however recommended in the guidelines that a 35 m taper and 5 m deceleration lane should be provided.

The detailed design of the filling access will address the impact of the gradient on the site access once the surveys have been obtained. The basic access distance requirements will be adjusted if necessary, according to Table 10.1 from the guidelines.

It should be noted that adjusting the deceleration lane distance will be possible, if necessary.

### 6.4 FULL ACCESS OFF SOUTHGATE ROAD (ACCESS 2)

As mentioned in Section 1, the full access off Southgate Road was previously approved. It is proposed that the access should be located 50 m east from the Aerodrome Road / Southgate Road intersection. The queue length on Southgate Road was taken into consideration when determining the position of the access. This is discussed in detail in the following Section 10.

### 6.5 GEOMETRIC DESIGN

The design of the marginal access will conform to the requirements as shown on the typical drawings. The typical drawing FIG: C3 contained in ITS ${ }^{(1)}$ guidelines are included herewith in Annexure C of this report were used.

## 7. SITE LAYOUT

A Site Development Plan (SDP) was not available at the time of this study. A detailed SDP still needs to be prepared by an Architect and should be reviewed by the Traffic Engineer for compliance with the following requirements of the ITS Guidelines for access to filling station(1).

### 7.1 HEAVY VEHICLES

Sufficient manoeuvring and loading space should be provided to accommodate trucks at the entry and within the site. The entry radii as per the drawing 8101 - TIA - FIG 3 attached in Annexure A should be sufficient to accommodate heavy vehicles at the access. The SDP should be evaluated, using AutoTurn (or similar software) to ensure that truck turning movements can be accommodated on the proposed layout.

### 7.2 PARKING

Parking should be provided in terms of the relevant Town-Planning Scheme. Details of the parking requirements will be addressed as part of the Site Development Plan (SDP) submission.

## 8. STUDY AREA INVESTIGATED

The traffic engineer previously conducted a site visit on 11 March 2021 to investigate the road infrastructure and intersections. The relevant photographs of the study area are shown in Annexure E.

## 9. TRAFFIC VOLUMES

### 9.1 TRAFFIC DATA

Classified traffic count surveys were conducted by Unitraf on 13 April 2021 and 20 April 2021. 24 Hour traffic counts were conducted at the Chris Hani Road / Aerodrome Road intersection (Intersection 1). Weekday AM and PM peak period traffic counts were conducted at the Aerodrome Road / Southgate Road intersection (Intersection 2).

The filling station is proposed to have a marginal access off Chris Hani Road and a full access off Southgate Road.

2018 traffic count data for Chris Hani Road / Aerodrome Road intersection was made available to K\&T. The historic data was escalated over 3 years at an annual growth rate of $3 \%$ to obtain the 2021 volumes. The 2021 traffic counts were compared for the intersection and it was found that the northbound traffic volumes from the Chris Hani Road / Aerodrome Road intersection towards Aerodrome Road / Southgate Road intersection was much higher.

After consultation with Mr P Peska from the JRA, it was decided that the historic data would be used for Chris Hani Road / Aerodrome Road intersection. The traffic counts at intersections 1 and 2 were increased proportionally to obtain a worst-case scenario.

The link traffic volumes for the worst-case scenario are summarised in Table 2 below.
Table 2: Surveyed Traffic Volumes

| Road Name | Travel Direction | AM Peak Hour Volume <br> (vehicles) | PM Peak Hour Volume <br> (vehicles) |
| :---: | :---: | :---: | :---: |
| Chris Hani Road | Eastbound | 678 | 1117 |
| Aerodrome Road | Northbound | 1232 | 623 |
|  | Southbound | 538 | 927 |
| Southgate Road | Eastbound | 202 | 345 |
|  | Westbound | 105 | 364 |

### 9.2 TRIP GENERATION

Filing stations in general do not generate new trips, but intercept the daily traffic from the adjacent road. Therefore, in terms of $\mathrm{BB2}^{(1)}$, the traffic report is not required to consider the trip generation, distribution and assignment of trips. Layout 1 shows the graph extracted from the ITS Guidelines for access to filling station was used to estimate the interception rate.


Layout 1: Interception Rate versus Passerby Traffic
The interception rates and design volumes are summarised in Table 3.
Table 3: Filling Station Trip Interception

| Road Name | Travel Direction | AM Peak <br> Interception <br> Rate | AM Peak <br> Volume | PM Peak <br> Interception <br> Rate | PM <br> Peak <br> Volume |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Chris Hani Road | Eastbound | 7.13 | 48 | 6.91 | 77 |
| Aerodrome Road | Northbound | 6.86 | 85 | 7.16 | 45 |
|  | Southbound | 7.21 | 39 | 7.01 | 65 |
| Southgate Road | Eastbound | 7.38 | 15 | 7.31 | 25 |
|  | Westbound | 7.43 | 8 | 7.30 | 27 |

## 10. TRAFFIC OPERATIONS

### 10.1 ANALYSIS SCENARIOS

The critical peak hours (weekday AM and PM) were considered in the SIDRA analysis. The following analysis scenarios were considered for each of the aforementioned peak hours:

- Base Year: 2021 AM and PM peak hour existing traffic demand (refer to Diagrams A1 \& A2 in Annexure C);
- Scenario 1: 2026 AM and PM peak hour background traffic volumes (refer to Diagrams B1 \& B2 in Annexure C);


### 10.2 SIDRA ANALYSIS

The traffic operations at the Chris Hani Road / Aerodrome Road intersection are not expected to affect the traffic operations at the marginal access (Access 1) therefore SIDRA analysis was done for the Aerodrome Road / Southgate Road intersection with the aforementioned scenarios to determine if there will be a significant queue along Southgate Road that could potentially affect the full access (Access 2).

### 10.2.1 Aerodrome Road / Southgate Road intersection

The Aerodrome Road / Southgate Road intersection is a T-junction with priority stop on the eastern approach. The intersection is proposed to be signalised by latent developments. The existing configuration (Layout 2) was analysed for the base year and the proposed latent upgrades (Layout 3) were analysed for scenario 1.

The capacity analyses indicates that the right-turn movement will operate at LOS F during both peak hours for the base year. The proposed signalisation by the latent developments will improve the LOS of the aforementioned movement to within acceptable ranges for Scenario 1. The 95 ${ }^{\text {th }}$ percentile queue length along Southgate Road is expected to be 42 m in the PM peak hour (worse case). The proposed access will be located 50 m from the eastern approach of Aerodrome Road / Southgate Road intersection which is considered sufficient in this case.

The SIDRA summaries are included Annexure D.


Layout 2: Aerodrome Road / Southgate Road Existing Configuration


Layout 3: Aerodrome Road / Southgate Road Latent Configuration

## 11. CONCLUSIONS \& RECOMMENDATIONS

### 11.1 CONCLUSIONS

The following can be concluded from the report:

1. A filling station is proposed on The Farm 751 Registration Division I.Q, province of Gauteng.
2. A marginal access (Access 1) to the filling station is proposed off Chris Hani Road, which is a class 2 route and the access was designed in terms of ITS ${ }^{(1)}$ guidelines. The access was designed in line with typical drawing FIG No: C3 from ITS ${ }^{(1)}$ guidelines. The proposed access point is sufficiently far away from the adjacent intersection and is not expected to have an impact on or be impacted by the operating conditions at the adjacent intersection.
3. A full access (Access 2) is proposed off Southgate Road which is a class 5 route. The access was previously approved. It is proposed that the access should be located at a minimum distance of 50 m east measured from the edge of the bellmouth along Southgate Road. The SIDRA capacity analysis for Aerodrome Road / Southgate Road intersection indicates that there will not be a queue spillback past the proposed access position.

### 11.2 RECOMMENDATIONS

Taking the report and above conclusions into account, it is recommended that the proposed access layout as per figure 8101-TIA - FIG 3 should be approved.

## ANNEXURE A

## FIGURES





## ANNEXURE B

## LATENT UPGRADES




## ANNEXURE C

DIAGRAMS






## ANNEXURE D

SIDRA SUMMARIES

## MOVEMENT SUMMARY

Site: 2 [Aerodome Road / Southgate Road - 2021 AM Base Year]
2021 AM Base Year
Existing Configuration
Site Category: (None)
Stop (Two-Way)


Site Level of Service (LOS) Method: Delay \& v/c (HCM 2010). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010).
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

Site: 2 [Aerodome Road / Southgate Road - 2021PM Base Year]
2021 PM Base Year
Existing Configuration
Site Category: (None)
Stop (Two-Way)

| $\begin{array}{ll} \hline \text { Mov } & \text { Turn } \\ \text { ID } & \\ \hline \end{array}$ | Deman <br> Total veh/h | $\begin{aligned} & \text { lows } \\ & \text { HV } \\ & \text { \% } \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed km/h |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| South: Aerodome Road 0 en |  |  |  |  |  |  |  |  |  |  |  |
| 2 T1 | 523 | 0.0 | 0.237 | 0.8 | LOS A | 1.3 | 9.2 | 0.07 | 0.06 | 0.08 | 55.6 |
| 3 R2 | 100 | 0.0 | 0.237 | 13.9 | LOS B | 1.3 | 9.2 | 0.76 | 0.62 | 0.83 | 43.6 |
| Approach | 623 | 0.0 | 0.237 | 2.9 | NA | 1.3 | 9.2 | 0.18 | 0.15 | 0.20 | 51.0 |
| East: Southgate Road |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 173 | 0.0 | 0.187 | 9.5 | LOS A | 0.7 | 5.2 | 0.38 | 0.90 | 0.38 | 45.8 |
| 6 R2 | 191 | 0.0 | 2.535 | 740.5 | LOS F | 44.5 | 311.4 | 1.00 | 2.33 | 6.76 | 2.9 |
| Approach | 364 | 0.0 | 2.535 | 393.1 | LOS F | 44.5 | 311.4 | 0.70 | 1.65 | 3.72 | 5.0 |
| North: Aerodome Road |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 245 | 0.0 | 0.256 | 5.5 | LOS A | 0.0 | 0.0 | 0.00 | 0.30 | 0.00 | 53.7 |
| 8 T1 | 754 | 0.0 | 0.256 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.10 | 0.00 | 56.4 |
| Approach | 999 | 0.0 | 0.256 | 1.4 | NA | 0.0 | 0.0 | 0.00 | 0.14 | 0.00 | 55.2 |
| All Vehicles | 1986 | 0.0 | 2.535 | 73.6 | NA | 44.5 | 311.4 | 0.19 | 0.42 | 0.75 | 13.7 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 2010). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010).
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

## Site: 2 [Aerodome Road / Southgate Road - 2026 AM Scenario 1]

2026 AM Scenario 1
Latent Road Upgrades
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 80 seconds (Site Optimum Cycle Time - Minimum Delay)


Site Level of Service (LOS) Method: Delay \& v/c (HCM 2010). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay and $\mathrm{v} / \mathrm{c}$ ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010).
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

## Site: 2 [Aerodome Road / Southgate Road - 2026 PM Scenario 1]

2026 PM Scenario 1
Latent Road Upgrades
Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 60 seconds (Site Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{array}{ll} \hline \text { Mov } & \text { Turn } \\ \hline \text { ID } & \\ \hline \end{array}$ | Demand <br> Total veh/h | $\begin{gathered} \text { Flows } \\ \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate | Aver. No. A Cycles | Average Speed km/h |
| South: Aerodome Road |  |  |  |  |  |  |  |  |  |  |  |
| 2 T1 | 607 | 0.0 | 0.509 | 8.9 | LOS A | 10.3 | 72.4 | 0.66 | 0.59 | 0.66 | 36.6 |
| 3 R2 | 116 | 0.0 | 0.509 | 20.5 | LOS C | 3.4 | 23.8 | 0.78 | 0.76 | 0.78 | 38.0 |
| Approach | 723 | 0.0 | 0.509 | 10.7 | LOS B | 10.3 | 72.4 | 0.68 | 0.62 | 0.68 | 37.1 |
| East: Southgate Road |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 201 | 0.0 | 0.476 | 27.8 | LOS C | 5.3 | 37.0 | 0.91 | 0.80 | 0.91 | 32.7 |
| 6 R2 | 221 | 0.0 | 0.523 | 28.0 | LOS C | 5.9 | 41.2 | 0.92 | 0.81 | 0.92 | 33.8 |
| Approach | 422 | 0.0 | 0.523 | 27.9 | LOS C | 5.9 | 41.2 | 0.92 | 0.80 | 0.92 | 33.3 |
| North: Aerodome Road |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 284 | 0.0 | 0.517 | 14.2 | LOS B | 10.3 | 72.2 | 0.66 | 0.68 | 0.66 | 44.9 |
| 8 T1 | 874 | 0.0 | 0.517 | 8.6 | LOSA | 10.6 | 74.0 | 0.66 | 0.62 | 0.66 | 36.0 |
| Approach | 1158 | 0.0 | 0.517 | 10.0 | LOS A | 10.6 | 74.0 | 0.66 | 0.63 | 0.66 | 39.7 |
| All Vehicles | 2303 | 0.0 | 0.523 | 13.5 | LOS B | 10.6 | 74.0 | 0.71 | 0.66 | 0.71 | 36.9 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 2010). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay and $\mathrm{v} / \mathrm{c}$ ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010).
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## ANNEXURE E

## PHOTOGRAPHS



Photograph 1: Aerodrome Road / Southgate Road Intersection - Northern leg


Photograph 2: Aerodrome Road / Southgate Road Intersection - Eastern leg


Photograph 3: Chris Hani Road / Aerodrome Road Intersection - Northern leg


Photograph 4: Chris Hani Road / Aerodrome Road Intersection - Eastern leg


Photograph 5: Chris Hani Road / Aerodrome Road Intersection - Western leg

## Access Alternatives Explored: Scenario 2 being the most feasible one for the site

| Scenarios | Access Description | Pros | Cons |
| :---: | :---: | :---: | :---: |
| Scenario 1 (Drawing 8101 - TIAA - FIG 1) As per SDP drawing | - Full access off Southgate Road <br> - Full access off Chris Hani Road | Can intercept all streams of traffic from Chris Hani Road and Southgate Road. | The proposed full access off Chris Hani Road will be shared with SAB and this poses a safety concern. |
| Scenario 2 (Drawing 8101 - TIAA - FIG 2) | - Full Access off Southgate Road <br> - Marginal Access off Chris Hani Road | Dedicated access to filling station off Chris Hani Road. <br> Less conflict points which benefits safety. <br> Cost saving as there is already a physical median along Chris Hani Road. | Can only intercept eastbound traffic from Chris Hani Road. |
| Scenario 3 (Drawing 8101 - TIAA - FIG 3) | - Marginal Access off Aerodome Road <br> - Marginal Access off Chris Hani Road | Access off Aerodome Road could be possible. | Costly option as it would require a physical median barrier to be built along Aerodome Road. <br> Can only intercept southbound traffic from Aerodome Road and eastbound traffic from Chris Hani Road. |





